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November 30, 2020

Mr. Andrew R. Wheeler, EPA Administrator Environmental Protection Agency 1200 Pennsylvania Avenue, N.W. Mail Code 5304-P Washington, DC 20460

Subject: Cardinal Power Plant - FAR II Alternative Closure Demonstration, Revision 1

Dear Administrator Wheeler:

Cardinal Operating Company hereby submits a revised demonstration to the U.S. Environmental Protection Agency (EPA) for approval for a site-specific alternative deadline to initiate closure pursuant to 40 CFR § 257.103(f)(1) for Fly Ash Reservoir II located at Cardinal Power Plant in Brilliant, Ohio.

Cardinal is requesting an extension pursuant to 40 CFR § 257.103(f)(1) to allow the impoundment to continue receiving CCR and non-CCR waste streams after April 11, 2021, in order to complete conversion to a dry fly ash system for future disposal at the on-site FAR I RSW Landfill.

Cardinal's original demonstration was submitted electronically to US EPA on October 30, 2020. The revised demonstration includes additional descriptions, clarifications and details to better describe Cardinal's CCR program compliance.

Enclosed is a demonstration prepared by Sargent & Lundy that addresses all of the criteria in 40 CFR § 257.103(f)(1)(i)-(iii) and contains the compliance documentation required by 40 CFR § 257.103(f)(1)(iv). As allowed by the agency, in lieu of hard copies of these documents, electronic files were submitted to Kristen Hillyer, Frank Behan, and Richard Huggins via email.

If you have any questions regarding this submittal, please contact Nick Kasper at (614) 681-5160 or nkasper@ohioec.org.

Sincerely,

Thomas M. Alban Vice President

The M. alban

cc: Kristen Hillyer Frank Behan Richard Huggins



Cardinal Power Plant Fly Ash Reservoir II

Demonstration for a Site-Specific Alternative to Initiation of Closure Deadline

Report SL-015643

Revision 1

November 30, 2020

Issue Purpose: Use

Project No.: 13770-007

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EXECUTIVE SUMMARY

Fly Ash Reservoir (FAR) II at the Cardinal Power Plant in Brilliant, Ohio does not meet the liner design criteria or the uppermost aquifer location criteria promulgated by 40 CFR Part 257 Subpart D ("the EPA CCR Rule"). Therefore, the Cardinal Power Plant must cease placing the CCR and non-CCR waste streams currently sent to FAR II as soon as technically feasible and no later than April 11, 2021, unless an alternative deadline is granted by the EPA in accordance with 40 CFR 257.103. After evaluating several on- and off-site alternative disposal solutions for the waste streams currently sent to FAR II – both permanent and temporary – the Cardinal Operating Company has concluded that no alternative disposal is available for the waste streams currently being sent to FAR II, and that it was technically infeasible to obtain alternative disposal capacity for these waste streams on or off-site by April 11, 2021. Accordingly, pursuant to 40 CFR 257.103(f)(1)(iv)(A), the Cardinal Operating Company has prepared the following workplan detailing its development of alternative disposal capacity to replace FAR II.

The Cardinal Power Plant currently sends the following coal combustion residual (CCR) and non-CCR waste streams to FAR II: fly ash transport water (CCR), excess process water from the station's Bottom Ash Pond Complex (non-CCR), and leachate and contact storm water run-off from the station's landfill (non-CCR). After evaluating several options for providing permanent alternative disposal capacity to FAR II for these waste streams, the Cardinal Operating Company elected to install a multiple technology system: convert the Cardinal Power Plant's wet fly ash-handling system to a dry system to utilize the plant's landfill for fly ash disposal, and install a waste water treatment system for the landfill's leachate and contact storm water run-off prior to discharge. In addition to providing compliance with the EPA CCR Rule, the dry fly ash system also eliminates the generation of fly ash transport water at the Cardinal Power Plant and therefore inherently provides compliance with the EPA's zero liquid discharge standard for fly ash transport water in the agency's recently revised effluent limitation guidelines for steam electric power generating stations.

The dry fly ash system is currently being constructed at all three of the Cardinal Power Plant's generating units, and the waste water treatment system is currently being designed and permitted for the landfill leachate and contact storm water run-off. Both systems are scheduled to be installed and operational by June 7, 2021. Accordingly, the Cardinal Operating Company is requesting the EPA allow FAR II to continue receiving the CCR and non-CCR waste streams currently managed therein until June 7, 2021.

1.0 DEVELOPMENT OF ALTERNATIVE CAPACITY

This section presents the option selected by the Cardinal Operating Company to provide alternative disposal capacity to Fly Ash Reservoir II for the CCR and non-CCR waste streams managed therein. In addition, this section provides background information on the Cardinal Power Plant, Fly Ash Reservoir II and the waste streams managed therein, the adverse impact to plant operations if Fly Ash Reservoir II was shutdown, the process the Cardinal Operating Company undertook to select the alternative disposal capacity currently being developed, and a narrative of the alternative disposal capacity design. Finally, an explanation and justification for the time being requested to operate Fly Ash Reservoir II beyond April 11, 2021 is also provided in this section.

1.1 BACKGROUND INFORMATION

1.1.1 CARDINAL POWER PLANT

The Cardinal Operating Company operates the Cardinal Power Plant ("Cardinal"), which is a coal-fired steam electric power generating station located in Brilliant, Ohio, adjacent to the Ohio River. The station's address is 306 County Road 7E, Brilliant, Ohio 43913. The plant consists of three operating units – Units 1, 2, and 3 – and has a combined nameplate capacity of approximately 1,800 MW.

1.1.1.1 PLANT OWNERSHIP & SERVICE AREA

Unit 1 is owned by AEP Generation Resources Inc., the competitive generation subsidiary of American Electric Power. Units 2 and 3 are owned by Buckeye Power, Inc., a generation and transmission cooperative that operates the Cardinal Operating Company. The plant operates as a base-load generation asset to meet the day-to-day electricity demands of the 25 electric cooperatives that own and govern Buckeye Power, Inc. as well as the local communities serviced by AEP Generation Resources Inc.

1.1.1.2 FLY ASH-HANDLING SYSTEM

Fly ash produced by all three of the station's units is initially collected by the hoppers underneath the casings of each unit's electrostatic precipitators (ESPs). The fly ash is then vacuum-pneumatically conveyed from these hoppers to one of two Hydroveyor® exhausters, where the dry fly ash is mixed with water recycled from the Recirculation Pond in the station's Bottom Ash Pond (BAP) Complex and the conveying air is discharged to the environment. The fly ash-water mixture (slurry) is then temporarily stored in tanks before ultimately being sluiced to the plant's existing Fly Ash Reservoir (FAR) II via three slurry pumps.

1.1.2 FLY ASH RESERVOIR II

1.1.2.1 RESERVOIR CHARACTERISTICS

FAR II is a coal combustion residual (CCR) surface impoundment that occupies a narrow valley due north of Cardinal Units 1 and 2. The reservoir was created by constructing a dam across the valley to retain the fly ash sluice water. A figure of the Cardinal facility and associated CCR units is presented in Figure 1.

1.1.2.2 RESERVOIR INFLOWS & OPERATIONS

Based on the Fact Sheet submitted with Cardinal Operating Company's 2018 National Pollution Discharge Elimination System (NPDES) permit application for the Cardinal Power Plant (Ref. 3), an average of approximately 8.9 million gallons of fly ash transport water (FATW) is sent to FAR II per day. In addition to FATW, the plant also conveys the following low-volume waste (LVW) streams to FAR II at a collective average of approximately 2.9 million gallons per day (MGD):

- Excess process water recycled from the Recirculation Pond to control the water level in BAP Complex (2.0 MGD), and
- Leachate collected and removed from the adjacent FAR I Landfill (0.1 MGD),
- Contact storm water run-off from the adjacent FAR I Landfill (0.8 MGD).

Table 1 summarizes the waste streams currently sent to FAR II. Per the aforementioned NPDES permit application, the maximum flows listed in the table are based on the 10-year, 24-hour storm event for the site. The water balance diagram associated with Cardinal's NPDES Permit 0IB00009*XD is presented in Figure 2.

Table 1 - Inflows into Cardinal Fly Ash Reservoir II

Waste Stream	Description	Average Flow, MGD (Max. Flow, MGD)
CCR Waste Streams	8.93	
Unit 1 & 2 Fly Ash Transport Water	Sluice water containing fly ash particles from the Unit 1 and 2 ESPs	5.76
Unit 3 Fly Ash Transport Water	Sluice water containing fly ash particles from the Unit 3 ESP	3.17
Non-CCR Waste Streams		2.90 (28.60)
Excess Process Water Through Unit 1 & 2 Fly Ash-Handling Systems	Water removed from the BAP Complex to control the pond's water level that bypasses the Unit 1 and 2 Hydroveyors®	0.96 (2.88)
Excess Process Water Through Unit 3 Fly-Ash Handling System	Water removed from the BAP Complex to control the pond's water level that bypasses the Unit 3 Hydroveyors®	1.06 (3.17)
FAR I Landfill Leachate	Leachate collected and removed from FAR I Landfill	0.09 (0.41)
FAR I Landfill Storm Water Run-Off	Contact storm water run-off from FAR I Landfill	0.79 (22.14)

Source: Cardinal 2018 NPDES Permit Application Fact Sheet (Ref. 3)

1.1.2.2.1 FLY ASH TRANSPORT WATER

The primary purpose of FAR II is to store fly ash produced by Cardinal during power-generating operations. FATW enters the northern end of the reservoir, and the suspended fly ash particles undergo sedimentation as the water flows towards the outfall at the southern end of the pond. The pond effluent then discharges through the station's NPDES-permitted Outfall 019.

1.1.2.2.2 EXCESS PROCESS WATER

In order to control the water level in the BAP Complex, particularly during unit outages and significant storm events, Cardinal will recover additional water from the complex's Recirculation Pond and route it to FAR II while bypassing the fly ash-handling systems. This process allows the station to limit discharges through NPDES-permitted Outfall 023 at the southern end of the Recirculation Pond, especially after significant storm events.

1.1.2.2.3 FAR I LANDFILL LEACHATE & CONTACT STORM WATER RUN-OFF

Cardinal's FAR I Landfill has a perimeter containment berm system that prevents storm water that comes into contact with solid waste ("contact storm water") from leaving the active landfill area. A perimeter drainage ditch inside and adjacent to the containment berm route collected contact water to chimney drains that flow into the landfill's leachate collection pipes. Contact water and leachate are ultimately conveyed to FAR II, which serves as FAR I Landfill's leachate treatment pond.

1.1.2.3 APPLICABLE REGULATIONS

1.1.2.3.1 FEDERAL CCR RULE

FAR II has been regulated by the EPA CCR Rule (40 CFR Part 257 Subpart D, Ref. 1) since the rule went into effect in October 2015. Per the 2016 Water Infrastructure Improvements for the Nation (WIIN) Act, FAR II will continue to be subject to the requirements prescribed in the EPA CCR Rule until the EPA approves a CCR permit program developed and submitted by the Ohio EPA. Because the Ohio EPA has yet to submit a proposed CCR permit program to the EPA, Ohio is currently considered a Nonparticipating State per 40 CFR 257.53. Consequently, this workplan and the alternative closure deadline requested herein for FAR II are subject to the approval of the EPA.

1.1.2.3.2 FEDERAL ELG RULE

In addition to the preceding EPA CCR Rule, the operation of FAR II – specifically discharges through NPDES-permitted Outfall 019 – is also subject to compliance with the EPA's effluent limitation guidelines for steam electric power plants ("ELG Rule"). The 2015 update to the ELG Rule (Ref. 8) set new limits for discharging FATW and other waste streams generated by steam electric power plants to waters of the U.S. Pursuant to 40 CFR 423.13(h)(1)(i), the ELG Rule establishes a zero-liquid discharge (ZLD) standard for Cardinal's FATW – including any LVW streams that come into contact with FATW – unless the FATW is used in a flue gas desulfurization (FGD) scrubber. In this case, the waste water may be discharged in accordance with the ELGs specified in 40 CFR 423.13(g)(1)(i) for FGD waste water.

Cardinal will be subject to the ZLD standard for FATW promulgated by the updated ELG Rule upon incorporation into the facility's NPDES permit by a date determined by the Ohio EPA. Pursuant to the station's NPDES permit, the Ohio EPA has approved a compliance end date of December 31, 2023. This complies with the new 40 CFR 423.13(k)(1)(i), which requires this NPDES permit modification to occur no later than December 31, 2023.

1.1.2.4 FUTURE HANDLING OF FLY ASH

FAR II does not meet the liner design criteria promulgated by 40 CFR 257.71(a) and is therefore considered to be an unlined CCR surface impoundment. In addition, the pond's base is within five feet of the upper limit of the site's uppermost aquifer (Ref. 11). Thus, per 40 CFR 257.101(a)(1), (a)(3), (b)(1)(i), and (b)(4), Cardinal must cease placing the CCR and non-CCR waste streams listed in Table 1 into FAR II as soon as technically feasible and no later than April 11, 2021, unless an alternative deadline is granted by the EPA.

As detailed herein, the Cardinal Operating Company is requesting that the EPA allow Cardinal to continue sending certain CCR and non-CCR waste streams to FAR II after April 11, 2021 while it develops alternative capacity to replace the reservoir because:

- No existing alternative disposal capacity is available on- or off-site for these waste streams, and
- It was technically infeasible to develop the alternative capacity selected by April 11, 2021 for these
 waste streams.

1.1.3 ADVERSE IMPACT TO PLANT OPERATIONS WITHOUT FAR II

In order to generate power at Cardinal, it is necessary to dispose of the fly ash produced from the combustion of pulverized coal in the station's boilers. Without suitable replacements for the FAR II, the Cardinal plant would be forced to stop power-generating operations. Additionally, if FAR II was forced to initiate closure, Cardinal would be subject to NPDES violations. Treatment systems designed to treat the LVW streams (landfill leachate, contact stormwater and non-contact stormwater) to meet effluent limits would not have been constructed, and the waste streams would have no permitted outfall.

1.2 GENERAL STRATEGY FOR COMPLIANCE WITH EPA REGULATIONS

The Cardinal Operating Company has evaluated several different handling and/or disposal alternatives for Cardinal's CCR and non-CCR waste streams since 2016, shortly after the EPA's new CCR Rule and the 2015 amendment to its ELG Rule both became effective. Given the ZLD standards established for both FATW and bottom ash transport water (BATW) in the 2015 ELG Rule (Ref. 8), which included (and still include) non-CCR waste streams that are commingled with FATW and BATW, the Cardinal Operating Company evaluated alternatives that either eliminated these waste streams or allowed for them to be recirculated back into plant systems. In this evaluation of ELG Rule compliance options, the Cardinal Operating Company also sought solutions that would be compliant with the new EPA CCR Rule. In essence, the Cardinal Operating Company has been seeking holistic solutions in regard to complying with both the EPA CCR and ELG Rules for alternative handling and/or disposal of Cardinal's CCR and non-CCR waste streams.

1.3 ALTERNATIVE DISPOSAL SOLUTIONS CONSIDERED

Prior to the August 2018 *Utility Solid Waste Activities Group* (*USWAG*) decision by the U.S. Court of Appeals for the D.C. Circuit (Ref. 2), in which the Court ordered the provisions in the EPA CCR Rule allowing unlined ash ponds to continue operating be vacated and remanded, the Cardinal Operating Company started evaluating several different methods for disposing of Cardinal's fly ash in lieu of sluicing it to FAR II. This study expanded a similar assessment performed in 2016 to identify additional fly ash storage capacity and, in accordance with the Cardinal Operating Company's desire for a holistic solution, assessed not only permanent disposal solutions for Cardinal's FATW but also the LVW streams managed by the reservoir. This assessment is summarized in Section 1.3.3.

Pursuant to the recently-revised alternative closure requirements for CCR surface impoundments in the EPA CCR Rule, the Cardinal Operation Company also evaluated whether existing capacity is available on- or off-site for each waste stream currently sent to FAR II. For those streams where existing capacity is not available, the Cardinal Operating Company evaluated whether it was technically feasible to obtain alternative disposal capacity – either temporary or permanent – by April 11, 2021. The following subsections discuss the alternative disposal solutions considered for each waste stream managed in FAR II and how these waste streams were ultimately dispositioned.

1.3.1 EXISTING ON-SITE DISPOSAL SOLUTIONS

1.3.1.1 FLY ASH TRANSPORT WATER

Because FATW is a CCR waste stream, it must be disposed of in an active CCR unit. As documented on the Cardinal Operating Company's public CCR website (Ref. 4), Cardinal has three CCR units on-site: FAR II, the BAP Complex, and FAR I Landfill. The BAP Complex is comprised of two CCR surface impoundments used by the station to store and treat its BATW and several miscellaneous non-CCR waste streams. However, like FAR II, both ponds in the BAP Complex are not compliant with the EPA CCR Rule's liner design criteria and are therefore subject to the closure-for-cause requirements promulgated by 40 CFR 257.101. Moreover, the BAP Complex is significantly smaller than FAR II (approximately 25 acres versus 160 acres) and was not designed to provide adequate detention time for the finer ash particles in Cardinal's FATW prior to being discharged in accordance with the station's NPDES permit. Finally, the BAP Complex was not designed to contain Cardinal's total daily volume of FATW and BATW. Thus, the BAP Complex would not be an acceptable alternative disposal facility for Cardinal's fly ash even if the necessary mechanical equipment and piping were installed to divert FATW from FAR II to the BAP Complex.

Located adjacent to FAR II, the station's FAR I Landfill is an EPA CCR Rule-compliant disposal facility that is primarily used by the station to dispose of the gypsum byproduct from its FGD systems. This landfill has also been used to dispose of bottom ash that has been dredged from the BAP Complex and subsequently

dewatered. While the landfill may receive fly ash and has sufficient capacity to accommodate Cardinal's daily generation of fly ash, the Ohio EPA prohibits industrial solid waste landfills like FAR I Landfill from receiving bulk or noncontainerized liquids wastes like Cardinal's FATW (Ref. 10). Thus, the station cannot utilize its landfill for directly disposing of its fly ash while it has a wet fly ash-handling system. Cardinal would need a fly ash dewatering system or an entirely dry fly ash-handling system to directly send its fly ash to FAR I Landfill. Because Cardinal does not currently have these systems, the station does not presently have the means to directly dispose of its fly ash in FAR I Landfill.

In summary, there is no alternative on-site disposal capacity to FAR II available for Cardinal's wet-generated fly ash because:

- The station's only other wet CCR disposal facility, the BAP Complex, is not compliant with the EPA
 CCR Rule's liner design criteria and, like FAR II, is subject to closure for cause, and
- Neither a dry fly ash-handling system nor a fly ash dewatering system are present at the station to allow for Cardinal to utilize its on-site CCR landfill, FAR I Landfill.

1.3.1.2 NON-CCR WASTE STREAMS

1.3.1.2.1 EXCESS PROCESS WATER FROM BAP COMPLEX

Excess process water recovered from the BAP Complex and sent to FAR II to maintain the former's water level inherently has an alternative disposal solution at Cardinal: the BAP Complex from which it comes. However, the station needs to remove this water during unit outages (all three units have outages scheduled for spring 2021) and during significant storm events. Otherwise, it is unlikely the BAP Complex would provide adequate detention time for the surplus volume of wastewater to meet Cardinal's NPDES permit requirements before having to discharge through Outfall 023 to avoid overtopping. Thus, this water needs to continue being sent to FAR II to avoid NPDES permit violations.

It should be noted that management of excess water sent to the BAP Complex after FAR II has initiated closure will be the responsibility of the contractor repurposing the BAP Complex in accordance with the Cardinal Operating Company's demonstration for a site-specific alternative initiation of closure deadline for the BAP Complex. Per the Cardinal Operating Company's workplan for repurposing the BAP Complex, that contractor is expected to mobilize to the site as FAR II is taken out of service and start drawing down the water level in the BAP Complex. All discharges from the BAP Complex at that time will be performed in accordance with Cardinal's NPDES permit.

1.3.1.2.2 FAR I LANDFILL LEACHATE & CONTACT STORM WATER RUN-OFF

The leachate and contact storm water from FAR I Landfill require adequate detention time for sedimentation to reduce the concentration of total suspended solids (TSS) to adequate discharge levels in accordance with

the station's NPDES permit. Given its size, the reservoir has historically been able to accept and treat this secondary waste stream in addition to the FATW that it treats. The only other existing facility at the Cardinal station that might be able to accept leachate and contact storm water run-off from FAR I Landfill would be the BAP Complex. However, as previously stated, this facility cannot accept new waste streams since it is subject to the closure-for-cause requirements promulgated by 40 CFR 257.101. Thus, the BAP Complex would not be an acceptable alternative disposal facility for the leachate and contact storm water run-off from FAR I Landfill even if the necessary mechanical equipment and piping were installed to divert this waste stream from FAR II to the BAP Complex.

While Cardinal has two, small coal pile runoff ponds, these ponds are not an alternative to FAR II. The ponds are less than an acre in size and are designed to receive various runoffs and facility sumps. They do not have capacity to handle additional waste streams. If these ponds were to accept leachate, they would need to be lined and have a groundwater monitoring system installed pursuant to Ohio Residual Solid Waste regulations.

1.3.2 EXISTING OFF-SITE DISPOSAL SOLUTIONS

1.3.2.1 TEMPORARY OFF-SITE DISPOSAL FACILITIES

Although the EPA itself has acknowledged that it is not feasible to transport wet-generated CCR to an off-site disposal facility (Ref. 5), the Cardinal Operating Company performed its due diligence and evaluated the feasibility of temporarily transporting the average daily volume of FATW, FAR I Landfill leachate, and contact storm water run-off from FAR I Landfill to an off-site disposal facility until a permanent disposal facility could be installed on-site. As previously mentioned, landfills are generally not permitted to receive bulk or noncontainerized liquids, so only waste water treatment plants (WWTPs) could be considered as potential disposal facilities for the waste water flows considered in this evaluation.

Although not covered in this workplan, the Cardinal Operating Company is also requesting an alternate deadline for ceasing flows to the BAP Complex. Consequently, CCR and non-CCR waste streams sent to this pond would also need to be transported to an off-site treatment facility if alternative disposal capacity does not currently exist on-site. As demonstrated in the corresponding workplan for the BAP Complex, Cardinal does not currently have alternative means of disposing the flows presented in Table 2. As shown in the table, an average flow of approximately 12.1 MGD of CCR and non-CCR waste water would need to be sent to a temporary facility off-site in addition to the noted FAR II waste streams.

To be a viable option, a WWTP would need to receive the average daily volume of the preceding CCR and non-CCR waste streams from FAR II and the BAP Complex, in addition to the waste water volume the WWTP currently treats. Therefore, per Table 1 and Table 2, the WWTP (or combination of WWTPs) would need to be capable of receiving an average flow of 21.9 MGD.

Table 2 - Inflows into the Cardinal Bottom Ash Pond Complex Requiring Alternative Disposal

Waste Stream	Description	Average Flow, MGD
CCR Waste Streams		4.14
Unit 1 & 2 Bottom Ash Transport Water	Sluice water containing bottom ash particles from the Unit 1 and 2 boilers	2.30
Unit 3 Bottom Ash Transport Water	Sluice water containing bottom ash particles from the Unit 3 boiler	1.84
Non-CCR Waste Streams		7.98
Unit 1 & 2 Plant Services Waste Water	Waste water from the process water used to operate equipment in Units 1 and 2 (e.g., heat exchangers)	4.32
Unit 3 Cooling Tower Blowdown	Waste water used to remove minerals collected in the Unit 3 cooling tower basin	1.58
Unit 3 Cooling Tower Basin Overflow Overflow water from the Unicooling tower basin		1.83
Unit 3 Sump and Drain Water	Contact storm water collected by sumps and drains in the Unit 3 power block	0.02
Coal Pile Run-Off Pond Overflow	Waste water collected by the station's Coal Pile Run-Off Pond. Includes contact storm water from: Coal pile, Coal truck unloading area, Unit 1, 2, and 3 FGD areas (including gypsum pile, limestone pile, and marine area run-off), and Unit 1 and 2 power block sumps and drains.	0.23

Source: Cardinal 2018 NPDES Permit Application Fact Sheet (Ref. 3)

Inquiries were placed with 11 WWTPs within 50 miles of the station to determine if any plants in the region were capable of handling the total or a significant portion of the 21.9 MGD of ash transport and non-CCR waste water from Cardinal. Of the four WWTPs that responded, one indicated that the facility could not accept external waste streams and two had a combined capacity of less than 10 MGD. These facilities treat water from other sources and are unable to guarantee the ability to treat Cardinal's waste streams on regular basis, while meeting their permit obligations. This would put Cardinal at risk for NPDES violations. Additionally, the WWTPs are not equipped to handle the additional truck traffic required to receive Cardinal's waste streams. Off-site WWTPs are not a viable treatment option for Cardinal's waste streams.

Even if one or more WWTPs had sufficient capacity, the Cardinal Operating Company would need to identify a means of transporting the waste water from Cardinal. Given the station's existing ash-handling infrastructure, trucks with tank trailers would likely be the only transportation method that could be established for the station's ash transport and non-CCR waste streams prior to the April 11, 2021 deadline for ceasing all flows into Cardinal's ash ponds.

In this scenario, fly ash slurry temporarily stored in the existing tanks downstream of the station's Hydroveyors® would be directly pumped into the trucks' tank trailers. Meanwhile, based on an average continuous flow rate of 8,400 gpm, new tanks be installed at some interception point upstream of the BAP Complex to temporarily store the BATW and non-CCR waste streams currently going into the BAP Complex prior to being pumped into tank trucks. A similar system would be established near FAR I Landfill to collect its leachate and contact storm water run-off. It should be noted that this scenario would require Cardinal to identify and obtain an alternate source of water for the fly ash-handling system in lieu of the water currently recycled from the BAP Complex. The nearest alternative water source would be the Ohio River. However, Cardinal does not have infrastructure to use this water. Utilizing this option would require regulatory approvals from Ohio EPA (permit to install, NPDES modification) and Ohio Department of Natural Resources. Additionally, there may be 316(b) implications associated with this withdrawal. Given regulatory uncertainty and time required to obtain these permits, Cardinal did not proceed with this option.

Ohio state law limits the overall gross vehicle weight to 80,000 pounds (Ref. 6). Considering the weight of the CCR solids in the waste water being transferred to a WWTP and assuming an empty tank trailer weight of 12,000 pounds, a 7,000-gallon tank trailer would be the maximum tank trailer that would be permitted to transport waste water to an off-site WWTP. Therefore, over 3,100 daily trips would be required to transport 21.9 MGD of ash transport and non-CCR waste water to a WWTP. Even if Cardinal implemented an alternate means of handling its non-CCR waste water, it would require more than 1,800 daily trips to transport the 13.1 MGD of FATW and BATW generated by the station.

Even if the station could support the number of tank trucks to keep up with its daily production rate of ash transport and non-CCR waste water, there would be significant logistics concerns in coordinating that many trips to and from the station's property. The only way trucks can access the Cardinal site is via Ohio State Route 7 (SR-7). Based on traffic data compiled by the Ohio Department of Transportation (Ref. 7), the average annual daily traffic (AADT) in 2019 for commercial trucks along SR-7 near Cardinal was 1,770 trucks. Therefore, the 3,100 trips required to transport Cardinal's daily volume of ash transport and non-CCR waste water to an off-site WWTP would almost triple the daily volume of truck traffic currently on SR-7. This would impose significant congestion issues on this four-lane road along the Ohio River, an increased potential for traffic accidents, and an increase in air pollution emissions. Thus, in addition to being harmful to human health and the environment, it is impractical to route 3,100-trips worth of trucks per day to an off-site WWTP for several months until alternative ash disposal facilities are installed on-site.

Based on the lack of regional WWTPs available to process or even handle Cardinal's daily volume of ash transport and non-CCR waste water, and based on the impracticality and risks of coordinating the number of truck trips required to handle this volume of waste water, the Cardinal Operating Company has reached the same conclusion as the EPA (Ref. 5) regarding the off-site transportation of wet-generated ash: it is not feasible.

1.3.3 NEW ON-SITE DISPOSAL SOLUTIONS

Based on the preceding evaluations, no alternative disposal capacity currently exists on- or off-site for Cardinal's FATW, FAR I Landfill leachate, and contact storm water run-off from FAR I Landfill. Consequently, the Cardinal Operating Company has been actively developing alternative disposal capacity for these waste streams. This subsection presents the process the Cardinal Operating Company underwent to ultimately select the alternative disposal capacity to replace FAR II.

1.3.3.1 EVALUATION OF ASH DISPOSAL METHODS

As previously stated, the Cardinal Operating Company commenced a study in the third quarter of 2018 that evaluated several different methods for disposing of Cardinal's fly ash in lieu of sluicing it to FAR II. This study was an expansion of a similar assessment performed two years earlier and included the following fly ash-handling technologies:

- Install geotextile filter tubes at FAR I Landfill,
- Construct a new surface impoundment on undeveloped land,
- Construct a concrete settling tank at FAR I Landfill, and
- Convert Cardinal's fly ash-handling system to a dry, vacuum-pneumatic system (with final disposal in FAR I Landfill).

1.3.3.1.1 GEOTEXTILE FILTER TUBES

Geotextile filter tubes are containers with oval-shaped cross sections that are composed of engineered fabric that can filter out fine particles within water. Thus, FATW lines could be routed directly to a series of these tubes to filter fly ash particles out of the transport water. As the fly ash particles are consolidated within each tube, filtered sluice water would percolate out of each tube onto an impermeable pad with appropriate run-off control measures. Once a tube is full of fly ash particles, FATW would be redirected to another tube while the full tube continues to dewater. After the filtered ash has been sufficiently dewatered, the full tube would be cut open and its contents loaded onto trucks for final disposal in FAR I Landfill.

For Cardinal, a series of geotextile filter tubes could be installed within the existing FAR I Landfill area. The tubes could be installed in a series of self-contained bays that would facilitate sequential operation of the tubes: one bay would feature a tube actively receiving FATW, a second bay would feature a tube being

dewatered, and a third bay would feature a tube being reclaimed for landfilling. Collected filtrate from dewatering could be gravity-drained to a collection sump that would ultimately convey water to a new recirculation water storage tank. To comply with the revised ELG Rule, a new recirculation water system would be installed to pump water back to all three units for re-use in the existing fly ash-handling system.

While geotextile filter tubes have been used as a method for dewatering bottom ash ponds, this option could be considered a "first-of-a-kind" technology for dewatering a power plant's daily product of fly ash. Consequently, this option would have a lot of uncertainties, especially as it pertains to dewatering and filtering out very fine fly ash particles. Filter aids such as coagulants and polymers may be required to coagulate the ash particles together, making them easier to filter; a series of tests would likely be required to determine the appropriate aids. This option would also require active monitoring to ensure the tube being filled is being done so uniformly and that all the tubes are being filled systematically. Finally, there would be challenges in operating and dewatering these tubes during below-freezing weather conditions and significant rain events.

Based on the permitting, engineering and design, procurement, and construction activities required to implement this option, the Cardinal Operating Company estimated that a new geotextile tube facility would take approximately 2.5 years to construct. The primary driver of this schedule would be the permitting required to modify the existing wet fly ash-handling system with this new treatment option.

1.3.3.1.2 NEW SURFACE IMPOUNDMENT

The Cardinal Operating Company also considered replacing FAR II with a new surface impoundment. Two potential locations on the station's property were identified as suitable for a new ash pond provided new dams were constructed to obtain the necessary long-term storage capacity. Pursuant to the EPA CCR Rule, the new ash pond would be lined with a composite liner system consisting of a geomembrane underlain by a compacted clay liner with a permeability no greater than 1×10^{-7} cm/sec. A groundwater monitoring program for the new ash pond would be implemented, including the installation of upstream and downstream monitoring wells, to sample and test groundwater in accordance with the EPA CCR Rule. Like the geotextile filter tube option, a recirculation system for FATW would be installed for this option.

Although ash ponds are a proven technology for ash disposal, constructing a new surface impoundment would require a significantly longer design, permitting, and construction effort than the other options considered. Except for the plant proper, Cardinal's property is predominately hilly terrain. So while the two locations identified as potential sites for a new ash pond are currently undeveloped, it would require extensive design and construction efforts to modify the station's FATW piping, to install an EPA CCR Rule-composite liner system, and to construct the earth dams required to form a reservoir. This option would also require sufficient time to adequately establish the background groundwater conditions in accordance with the

EPA CCR Rule's groundwater monitoring requirements. Finally, a significant amount of return piping would need to be installed to comply with the revised EPA ELG Rule. Overall, it was estimated that this option would take just over 3 years to develop from engineering and design through construction and commissioning.

Given the prolonged schedule required to design, permit, and construct a new surface impoundment relative to the other options evaluated, this option was removed from consideration as an alternative disposal option to replace FAR II.

1.3.3.1.3 CONCRETE SETTLING TANKS

In lieu of a traditional ash pond, fly ash could be settled out of transport water by using self-supporting, cast-in-place reinforced concrete tanks. This option would feature a series of primary tanks where most of the ash particles would settle. Water from the primary tanks would overflow into a surge tank for settling of the finer ash particles.

Like the previous two options, FATW in the surge tank would ultimately be recirculated back to the station to comply with the revised ELG Rule. Cardinal would sluice FATW to one primary tank at a time, switching to an empty tank as a given tank reaches capacity. Equipment would then be used to manually segregate and manipulate the ash in the full tank to promote dewatering. After this initial dewatering, ash would be recovered and transferred to an adjacent concrete pad to completely dewater. Like the pad proposed for the geotextile filter tube option, this dewatering pad would feature appropriate run-off control measures; it would also be sloped such that water drains back to the primary tank. Once the ash is sufficiently dry, it would be loaded onto trucks and disposed of in FAR I Landfill.

While concrete settling tanks have been used to handle bottom ash, this technology, like geotextile filter tubes, could be considered "first of a kind" for regularly handling fly ash. The finer fly ash particles require a longer detention time than bottom ash to settle out of transport water, and therefore it is questionable whether fly ash can be adequate dewatered in a concrete settling tank. Filter aids such as coagulants and polymers could be introduced to the FAW, but this would require adequate bench testing to assure operational reliability. Moreover, given the inherent ability of fly ash to retain water, it is anticipated that the amount of manual labor required to dewater this material would be significant. Dewatering would also be further inhibited by adverse weather conditions, especially during the winter. Finally, the operation of these tanks and subsequent dewatering of ash collected therein would not be technically feasible during below-freezing weather conditions and excessive rain events.

Given this option's similarities to the geotextile filter tube option, it was anticipated that this option would take a similar amount of time to implement. Thus, the Cardinal Operating Company estimated that it would take

approximately 2.5 years to construct new concrete settling tanks and the ancillary operating equipment to handle Cardinal's fly ash.

1.3.3.1.4 DRY FLY ASH CONVERSION

Finally, the Cardinal Operating Company evaluated the conversion of Cardinal's existing wet fly ash-handling system to a dry system. In addition to providing inherent compliance with the ELG Rule, this would enable Cardinal to utilize FAR I Landfill as the alternative disposal capacity for FAR II without the operational risks of geotextile filter tubes or concrete settling tanks. This option would entail a new vacuum-pneumatic system for each unit that would utilize air flow and conveying pipes to transfer fly ash from the units' ESPs to filter separators, which would separate the conveying air from the fly ash. The fly ash would ultimately be discharged into a silo for temporary storage until the material is loaded onto trucks for either on-site disposal in FAR I Landfill or beneficial re-use.

For this option, the primary schedule driver would be the procurement of the temporary storage silos and the other fly ash-handling equipment required for the project. While some time would be required to apply for and receive an air permit for this option, the overall permitting effort would not be as extensive as the preceding three options. Overall, it was estimated that it would take a little over 2 years to convert Cardinal's wet fly ash-handling system into a dry system.

1.3.3.2 OPTION SELECTED

Ultimately, the Cardinal Operating Company elected to comply with the EPA CCR and ELG Rules by installing a new vacuum-pneumatic fly ash conveying system at each unit, temporarily storing dry fly ash in new storage silos, and ultimately transporting the stored fly ash to Cardinal's existing FAR I Landfill for final disposal. This option provided the station with a proven technology widely utilized by power plants that also eliminates FATW which inherently provides compliance with the EPA ELG Rule's ZLD standard for this waste stream.

Although a conversion to a dry fly ash-handling system provides access to alternative disposal capacity for Cardinal's fly ash (FAR I Landfill), this system does not provide alternative storage for FAR I Landfill's leachate or contact storm water run-off. Accordingly, the Cardinal Operating Company has also been developing alternative disposal capacity for these non-CCR waste streams. Specifically, a new leachate collection system is being developed to treat the leachate and contact storm water run-off from FAR I Landfill. In essence, the Cardinal Operating Company has opted to replace FAR II with a multiple technology system that consists of converting the station's wet fly ash-handling system to a dry system and installing a new waste water treatment system for FAR I Landfill's leachate and contact storm water run-off.

1.3.3.3 JUSTIFICATION OF OPTION SELECTED

Of the new, permanent on-site disposal alternatives considered to replace FAR II, the multiple technology system selected – convert Cardinal's wet fly ash-handling system to a dry system to utilize FAR I Landfill and install a waste water treatment facility for FAR I Landfill's leachate and contact storm water run-off – is the alternative disposal capacity that could be implemented the fastest and is technically feasible. In addition, both components ultimately provide the Cardinal station with a holistic solution for compliance with the EPA CCR and ELG Rules. Both systems provide alternative disposal capacity for the various waste streams currently managed by FAR II (EPA CCR Rule compliance), and the dry fly ash system eliminates the generation of FATW at the station and therefore inherently eliminates the future discharge of FATW from the Cardinal plant (EPA ELG Rule compliance).

As discussed in their respective summaries, geotextile filter tubes and concrete settling tanks would have operational risks during inclement weather (especially during the winter) and are not proven technologies for handling a power plant's daily generation of fly ash. And while a new ash pond could be constructed on undeveloped land on Cardinal's property, the hilly terrain and distance from the plant would require just over three years to design, permit, and construct the FATW piping to and from the impoundment, the composite liner system, the dams necessary to form a reservoir, and the well network for groundwater monitoring (including establishing background levels). Conversely, a vacuum-pneumatic conveying system requires less time to construct at the Cardinal plant (just over two years) and is a proven dry fly ash-handling technology that is widely utilized in the power industry.

1.4 CONCEPTUAL DESIGN OF ALTERNATIVE DISPOSAL CAPACITY

This section describes the conceptual designs for Cardinal's dry fly ash conversion and the waste water treatment for FAR I Landfill's leachate and contact storm water run-off.

1.4.1 DRY FLY ASH CONVERSION

1.4.1.1 CONCEPTUAL DESIGN OF VACUUM-PNEUMATIC CONVEYING SYSTEM

In general, the new vacuum-pneumatic system for each unit will utilize air flow and conveying pipes to transfer fly ash from the units' ESPs to filter separators, which will separate the conveying air from the fly ash. The fly ash will then be discharged into a silo for temporary storage until the material is loaded onto trucks for either on-site disposal at Cardinal's existing landfill or beneficial re-use.

The fly ash vacuum-pneumatic conveying system for each unit will utilize the existing branch line pipes and the ash feed valves located under the two precipitators. New conveying pipes will be installed between each unit's fly ash collection hoppers (one pipe per precipitator casing) to a new storage silo dedicated to each unit. A crossover pipe downstream of the precipitator will allow fly ash to be conveyed to the given unit's

storage silo by either conveying pipe. New vacuum exhausters will be installed to provide the conveying vacuum to the storage silos and will be cross-tied to allow the station to switch between vacuum conveying trains or operating filter separators. Fly ash will be separated from the conveying air via one of two filter separators installed on top of each storage silo.

To empty each silo, the fly ash will be aerated by fluidizing blowers and air heaters at the bottom of the silo. The discharge of the silo will be provided with a water-based ash conditioner (i.e. pin mixer) for loading trucks hauling the fly ash to Cardinal's existing landfill for disposal. The plant's existing service water system will supply the water to the pin mixers on each silo. Each silo's discharge will also be equipped with a telescoping spout for loading dry ash into enclosed truck trailers.

A new transformer and motor control center (MCC) will be installed to power the auxiliary equipment for each unit's vacuum-pneumatic conveying system. Each unit's MCC will be installed within a new power distribution center (PDC) near the given unit's vacuum exhausters and storage silo. Finally, the existing fly ash system programmable logic controls (PLCs) will be migrated to the station's distributed control system (DCS), and new DCS controllers and input/output (I/O) hardware will be added where needed for the new fly ash system and integrated into the existing station DCS.

1.4.1.2 IMPACTS TO STATION WATER BALANCE

Historically, Cardinal has discharged FATW and non-CCR waste streams sent to FAR II through NPDES-permitted Outfall 019. As previously mentioned, contact storm water run-off and leachate from FAR I Landfill also drain to FAR II. Upon revising the station's NPDES permit in accordance with the revised ELG Rule (see Section 1.1.2), these streams would be considered FATW in accordance with 40 CFR 423.13(h)(1)(i) and thus could not be discharged to waters of the U.S. Accordingly, the new vacuum-pneumatic conveying system eliminates FATW sent to and discharged from FAR II. In addition, Cardinal Operating Company is in the process of designing and permitting a new waste water treatment system for the contact storm water run-off and leachate from FAR I Landfill such that these waste streams will no longer be sent to FAR II.

1.4.2 TREATMENT FOR FAR I LANDFILL WASTE STREAMS

In addition to treating Cardinal's FATW, FAR II also serves as the treatment pond for FAR I Landfill's leachate and contact storm water run-off prior to discharge to the Ohio River via NPDES-permitted Outfall 019. Given its size, FAR II provides adequate detention time to treat the TSS in the landfill's leachate and contact storm water run-off via sedimentation. Therefore, the replacement disposal capacity for these waste streams would need to provide similar treatment. Indeed, the Cardinal Operating Company is currently developing a waste water treatment system comprised of settling and treatment tanks to handle and treat FAR I Landfill's leachate and contact storm water run-off prior to discharge through Outfall 019.

The Cardinal Operating Company plans to install the settling and treatment tanks at the FAR I Landfill. New piping will convey the landfill's leachate and contact storm water run-off to these tanks by tying into the existing piping currently conveying these waste streams into FAR II. New piping will also be installed to convey treated effluent from the tanks to the existing Outfall 019. These tanks will be adequately sized to promote settling of the TSS in both waste streams prior to NPDES-permitted concentrations.

1.5 EXPLANATION & JUSTIFICATION OF TIME REQUESTED.

Per the visual timeline representation and narrative discussion of the project schedule presented in Sections 2.0 and 3.0, respectively, the Cardinal Operating Company is requesting that the EPA allow FAR II to continue operating until June 7, 2021, when the vacuum-pneumatic conveying systems for all three units will be operational. During this period, the following CCR and non-CCR waste streams would be placed into FAR II since they do not currently have alternative disposal options at Cardinal or offsite:

- Unit 1 and 2 FATW,
- Unit 3 FATW,
- Excess process water from the BAP Complex,
- FAR I Landfill leachate, and
- FAR I Landfill contact storm water run-off.

The Cardinal Operating Company is requesting this additional time to continue operating FAR II not only because of the time required to develop the selected alternative disposal capacity, but also because of the time required to perform the preliminary engineering for the project and the time required to secure project funding from the electric cooperatives for which it serves. These items are discussed in the following paragraphs. A detailed explanation and justification for the time required to convert Cardinal's wet fly ash-handling system to a dry system and to install the waste water treatment system for FAR I Landfill's leachate and contact storm water run-off, starting with the engineering and design phase, are provided in the narrative of the project schedule in Section 3.0.

Finally, pursuant to the recently-revised alternative closure requirements in the EPA CCR Rule, the Cardinal Operating Company also evaluated whether temporary storage could be provided for the preceding CCR and non-CCR waste streams that will be sent to FAR II until the new vacuum-pneumatic conveying systems are operational. This evaluation is summarized at the end of this section.

1.5.1.1 PLANNING & INITIAL DESIGN

As previously stated, the Cardinal Operating Company has been actively evaluating different means of handling and disposing of its fly ash since even before the *USWAG* decision in August of 2018. Shortly after completing the aforementioned conceptual assessment of different fly ash-handling technologies in late 2018, the Cardinal Operating Company opted to convert Cardinal's wet fly ash-handling system to a dry

system and initiated a more detailed study for the new vacuum-pneumatic fly ash conveying system required to do so. This preliminary engineering phase included developing conceptual design drawings such as general arrangements, electrical one-lines, and process flow diagrams; updating the station's water balance to reflect the conversion to dry fly ash handling; and estimating the capital and operation and maintenance (O&M) costs for the dry system. This study was completed in late 2018.

1.5.1.2 PROJECT FUNDING & INITIATION

The capital and O&M cost estimates developed during the dry fly ash conversion study were ultimately used to obtain the necessary funding for the project. The dry fly ash conversion project at Cardinal could not commence until the appropriate funds were approved and allocated.

In general, funding for environmental compliance projects is not approved until the corresponding environmental regulations are finalized. While this project addresses revisions to the EPA CCR Rule in response to the August 2018 *USWAG* decision (and subsequent October 2018 mandate by the U.S. Court of Appeals for the D.C. Circuit) (Ref. 2), this project also provides operational changes required to comply with the EPA ELG Rule regulations for FATW. Therefore, funding for the entire project was able to be approved prior to the EPA finalizing its updates to its CCR Rule in response to the aforementioned court mandate. However, it was not possible to start the project sooner on the basis of forecasted changes to the EPA CCR Rule due to the October 2018 court mandate given the project approval process utilized by Cardinal Operating Company.

Ultimately, the Cardinal Operating Company secured the funding required to initiate the dry fly ash conversion project shortly after finalizing the detailed study in late 2018. Detailed engineering and design commenced in March 2019 and construction is currently ongoing at all three units of the station. See Sections 2.0 and 3.0 for the visual timeline representation and narrative discussion of the project schedule, respectively. For the progress made to date on the dry fly ash conversion project, see Section 4.0.

1.5.1.3 TEMPORARY DISPOSAL OF WASTE STREAMS

The Cardinal Operating Company considered two temporary disposal solutions for the CCR and non-CCR waste streams that will continue to be sent to FAR II until the new vacuum-pneumatic conveying systems and landfill leachate treatment system are operational at all three units on June 7, 2021: tanks and water treatment trailers.

1.5.1.3.1 STORAGE TANKS

Based on the Cardinal Operating Company's current forecast of obtaining permanent alternative disposal capacity to replace FAR II, enough tanks would need to be procured and installed at the site to provide storage of waste water produced by the plant for approximately eight months. Given an average daily inflow

of 9.8 MGD into FAR II for Cardinal's FATW, FAR I Landfill leachate, and contact storm water run-off from FAR I Landfill (see Table 1), these temporary tanks would need to provide almost 2.4 billion gallons-worth of storage. It is not technically feasible to install this many tanks at the Cardinal site to provide temporary storage.

Less storage capacity would be required if the tank contents could be regularly discharged or recirculated, but the tanks would need to be large enough to promote sedimentation of the TSS in the waste streams. Given that fly ash particles are generally very fine and given the daily volume of FATW produced at Cardinal, this is not a technically feasible option for Cardinal's FATW. The number and size of these tanks could be controlled if the waste could be transported off-site, but the logistics required for off-site transport, even if off-site disposal capacity was available, also make this temporary solution technically infeasible (see Section 1.3.2).

As previously stated, this option is being implemented as the permanent alternative disposal solution for the leachate and contact storm water run-off from FAR I Landfill. However, the leachate treatment system will not be operational until June 7, 2021 given the time required to design, permit, and construct this system. The primary schedule driver is modifying the station's NPDES permit for this alternative treatment system for these waste streams, which Cardinal anticipates taking six months based on recent experience with similar PTIs submitted to the Ohio EPA.

1.5.1.3.2 WASTE WATER TREATMENT TRAILERS

While it is technically infeasible to use tanks to temporarily treat the large flow of FATW currently going into FAR II, waste water treatment trailers from a vendor that specializes in such technology may be capable of treating Cardinal's FATW. The amount of waste water a trailer can treat is dependent on water chemistry, but 1 MGD is generally achievable. So, it would take approximately nine trailers to treat Cardinal's daily average generation of FATW (8.93 MGD). Despite its ability to treat this large flow, waste water treatment trailers are not appropriate for waste streams with restricted discharge rates since a trailer would not have the size required to detain treated waste water. Thus, temporary storage tanks would still need to be installed downstream of these trailers to detain treated FATW which, per the previous subsection, is not technically feasible at Cardinal. Consequently, a network of waste water treatment trailers is also a technically infeasible solution for providing temporary storage capacity for Cardinal's FATW in lieu of FAR II while the station is being retrofitted with a dry fly ash-handling system.

2.0 PROJECT SCHEDULE: VISUAL TIMELINE

This section presents a visual timeline representation of the Cardinal Operating Company's schedule for converting Cardinal's current wet fly ash-handling system to a dry system. Pursuant to 40 CFR 257.103(f)(iv)(1)(A)(2), the following visual timeline representation of the project schedule shows:

- How each phase and the steps within that phase interact with or are dependent on each other and the other phases,
- All of the steps and phases that can be completed concurrently,
- The total time needed to convert Cardinal's wet fly ash-handling system to a dry system, and
- How long each phase and step within each phase will take.

As shown in its visual timeline representation, the project schedule is divided into the following phases:

- · Engineering & Design,
- Permitting,
- · Procurement, and
- Construction.

In accordance with 40 CFR 257.103(f)(iv)(1)(A)(2)(iv), the project schedule includes the following sub-phases in the Procurement and Construction phases:

Procurement:

- Fabrication and delivery of piles, concrete (including rebar), structural steel, the fly ash system, a pre-fabricated PDC building, and transformers.
- Selection of a contractor to install the piles supporting the base mats for the new fly ash storage silos.
- Selection of a contractor to install civil works (e.g., roads, site grading) and substructures,
- Selection of a contractor to install the dry fly ash system and its ancillary components (i.e., general work contractor).
- Selection of a contractor to install the electrical components.

Construction:

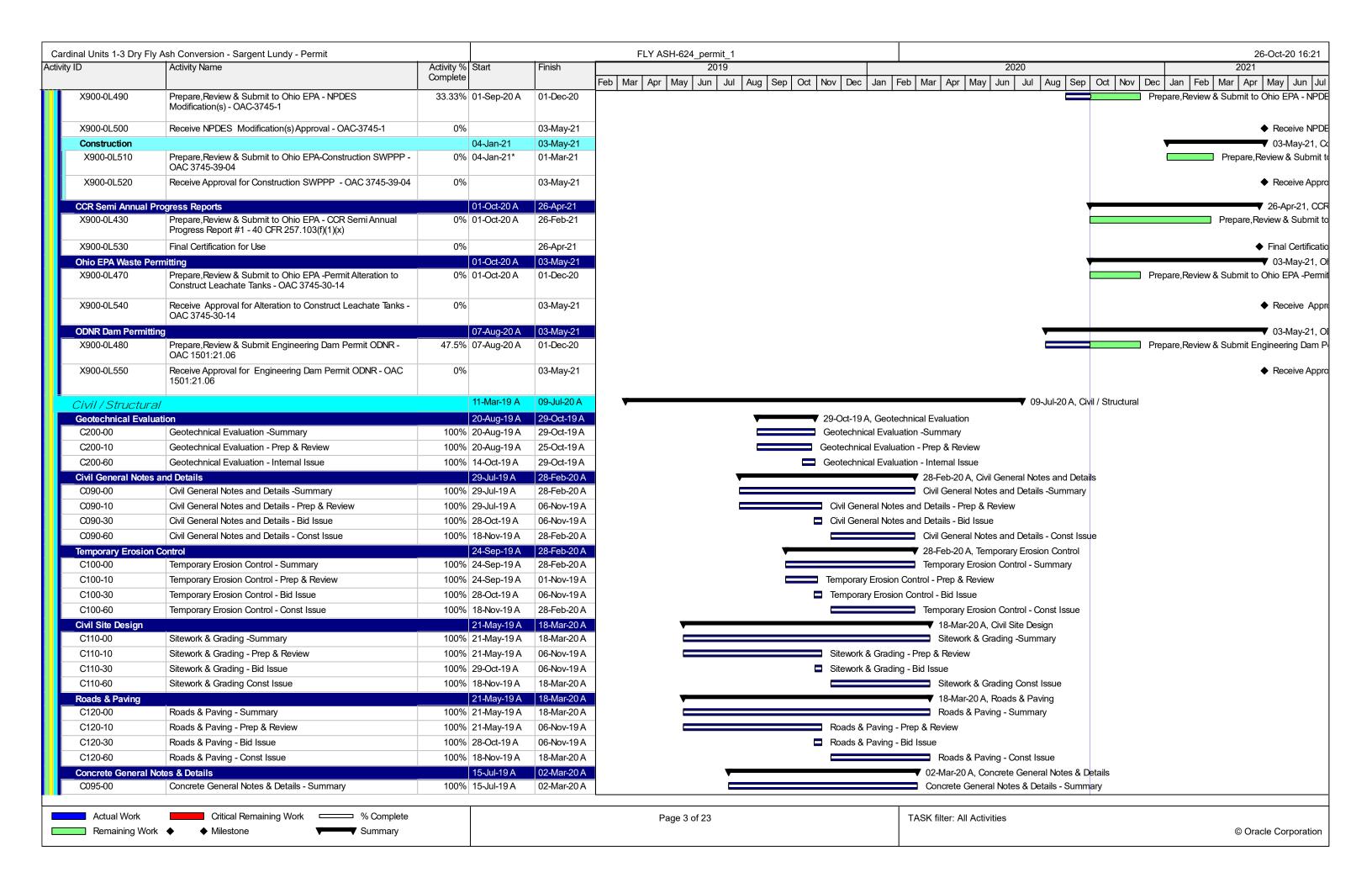
- Commissioning (i.e., start-up and implementation) of the Unit 1, 2, and 3 dry fly ash systems and components, and
- Substantial completion (i.e., tuning and optimization) of the Unit 1, 2, and 3 dry fly ash systems and components.

See Section 3.0 for the corresponding narrative discussion of the project schedule.

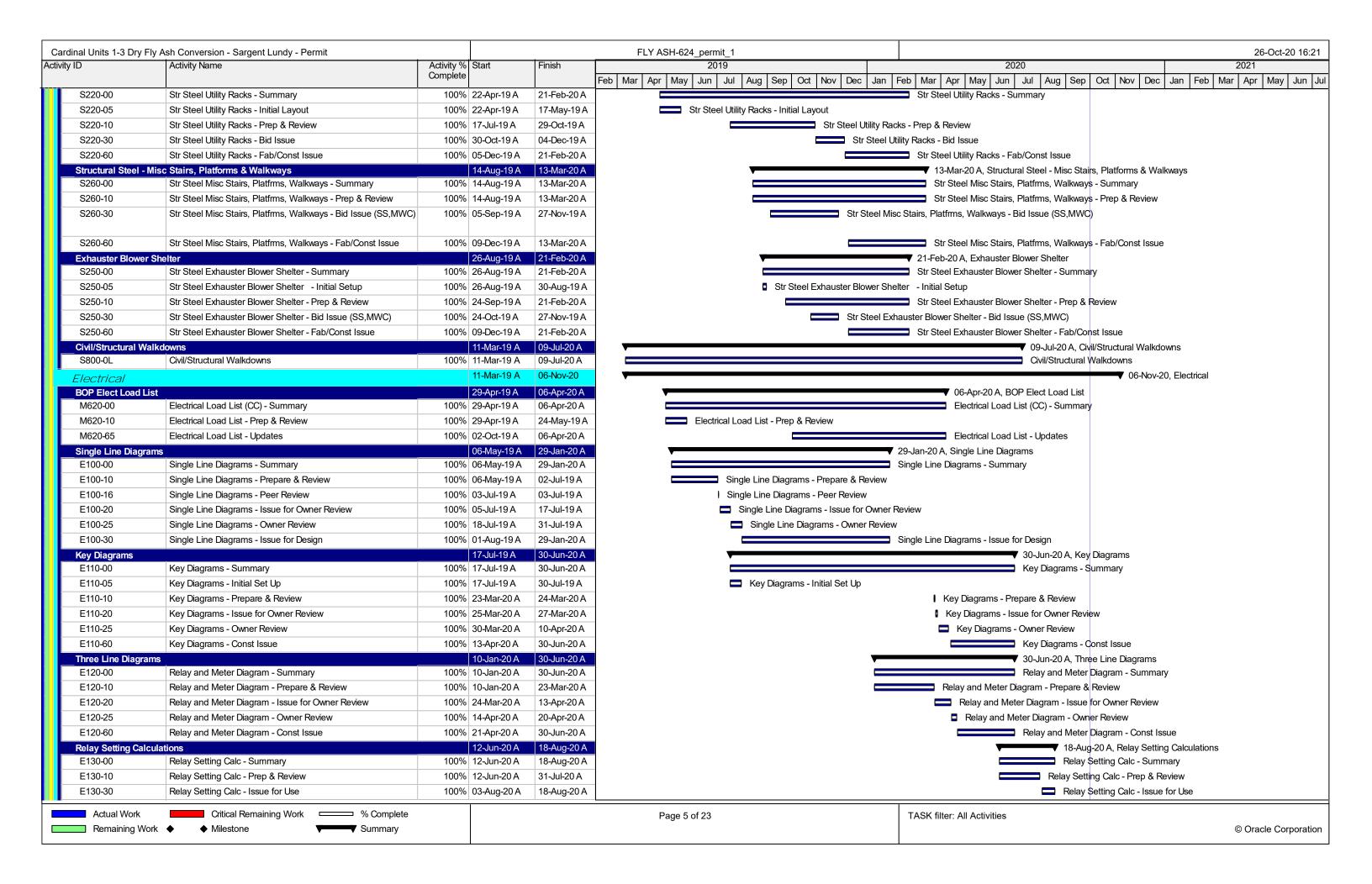
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G130-00	Cost Estimate - Summary	100% 11-Mar-19 /		Cost Estimate - Summary		
G130-10	Update Cost Estimate - Prep & Review	100% 11-Mar-19 /		Update Cost Estimate - Prep & Review		
G130-20	Cost Estimate - Issue for Use	100% 04-Jul-19 A		Cost Estimate - Issue for Use		
Permitting Suppo		24-Jun-19 A		22-Aug-19 A, Permitting Support		
G140-0L Permit List	Permitting Support	100% 24-Jun-19 <i>F</i>		Permitting Support 26-Jun-19 A, Permit List		
G142-0L	Develop Permit List - WBS Summary	100% 11-Jun-19 A		Develop Permit List - WBS Summary		
G142-10	Permit Application Prep & Review	100% 11-Jun-19 A		Permit Application Prep & Review		
G142-20	Permit Application - Issue to the Agency	100% 26-Jun-19 A		Permit Application - Issue to the Agency		
Air Emissions Pl		11-Mar-19 /		10-May-19 A, Air Emissions Plan		
G144-00	Air Emissions Plan	100% 11-Mar-19 /		Air Emissions Plan		
G144-10	Air Emissions Plan - Prep & Review	100% 11-Mar-19 A		Air Emissions Plan - Prep & Review		
G144-20	Air Emissions Plan - Issue for Use	100% 06-May-19		☐ Air Emissions Plan - Issue for Use		
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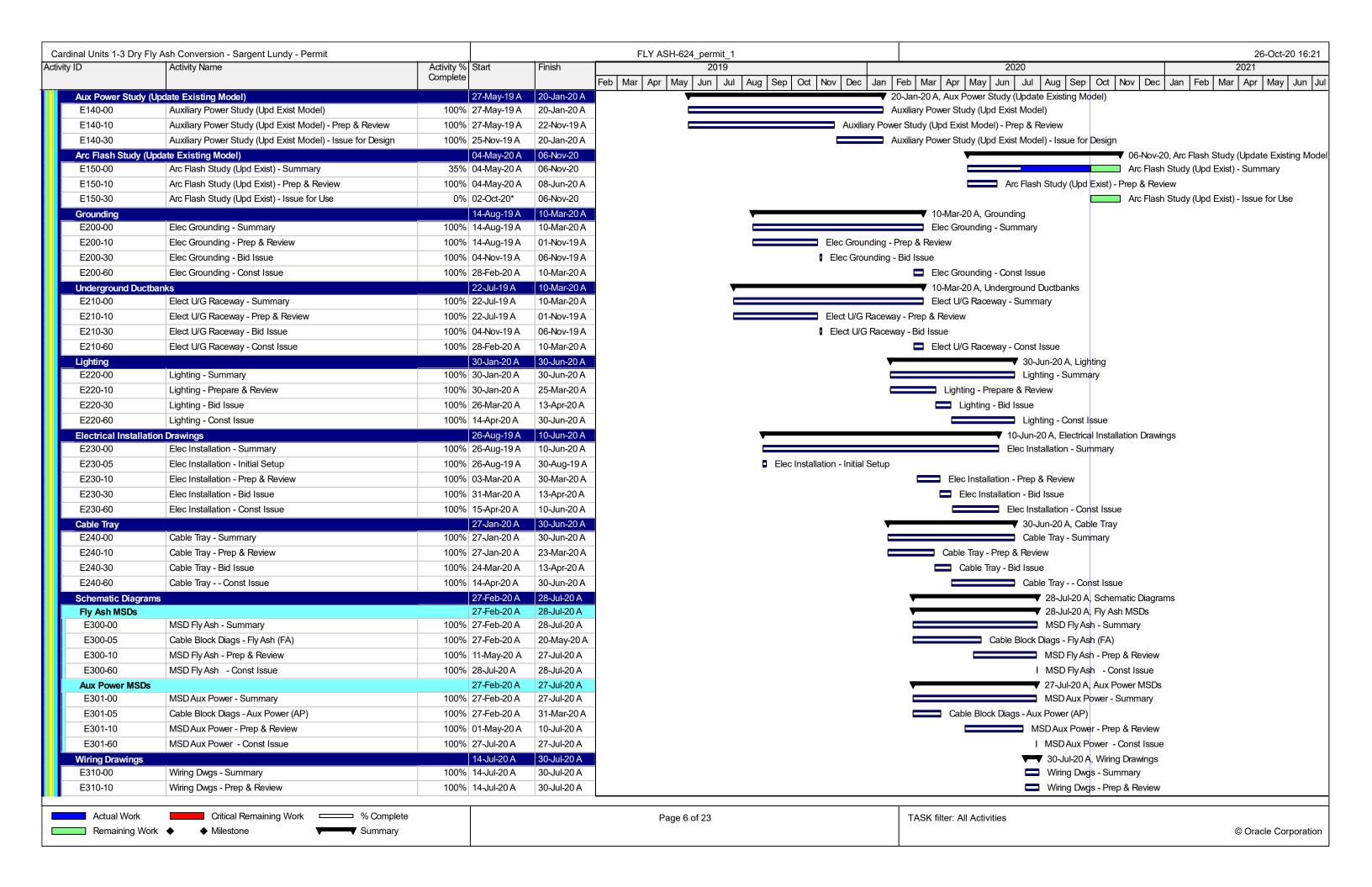
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Design Basis		11-Mar-19 A	29-Jul-19 A	▼ 29-Jul-19 A, Design Basis		
G100-00	Design Basis - Summary	100% 11-Mar-19 A	29-Jul-19 A	Design Basis - Summary		
G100-10	Design Basis - Prep & Review	100% 11-Mar-19 A	10-May-19 A	Design Basis - Prep & Review		
G100-20	Design Basis - Issue for Owner Review	100% 13-May-19 A	17-May-19 A	Design Basis - Issue for Owner Review		
G100-25	Design Basis - Owner Review	100% 20-May-19 A	12-Jul-19 A	Design Basis - Owner Review		
G100-30	Design Basis - Incorp Comms/Issue for Design	100% 15-Jul-19 A	29-Jul-19 A	Design Basis - Incorp Comms/Issue for Design		
Model		11-Mar-19 <i>A</i>		▼ 10-Feb-20 A, Model		
G110-0L	Plant & Equipment Modeling	100% 11-Mar-19 A	10-Feb-20 A	Plant & Equipment Modeling		
	ocation/Redesign					
Cost Estimate	0.45% 4.0	11-Mar-19 A		08-Jul-19 A, Cost Estimate		
G130-00	Cost Estimate - Summary	100% 11-Mar-19 A		Cost Estimate - Summary		
G130-10	Update Cost Estimate - Prep & Review	100% 11-Mar-19 A		Update Cost Estimate - Prep & Review		
G130-20	Cost Estimate - Issue for Use	100% 04-Jul-19 A	08-Jul-19 A	Cost Estimate - Issue for Use		
Permitting Suppo G140-0L	Permitting Support	24-Jun-19 A 100% 24-Jun-19 A		22-Aug-19 A, Permitting Support Permitting Support		
Permit List	Permitting Support	11-Jun-19 A		✓ 26-Jun-19 A, Permit List		
G142-0L	Develop Permit List - WBS Summary	100% 11-Jun-19 A		Develop Permit List - WBS Summary		
G142-10	Permit Application Prep & Review	100% 11-Jun-19 A		Permit Application Prep & Review		
G142-20	Permit Application - Issue to the Agency	100% 26-Jun-19 A		Permit Application - Issue to the Agency		
Air Emissions Pl		11-Mar-19 A		10-May-19 A, Air Emissions Plan		
G144-00	Air Emissions Plan	100% 11-Mar-19 A		Air Emissions Plan		
G144-10	Air Emissions Plan - Prep & Review	100% 11-Mar-19 A		Air Emissions Plan - Prep & Review		
G144-20	Air Emissions Plan - Issue for Use	100% 06-May-19 A		. □ Air Emissions Plan - Issue for Use		
Air Emissions Ca	alculatiion	13-May-19 A	-	▼ 21-Jun-19 A, Air Emissions Calculatiion		
G146-00	Air Emissions Calculatiion	100% 13-May-19 A	21-Jun-19 A	Air Emissions Calculatiion		
G146-10	Air Emissions Calculatiion - Prep & Review	100% 13-May-19 A	21-Jun-19 A	Air Emissions Calculatiion - Prep & Review		
G146-20	Air Emissions Calculatiion - Issue for Use	100% 21-Jun-19 A	21-Jun-19 A	Air Emissions Calculatiion - Issue for Use		
			00.1			
Dwg Closeout/Fil X800-0L	Dwg Closeout/File Return	11-May-21 0% 11-May-21	08-Jun-21 08-Jun-21			
7000-0F	Dwg Goseout/File Retuil1	070 11-iviay-21	00-JUH-2 I			
Home Office Con	struction Support	17-Feb-20 A	10-May-21	▼	▼ 10-M	
X900-0L	Home Office Construction Support	51.12% 17-Feb-20 A	10-May-21		Home	
Actual Work	Critical Remaining Work			Page 1 of 23 TASK filter: All Activities		
	Vork ♦ Milestone ▼ Summary			170K met. All Activities	© Oracle Corpo	

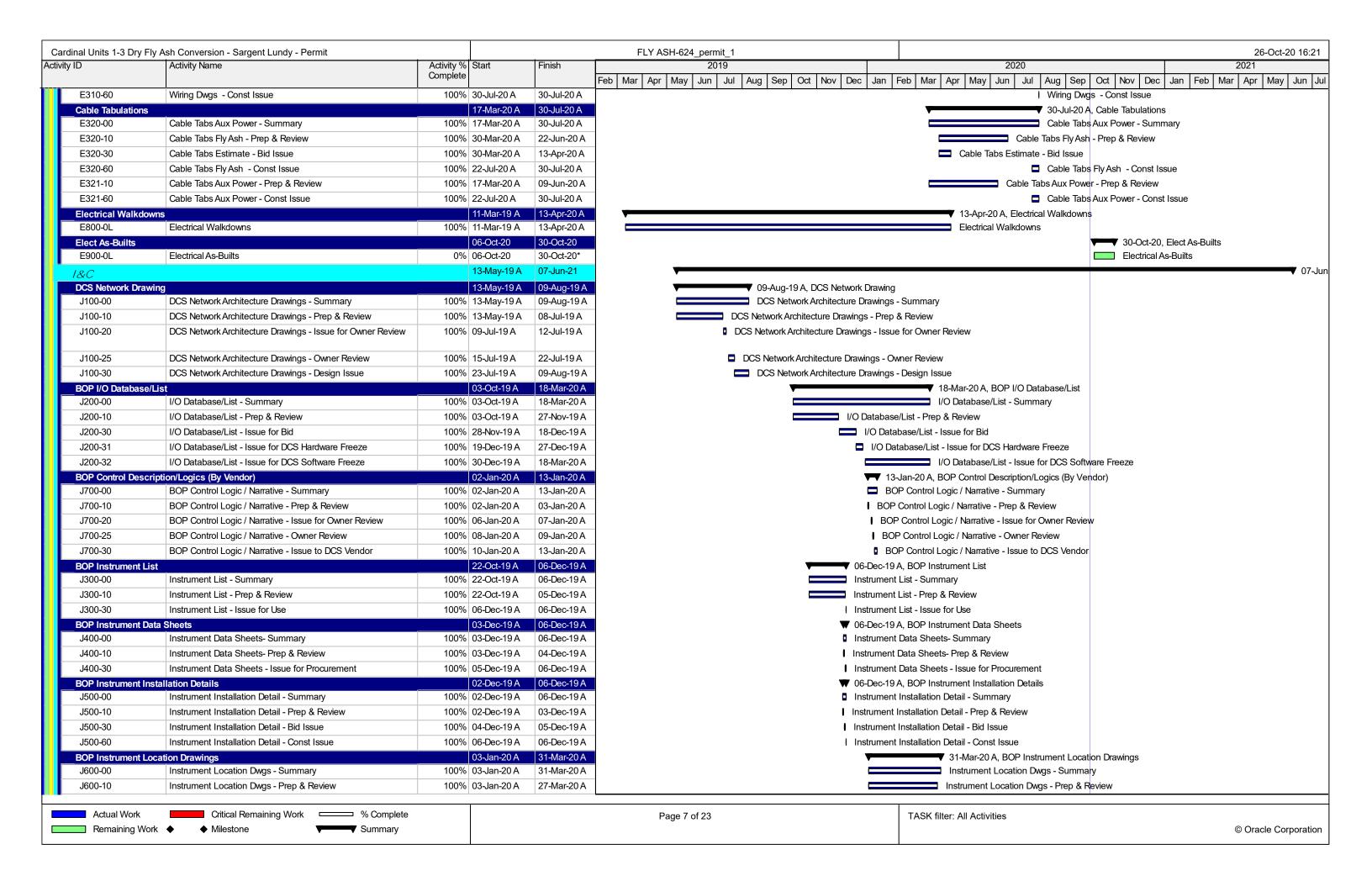
<u>·</u>	ly Ash Conversion - Sargent Lundy - Permit			FLY ASH-624_permit_1	26-Oct-20 1
D	Activity Name	Activity % Start Complete	Finish	2019 2020	2021
		·		eb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Jun Jul Aug S	Sep Oct Nov Dec Jan Feb Mar Apr May Ju
andfill Leach	nate	08-Sep-20 A	20-May-21	· · · · · · · · · · · · · · · · · · ·	▼ 20-1
Leachate Treatme	nt System - Phase 1	08-Sep-20 A	21-Jan-21	•	▼ 21-Jan-21, Leachate Treatme
X900-0L085	Kick Off Meeting	100% 09-Sep-20 A	09-Sep-20 A	· · · · · · · · · · · · · · · · · · ·	Kick Off Meeting
X900-0L090	Perform Site Visit	0% 14-Oct-20	16-Oct-20		Perform Site Visit
X900-0L100	Leachate Tank/Pump Information (GA,Location, Foundation design etc.)	0% 19-Oct-20	20-Oct-20		Leachate Tank/Pump Information (GA,Location,
X900-0L110	Develop & Finalize P&ID	0% 05-Oct-20	23-Oct-20		Develop & Finalize P&ID
X900-0L120	Leachate Management System Geotech Studies/Survey/Review & Comment Existing Data	0% 12-Oct-20	26-Oct-20		Leachate Management System Geotech Studi
X900-0L130	Permit Drawings Sealed & Provide for NPDES PIT Submittal to Ohio EPA	0% 26-Oct-20	30-Oct-20		Permit Drawings Sealed & Provide for NPDES
X900-0L140	Owner Design Reveiw of Leachate Tank Design	0% 29-Oct-20	30-Oct-20		Owner Design Reveiw of Leachate Tank Design
X900-0L150	Develop Leachate Tank Issue For Bid Pacakge	0% 12-Oct-20	06-Nov-20		Develop Leachate Tank Issue For Bid Pacak
X900-0L160	Develop and Finalize Pipe Routing	0% 26-Oct-20	13-Nov-20		Develop and Finalize Pipe Routing
X900-0L170	Develop & Finalize Sump Details	0% 26-Oct-20	13-Nov-20		Develop & Finalize Sump Details
X900-0L170	Owner Design Review of CDOX Design	0% 12-Nov-20	13-Nov-20		Owner Design Review of CDOX Design
X900-0L190	Develop CDOX Issue for BID Package	0% 02-Nov-20	20-Nov-20		Develop CDOX Issue for BID Package
X900-0L200	Owner Design Review of Mechanical Design	0% 20-Nov-20	23-Nov-20		Owner Design Review of Mechanical Des
X900-0L210	Develop Mechanical Issue for BID Package	0% 12-Oct-20	30-Nov-20		Develop Mechanical Issue for BID Pack
X900-0L220	PTI Application Support	25.71% 08-Sep-20 A	15-Dec-20	E .	PTI Application Support
X900-0L230	Develop Leachate Tank IFC Package	0% 18-Nov-20	21-Jan-21	·	Develop Leachate Tank IFC
X900-0L240	Develop Mechanical IFC Package	0% 04-Nov-20	21-Jan-21		Develop Mechanical IFC Pag
Landfill Leachate I		09-Nov-20	29-Apr-21		▼ 29-Apr-
X900-0L250	Bid, Evaluate & Award Mechanical Scope (Includes Pipe, Pump, & Accessories)	0% 09-Nov-20	07-Jan-21		Bid, Evaluate & Award Mechan
X900-0L310	Bid, Evaluate & Award Leachate Tanks	0% 09-Nov-20	07-Jan-21		Bid, Evaluate & Award Leachate
X900-0L320	Bid, Evaluate & Award Site Prep Scope	0% 09-Nov-20	07-Jan-21		Bid, Evaluate & Award Site Prep
X900-0L330	Bid, Evaluate & Award CDOX System	0% 23-Nov-20	21-Jan-21		Bid, Evaluate & Award CDO
X900-0L340	Pipe Fabrication and Delivery	0% 08-Jan-21	01-Apr-21		Pipe Fabricati
X900-0L350	Pump Fabrication & Delivery	0% 08-Jan-21	01-Apr-21		Pump Fabrica
X900-0L360	CDOX System Fabrication & Delivery	0% 22-Jan-21	29-Apr-21		CDOX S
Landfill Leachate	Construction	01-Feb-21	20-May-21		▼ 20-
X900-0L300	Site Prep Scope (including Foundations)	0% 01-Feb-21	26-Feb-21		Site Prep Scope (incl
X900-0L370	Tank Installation	0% 15-Feb-21	07-May-21		Tank In
X900-0L380	Reroute Mechnical Construction & Commissioning	0% 26-Feb-21	13-May-21		Reroi
X900-0L390	CDOX System & Installation & Commissioning	0% 30-Apr-21	20-May-21		CDC
Environmentai	I Permitting LF/FAR II	01-Jun-19 A	03-May-21		▼ 03-May
Ohio EPA Surface	Water Permitting	01-Jun-19 A	03-May-21		▼ 03-May
X900-0L400	Prepare,Review & Submit to Ohio EPA-PTI for Closure of FAR II - OAC 3745-42-03	100% 01-Jun-19 A	28-Oct-19 A	Prepare,Review & Submit to Ohio EPA-PTI for Closure of FAR II	- OAC 3745-42-03
X900-0L410	Receive PTI Approval - OAC 3745-42-03	100%	21-Feb-20 A	◆ Receive PTI Approval - OAC 3745-42-03	3
X900-0L420	Prepare,Review & Submit to Ohio EPA - PTI for Leachate Conveyance - OAC 3745-42-03	0% 01-Oct-20 A	01-Dec-20		Prepare,Review & Submit to Ohio EPA
X900-0L460	Receive Leachate Conveyance Approval - OAC 3745-42-03	0%	03-May-21		◆ Receiv
Actual Work	Critical Remaining Work			Page 2 of 23 TASK filter: All Activities	

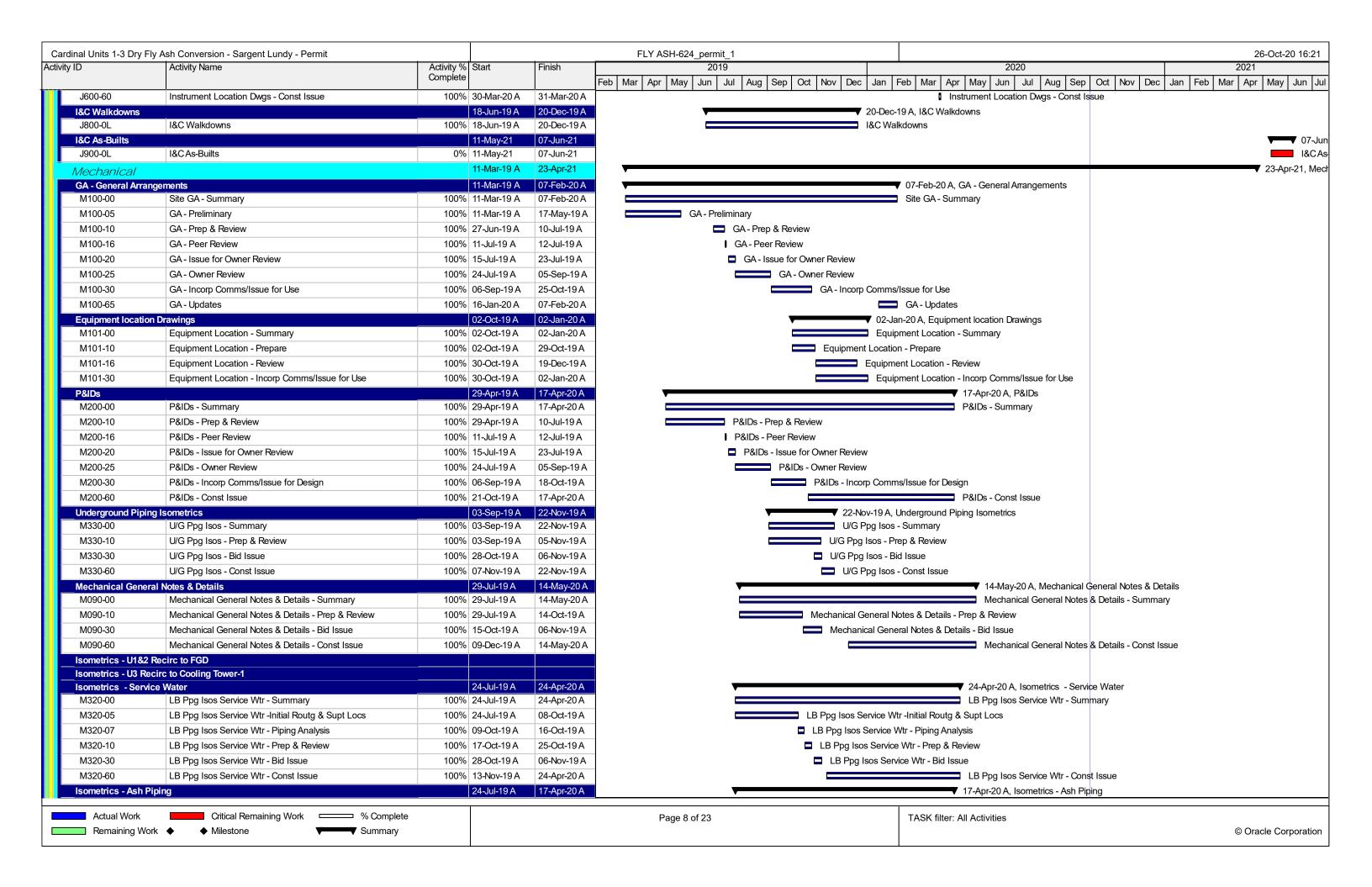


ID	ly Ash Conversion - Sargent Lundy - Permit Activity Name	Activity % Start	Finish	2019	2020 2021
D	Activity Name	Complete	I II IISI		Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May
C095-10	Concrete General Notes & Details - Prep & Review	100% 15-Jul-19	A 02-Oct-19 A		es & Details - Prep & Review
C095-30	Concrete General Notes & Details - Bid Issue (Substr)	100% 03-Oct-1			General Notes & Details - Bid Issue (Substr)
C095-31	Concrete General Notes & Details - Bid Issue (Pile)	100% 04-Oct-1			al Notes & Details - Bid Issue (Pile)
C095-60	Concrete General Notes & Details - Const Issue	100% 05-Dec-1			Concrete General Notes & Details - Const Issue
Silo Foundations		26-Aug-1			▼ 13-Mar-20 A, Silo Foundations
S100-00	Silo Fdn - Summary	100% 26-Aug-1		<u></u>	Silo Fdn - Summary
S100-05	Silo Fdn - Initial Setup	100% 26-Aug-1		□ Silo Fdn - Initial Setup	·
S100-10	Silo Fdn - Prep & Review	100% 17-Sep-1	_	Silo Fdn - Prep & R	Review
S100-30	Silo Fdn - Bid Issue (Substructure)	100% 22-Oct-1		_	- Bid Issue (Substructure)
S100-31	Silo Fdn - Bid Issue (Pile)	100% 22-Oct-1	01-Nov-19 A	□ Silo Fdn - Bid Iss	·
S100-60	Silo Fdn - Const Issue (Pile)	100% 04-Nov-1			Silo Fdn - Const Issue (Pile)
S100-61	Silo Fdn - Const Issue (Substructure)	100% 04-Nov-1			Silo Fdn - Const Issue (Substructure)
PDC & Transforme	,	30-Sep-1			02-Mar-20 A. PDC & Transformers Foundations
S110-00	PDC & Transformers Fdn - Summary	100% 30-Sep-1		<u> </u>	PDC & Transformers Fdn - Summary
S110-10	PDC & Transformers Fdn - Prep & Review	100% 30-Sep-1	9 A 20-Nov-19 A	PDC & Trans	sformers Fdn - Prep & Review
S110-30	PDC & Transformers Fdn - Bid Issue	100% 15-Oct-1			ansformers Fdn - Bid Issue
S110-60	PDC & Transformers Fdn - Const Issue	100% 05-Dec-1			PDC & Transformers Fdn - Const Issue
Itility Racks Foun	2	20-May-1			▼ 02-Mar-20 A. Utility Racks Foundations
S120-00	Utility Rack Fdn - Summary	100% 20-May-1			Utility Rack Fdn - Summary
S120-05	Utility Rack Fdn - Initial Layout	100% 20-May-1		Utility Rack Fdn - Initial Layout	
S120-10	Utility Rack Fdn - Prep & Review	100% 04-Sep-1		Utility Rack Fdn -	Prep & Review
S120-30	Utility Rack Fdn - Bid Issue (Substr)	100% 01-Nov-1		_	ck Fdn - Bid Issue (Substr)
S120-60	Utility Rack Fdn - Const Issue	100% 05-Dec-1			Utility Rack Fdn - Const Issue
	pad Crossing Foundation	06-May-1			13-Mar-20 A, U2 Pipe Trench Road Crossing Foundation
S130-00	U2 Pipe Trench Road Crossing Fdn - Summary	100% 06-May-1			U2 Pipe Trench Road Crossing Fdn - Summary
S130-05	U2 Pipe Trench Road Crossing Fdn - Initial Layout	100% 06-May-1	9 A 03-Jun-19 A	U2 Pipe Trench Road Crossing Fdn - Initial Layou	ut
S130-10	U2 Pipe Trench Road Crossing Fdn - Prep & Review	100% 19-Sep-1	9 A 25-Oct-19 A	U2 Pipe Trench Ro	oad Crossing Fdn - Prep & Review
S130-30	U2 Pipe Trench Road Crossing Fdn - Bid Issue	100% 28-Oct-1	04-Dec-19 A	U2 Pipe T	Trench Road Crossing Fdn - Bid Issue
S130-60	U2 Pipe Trench Road Crossing Fdn - Const Issue	100% 05-Dec-1	9 A 13-Mar-20 A		U2 Pipe Trench Road Crossing Fdn - Const Issue
/lisc Housekeepir	ng Pads & Pipe Supports Foundation	20-May-1		-	02-Mar-20 A, Misc Housekeeping Pads & Pipe Supports Foundation
S140-00	Misc Housekeeping Pads & Pipe Supports Fdn - Summary	100% 20-May-1			Misc Housekeeping Pads & Pipe Supports Fdn - Summary
S140-05	Misc Housekeeping Pads & Pipe Supports Fdn - Initial Layou	100% 20-May-1	9 A 17-Jun-19 A	Misc Housekeeping Pads & Pipe Supports Fo	dn - Initial Layout
S140-10	Misc Housekeeping Pads & Pipe Supports Fdn - Prep & Review	100% 26-Sep-1	9 A 23-Oct-19 A	Misc Housekeeping	ng Pads & Pipe Supports Fdn - Prep & Review
S140-30	Misc Housekeeping Pads & Pipe Supports Fdn - Bid Issue	100% 24-Oct-1	04-Dec-19 A	Misc House	sekeeping Pads & Pipe Supports Fdn - Bid Issue
S140-60	Misc Housekeeping Pads & Pipe Supports Fdn - Const Issue	100% 05-Dec-1	9 A 02-Mar-20 A		Misc Housekeeping Pads & Pipe Supports Fdn - Const Issue
xhauster Blower	Foundation	30-Sep-1	9 A 12-Mar-20 A	▼	12-Mar-20 A, Exhauster Blower Foundation
S150-00	Exhauster Blower Fdn - Summary	100% 30-Sep-1	9 A 12-Mar-20 A		Exhauster Blower Fdn - Summary
S150-10	Exhauster Blower Fdn - Prep & Review	100% 30-Sep-1	9 A 30-Oct-19 A	Exhauster Blower	er Fdn - Prep & Review
S150-30	Exhauster Blower Fdn - Bid Issue	100% 31-Oct-1	04-Dec-19 A	Exhauste	er Blower Fdn - Bid Issue
S150-60	Exhauster Blower Fdn - Const Issue	100% 05-Dec-1	9 A 12-Mar-20 A		Exhauster Blower Fdn - Const Issue
Structural Steel G	eneral Notes & Details	15-Jul-19	A 20-Feb-20 A	▼	20-Feb-20 A, Structural Steel General Notes & Details
S090-00	Structural Steel General Notes & Details - Summary	100% 15-Jul-19	A 20-Feb-20 A		Structural Steel General Notes & Details - Summary
S090-10	Structural Steel General Notes & Details - Prep & Review	100% 15-Jul-19	A 31-Oct-19 A	Structural Steel C	General Notes & Details - Prep & Review
S090-30	Structural Steel General Notes & Details - Bid Issue (SS,MWC)	100% 01-Nov-1	9 A 27-Nov-19 A	Structural S	Steel General Notes & Details - Bid Issue (SS,MWC)
S090-60	Structural Steel General Notes & Details - Fab/Const Issue	100% 28-Nov-1			Structural Steel General Notes & Details - Fab/Const Issue
Structural Steel - I	Utility Racks	22-Apr-1	9A 21-Feb-20 A	·	▼ 21-Feb-20 A, Structural Steel - Utility Racks
Actual Work	Critical Remaining Work				TASK filter: All Activities

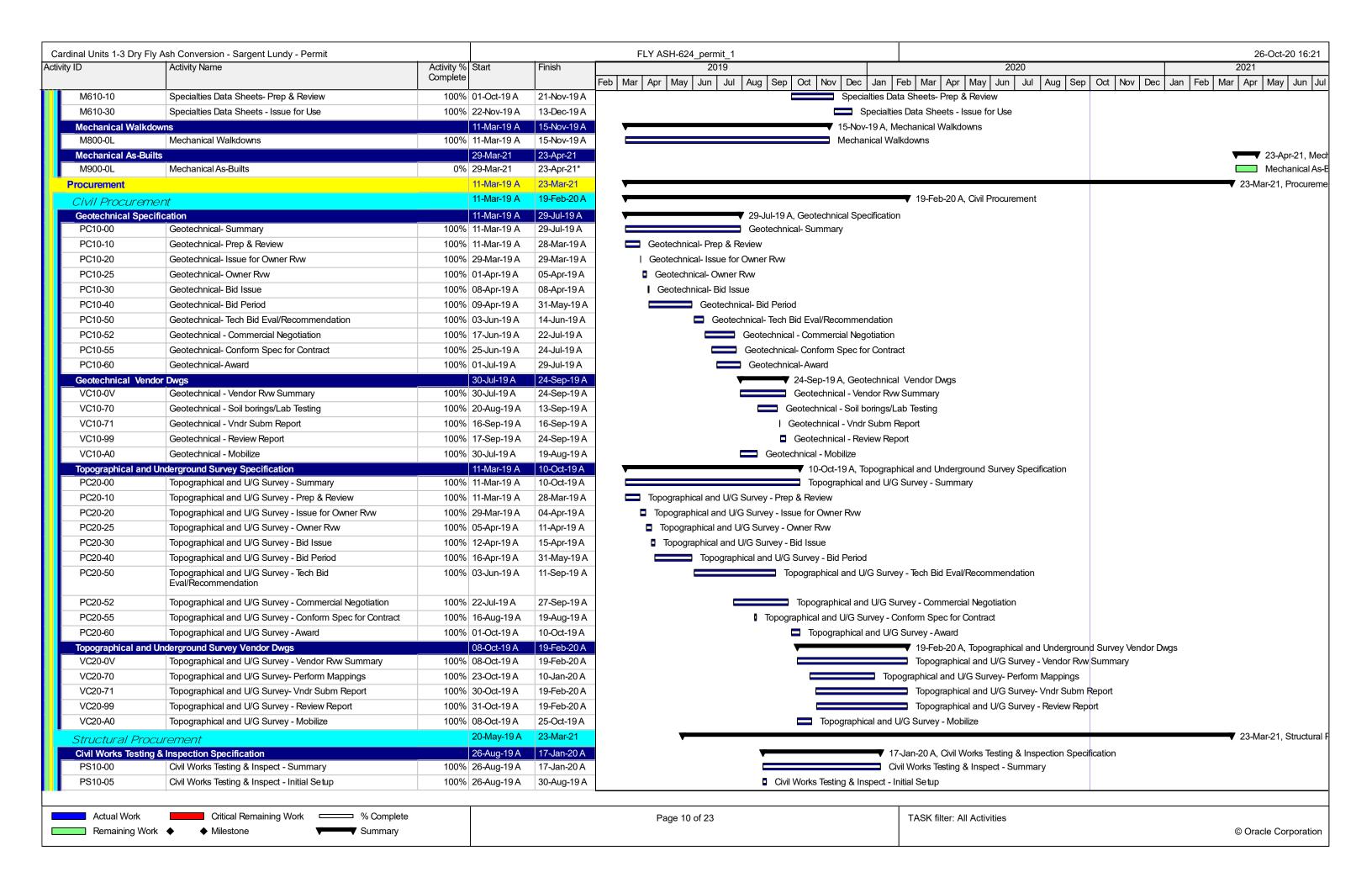


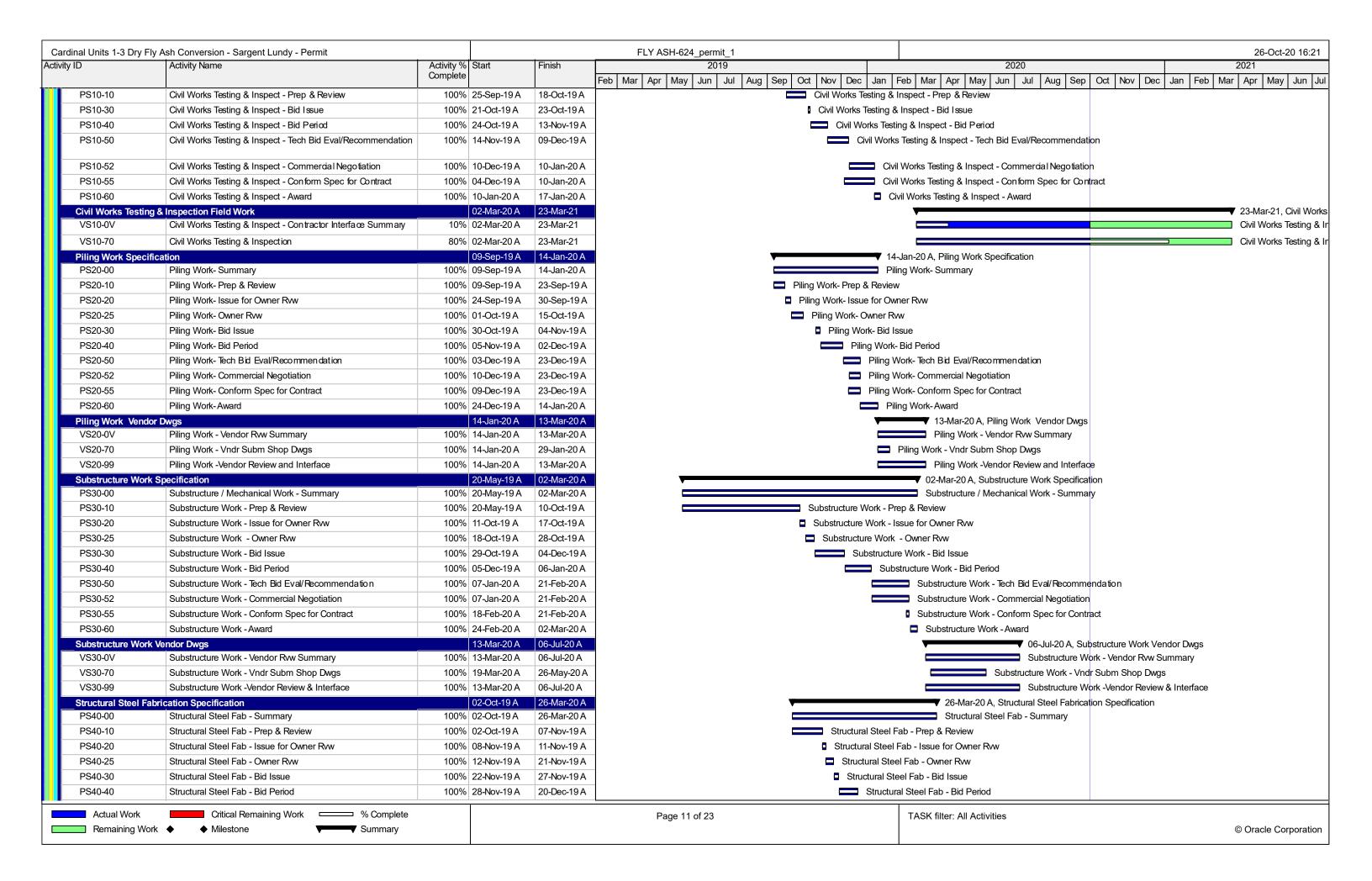


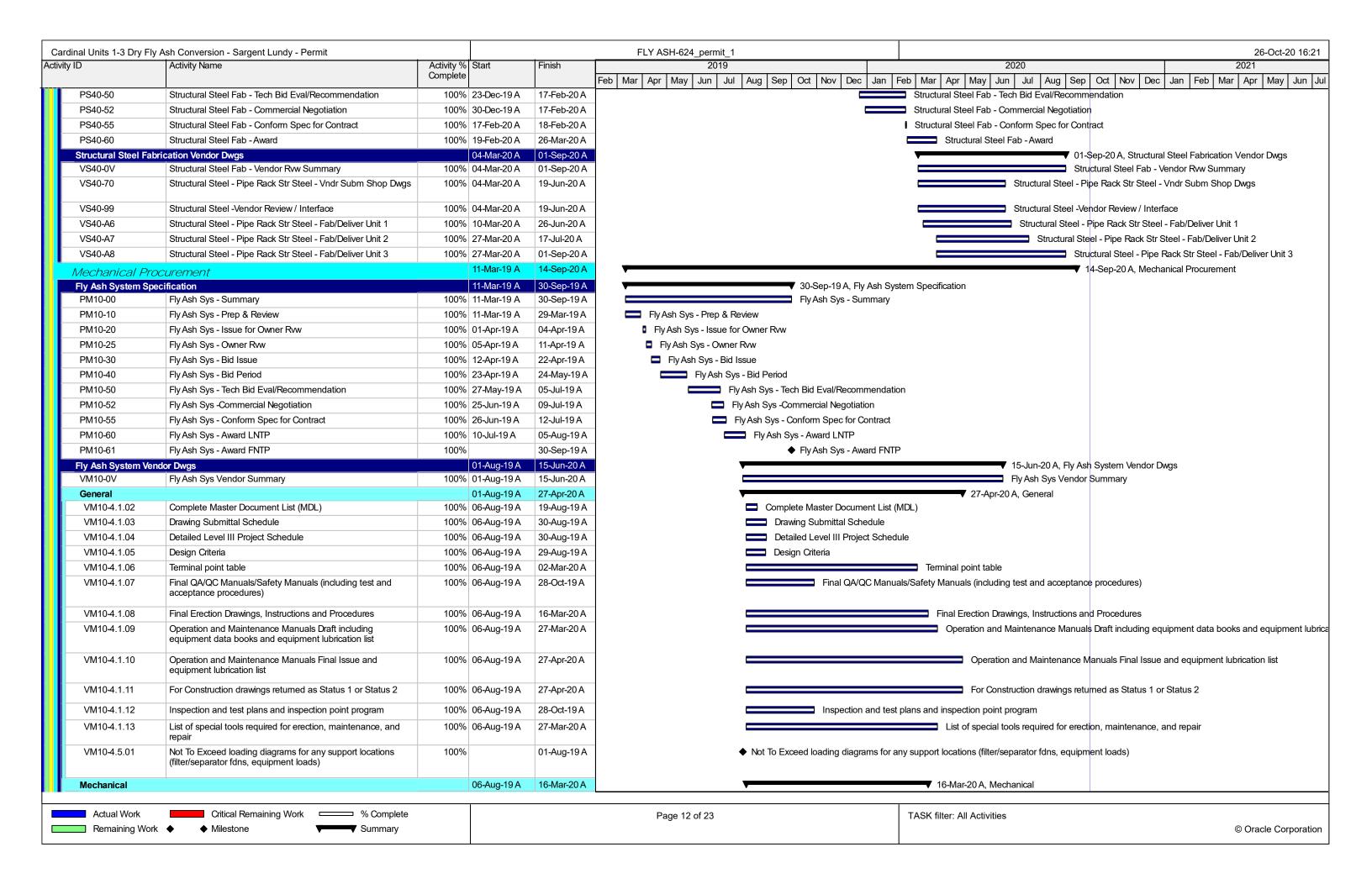




rdinal Units 1-3 Dry	<u> </u>	A otivity (0/	Ctort	Linioh	2019 2020 2021
ity ID	Activity Name	Activity % Complete	Start	Finish	Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Jun
M340-00	Ash Piping - Summary	100%	24-Jul-19 A	17-Apr-20 A	Ash Piping - Summary
M340-05	Ash Piping - Initl Routg & Supt Locs	100%	24-Jul-19 A	30-Sep-19 A	Ash Piping - Initl Routg & Supt Locs
M340-07	Ash Piping - Piping Analysis	100%	31-Jul-19 A	11-Oct-19 A	Ash Piping - Piping Analysis
M340-10	Ash Piping - Prep & Review	100%	14-Oct-19 A	06-Nov-19 A	Ash Piping - Prep & Review
M340-29	Ash Piping - Issue BOM for Bid	100%	14-Oct-19 A	06-Nov-19 A	Ash Piping - Issue BOM for Bid
M340-30	Ash Piping - Bid Issue	100%	07-Nov-19 A	07-Nov-19 A	■ I Ash Piping - Bid Issue
M340-60	Ash Piping - Const Issue	100%	11-Nov-19 A	17-Apr-20 A	Ash Piping - Const Issue
Supports - U1&2	Recirc to FGD circ to Cooling Tower (NR)		19-Aug-19 A	23-Aug-19 A	▼ 23-Aug-19 A, Supports - U3 Recirc to Cooling Tower (NR)
_ Supports - 03 Re M410-00	Unit 3 Recirc to Cooling Tower (NR) - Summary		19-Aug-19 A	23-Aug-19 A	
M410-05	Unit 3 Recirc to Cooling Tower (NR) - Initial Setup		19-Aug-19 A	23-Aug-19 A	
	2 , , ,		28-Oct-19 A	18-Nov-19 A	
M420-00	Service Water (ONLY U/G SERVICE WATER WILL BE ISSUED) LB Ppg Supts Service Wtr - Summary		28-Oct-19 A 28-Oct-19 A	18-Nov-19 A	18-Nov-19 A, DNU - Supports - Service Water (ONLY U/G SERVICE WATER WILL BE ISSUED) LB Ppg Supts Service Wtr - Summary
M420-10	LB Ppg Supts Service Wtr - Prep & Review		28-Oct-19 A	04-Nov-19 A	□ LB Ppg Supts Service Wtr - Strimary □ LB Ppg Supts Service Wtr - Prep & Review
	, , ,				
M420-30	LB Ppg Supts Service Wtr - Bid Issue		01-Nov-19 A	06-Nov-19 A	LB Ppg Supts Service Wtr - Bid Issue
M420-60	LB Ppg Supts Service Wtr - Const Issue		07-Nov-19 A	18-Nov-19 A	LB Ppg Supts Service Wtr - Const Issue
Supports - SB Ins			28-Oct-19 A	18-Nov-19 A	
M440-00	SB Installation Guide - Summary		28-Oct-19 A	18-Nov-19 A	SB Installation Guide - Summary
M440-10	SB Installation Guide - Prep & Review		28-Oct-19 A	11-Nov-19 A	SB Installation Guide - Prep & Review
M440-29	SB Installation Guide - Issue BOM for Bid		28-Oct-19 A	06-Nov-19 A	SB Installation Guide - Issue BOM for Bid
M440-30	SB Installation Guide - Bid Issue		12-Nov-19 A	18-Nov-19 A	SB Installation Guide - Bid Issue
Supports - Ash P	<u> </u>		26-Aug-19 A	17-Apr-20 A	17-Apr-20 A, Supports - Ash Piping
M450-00	Ash Piping - Summary		26-Aug-19 A	17-Apr-20 A	Ash Piping - Summary
M450-05	Ash Piping - Initial Setup		26-Aug-19 A	30-Aug-19 A	Ash Piping - Initial Setup
M450-10	Ash Piping - Prep & Review	100%	19-Nov-19 A	08-Jan-20 A	Ash Piping - Prep & Review
M450-29	Ash Piping - Issue BOM for Bid	100%	08-Jan-20 A	08-Jan-20 A	I Ash Piping - Issue BOM for Bid
M450-30	Ash Piping - Bid Issue	100%	24-Dec-19 A	08-Jan-20 A	Ash Piping - Bid Issue
M450-60	Ash Piping - Const Issue	100%	17-Jan-20 A	17-Apr-20 A	Ash Piping - Const Issue
Pipe Analysis - A	sh Piping & Service Water		26-Aug-19 A	13-Dec-19 A	▼ 13-Dec-19 A, Pipe Analysis - Ash Piping & Service Water
M480-00	Pipe Analysis - Ash Piping & Service Water - Summary	100%	26-Aug-19 A	13-Dec-19 A	Pipe Analysis - Ash Piping & Service Water - Summary
M480-60	Ash Piping & Service Water - Final Analysis	100%	26-Aug-19 A	13-Dec-19 A	Ash Piping & Service Water - Final Analysis
Structural Steel -	Pipe Supports & Aux Steel		06-May-19 A	21-Feb-20 A	▼ 21-Feb-20 A, Structural Steel - Pipe Supports & Aux Steel
S240-00	Str Steel Pipe Supports & Aux Steel - Summary	100%	06-May-19 A	21-Feb-20 A	Str Steel Pipe Supports & Aux Steel - Summary
S240-05	Str Steel Pipe Supports & Aux Steel - Initial Layout	100%	06-May-19 A	23-Aug-19 A	Str Steel Pipe Supports & Aux Steel - Initial Layout
S240-10	Str Steel Pipe Supports & Aux Steel - Prep & Review	100%	26-Aug-19 A	21-Feb-20 A	Str Steel Pipe Supports & Aux Steel - Prep & Review
S240-30	Str Steel Pipe Supports & Aux Steel - Bid Issue (MWC)	100%	14-Nov-19 A	27-Nov-19 A	Str Steel Pipe Supports & Aux Steel - Bid Issue (MWC)
S240-60	Str Steel Pipe Supports & Aux Steel - Fab/Const Issue	100%	28-Nov-19 A	21-Feb-20 A	Str Steel Pipe Supports & Aux Steel - Fab/Const Issue
Demo Dwgs - ES			01-Oct-19 A	08-Jan-20 A	
M500-00	Demo Dwgs - ESP Ppg - Summary		01-Oct-19 A	08-Jan-20 A	Demo Dwgs - ESP Ppg - Summary
M500-10	Demo Dwgs - ESP Ppg - Prep & Review		01-Oct-19 A	13-Dec-19 A	Demo Dwgs - ESP Ppg - Prep & Review
M500-30	Demo Dwgs - ESP Ppg - Bid Issue		31-Oct-19 A	06-Nov-19 A	□ Demo Dwgs - ESP Ppg - Bid Issue
M500-60	Demo Dwgs - ESP Ppg - Const Issue		17-Dec-19 A	08-Jan-20 A	Demo Dwgs - ESP Ppg - Const Issue
	peline, Valves, Specialties		01-Oct-19 A	07-Feb-20 A	
M600-00	Equip, Pipeline, Valves Specialties Lists - Summary		01-Oct-19 A	07-Feb-20 A	Equip, Pipeline, Valves, Specialities Equip, Pipeline, Valves Specialities Lists - Summary
M600-10	Equip, Pipeline, Valves Specialties Lists- Prep & Review		01-Oct-19 A	03-Jan-20 A	Equip, Pipeline, Valves Specialties Lists- Prep & Review
M600-30	Equip, Pipeline, Valves Specialties Lists - Issue for Use		06-Jan-20 A	07-Feb-20 A	Equip, Pipeline, Valves Specialties Lists - Issue for Use
Specialty Data S			01-Oct-19 A	13-Dec-19 A	
M610-00	Specialties Data Sheets - Summary		01-Oct-19 A	13-Dec-19 A	Specialties Data Sheets - Summary
	<u> </u>				· · · · · · · · · · · · · · · · · · ·

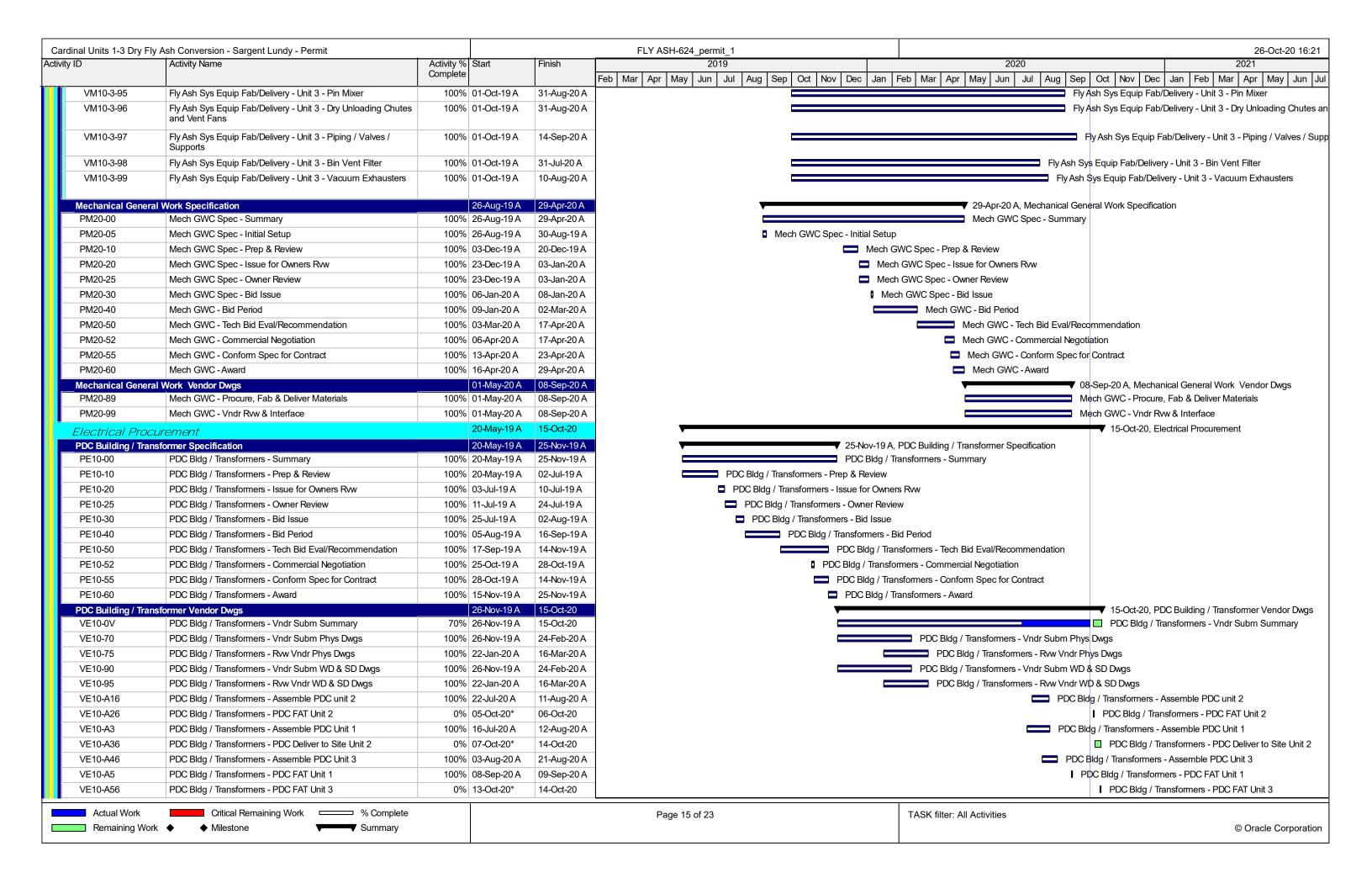






	Ash Conversion - Sargent Lundy - Permit			1	FLY ASH-624_permit_1	
ty ID	Activity Name	Activity % Complete	Start	Finish	2019	2020 2021
VM10-4.2.01	Initial - Key Process Flow Diagrams	·	06-Aug-19 A	06-Sep-19 A	Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec 3	Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May
VM10-4.2.02	Final - Key Process Flow Diagrams		06-Aug-19 A	16-Oct-19 A	Final - Key Proc	
VM10-4.2.03	Initial - Piping & Instrumentation Diagrams		06-Aug-19 A	16-Sep-19 A	Initial - Piping & Instru	· ·
VM10-4.2.04	Final - Piping & Instrumentation Diagrams		06-Aug-19 A	02-Mar-20 A		Final - Piping & Instrumentation Diagrams
VM10-4.2.05	Initial - General Arrangement Drawings including outline plan		06-Aug-19 A	22-Aug-19 A		nt Drawings including outline plan and elevation
	and elevation		3	J -		
VM10-4.2.06	Final - General Arrangement Drawings including outline plan and elevation	100%	06-Aug-19 A	02-Mar-20 A		Final - General Arrangement Drawings including outline plan and elevation
VM10-4.2.07	Mechanical Equipment, Valve and Line Lists	100%	06-Aug-19 A	21-Oct-19 A	Mechanical Eq	uipment, Valve and Line Lists
VM10-4.2.08	Initial - 2D piping arrangement drawings	100%	06-Aug-19 A	02-Mar-20 A		Initial - 2D piping arrangement drawings
VM10-4.2.09	Final - 2D piping arrangement drawings	100%	06-Aug-19 A	09-Mar-20 A		Final - 2D piping arrangement drawings
VM10-4.2.10	Vendor Certified outline and arrangement drawings of mechanical equipment	100%	06-Aug-19 A	09-Mar-20 A		Vendor Certified outline and arrangement drawings of mechanical equipment
VM10-4.2.11	Mechanical Equipment Data Sheets	100%	06-Aug-19 A	16-Mar-20 A		Mechanical Equipment Data Sheets
VM10-4.2.12	Exhauster physical certified drawings	100%	06-Aug-19 A	09-Mar-20 A		Exhauster physical certified drawings
VM10-4.2.13	Equipment Supplier Drawings	100%	06-Aug-19 A	13-Jan-20 A		Equipment Supplier Drawings
Electrical			06-Aug-19 A	31-Mar-20 A	<u> </u>	▼ 31-Mar-20 A, Electrical
VM10-4.3.01	Initial - Electrical Load Lists	100%	06-Aug-19 A	30-Sep-19 A	Initial - Electrical Lo	pad Lists
VM10-4.3.02	Final - Electrical Load Lists	100%	06-Aug-19 A	28-Oct-19 A	Final - Electric	cal Load Lists
VM10-4.3.03	Initial - Electrical One Line Diagrams	100%	06-Aug-19 A	30-Sep-19 A	Initial - Electrical Or	ne Line Diagrams
VM10-4.3.04	Final - Electrical One Line Diagrams	100%	06-Aug-19 A	04-Nov-19 A	Final - Elect	rical One Line Diagrams
VM10-4.3.05	Initial - Schematic Diagrams	100%	06-Aug-19 A	02-Mar-20 A		Initial - Schematic Diagrams
VM10-4.3.06	Final - Schematic Diagrams	100%	06-Aug-19 A	06-Jan-20 A		Final - Schematic Diagrams
VM10-4.3.07	Initial - Wiring Diagrams	100%	06-Aug-19 A	31-Mar-20 A		Initial - Wiring Diagrams
VM10-4.3.08	Final - Wiring Diagrams	100%	06-Aug-19 A	06-Jan-20 A		Final - Wiring Diagrams
VM10-4.3.09	Electrical Equipment Data Sheets	100%	06-Aug-19 A	30-Mar-20 A		Electrical Equipment Data Sheets
VM10-4.3.10	Motor Data Sheets	100%	06-Aug-19 A	25-Nov-19 A	Motor D	Data Sheets
VM10-4.3.11	Motor Curves	100%	06-Aug-19 A	02-Mar-20 A		Motor Curves
I&C			06-Aug-19 A	15-Jun-20 A	▼	▼ 15-Jun-20 A, I&C
VM10-4.4.02	Initial - Instrument List/Database	100%	06-Aug-19 A	14-Oct-19 A	Initial - Instrume	nt List/Database
VM10-4.4.03	Final - Instrument List/Database	100%	06-Aug-19 A	04-Nov-19 A	Final - Instru	ument List/Database
VM10-4.4.04	Initial - I/O List/Database	100%	06-Aug-19 A	28-Oct-19 A	Initial - I/O Lis	
VM10-4.4.05	Final - I/O List/Database		06-Aug-19 A	05-Jun-20 A		Final - I/O List/Database
VM10-4.4.06	Initial - Instrument Data Sheets		06-Aug-19 A	02-Mar-20 A		Initial - Instrument Data Sheets
VM10-4.4.07	Final - Instrument Data Sheets		06-Aug-19 A	15-Jun-20 A		Final - Instrument Data Sheets
VM10-4.4.08	Initial - Instrument installation details and location drawings		06-Aug-19 A	02-Mar-20 A		Initial - Instrument installation details and location drawings
VM10-4.4.09	Final - Instrument installation details and location drawings		06-Aug-19 A	15-Jun-20 A		Final - Instrument installation details and location drawings
VM10-4.4.10	Initial - Functional Description		06-Aug-19 A	02-Mar-20 A		Initial - Functional Description
VM10-4.4.11	Final - Final - Functional Description		06-Aug-19 A	01-Jun-20 A		Final - Final - Functional Description
VM10-4.4.12	Initial - Control Logic Diagrams		06-Aug-19 A	16-Mar-20 A		
VM10-4.4.13	Final - Control Logic Diagrams		06-Aug-19 A	15-Jun-20 A		Final - Control Logic Diagrams
VM10-4.4.14	Initial - DCS Graphic Screen Sketches		06-Aug-19 A	16-Mar-20 A		Initial - DCS Graphic Screen Sketches
VM10-4.4.15	Final - DCS Graphic Screen Sketches		06-Aug-19 A	15-Jun-20 A		Final - DCS Graphic Screen Sketches
VM10-4.5.02	Final loading diagrams for Foundations including Anchor Rod setting plans		06-Aug-19 A 06-Aug-19 A	16-Mar-20 A 16-Mar-20 A		16-Mar-20 A, Structural Final loading diagrams for Foundations including Anchor Rod setting plans
VM10-4.5.02.a.1	Preliminary loading diagrams for Fnds-For Silo	100%	06-Aug-19 A	02-Mar-20 A		Preliminary loading diagrams for Fnds-For Silo
Actual Work	Critical Remaining Work % Complete				Page 13 of 23	TASK filter: All Activities

<u> </u>	Ash Conversion - Sargent Lundy - Permit	۸ مان شد ۱۹۰	Ctort	Tipich	FLY ASH-624_permit_1	26-Oct-20 10
ty ID	Activity Name	Activity % Complete		Finish	2019 Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec	2020 2021 Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Ju
VM10-4.5.02.a.2	Preliminary loading diagrams for Fnds-For Stair Tower	100%	06-Aug-19 A	11-Mar-20 A		Preliminary loading diagrams for Fnds-For Stair Tower
VM10-4.5.03	Initial - Silo design drawings	100%	06-Aug-19 A	02-Mar-20 A		Initial - Silo design drawings
VM10-4.5.04	Silo not to exceed loads	100%	06-Aug-19 A	06-Mar-20 A		Silo not to exceed loads
VM10-4.5.05	Final - Silo design drawings	100%	06-Aug-19 A	06-Mar-20 A		Final - Silo design drawings
VM10-4.5.06	Pipe fitting properties, i.e. elbows, pipe spools etc. Info required for designing the pipe route	100%	06-Aug-19 A	02-Sep-19 A	Pipe fitting properties,	i.e. elbows, pipe spools etc. Info required for designing the pipe route
Fly Ash System Equ	ipment Delivery		05-Aug-19 A	14-Sep-20 A	·	▼ 14-Sep-20 A, Fly Ash System Equipment Delivery
VM10-V0	Fly Ash Sys Equip Delivery	100%	05-Aug-19 A	14-Sep-20 A		Fly Ash Sys Equip Delivery
Unit 1			05-Aug-19 A	20-Jul-20 A	▼	▼ 20-Jul-20 A, Unit 1
VM10-1-91	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Fly Ash Silo & Support Steel	100%	05-Aug-19 A	19-Jun-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Fly Ash Silo & Support Steel
VM10-1-92	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Vacuum Exhausters	100%	05-Aug-19 A	15-Jun-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Vacuum Exhausters
VM10-1-93	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Fluidizing Air System	100%	01-Oct-19 A	06-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Fluidizing Air System
VM10-1-94	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Compressed Air System & Pipe	100%	01-Oct-19 A	06-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Compressed Air System & Pip
VM10-1-95	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Pin Mixer	100%	01-Oct-19 A	06-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Pin Mixer
VM10-1-96	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Dry Unloading Chutes and Vent Fans	100%	01-Oct-19 A	06-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Dry Unloading Chutes and Ve
VM10-1-97	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Piping / Valves / Supports	100%	01-Oct-19 A	20-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Piping / Valves / Supports
VM10-1-98	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Filter Separators	100%	01-Oct-19 A	16-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Filter Separators
VM10-1-99	Fly Ash Sys Equip Fab/Delivery - Unit 1 - Bin Vent Filter	100%	05-Aug-19 A	02-Jun-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 1 - Bin Vent Filter
Unit 2			06-Aug-19 A	17-Aug-20 A	-	▼ 17-Aug-20 A, Unit 2
VM10-2-91	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Fly Ash Silo & Support Steel		06-Aug-19 A	22-Jun-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Fly Ash Silo & Support Steel
VM10-2-92	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Vacuum Exhausters	100%	01-Oct-19 A	13-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Vacuum Exhausters
VM10-2-93	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Fluidizing Air System	100%	01-Oct-19 A	03-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Fluidizing Air System
VM10-2-94	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Compressed Air System & Pipe	100%	01-Oct-19 A	03-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Compressed Air System
VM10-2-95	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Pin Mixer	100%	01-Oct-19 A	03-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Pin Mixer
VM10-2-96	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Dry Unloading Chutes and Vent Fans	100%	01-Oct-19 A	03-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Dry Unloading Chutes a
VM10-2-97	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Piping / Valves / Supports	100%	01-Oct-19 A	17-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Piping / Valves / Sup
VM10-2-98	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Filter Separators	100%	01-Oct-19 A	03-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Filter Separators
VM10-2-99	Fly Ash Sys Equip Fab/Delivery - Unit 2 - Bin Vent Filter	100%	01-Oct-19 A	03-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 2 - Bin Vent Filter
Unit 3			01-Oct-19 A	14-Sep-20 A	▼	14-Sep-20 A, Unit 3
VM10-3-91	Fly Ash Sys Equip Fab/Delivery - Unit 3 - Fly Ash Silo & Support Steel	100%	01-Oct-19 A	20-Jul-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 3 - Fly Ash Silo & Support Ste
VM10-3-92	Fly Ash Sys Equip Fab/Delivery - Unit 3 - Vacuum Exhausters	100%	01-Oct-19 A	10-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 3 - Vacuum Exhausters
VM10-3-93	Fly Ash Sys Equip Fab/Delivery - Unit 3 - Fluidizing Air System	100%	01-Oct-19 A	31-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 3 - Fluidizing Air Systo
VM10-3-94	Fly Ash Sys Equip Fab/Delivery - Unit 3 - Compressed Air System & Pipe	100%	01-Oct-19 A	31-Aug-20 A		Fly Ash Sys Equip Fab/Delivery - Unit 3 - Compressed Air S
Actual Work	Critical Remaining Work				Page 14 of 23	TASK filter: All Activities
Actual Work Remaining Work	-				Page 14 of 23	TASK filter: All Activities © Oracle

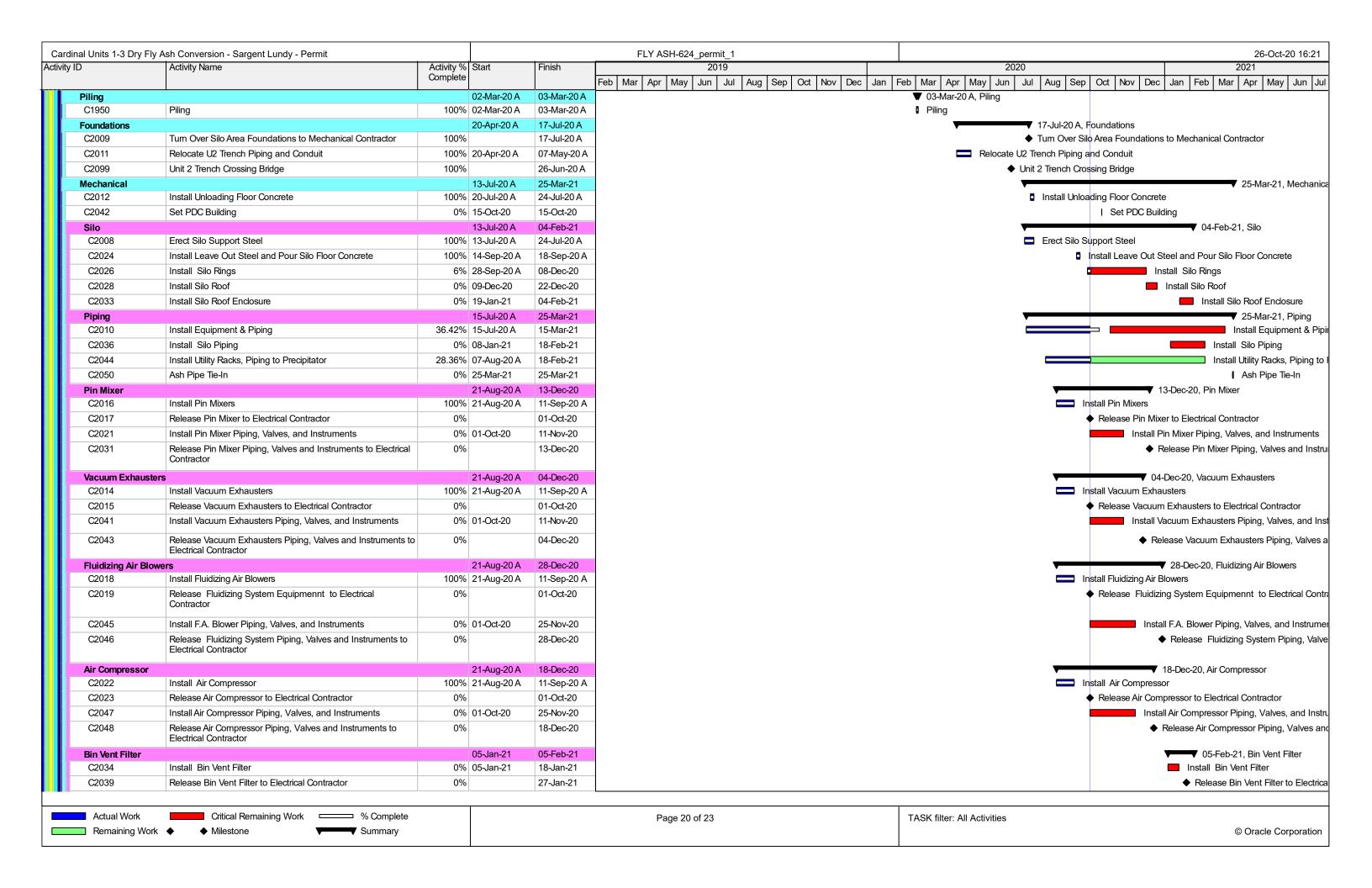


	Fly Ash Conversion - Sargent Lundy - Permit	A ativity 0/	Stort	Finish	FLY ASH-624_permit_1 26-Oct-2 2019 2020 2021
ID	Activity Name	Activity % Complete	Olall	FILISH	eb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May
VE10-A6	PDC Bldg / Transformers - PDC Deliver to Site unit 1	100%	23-Sep-20 A	24-Sep-20 A	PDC Bldg / Transformers - PDC Deliver to Site unit
VE10-A66	PDC Bldg / Transformers - PDC Deliver to Site Unit 3		15-Oct-20*	15-Oct-20	I PDC Bldg / Transformers - PDC Deliver to Site
VE10A-A1	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip		17-Mar-20 A	16-Jul-20 A	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip To
	To PDC Shop Unit 1				
VE10A-A2	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip To PDC Shop Unit 2	100%	30-Apr-20 A	10-Jul-20 A	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip To
VE10A-A3	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip To PDC Shop Unit 3	100%	12-May-20 A	22-Jul-20 A	PDC Bldg / Transformers - Procure/Fab/Deliv PDC Comp/Equip
Electrical General	I Work Specification		02-Mar-20 A	09-Jul-20 A	09-Jul-20 A, Electrical General Work Specification
PE20-00	Elect GWC Spec - Summary	100%	02-Mar-20 A	09-Jul-20 A	Elect GWC Spec - Summary
PE20-10	Elect GWC Spec - Prep & Review	100%	02-Mar-20 A	26-Mar-20 A	Elect GWC Spec - Prep & Review
PE20-20	Elect GWC Spec - Issue for Owners Rvw	100%	27-Mar-20 A	01-Apr-20 A	☐ Elect GWC Spec - Issue for Owners Rvw
PE20-25	Elect GWC Spec - Owner Review	100%	25-Mar-20 A	06-Apr-20 A	■ Elect GWC Spec - Owner Review
PE20-30	Elect GWC Spec - Bid Issue	100%	07-Apr-20 A	14-Apr-20 A	□ Elect GWC Spec - Bid Issue
PE20-40	Elect GWC - Bid Period	100%	15-Apr-20 A	01-Jun-20 A	Elect GWC - Bid Period
PE20-50	Elect GWC - Tech Bid Eval/Recommendation		01-Jun-20 A	19-Jun-20 A	Elect GWC - Tech Bid Eval/Recommendation
PE20-52	Elect GWC - Commercial Negotiation		15-Jun-20 A	26-Jun-20 A	■ Elect GWC - Commercial Negotiation
PE20-55	Elect GWC - Conform Spec for Contract	100%	29-Jun-20 A	06-Jul-20 A	■ Elect GWC - Conform Spec for Contract
PE20-60	Elect GWC -Award	100%	09-Jul-20 A	09-Jul-20 A	Elect GWC - Award
Electrical General	l Work Vendor Dwgs		01-Jul-20 A	03-Sep-20 A	03-Sep-20 A, Electrical General Work Vendor Dwgs
VE20-0V	Elect GWC Fab & Deliver - Summary		01-Jul-20 A	03-Sep-20 A	Elect GWC Fab & Deliver - Summary
VE20-70	Elect GWC - Vndr Subm Phys Dwgs	100%	01-Jul-20 A	03-Sep-20 A	Elect GWC - Vndr Subm Phys Dwgs
VE20-98	Elect GWC - Procure/Fab/Deliver Materials	100%	01-Jul-20 A	03-Sep-20 A	Elect GWC - Procure/Fab/Deliver Materials
&C Procurem	nent		29-Jul-19 A	21-Aug-20 A	▼ 21-Aug-20 A, I&C Procurement
CS Modification			29-Jul-19 A	20-Mar-20 A	▼ 20-Mar-20 A, DCS Modification Specification
PJ10-00	DCS - Summary	100%	29-Jul-19 A	20-Mar-20 A	DCS - Summary
PJ10-05	DCS - Initial Setup	100%	29-Jul-19 A	16-Aug-19 A	DCS - Initial Setup
PJ10-10	DCS - Prep & Review	100%	21-Oct-19 A	11-Nov-19 A	DCS - Prep & Review
PJ10-20	DCS - Issue for Owners Review for Rvw	100%	15-Oct-19 A	21-Oct-19 A	DCS - Issue for Owners Review for Rvw
PJ10-25	DCS - Owner Review	100%	18-Oct-19 A	31-Oct-19 A	DCS - Owner Review
PJ10-30	DCS - Bid Issue	100%	12-Nov-19 A	22-Nov-19 A	DCS - Bid Issue
PJ10-40	DCS - Bid Period	100%	04-Dec-19 A	13-Jan-20 A	DCS - Bid Period
PJ10-50	DCS - Tech Bid Eval/Recommendation	100%	14-Jan-20 A	03-Mar-20 A	DCS - Tech Bid Eval/Recommendation
PJ10-52	DCS - Commercial Negotiation	100%	13-Jan-20 A	18-Mar-20 A	DCS - Commercial Negotiation
PJ10-55	DCS - Conform Spec for Contract	100%	04-Mar-20 A	18-Mar-20 A	DCS - Conform Spec for Contract
PJ10-60	DCS - Award	100%	09-Mar-20 A	20-Mar-20 A	DCS - Award
OCS Modification	Vendor Dwgs		27-Dec-19 A	21-Aug-20 A	▼ 21-Aug-20 A, DCS Modification Vendor Dwgs
VJ10-0V	DCS - Vendor Rvw/Interface Summary	100%	27-Dec-19 A	21-Aug-20 A	DCS - Vendor Rvw/Interface Summary
Hardware			27-Dec-19 A	21-Aug-20 A	▼ 21-Aug-20 A, Hardware
VJ10H1-0V	DCS HW - Vendor Rvw/Interface Summary		27-Dec-19 A	21-Aug-20 A	DCS HW - Vendor Rvw/Interface Summary
	DCS HW - Vndr Submit System Layout Drawings		30-Mar-20 A	06-May-20 A	DCS HW - Vndr Submit System Layout Drawings
VJ10H1-1120	DCS HW - SL Submit I/O List for I/O Freeze		27-Dec-19 A	03-Apr-20 A	DCS HW - SL Submit I/O List for I/O Freeze
VJ10H1-1160			00 1400 4	03-Apr-20 A	DCS HW - ALL I/O Freeze
VJ10H1-1160 VJ10H1-1170	DCS HW - ALL I/O Freeze		30-Mar-20 A	· ·	
VJ10H1-1160 VJ10H1-1170 VJ10H1-1180	DCS HW - Vndr Submit Cabinet Detail Drawings	100%	06-Apr-20 A	04-May-20 A	DCS HW - Vndr Submit Cabinet Detail Drawings
VJ10H1-1160 VJ10H1-1170	DCS HW - Vndr Submit Cabinet Detail Drawings DCS HW - SL/Client Review & Comment on Cabinet Detail Drawings	100% 100%	06-Apr-20 A 07-May-20 A	04-May-20 A 07-May-20 A	DCS HW - SL/Client Review & Comment on Cabinet Detail Drawings
VJ10H1-1160 VJ10H1-1170 VJ10H1-1180	DCS HW - Vndr Submit Cabinet Detail Drawings DCS HW - SL/Client Review & Comment on Cabinet Detail	100% 100%	06-Apr-20 A	04-May-20 A	
VJ10H1-1160 VJ10H1-1170 VJ10H1-1180 VJ10H1-1190	DCS HW - Vndr Submit Cabinet Detail Drawings DCS HW - SL/Client Review & Comment on Cabinet Detail Drawings	100% 100%	06-Apr-20 A 07-May-20 A	04-May-20 A 07-May-20 A	DCS HW - SL/Client Review & Comment on Cabinet Detail Drawings

rdinal Units 1-3 Dry F ty ID	Activity Name	Activity %	Start	Finish	2019	2020 2021
,		Complete			Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec J	
VJ10H1-1240	DCS HW - Vndr Hardware Fabrication & Test Setup	100%	16-Jun-20 A	24-Jun-20 A		DCS HW - Vndr Hardware Fabrication & Test Setup
VJ10H1-1250	DCS HW - Hardware FAT	100%	03-Aug-20 A	07-Aug-20 A		■ DCS HW - Hardware FAT
VJ10H1-1260	DCS HW - Vndr Clean-up and pack Hardware	100%	10-Aug-20 A	14-Aug-20 A		DCS HW - Vndr Clean-up and pack Hardware
VJ10H1-1270	DCS HW - Vndr Ship Hardware	100%	17-Aug-20 A	21-Aug-20 A		DCS HW - Vndr Ship Hardware
VJ10H1-1280	DCS HW - Receive Hardware on-site	100%	21-Aug-20 A	21-Aug-20 A		DCS HW - Receive Hardware on-site
Software			13-Apr-20 A	14-Aug-20 A		▼ 14-Aug-20 A, Software
VJ10S1-0V	DCS SW- Vendor Rvw/Interface Summary		13-Apr-20 A	14-Aug-20 A		DCS SW- Vendor Rw/Interface Summary
VJ10S1-1300	DCS SW - Vndr DCS Software Development	100%	13-Apr-20 A	22-May-20 A		DCS SW - Vndr DCS Software Development
VJ10S1-1330	DCS SW - Vndr Submit Control Sheets & Graphics for review	100%	22-May-20 A	22-May-20 A		DCS SW - Vndr Submit Control Sheets & Graphics for review
VJ10S1-1340	DCS SW - SL/Client Review & Comment on Control Sheets & Graphics	100%	22-May-20 A	05-Jun-20 A		DCS SW - SL/Client Review & Comment on Control Sheets & Graphics
VJ10S1-1350	DCS SW -ALL Design Review Meeting	100%	17-Jun-20 A	19-Jun-20 A		DCS SW - ALL Design Review Meeting
VJ10S1-1360	DCS SW -ALL Software Freeze	100%	08-Jun-20 A	08-Jun-20 A		DCS SW - ALL Software Freeze
VJ10S1-1370	DCS SW - Vndr Implement Design Review Comments	100%	17-Jun-20 A	19-Jun-20 A		DCS SW - Vndr Implement Design Review Comments
VJ10S1-1390	DCS SW - Software FAT	100%	20-Jul-20 A	24-Jul-20 A		DCS SW - Software FAT
VJ10S1-1400	DCS SW - Clean-up Software	100%	27-Jul-20 A	11-Aug-20 A		DCS SW - Clean-up Software
VJ10S1-1410	DCS SW - Ship Software	100%	12-Aug-20 A	14-Aug-20 A		DCS SW - Ship Software
VJ10S1-1420	DCS SW - Receive Software on-site		14-Aug-20 A	14-Aug-20 A		I DCS SW - Receive Software on-site
Instrumentation B0	DM					
Instrumentation B0	DMVendor Dwgs					
roject Administra	tion & Management Tasks		11-Mar-19 A	31-Jul-20 A	V	▼ 31-Jul-20 A, Project Administration & Management Tasks
External Statu	is Meetinas		08-Apr-19 A	30-Jun-20 A	▼	▼ 30-Jun-20 A, External Status Meetings
X020-0L	External Status Meetings - Summary	100%	08-Apr-19 A	30-Jun-20 A		External Status Meetings - Summary
Project Status	Meetings		25-Mar-19 A	30-Jun-20 A	V	30-Jun-20 A, Project Status Meetings
X030-0L	Project Status Meetings (Conference Calls) - Summary	100%	25-Mar-19 A	30-Jun-20 A		Project Status Meetings (Conference Calls) - Summary
Internal Team			18-Mar-19 A	30-Jun-20 A		30-Jun-20 A, Internal Team Meetings
X040-0L	Internal Team Meetings - Summary	100%	18-Mar-19 A	30-Jun-20 A		Internal Team Meetings - Summary
		10070	25-Mar-19 A	30-Jun-20 A		30-Jun-20 A, Schedule Development and Maintenance
<i>Scheaule Deve</i> X050-0L	elopment and Maintenance	1000/	25-Mar-19 A	30-Jun-20 A		Schedule Development and Maintenance - Summary
	Schedule Development and Maintenance - Summary			1 1 1 1		,
Monthly Progr			08-Apr-19 A	30-Jun-20 A		▼ 30-Jun-20 A, Monthly Progress reports
X060-0L	Monthly Progress reports - Summary	100%	08-Apr-19 A	30-Jun-20 A		Monthly Progress reports - Summary
	stration Support & Document Control		18-Mar-19 A	30-Jun-20 A		▼ 30-Jun-20 A, Project Adminstration Support & Document Control
X070-0L	Project Adminstration Support & Document Control - Summary	100%	18-Mar-19 A	30-Jun-20 A		Project Adminstration Support & Document Control - Summary
Model review .			09-Jun-19 A	31-Jul-20 A	▼	▼ 31-Jul-20 A, Model review meetings
X080-0L	Model review meetings - Summary	100%	09-Jun-19 A	31-Jul-20 A		Model review meetings - Summary
	istration & Management		11-Mar-19 A	30-Jun-20 A	▼	▼ 30-Jun-20 A, Project Administration & Management
X100-0L	Project Administration & Management - Summary	100%	11-Mar-19 A	30-Jun-20 A		Project Administration & Management - Summary
Project Contin	gency		20-Dec-19 A	23-Jun-20 A	▼	▼ 23-Jun-20 A, Project Contingency
X990-0L	Project Contingency - Summary	100%	20-Dec-19 A	23-Jun-20 A		Project Contingency - Summary
Construction			21-Sep-19 A	21-Nov-21	▼	
Unit 1			21-Sep-19 A	21-Nov-21	▼	
Unit 1 - Outages			21-Sep-19 A	21-Nov-21	▼	
OUTU1.001	Unit No 1 - 2019 Fall Outage		21-Sep-19 A	29-Sep-19 A	☐ Unit No 1 - 2019 Fa	all Outage
OUTU1.002	Unit No 1 - 2020 Spring Outage	100%	14-Mar-20 A	10-May-20 A		Unit No 1 - 2020 Spring Outage
OUTU1.003	Unit No 1 - 2020 Fall Outage	20%	29-Sep-20 A	08-Oct-20*		Unit No 1 - 2020 Fall Outage
			I			

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ID	Activity Name	Activity % Complete	Start	Finish	2019	2020	2021
OLITI IA OOA	Lieit Ne. 4. 2004 Coning Oute as	·	47 A 04*	25 Am 24*	Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jar	n Feb Mar Apr May Jun Jul Aug Ser	o Oct Nov Dec Jan Feb Mar Apr May J
OUTU1.004	Unit No 1 - 2021 Spring Outage		17-Apr-21*	25-Apr-21*			■ Unit No
OUTU1.005	Unit No 1 - 2021 Fall Outage		30-Oct-21*	21-Nov-21	_		40 1 04 11 74 0 1 17
Unit 1 - Construction	1		17-Jan-20 A 17-Jan-20 A	12-Jan-21 27-Feb-20 A		27 Fala 20 A. Dilina	▼ 12-Jan-21, Unit 1 - Construction
Piling C0000	Piling Work - Mobilize		17-Jan-20 A 17-Jan-20 A	27-Feb-20 A 29-Jan-20 A	Y	✓ 27-Feb-20 A, Piling Piling Work - Mobilize	
C0050	Piling		30-Jan-20 A	27-Feb-20 A		Piling	
Foundations	rillig		29-May-20 A	29-May-20 A		▼ 29-May-20 A. Foundat	tions
C1009	Turn Over Silo Area Foundations to Mechanical Contractor	100%		29-May-20 A		· · · · · · · · · · · · · · · · · · ·	oundations to Mechanical Contractor
Mechanical	Taill Over Cite / tea Carinations to Medianical Centractor		12-May-20 A	12-Jan-21		Tall Over che / tea / te	12-Jan-21, Mechanical
C1007	Mechanical GWC - Mobilize		12-May-20 A	29-May-20 A		Mechanical GWC - Mo	
C1012	Install Unloading Floor Concrete		24-Jun-20 A	30-Jun-20 A		☐ Install Unloading	
C1042	Set PDC Building		25-Sep-20 A	25-Sep-20 A			Set PDC Building
Silo	Get i Do Building		22-Jun-20 A	28-Dec-20			28-Dec-20, Silo
C1008	Erect Silo Support Steel		22-Jun-20 A	06-Jul-20 A		Erect Silo Sup	The state of the s
C1000	Install Leave Out Steel and Pour Silo Floor Concrete		04-Aug-20 A	10-Aug-20 A			Leave Out Steel and Pour Silo Floor Concrete
C1024	Install Silo Rings		03-Aug-20 A	14-Sep-20 A			Install Silo Rings
C1028	Install Silo Roof		23-Sep-20 A	28-Sep-20 A			☐ Install Silo Roof
C1028	Install Silo Roof Enclosure		16-Nov-20	28-Dec-20		·	Install Silo Roof Enclosure
	Install Silo Piping						
C1036	Install 5110 Piping		29-Sep-20 A	28-Dec-20			Install Silo Piping
Piping C1010	Install Equipment & Piping		01-Jul-20 A 01-Jul-20 A	12-Jan-21 28-Dec-20		<u> </u>	▼ 12-Jan-21, Piping Install Equipment & Piping
C1010	Install Utility Racks, Piping to Precipitator		01-Jul-20 A	30-Nov-20			Install Utility Racks, Piping to Precipitate
C1050	Ash Pipe Tie-In		12-Jan-21	12-Jan-21			Ash Pipe Tie-In
Pin Mixer C1016	Install Pin Mixers		22-Jul-20 A 22-Jul-20 A	06-Nov-20		■ Install Pi	06-Nov-20, Pin Mixer
	111212111111111111111111111111111111111			03-Aug-20 A			
C1017	Release Pin Mixer to Electrical Contractor	100%		24-Aug-20 A			ease Pin Mixer to Electrical Contractor
C1021	Install Pin Mixer Piping, Valves, and Instruments		28-Aug-20 A	06-Nov-20			motali i in misor i iping, varvos, and motali
C1031	Release Pin Mixer Piping, Valves and Instruments to Electrical Contractor	0%		06-Nov-20			◆ Release Pin Mixer Piping, Valves and Instru
Vacuum Exhauste	ers		15-Jul-20 A	29-Oct-20			29-Oct-20, Vacuum Exhausters
C1014	Install Vacuum Exhausters	100%	15-Jul-20 A	24-Jul-20 A		☐ Install Vac	uum Exhausters
C1015	Release Vacuum Exhausters to Electrical Contractor	100%		25-Aug-20 A		♦ Rel	ease Vacuum Exhausters to Electrical Contractor
C1041	Install Vacuum Exhausters Piping, Valves, and Instruments	47.5%	11-Aug-20 A	29-Oct-20			Install Vacuum Exhausters Piping, Valves, and
C1043	Release Vacuum Exhausters Piping, Valves and Instruments to Electrical Contractor	0%		29-Oct-20			◆ Release Vacuum Exhausters Piping, Valves a
Fluidizing Air Blow			22-Jul-20 A	20-Nov-20		—	▼ 20-Nov-20, Fluidizing Air Blowers
C1018	Install Fluidizing Air Blowers	100%	22-Jul-20 A	03-Aug-20 A		☐ Install Fl	uidizing Air Blowers
C1019	Release Fluidizing System Equipmennt to Electrical Contractor	100%		24-Aug-20 A		♦ Rele	ease Fluidizing System Equipmennt to Electrical Contra
C1045	Install F.A. Blower Piping, Valves, and Instruments	17.78%	25-Aug-20 A	20-Nov-20			Install F.A. Blower Piping, Valves, and Ins
C1046	Release Fluidizing System Piping, Valves and Instruments to Electrical Contractor	0%		20-Nov-20			◆ Release Fluidizing System Piping, Valve
Air Compressor			22-Jul-20 A	13-Nov-20		-	13-Nov-20, Air Compressor
C1022	Install Air Compressor		22-Jul-20 A	03-Aug-20 A		Install A	· ·
C1023	Release Air Compressor to Electrical Contractor	100%		24-Aug-20 A		♦ Rela	ease Air Compressor to Electrical Contractor
C1047	Install Air Compressor Piping, Valves, and Instruments	36%	25-Aug-20 A	13-Nov-20			Install Air Compressor Piping, Valves, and
C1048	Release Air Compressor Piping, Valves and Instruments to Electrical Contractor	0%		13-Nov-20			◆ Release Air Compressor Piping, Valves an
Bin Vent Filter			01-Oct-20	07-Dec-20			07-Dec-20, Bin Vent Filter
Actual Work	Critical Remaining Work				Page 18 of 23	TASK filter: All Activities	

	y Ash Conversion - Sargent Lundy - Permit					
y ID	Activity Name	Activity %		Finish	2019	2020 2021
		Complete			Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Ja	
C1034	Install Bin Vent Filter	0%	01-Oct-20	14-Oct-20		Install Bin Vent Filter
C1039	Release Bin Vent Filter to Electrical Contractor	0%		24-Nov-20		◆ Release Bin Vent Filter to Electrical Cont
C1051	Release Bin Vent Filter Piping, Valves and Instruments to Electrical Contractor	0%		07-Dec-20		◆ Release Bin Vent Filter Piping, Valves
Filter Separtors			01-Oct-20	10-Dec-20		10-Dec-20, Filter Separtors
C1032	Install Filter Separators	0%	01-Oct-20	14-Oct-20		Install Filter Separators
C1037	Release Filter Separators to Electrical Contractor	0%		24-Nov-20		♦ Release Filter Separators to Electrical C
C1049	Release Filter Separators Piping, Valves and Instruments to Electrical Contractor	0%		10-Dec-20		◆ Release Filter Separators Piping, Val
Electrical			10-Aug-20 A	08-Jan-21		▼ 08-Jan-21, Electrical
C1020	Electrical GWC - Mobilize	100%	10-Aug-20 A	21-Aug-20 A		■ Electrical GWC - Mobilize
C1025	LOTO & Install DCS Hardware	100%	16-Sep-20 A	17-Sep-20 A		LOTO & Install DCS Hardware
C1027	Transfer Precip Valve Wiring from PLC to DCS	100%	18-Sep-20 A	20-Sep-20 A		Transfer Precip Valve Wiring from PLC to DCS
C1030	Electrical Contractor Install Electrical	14.1%	16-Sep-20 A	08-Jan-21		Electrical Contractor Install Ele
C1035	Electrical Tie-In Outage		29-Sep-20 A	08-Oct-20		☐ Electrical Tie-In Outage
C1080	Perform Maintenance on Existing ESP Valves		01-Sep-20 A	10-Sep-20 A		□ Perform Maintenance on Existing ESP Valves
Turnover to Comn	-		27-Nov-20	05-Jan-21		▼ 05-Jan-21, Turnover to Commi
C1031C	Pin Mixer Turnover to Commissioning	0%		27-Nov-20*		◆ Pin Mixer Tumover to Commissioning
C1043C	Vacuum Exhausters Turnover to Commissioning	0%		21-Dec-20*		◆ Vacuum Exhausters Turnover to 0
C1046C	Fluidizing System Turnover to Commissioning	0%		07-Dec-20*		◆ Fluidizing System Tumover to Comm
C1048C	Air Compressor Turnover to Commissioning	0%		14-Dec-20*		◆ Air Compressor Turnover to Commi
C1049C	Filter Separators Turnover to Commissioning	0%		17-Dec-20		◆ Filter Separators Turnover to Comm
C1050A	Wet and Dry Unloading Complete System Turnover to Commissioning	0%		31-Dec-20		◆ Wet and Dry Unloading Comple
C1050B	Vacuum Conveying Complete System Tumover to Commissioning	0%		05-Jan-21		◆ Vacuum Conveying Complete
C1051C	Bin Vent Filter Tumover to Commissioning	0%		29-Dec-20*		◆ Bin Vent Filter Turnover to Comr
Commissioning			21-Sep-20 A	12-Jan-21		12-Jan-21, Commissioning
C1029	Commision Precip Valve in DCS	100%	21-Sep-20 A	25-Sep-20 A		Commision Precip Valve in DCS
C1040	Commissioning	0%	19-Oct-20	12-Jan-21		Commissioning
C1052	Commission Pin Mixer Subsystem	0%	28-Nov-20	07-Dec-20		Commission Pin Mixer Subsystem
C1054	Commission F.A. Blowers Subsystem		08-Dec-20	17-Dec-20		Commission F.A. Blowers Subsyste
C1056	Commission Air Compressor Subsystem		15-Dec-20	24-Dec-20		Commission Air Compressor Sub
C1058	Commission Vacuum Exhausters Subsystem		22-Dec-20	26-Dec-20		Commission Vacuum Exhausters
C1060	Commission Filter Separtators Subsystem		30-Dec-20*	05-Jan-21		Commission Filter Separtators
C1062	Commission Fine Separations Subsystem Commission Bin Vent Filter Subsystem		30-Dec-20*	05-Jan-21		Commission Bin Vent Filter Sul
C1064	Commission Vacuum System		06-Jan-21	12-Jan-21		Commission Vacuum System
C1066	Commission Unloading System		06-Jan-21	12-Jan-21		Commission Vacuum System Commission Unloading Syste
	Commission Unloading System Commission Aux Power Subsystem					
C1090	CONTINUSSION AUX POWER SUDSYSTEM	0%	09-Oct-20 19-Oct-19 A	13-Oct-20 11-Oct-21		Commission Aux Power Subsystem
Unit 2						
Unit 2 - Outages	Limit Nia 2, 2040 Fall Outerra	4000/	19-Oct-19 A	11-Oct-21	Librit No. 2	2040 Fall Outage
OUTU2.001	Unit No 2 - 2019 Fall Outage		19-Oct-19 A	18-Nov-19 A	Unit No 2	- 2019 Fall Outage
OUTU2.002	Unit No 2 - 2020 Spring Outage		04-Apr-20 A	13-Apr-20 A		Unit No 2 - 2020 Spring Outage
OUTU2.003	Unit No 2 - 2020 Fall Outage		26-Sep-20 A	04-Oct-20*		☐ Unit No 2 - 2020 Fall Outage
OUTU2.004	Unit No 2 - 2021 Spring Outage		27-Mar-21*	24-May-21*		U
OUTU2.005	Unit No 2 - 2021 Fall Outage		02-Oct-21*	11-Oct-21*		
Unit 2 - Construction	on The Control of the		02-Mar-20 A	30-Mar-21		▼ 30-Mar-21, U
			1			
Actual Work	Critical Remaining Work				Page 19 of 23	TASK filter: All Activities



ators arators to Electrical Contractor arators Piping, Valves and Instruments to r B Hardware we Wiring from PLC to DCS Valve in DCS Subsystem	Activity % Complete	05-Feb-21 05-Feb-21 18-Jan-21 27-Jan-21 05-Feb-21	b Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr I	2020 2021 May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May Ju ◆ Release Bin Vent Filter P
ators arators to Electrical Contractor arators Piping, Valves and Instruments to r 6 Hardware we Wiring from PLC to DCS	0% 05-Jan-21 0% 05-Jan-21 0% 0%	05-Feb-21 18-Jan-21 27-Jan-21	b Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr I	
ators arators to Electrical Contractor arators Piping, Valves and Instruments to r 6 Hardware we Wiring from PLC to DCS	05-Jan-21 0% 05-Jan-21 0% 0%	05-Feb-21 18-Jan-21 27-Jan-21		◆ Release Bin Vent Filter P
arators to Electrical Contractor arators Piping, Valves and Instruments to r S Hardware ve Wiring from PLC to DCS	0% 05-Jan-21 0% 0%	18-Jan-21 27-Jan-21		
arators to Electrical Contractor arators Piping, Valves and Instruments to r S Hardware ve Wiring from PLC to DCS	0%	27-Jan-21		▼ 05-Feb-21, Filter Separto
rators Piping, Valves and Instruments to r 6 Hardware ve Wiring from PLC to DCS	0%			Install Filter Separators
B Hardware ve Wiring from PLC to DCS		05-Feb-21		◆ Release Filter Separators t
ve Wiring from PLC to DCS	21-Sep-20 A	1		◆ Release Filter Separators
ve Wiring from PLC to DCS		30-Mar-21		30-Mar-21, El
	100% 26-Sep-20 A	27-Sep-20 A		LOTO & Install DCS Hardware
	100% 28-Sep-20 A	30-Sep-20 A		Transfer Precip Valve Wiring from PLC to DCS
	0% 01-Oct-20	04-Oct-20		Commission Precip Valve in DCS Subsystem
r Install Electrical	6.06% 21-Sep-20 A	30-Mar-21		Electrical Con
age	55.56% 26-Sep-20 A	04-Oct-20		☐ Electrical Tie-In Outage
ce on Existing ESP Valves	55.56% 26-Sep-20 A	04-Oct-20		☐ Perform Maintenance on Existing ESP Valves
	19-Jan-21	22-Feb-21		22-Feb-21, Tumover
to Commissioning	0%	19-Jan-21		♦ Pin Mixer Turnover to Comm
s Turnover to Commissioning	0%	09-Feb-21		◆ Vacuum Exhausters Tu
umover to Commissioning	0%	27-Jan-21		◆ Fluidizing System Turnovei
nover to Commissioning	0%	04-Feb-21		◆ Air Compressor Turnover
mover to Commissioning	0%	09-Feb-21		◆ Filter Separators Tumov
ding Complete System Turnover to	0%	17-Feb-21		◆ Wet and Dry Unloadin
Complete System Tumover to	0%	22-Feb-21		◆ Vacuum Conveying 0
over to Commissioning	0%	09-Feb-21		◆ Bin Vent Filter Turnover
Work to Commissioning	23-Sep-20 A	25-Mar-21		25-Mar-21, Co
	4.72% 23-Sep-20 A	25-Mar-21		Commissionin
xer Subsystem	0% 20-Jan-21	29-Jan-21		Commission Pin Mixer Su
lowers Subsystem	0% 28-Jan-21	06-Feb-21		Commission F.A. Blower
mpressor Subsystem	0% 05-Feb-21	14-Feb-21		Commission Air Comp
m Exhausters Subsystem	0% 12-Feb-21	21-Feb-21		Commission Vacuum
Separtators Subsystem	0% 17-Feb-21	21-Feb-21		Commission Filter Se
ent Filter Subsystem	0% 14-Feb-21	18-Feb-21		Commission Bin Vent
m System	0% 23-Feb-21	25-Mar-21		Commission \
ding System	0% 24-Feb-21	28-Feb-21		Commission Unload
ower Subsystem	0% 24-Feb-21	17-Oct-20		Commission Aux Power Subsystem
wor Subsystem	0% 13-Oct-20	04-Oct-21		Continuesion Aux Power Subsystem
all Outage	05-Oct-19 A 100% 05-Oct-19 A	04-Oct-21 14-Oct-19 A	■ Unit No 3 - 2019 Fall Outage	
oring Outage	100% 05-Oct-19 A 100% 18-Apr-20 A	27-Apr-20 A	-	Unit No 3 - 2020 Spring Outage
	·	· ·		
all Outage	0% 10-Oct-20*	02-Nov-20*		Unit No 3 - 2020 Fall Outage
oring Outage	0% 01-May-21*	10-May-21*		■ Unit
all Outage	0% 25-Sep-21*	04-Oct-21*		
				A. Dilina
				A, Miling
			I Pling	▼ 20 Jul 20 A Foundations
				 ▼ 28-Jul-20 A Foundations ◆ Turn Over Silo Area Foundations to Mechanical Contractor
Foundations to Machanical Contractor	10070	20-Jui-20 A	I	▼ Tuni Over Silo Alea i outidations to ivied latitual contractor
	ations to Mechanical Contractor	04-Mar-20 A 04-Mar-20 A 100% 04-Mar-20 A 28-Jul-20 A ations to Mechanical Contractor 100%	04-Mar-20 A 05-Mar-20 A 100% 04-Mar-20 A 05-Mar-20 A 05-Mar-20 A 05-Mar-20 A 28-Jul-20 A 28-Jul-	04-Mar-20 A 05-Mar-20 A ▼ 05-Mar-20 A 100% 04-Mar-20 A 05-Mar-20 A I Piling 28-Jul-20 A 28-Jul-20 A 28-Jul-20 A ations to Mechanical Contractor 100% 28-Jul-20 A

•	y Ash Conversion - Sargent Lundy - Permit			Trimite !	FLY ASH-624_permit_1	26-Oct-20
D	Activity Name	Activity % Complete	Start	Finish	2019	2020 2021
Mechanical		·	19-Aug-20 A	10-May-21	Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Ja	an Feb Mar Apr May Jun Jul Aug Sep Oct Nov Dec Jan Feb Mar Apr May J ▼ 10-N
C3012	Install Unloading Floor Concrete		03-Sep-20 A	10-May-21 10-Sep-20 A		■ Install Unloading Floor Concrete
C3042	Set PDC Building		15-Oct-20	15-Oct-20		Set PDC Building
Silo			19-Aug-20 A	29-Apr-21		29-Apr
C3008	Erect Silo Support Steel		19-Aug-20 A	01-Sep-20 A		Erect Silo Support Steel
C3024	Install Leave Out Steel and Pour Silo Floor Concrete		05-Nov-20	11-Nov-20		☐ Install Leave Out Steel and Pour Silo Floor
C3026	Install Silo Rings	0%	18-Nov-20	19-Jan-21		Install Silo Rings
C3028	Install Silo Roof	0%	20-Jan-21	02-Feb-21		Install Silo Roof
C3033	Install Silo Roof Enclosure	0%	16-Apr-21	29-Apr-21		Install
C3036	Install Silo Piping	0%	19-Mar-21	29-Apr-21		Install
Piping			01-Oct-20 A	10-May-21		▼ 10-N
C3010	Install Equipment & Piping	11.26%	01-Oct-20 A	06-May-21		Instal
C3044	Install Utility Racks, Piping to Precipitator	0%	30-Oct-20	29-Apr-21		Install
C3050	Ash Pipe Tie-In	0%	01-May-21	10-May-21		■ Ash
Pin Mixer			08-Oct-20	17-Feb-21		▼ 17-Feb-21, Pin Mixer
C3016	Install Pin Mixers	0%	08-Oct-20	21-Oct-20		Install Pin Mixers
C3017	Release Pin Mixer to Electrical Contractor	0%		03-Dec-20		◆ Release Pin Mixer to Electrical Contra
C3021	Install Pin Mixer Piping, Valves, and Instruments	0%	04-Dec-20	02-Feb-21		Install Pin Mixer Piping, \
C3031	Release Pin Mixer Piping, Valves and Instruments to Electrical Contractor	0%		17-Feb-21		◆ Release Pin Mixer Pip
Vacuum Exhaus			29-Sep-20 A	04-Feb-21		▼ 04-Feb-21, Vacuum Exh
C3014	Install Vacuum Exhausters		29-Sep-20 A	07-Oct-20		Install Vacuum Exhausters
C3015	Release Vacuum Exhausters to Electrical Contractor	0%		01-Dec-20		♦ Release Vacuum Exhausters to Electr
C3041	Install Vacuum Exhausters Piping, Valves, and Instruments Release Vacuum Exhausters Piping, Valves and Instruments to	0%	02-Dec-20	29-Jan-21		Install Vacuum Exhauster
C3043	Electrical Contractor			04-Feb-21		◆ Release Vacuum Exhau
Fluidizing Air Blo			22-Oct-20	19-Feb-21		▼ 19-Feb-21, Fluidizing
C3018	Install Fluidizing Air Blowers		22-Oct-20	04-Nov-20		Install Fluidizing Air Blowers
C3019	Release Fluidizing System Equipmennt to Electrical Contractor	0%		19-Nov-20		◆ Release Fluidizing System Equipmennt
C3045	Install F.A. Blower Piping, Valves, and Instruments	0%	20-Nov-20	04-Feb-21		Install F.A. Blower Piping
C3046	Release F.A. Blower Piping, Valves and Instruments to Electrical Contractor	0%		19-Feb-21		◆ Release F.A. Blower
Air Compressor			22-Oct-20	12-Feb-21		▼ 12-Feb-21, Air Compre
C3022	Install Air Compressor		22-Oct-20	04-Nov-20		Install Air Compressor
C3023	Release Air Compressor to Electrical Contractor	0%		19-Nov-20		◆ Release Air Compressor to Electrical Cor
C3047	Install Air Compressor Piping, Valves, and Instruments		20-Nov-20	04-Feb-21		Install Air Compressor Pi
C3048	Release Air Compressor Piping, Valves and Instruments to Electrical Contractor	0%		12-Feb-21		◆ Release Air Compress
Bin Vent Filter			03-Feb-21	19-Mar-21		▼ 19-Mar-21, Bin
C3034	Install Bin Vent Filter	0%	03-Feb-21	16-Feb-21		Install Bin Vent Filter
C3039	Release Bin Vent Filter to Electrical Contractor	0%		10-Mar-21		◆ Release Bin Vent
C3051	Release Bin Vent Filter Piping, Valves and Instruments to Electrical Contractor	0%		19-Mar-21		♦ Release Bin Ve
Filter Separtors			03-Feb-21	26-Mar-21		▼ 26-Mar-21, Fi
C3032	Install Filter Separators		03-Feb-21	16-Feb-21		Install Filter Separato
C3037	Release Filter Separators to Electrical Contractor	0%		10-Mar-21		◆ Release Filter Se
Actual Work	Critical Remaining Work				Page 22 of 23	TASK filter: All Activities
Remaining Wo					·g	© Oracle Corp

3.0 PROJECT SCHEDULE: NARRATIVE DISCUSSION

This section presents a narrative of the project steps and sequencing necessary to develop the alternative disposal capacity selected to replace the existing FAR II. This narrative follows and supplements the visual timeline representation of the project schedule provided in Section 2.0.

Section 3.1 presents the engineering and design activities for the balance-of-plant components to support the dry fly ash system and for the dry fly ash system itself. Section 3.2 Section 3.2 discusses the steps required to procure the vacuum-pneumatic conveying system and its auxiliary components and the contracting strategy employed for this project. Finally, Section 3.4 presents the construction activities for this project and the general sequence in which those activities will be executed, ultimately concluding with the commissioning of the new dry fly ash-handling system.

See Section 4.0 for a narrative discussion of the progress the Cardinal Operating Company has made to date in developing this alternative disposal capacity for FAR II.

3.1 ENGINEERING & DESIGN

The engineering and design phase of the project was divided into two sub-phases: (1) balance-of-plant (BOP) and (2) fly ash system. The design work for these two sub-phases were awarded under two separate contracts, with the latter being a portion of the overall contract for the vacuum-pneumatic conveying system.

3.1.1 BALANCE-OF-PLANT COMPONENTS

Detailed engineering and design of the BOP components for the vacuum-pneumatic conveying system started in March of 2019, shortly after the completion of the dry fly ash conversion study (see Section 1.3.3). This work required coordination between several different engineering disciplines. The following subsections summarize the BOP engineering effort for this project as it pertained to each discipline.

3.1.1.1 **GENERAL**

The general engineering and design tasks for this project began once the project was initially authorized in March of 2019 and are scheduled to last approximately 15 months. This work primarily includes project planning activities and tasks that do not belong to a specific discipline.

3.1.1.1.1 PROJECT PLANNING

The initial project planning and engineering tasks began at the authorization of the project in March 2019 and were completed a few months thereafter. These activities included the development of the project design basis, the creation of a three-dimensional computer model to aid in engineering and design tasks, and

updating the project cost estimate per changes made to the design since the conceptual design study. The project design basis, which provides the design requirements of the project and design inputs for the engineering and design work, was drafted within two months of the project starting and was finalized at the end of July 2019. Meanwhile, the plant and equipment modeling work continued through the engineering and design work, with updates to the model being made in accordance with the progress made in the detailed engineering and design work. The modeling was substantially completed in early 2020 once most of the civil, structural, and mechanical design work was finished.

3.1.1.1.2 PERMITTING SUPPORT

The engineering tasks to support the Cardinal Operating Company's air emissions permit application also started once the project was initiated in March of 2019. Specifically, an air emissions plan and calculation were prepared to be included in the air emissions permit application submitted to the Ohio EPA. These documents took three months to complete, which enabled the Cardinal Operating Company to submit the corresponding permit application at the end of June 2019. Permitting support was provided as needed throughout the agency's review of the application. The air emissions permit for the project was ultimately approved at the end of August 2019, approximately two months after the application was submitted to the agency.

3.1.1.1.3 PROJECT CLOSE-OUT

The remaining general engineering and design activities, primarily construction support and drawing closeout, will be performed during and after construction of the new vacuum-pneumatic handling system and auxiliary components. Specifically, construction support is scheduled to last throughout construction at all three units (early 2020 at Unit 1 through spring 2021 at Unit 3), while drawing close-out will not start until construction is complete and the fly ash system vendor is optimizing and tuning the system for Cardinal's use. Both tasks will be completed by the day after start-up and implementation of the new dry fly ash system on June 7, 2021.

3.1.1.2 CIVIL & STRUCTURAL

The engineering and design work for the civil and structural aspects of the project began once the project was authorized in March of 2019. This discipline's work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing and/or designing:

- Civil sitework and grading plans;
- Roads and paving for the updated site layout;
- Foundations for:
 - Fly ash storage silos;
 - PDCs and transformers,

- Utility racks supporting new piping, cable tray, etc.,
- o Exhauster blowers, and
- Miscellaneous equipment pads and pipe supports;
- Structural steel for:
 - o Utility racks supporting new piping, cable tray, etc.,
 - Shelters for the exhauster blowers, and
 - o Miscellaneous stairs, platforms, and walkways.

In addition to the preceding engineering work, a geotechnical evaluation was performed to assess the soil boring data and laboratory test results received from the contractor that performed the subsurface investigation for the project. This data ultimately provided design inputs for the project's various foundations. Given its nature, this work was performed concurrently with the subsurface investigation at the site and concluded when the contractor issued its final report.

Due to the design inputs required from the subsurface investigation (see Section 3.3.1), fly ash system vendor, and other engineering disciplines, most of the detailed civil and structural engineering and design work started in the summer or fall of 2019. The early work primarily entailed developing initial layouts for the pipe racks supporting new piping for the dry fly ash system and the site in general. Each detailed design task was generally completed within a few months after it was started, with most design tasks being substantially complete by December 2019. Notably, the pile designs for the ash storage silo foundations were completed about a month earlier to facilitate earlier procurement of and ultimately an earlier start date for the contractor installing the piles relative to the other contractions hired to install the dry fly ash system and its ancillary components.

Issuance of construction drawings was generally scheduled to align with the awarding of the contract for which a given scope of work was included. The piling drawings were issued first in late January 2020 shortly after the piling contractor was selected. This enabled the piling contractor to mobilize to the site and start installing piles prior to the other construction work in 2020. The remaining foundation and civil work drawings were issued approximately six weeks later, shortly after the civil/substructure contractor was selected in early March 2020. Meanwhile, structural steel drawings for the pipe racks were released for fabrication in late February 2020 as the Cardinal Operating Company was conforming the structural steel fabrication contract with the designated fabricator.

Overall, the civil and structural engineering and design work was completed within about a year after the project began (*i.e.*, March 2019 to March 2020).

3.1.1.3 MECHANICAL

Like the civil and structural engineering design scope, the mechanical engineering and design work began once the project was authorized in March 2019. This discipline's work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing and/or designing:

- General arrangement drawings;
- Equipment location drawings;
- Service water, fly ash, and underground piping, including:
 - Pipe and instrumentation diagrams (P&IDs),
 - o Isometrics, and
 - Supports, including auxiliary steel; and
- Demolition work for existing electrostatic precipitator piping.

The early mechanical engineering work took approximately four months to complete (March 2019 through July 2019) and included preparing general arrangements of the project site and, similar to the structural and civil work, developing initial layouts for equipment, piping, and auxiliary steel. P&IDs corresponding to the proposed layouts were then prepared and subsequently issued to the Cardinal Operating Company for review.

As the Cardinal Operating Company was reviewing P&IDs, the underground, service water, and ash piping for the project were routed and analyzed. Upon establishing an initial route and during the initial analysis, preliminary pipe supports were located and designed in coordination with the structural and auxiliary steel design. Pipe analyses and isometrics were mostly completed by early November 2019 when the underground work was issued for bids from potential contractors for the substructure work. Demolition drawings for existing piping to be removed from the station's ESPs were also prepared in the fall of 2019. Finally, equipment location drawings were started in October 2019, shortly after the fly ash system vendor received a full notice to proceed with its design.

By December of 2019, the final analyses for the ash and service water piping were completed. The corresponding pipe support drawings and bills of materials were substantially completed about a month later, as were construction drawings for the pipe demolition work in the ESPs. At this time, equipment locations drawings had also been prepared and were ready to be issued. These mechanical drawings, in addition to those previously issued, were ultimately incorporated into the bid package issued to potential contractors that would be responsible for installing the project's mechanical equipment ("mechanical general work contractor").

Issuance of construction drawings was generally scheduled to align with the awarding of the mechanical general work contract in late April 2020. Drawings and design documents issued at that time included the project's P&IDs, service water and ash piping isometrics, and pipe supports.

Overall, the mechanical engineering and design work was completed about 13 months after the project began (*i.e.*, March 2019 to April 2020). As-built drawings are also scheduled to be prepared near the end of construction in the spring of 2021 and are anticipated to take approximately one month to complete.

3.1.1.4 ELECTRICAL

Like the previous disciplines, the electrical engineering and design work began once the project was authorized in March 2019. This discipline's work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing and/or designing:

- Project electrical load list,
- Single line and phasing diagrams,
- Auxiliary power and arc flash studies,
- Grounding,
- Underground duct banks,
- · Lighting,
- Electrical installation drawings, and
- Cable and cable tray routing,
- Updating the project electrical load list as necessary,
- Performing relay setting calculations, and
- Preparing electrical schematic diagrams, wiring drawings, and cable tabulations.

Given that most of the electrical work for the project will not be installed until relatively late in the construction schedule, the corresponding detailed electrical engineering and design work was not scheduled to start until January 2020. Notable exceptions to this were the initial set up of the electrical load list, key diagrams and electrical load drawings, all of which were done concurrently with the initial set up work by the other engineering disciplines. Single line diagrams were also developed throughout the summer of 2019 to conceptualize the overall auxiliary power design. Finally, electrical-related work within the substructure scope of work was also started in the summer of 2019 to be included in the bid package and subsequent construction issue in November 2019 and March 2020, respectively. This work included the designs for electrical grounding and the underground ductbanks.

In January 2020, work began on preparing the detailed wiring diagrams (*i.e.*, three-line diagrams) and the lighting design for the project. Cable tray routes were also starting to be established at this time. Approximately two months into this design work, the detailed design work for the electrical installation

drawings also started. This work was substantially completed by mid-April 2020 and was incorporated into the bid packaged issued to potential contractors that would be responsible for installing the project's electrical equipment ("electrical general work contractor").

Issuance of construction drawings was generally scheduled to align with the awarding of the electrical general work contract in July 2020. From February to July 2020, schematic diagrams and cable tabulations were prepared for the fly ash and auxiliary power systems, and the key diagrams, three-line diagrams, lighting design, and cable tray design were all finalized and issued for construction. Shortly thereafter, wiring drawings and relay setting calculations were also issued (late-July and mid-August 2020, respectively).

Overall, the electrical engineering and design work is scheduled to be completed about 20 months after the project began (*i.e.*, March 2019 to November 2020). As-built electrical drawings and an update to the existing arc flash study are both scheduled to be completed by the beginning of November 2020.

3.1.1.5 INSTRUMENTATION AND CONTROLS (I&C)

The engineering and design work for the project's instrumentation and controls (I&C) began in May 2019 after the project design basis had been drafted and issued to the Cardinal Operating Company for review. The work was divided into initial work and final work. Like the other engineering disciplines on this project, the I&C engineering and design work was performed concurrently with the other engineering and design work and included preparing and/or designing:

- DCS network architecture drawings;
- Control logic; and
- Instrument lists, data sheets, installation details, and location drawings.

Once the project design basis was drafted in mid-May 2019, preparation of the DCS network architecture drawings commenced. These drawings were finalized and issued for use approximately three months later.

Following the issuance of P&IDs for design by the mechanical discipline in late October 2019, the BOP instrument list, data sheets, and installation details were started. These documents took approximately six weeks to finalize.

Finally, the BOP control logic and instrument location drawings were started once the mechanical discipline issued the equipment location drawings for use in early January 2020. The BOP control logic was prepared within a week and a half and subsequently issued to the DCS vendor for use. Meanwhile, the BOP instrument location drawings were completed approximately three months later in late March 2020 and issued for construction to the vendor performing the DCS modifications.

Overall, the I&C engineering and design work is scheduled to be completed about 25 months after the discipline's work started in May 2019. While most of the engineering and design work has been substantially completed, as-built I&C drawings still need to be prepared. These as-built drawings will not be prepared until the fly ash system vendor has started tuning and optimizing the system for Cardinal in May 2021.

3.1.2 VACUUM-PNEUMATIC CONVEYING SYSTEM

Once the project was authorized in March of 2019, the Cardinal Operating Company began preparing a technical specification detailing the requirements for designing, furnishing, manufacturing, and delivering the vacuum-pneumatic conveying system for/to Cardinal. One month later, the Cardinal Operating Company started soliciting bids from potential vendors. The bidding period last approximately one month, and the Cardinal Operating Company took six weeks to evaluate the bids, select a vendor, and enter into commercial negotiations with the vendor. During commercial negotiations, the technical specification issued with the bid package was also conformed in accordance with the forthcoming contract requirements.

The vacuum-pneumatic conveying system contract was awarded in two phases. The first phase, which was awarded in early August 2019, was for the engineering and design scope of work. This limited notice-to-proceed (LNTP) also allowed the fly ash system vendor to start the project planning process. Because the fly ash storage silos would be the first pieces of equipment of the new dry fly ash-handling system to be installed on site, this LNTP also included the fabrication and delivery of the silos. The fly ash system vendor was awarded the full contract about two months later in later September 2019, which included the full fabrication and delivery scope of work.

Like the engineering and design work for the BOP components of the project, the fly ash system work required coordination between several different engineering disciplines. Accordingly, the following subsections summarize the engineering and design work performed by the dry fly ash system vendor as it pertained to each discipline.

3.1.2.1 **GENERAL**

The general engineering and design tasks for the vacuum-pneumatic conveying system began once the vendor received its LNTP from the Cardinal Operating Company in early August 2019. Like the general BOP engineering and design work, this work primarily includes planning level activities and tasks that do not belong to a specific discipline.

3.1.2.1.1 PROJECT PLANNING

Upon receiving a LNTP, the dry fly ash system vendor began developing a plan to execute the project.

Project planning activities included compiling a master document list, documenting the dry fly ash system

design criteria, and preparing schedules for drawing submittals and for executing the overall project. These tasks were all prepared concurrently with each other and were finalized by the end of August 2019.

3.1.2.1.2 QUALITY ASSURANCE / QUALITY CONTROL

Concurrent with completing its initial equipment and component lists, the fly ash system vendor prepared quality assurance/quality control (QA/QC) manuals and safety manuals for the system equipment. These manuals also included testing and acceptance procedures to be performed at the end of the project during the commissioning, optimization, and tuning phases. Corresponding inspection and test plans were also prepared. These QA/QC and commissioning planning documents took approximately three months to prepare and were all submitted to the Cardinal Operating Company at the end of October 2019.

3.1.2.1.3 CONSTRUCTION AND OPERATION AND MAINTENANCE (O&M) SUPPORT

To support the contractors installing its dry fly ash system design at Cardinal, the fly ash system vendor prepared a terminal point table; erection drawings, instructions, and procedures; and a list of special tools required for erection, maintenance, and repair. In addition, the vendor prepared operation and maintenance (O&M) manuals for the system and its components. These documents were prepared and submitted to the Cardinal Operating Company by the time the mechanical general work contractor began mobilizing the site to start installing the fly ash storage silo at Unit 1 (late April 2020).

3.1.2.2 MECHANICAL

The fly ash system vendor began performing the mechanical engineering and design work once the vendor was given a LNTP from the Cardinal Operating Company in early August 2019. This discipline's work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing, reviewing, and/or designing:

- · General arrangement drawings;
- · Process flow diagrams;
- P&IDs;
- Mechanical equipment, valve, and line lists;
- · Equipment supplier drawings;
- · Piping arrangement drawings;
- · Exhauster physical certified drawings; and
- Mechanical equipment data sheets.

At the onset of the dry fly ash system design, the vendor prepared general arrangement drawings, key process flow diagrams, and P&IDs for the system. These initial design documents were prepared concurrently and submitted to the Cardinal Operating Company for review within six weeks of receiving the

LNTP. The process flow diagrams were finalized approximately one month later in mid-October 2019. Shortly thereafter, the vendor submitted the mechanical equipment, valve, and line lists.

Throughout the fall and winter of 2019, the fly ash system vendor analyzed and designed the piping for the fly ash storage silo at each unit, which included conveyor piping, vacuum transport air piping, water piping, silo fluidizing piping, and compressed air piping. This engineering and design work was substantially completed in the first quarter of 2020, after which the general arrangement and piping arrangement drawings were finalized. By the beginning of March of 2020, approximately seven months after receiving the LNTP, the mechanical engineering and design work for the dry fly ash system piping was substantially completed.

3.1.2.3 ELECTRICAL

The fly ash system vendor also started the electrical engineering and design work when the vendor received the LNTP from the Cardinal Operating Company in early August 2019. This discipline's work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing and/or reviewing:

- Electrical load lists,
- Single line diagrams,
- Motor data sheets,
- Schematic and wiring diagrams,
- Motor curves, and
- Electrical equipment data sheets.

The vendor first prepared the electrical loads required for to power the system and the corresponding single line diagrams to conceptualize the system's auxiliary power design. The vendor completed these design documents less than two months after receiving the LNTP and subsequently submitted them to the Cardinal Operating Company for review. Like the mechanical arrangements and diagrams, these electrical design inputs were finalized about one month later. Finally, motor data sheets were prepared and submitted by the end of November 2019.

Through the fall and winter of 2019 and extending into the first quarter of 2020, the vendor prepared the schematic and wiring diagrams for the electrical design of the dry fly ash system. The initial sheets were submitted to the Cardinal Operating Company to review in early January 2020 and were finalized by the end of March 2020. Around the same time, the vendor finalized the performance curves for the system's motors as well as the electrical equipment data sheets. Thus, the vendor's electrical engineering and design work was substantially completed by the end of March 2020.

3.1.2.4 INSTRUMENTATION AND CONTROLS (I&C)

The I&C work for the dry fly ash system commenced right after the initial mechanical and electrical design documents were prepared and submitted for review in September 2020. This work was performed concurrently with the design and engineering work performed by the other disciplines and included preparing, reviewing, and/or designing:

- Instrument lists,
- Input and output lists,
- Instrument data sheets,
- Instrument installation details and location drawings,
- Functional descriptions,
- Control logic diagrams, and
- DCS graphic screens.

After preparing the initial P&IDs and process flow diagrams, the vendor began preparing a list of the instruments required for the system as well as input and output data. These initial design documents were submitted to the Cardinal Operating Company for review in October 2019, and the instrument list was finalized in early November 2019.

Concurrent with the mechanical and electrical engineering work, the vendor performed the I&C engineering work throughout the fall and winter of 2019. During this time, the vendor prepared a functional description for the system, control logic diagrams, and sketches for the DCS graphic screens. These documents were submitted for review in March 2020 as the vendor was finalizing the mechanical and electrical designs. Approximately three months later, in mid-June of 2020, the I&C engineering and design work was substantially completed.

3.1.2.5 STRUCTURAL

The structural engineering and design of the fly ash storage silo and corresponding stair tower also started in early August 2019 with the other engineering and design work and was performed concurrently with the other engineering disciplines. In addition to the design of the silo structure and stair tower, this work included preparing, reviewing, and/or designing pipe fittings and anchor rod setting plans for the silo structure.

To support the engineering and design of the silo foundation, the initial structural engineering and design work focused on preparing preliminary design drawings and loading diagrams. These preliminary design documents were submitted in September 2019. To support the issuance of the piling and substructure bid packages in November of 2019, the vendor calculated not-to-exceed loads to be used as inputs in the foundation design.

Final engineering and design work for the storage silo and stair tower structure continued through the first quarter of 2020. Like the mechanical and electrical engineering and design work, the structural engineering and design for the storage silo and stair tower structure were substantially completed by mid-March 2020. At this time, the vendor submitted final structural design drawings which included final structural steel member sizes, structural steel connection information, base plate information, and anchor rod setting plans.

3.2 PERMITTING

In order to install the new dry fly ash-handling system at Cardinal, the Cardinal Operating Company needed to obtain an air pollution permit-to-install (PTI) from the Ohio EPA. After drafting the project design basis in May of 2019, the Cardinal Operating Company began preparing the necessary PTI application forms and assembling relevant input data (e.g., pin mixer ash moisture content, fly ash throughput, anticipated truck traffic to FAR I Landfill). This work was performed concurrently with the air emissions plan and calculation in the engineering and design phase of the project, both of which were included in the PTI application.

The air pollution PTI application was submitted to the Ohio EPA in late June 2019, about one month after the permitting support work started. Approximately two months later, in late August 2019, the Ohio EPA issued the final PTI authorizing the Cardinal Operating Company to construct and operate the new dry fly ash-handling system at Cardinal.

3.3 PROCUREMENT

The procurement phase of the project schedule was generally divided amongst the scopes of work for the different engineering disciplines involved in this project. However, the procurement work for this project was generally executed in the following three subphases:

- 1. Site Investigations
- 2. Contractor Selection
- 3. Equipment Fabrication and Delivery

3.3.1 SITE INVESTIGATIONS

Two investigations were performed at the project site in the fall of 2019 to obtain geotechnical and underground utility data. This data was required for necessary design inputs for the foundation and substructure designs, civil site work, and underground utility routing. Two contractors were procured to perform these site investigations: a geotechnical consultant to perform the subsurface investigation and a surveyor to perform the underground surveys.

The procurement schedules for both contractors were generally concurrent with each other. Technical specifications for both contracts were prepared once the project was initiated in March of 2019 and took

approximately one month to prepare, review, and issue to the respective bidders. The bid periods for both contracts concluded at the end of May 2019.

3.3.1.1 SUBSURFACE INVESTIGATION

Given the need to obtain geotechnical data to support the foundation design work being performed in the fall of 2019, the subsurface investigation contract was awarded first. This contract was awarded at the end of July 2019 approximately two months after the bid period ended during which the Cardinal Operating Company evaluated the bids, selected a contractor, negotiated the commercial terms and conditions, and finally conformed the technical specification.

Upon being awarded the contract, the geotechnical contractor began mobilizing its crew, drill rigs, *etc.* to the project site, which took approximately one month. Once fully mobilized, the contractor began drilling soil borings and collecting soil samples for laboratory testing. Throughout the field and laboratory work, the contractor compiled the requested geotechnical data into a report which was issued in mid-September 2019. Afterwards, the geotechnical data was evaluated and incorporated into the foundation and subsurface designs (see Section 3.1.1).

3.3.1.2 UNDERGROUND SURVEY

While going through the process of awarding the subsurface investigation contract, the Cardinal Operating Company was also evaluating the bids received for the underground survey work. Commercial negotiations with the selected contractor were initiated just before the geotechnical contract was awarded and continued through the end of September 2019. The underground survey contract was then awarded approximately two weeks later in mid-October 2019.

The surveyor mobilized to the project site within three weeks after being awarded the contract. In addition to mapping underground utilities, the surveyor also prepared some topographic maps of the project site. This data was ultimately incorporated into the civil and substructure designs and provided to the substructure contractor for information. Accordingly, the surveys were performed throughout the engineering and design phase in the fall of 2019 and were finalized in mid-February 2020, just before the substructure contract was awarded in early March 2020.

3.3.2 CONTRACTOR SELECTION

The Cardinal Operating Company opted to hire four different contractors for the dry fly ash conversion project at Cardinal, each corresponding to a different phase of construction. As outlined in Section 3.4, construction was divided into the following four principal phases:

- 1. Piling
- 2. Foundations (i.e., Substructures)

- 3. Mechanical General Work
- 4. Electrical General Work

In addition to the four preceding installation contractors, the Cardinal Operating Company also hired a testing and inspection agency, a structural steel fabricator, a vendor for the electric PDC building and transformers, and a vendor for the DCS modifications.

The following subsections provide detailed narratives of the selection process for each of the preceding contractors and vendors and their respective timelines.

3.3.2.1 INSTALLATION CONTRACTORS

3.3.2.1.1 PILING WORK

Since the piles under the new fly ash storage silos would need to be installed before the other substructure elements in the area, the first installation contract that the Cardinal Operating Company awarded for the dry fly ash conversion project was that for the piling work. As previously stated in Section 3.1.1, the piling engineering and design work was mostly completed by November of 2019, and shortly thereafter the work was issued for bids from prospective piling contractors. Prior to issuing the bid package, the corresponding technical specification was prepared simultaneously with the piling design and took approximately two months to prepare, review, and finalize (September 2019 to November 2019).

The bid period for the piling work lasted about a month, after which the Cardinal Operating Company began evaluating the bids. By mid-December of 2019, the Cardinal Operating Company had selected a contractor and entered into commercial negotiations. Following negotiations and conforming of the technical specification for the contract, the Cardinal Operating Company awarded the piling contract in mid-January 2020, approximately six weeks after the corresponding bid period had ended.

Within two weeks after being awarded the contract, the piling contractor submitted several documents to the Cardinal Operating Company, including its plan for installing the piles required for the project (equipment, sequence, *etc.*), grout mix design, shop drawings for the reinforcing steel, and pile testing criteria. These documents were reviewed and subsequently approved prior to the first pile being installed in January of 2020.

3.3.2.1.2 SUBSTRUCTURE WORK

Because the storage silo foundation work could start after the piles had been installed, the second contract the Cardinal Operating Company awarded for this project was that for the substructure work. Given that the foundation engineering and design work was performed concurrently with the piling engineering and design, the substructure package was issued for bids about a month later (early December 2019). The technical

specification for the work was started in mid-May of 2019 after the project design basis was drafted and much of the initial project layout had been completed (including preliminary general arrangement drawings). Preparation of the specification continued through the summer and fall of 2019 concurrent with the corresponding engineering and design work.

Like the piling contract, the bid period for the substructure contract lasted about a month. The Cardinal Operating Company then spent January and February of 2019 evaluating the bids, selecting a contractor, entering into negotiations with the selected contractor, and conforming the technical specification with the contract. The substructure contract was ultimately awarded in early March 2020. This contract was awarded just as the piling work was being finished at Unit 1, so the substructure contractor was able to immediately mobilize to the Unit 1 area and start the foundation work for that unit's vacuum-pneumatic conveying system.

3.3.2.1.3 MECHANICAL GENERAL WORK

The third contract issued by the Cardinal Operating Company for the dry fly ash conversion project covered most of the aboveground installation work except for the electrical equipment and components to be installed later by the electrical general work contractor. This contract covered demolition of existing site structures or systems necessary to install the new dry fly ash system, installation of the fly ash storage silos and dry fly ash-handling equipment furnished by the fly ash system vendor, installation of BOP piping and supports, and erection of BOP structural steel. Accordingly, this contract was awarded after the structural and mechanical engineering and design work (both for BOP and for the dry fly ash system) were substantially completed in mid-April 2020.

Preparation of the mechanical general work specification began in December of 2019, just after the bid period for the piling work started. Given the amount of engineering and design work already completed to that point, the corresponding specification was prepared within a month, and the bid package was issued in early January 2020. Based on the large scope of work for this contract relative to the other contracts issued for this project, the bid period lasted until early March 2020, a duration of approximately two months. The Cardinal Operating Company then spent six weeks reviewing the bids it received for the work, selecting a contractor, and entering into commercial negotiations with that contractor. Shortly after concluding commercial negotiations, the technical specification was conformed in accordance with the contract. The mechanical general work contract was ultimately awarded at the end of April 2020, which allowed the contractor to start mobilizing to the site in May of 2020.

3.3.2.1.4 ELECTRICAL GENERAL WORK

Since the electrical equipment and components would not need to be installed until later in the construction schedule, the Cardinal Operating Company awarded the electrical general work contract last of the four installation contracts. The scope of this contract included furnishing, installing, and/or testing, multi-voltage

switchgear modifications to provide power to the fly ash storage silo area; DCS hardware in the control system and in the PDCs; all power, grounding, instrument and control cables to and from equipment in the fly ash storage silo area; all conduit and raceway; and all lighting, receptacles, and lighting protection in the fly ash storage silo area.

Preparation of the electrical general work specification began in early March of 2020, just as the grounding and underground ductbank designs were being issued for construction by the substructure contractor. Given the scope of work involved relative to the other three installation contracts, this specification took about six weeks to prepare and was issued in a bid package to prospective contractors in mid-April 2020. The bid period also lasted approximately six weeks, after which the Cardinal Operating Company began evaluating bids. The bid evaluation, commercial negation, and specification conformance period for this specification ultimately took about a month to complete, and the electrical general work contract was awarded in early July 2020.

3.3.2.2 VENDORS/SUPPLIERS

3.3.2.2.1 CIVIL WORKS TESTING AND INSPECTION

For this project, the Cardinal Operating Company hired an independent contractor to inspect and, where necessary, test the civil work performed by the installation contractors. This scope of work included testing and inspecting the following civil engineering-related works: concrete, earthwork, aggregate surfacing, and road pavement.

The technical specification for this work was prepared concurrently with the civil and structural engineering and design work during the fall of 2019 and was issued for bids from prospective testing and inspection agencies in mid-October 2019 (after the piling work and before the substructure work). The bid period lasted approximately three weeks, after which the Cardinal Operating Company evaluated the bids. A testing and inspection contractor was selected by early December 2019, after which commercial negotiations began. Approximately one month later, in mid-January of 2020, the testing and inspection specification was conformed and the contract was awarded. The timing of this contract was such that the testing and inspection contractor could mobilize to the project site by the time the substructure contractor was hired and had started mobilizing to the site (early March 2020).

3.3.2.2.2 STRUCTURAL STEEL FABRICATION

While the mechanical general work contractor would be responsible for erecting the structural steel, the Cardinal Operating Company hired a separate structural steel fabricator to furnish the structural steel needed for the project. This scope of work included furnishing the structural steel required for utility racks for piping,

the exhauster blower shelters, access platforms, and stairs. This contract also included various walkway components including grating, handrail, guard plate, ladders, stair treads, and stair stringers.

Preparation of the technical specification for the structural steel fabrication work began in October of 2019, shortly after the steel design work started for the exhauster blower shelters (the utility rack steel had been well underway at this point). The specification was then issued for bids from potential fabricators at the end of November 2019. The bid period lasted approximately three weeks, after which the Cardinal Operating Company started evaluating the bids. A structural steel fabricator was selected by the end of December 2019, after which commercial negotiations began and the specification was conformed. By the beginning of March 2020, the structural steel fabrication contract was awarded. This timeline ultimately provided the fabricator with a four-month window before the structural steel was to be erected for the Unit 1 utility rack (July 2020).

3.3.2.2.3 ELECTRIC PDCS AND TRANSFORMERS

Because the new dry fly ash system equipment installed at Cardinal Units 1, 2, and 3 will add new electrical loads to each unit, new electric power distribution equipment is required. This equipment will be located in a pre-fabricated PDC building installed at each unit. In addition, outdoor step-down transformers are also required to reduce the electrical voltage from the existing equipment to the new equipment. Accordingly, the Cardinal Operating Company hired a vendor to design, manufacture, furnish, test, inspect, and deliver three dry-type transformers, three PDCs, and all auxiliary equipment and components required for the PDCs.

Once the BOP electrical load list had been prepared and reviewed in late May of 2019, the technical specification for the electric PDCs and transformers was started. Just over two months later, in August of 2019, the bid package was issued to prospective vendors. Bidders were given approximately six weeks to submit their bids, which the Cardinal Operating Company began evaluating in mid-September 2019. A vendor was selected in late October 2019, with whom the Cardinal Operating Company began commercial negotiations and conforming the technical specification with the forthcoming contract. The contract was ultimately awarded in late November 2019, after the which the vendor began preparing schematic diagrams, wiring diagrams, and equipment layout and arrangement drawings.

3.3.2.2.4 DCS MODIFICATIONS

The Cardinal Operating Company opted have control and monitoring of the new dry fly ash system be implemented in the station DCS. Thus, the PLC programming and I/O hardware for the new components will be added and integrated into the station DCS. Meanwhile, all existing PLC programming for existing fly ash system equipment remaining in service will be migrated into the station DCS. To execute this work, the Cardinal Operating Company elected to hire a vendor Accordingly, the Cardinal Operating Company hired a

vendor to design, procure, fabricate, test, implement, configure, program, deliver, and provide commissioning support for these DCS modifications.

The Cardinal Operating Company began preparing the technical specification for the Cardinal DCS modifications in late July 2019, just after the DCS network drawings were issued for design. Preparation work continued through the preparation of P&IDs, which were issued for design in mid-October 2019. The specification was then finalized in late-November 2019 and issued for bids from prospective vendors. The bid period lasted approximately six weeks, concluding in mid-January 2020. The Cardinal Operating Company then spent approximately six weeks evaluating the bids, selecting a vendor, negotiating the contract with the selected vendor, and conforming the specification with the forthcoming contract requirements. The contract for performing the required DCS modifications was ultimately executed in mid-March 2020.

3.3.3 EQUIPMENT FABRICATION & DELIVERY

The major equipment and materials being fabricated for the dry fly ash conversion project at Cardinal are the equipment and materials for the dry fly ash system itself, structural steel for the utility racks, the PDCs and transformers, and the DCS modifications. The following subsections provide narratives of how these items will be fabricated and delivered to the Cardinal site.

3.3.3.1 FLY ASH SYSTEM

Concurrent with its engineering and design work, the fly ash vendor began procuring the equipment and materials needed for the dry fly ash system. In general, the vendor worked with other third-party vendors to procure and/or fabricate the equipment and piping required for the dry fly ash system consistent with its engineering and design. The procurement process would start with a material release and purchase order submitted to the specific third-party vendor, after which the third-party vendor would submit shop drawings to fly ash system vendor to review and approve. Once approve, the equipment was fabricated, then inspected, and finally shipped to the Cardinal site to be installed by the mechanical general work contractor. In general, this process took approximately six to eight months from material release to delivery at the Cardinal site.

The fly ash system vendor had equipment and material procured in accordance with its forecasted installation sequence. Because the Unit 1 fly ash storage silo, bin vent filter, and vacuum exhausters were scheduled to be installed first, these items were the first to be procured and were included in the LNTP that the Cardinal Operating Company gave to the vendor in early August 2019. Starting in early October 2019, the vendor began placing material orders for the dry unloading chutes and vent fans, fluidizing air system, compressed air system and piping, and the pin mixer for Unit 1. A few weeks later, the filter separators and the balance of the system piping (including valves and supports) were ordered. This procurement sequence facilitated the delivery of each piece of equipment and component to the Cardinal site in advance of when the mechanical general work contractor was scheduled to install it.

All of the equipment and components for the Unit 1 dry fly ash system were delivered to the site between June and July of 2020. Like most of this project, the procurement of the equipment and components for the Unit 2 and 3 dry fly ash systems followed a similar sequence as that for Unit 1 but with staggered starts by about two to four weeks. Consequently, the Unit 2 and 3 dry fly ash system equipment and components were delivered to the project site by mid-August 2020 and mid-September 2020, respectively. Overall, the equipment fabrication and delivery process for the dry fly ash systems at all three units took approximately 13 months to complete following the LNTP issued by the Cardinal Operating Company in August of 2019.

3.3.3.2 STRUCTURAL STEEL

To fabricate the structural steel for the project's utility racks, exhauster blower shelters, and miscellaneous platforms and stairs, the fabricator first prepared shop drawings based on the structural design drawings. These shop drawings were started in early March 2020 after the fabricator was awarded the contract and approximately two weeks after the corresponding design drawings had been issued for construction. Because construction of the Unit 1 dry fly ash system was scheduled to occur first, the fabricator began preparing the corresponding shop drawings for the structural steel to be erected at Unit 1, followed by the Unit 2 and 3 shop drawings a few weeks later. After the shop drawings were accepted by the Cardinal Operating Company, the fabricator began fabricating the steel components at its shop.

In general, the structural steel for all three units was fabricated in a concurrent fashion with staggered start times based on the anticipated erection date for each unit. The steel for Unit 1 was ultimately delivered to the site at the end of June 2020, approximately one week before it was erected by the mechanical general work contractor. The steel for Units 2 and 3 followed several weeks thereafter, arriving to the project site in mid-July 2020 and early September 2020, respectively.

3.3.3.3 ELECTRIC PDCS & TRANSFORMERS

Upon receiving the contract for the electric PDCs and transformers for the new dry fly ash system in late November 2019, the vendor began preparing the corresponding wiring diagrams, schematic diagrams, and physical drawings. These drawings were submitted to the Cardinal Operating Company for review approximately two to three months later and ultimately finalized by mid-March 2020.

Once the design drawings were approved, the vendor began fabricating the PDC equipment and the transformers. The vendor generally fabricated the equipment for all three units in a continuous fashion with staggered start dates for each unit. Because construction would start at Unit 1 first, the vendor started fabricating the Unit 1 equipment first and then continued with the equipment for Units 2 and 3. The PDC equipment for each unit was completed and delivered to the PDC assembly shop by the end of July 2020.

Following delivery of a unit's PDC equipment to the shop, that unit's PDC was assembled. This process generally took three to four weeks, and the PDCs for all three units were assembled by the end of August 2020. Prior to delivery to the site, each PDC will undergo a factory acceptance test (FAT) to ensure it was built and operates in accordance with the design specifications. Following a successful FAT, the PDC will be shipped to Cardinal for installation at its corresponding unit.

The PDCs and transformers underwent FATs in early September (Unit 1) and early October (Units 2 and 3). Approximately one week after their respective successful FATs, the PDCs and transformers for Units 1, 2, and 3 were delivered to the project site. The Unit 1 set arrived on September 24, 2020, while the PDCs and transformers for Units 2 and 3 arrived on October 14, 2020.

3.3.3.4 DCS HARDWARE & SOFTWARE

After being awarded the DCS modifications contract in late March 2020, the designated vendor began preparing cabinet detail drawings and system layout drawings. These drawings were submitted to the Cardinal Operating Company for review and approval, and they were ultimately finalized by early May 2020. During this time, the DCS modifications vendor was also developing the software for the new system and submitted the proposed control sheets and graphics for the DCS workstations to the Cardinal Operating Company in late May 2020. A design review meeting was then held in mid-June 2020, after which the vendor started finalizing the new DCS software and fabricating the new DCS hardware.

Approximately one month after the design review meeting with the Cardinal Operating Company, the vendor performed an FAT for the new DCS software to ensure it was built and operating in accordance with the design specifications. After performing the FAT and addressing issues identified during the FAT, the vendor shipped the software to the project site in mid-August 2020.

The FAT was performed for the DCS hardware approximately two weeks after the corresponding test for the DCS software. As was done for the DCS software, the vendor then addressed any issues identified in the hardware during the FAT before finally shipping the hardware to the project site. The DCS hardware was delivered to the Cardinal station in late August 2020, approximately one week after the DCS software arrived.

3.4 CONSTRUCTION

Construction of Cardinal's new dry fly ash system commenced in mid-January 2020 when the piling contractor mobilized to the project site. The construction schedule was set up in phases corresponding to the four installation contractors hired to install the various components of the dry fly ash system. For each unit, the piling for the new fly ash storage silo would be installed first, then the foundation work for the silo would commence, then the mechanical installation work would start, and finally the electrical installation work would

begin. While the mechanical and electrical installation work were generally performed concurrently at a given unit, the piling and substructure work were performed sequentially prior to the mechanical installation work starting.

Overall, construction was performed concurrently at all three units but with staggered starts. Construction started at Unit 1 first, followed by Unit 2 and then finally Unit 3. The dry fly ash systems for all three units are expected to take approximately 16 months to install, starting at Unit 1 in mid-January 2020 and ending at Unit 3 in mid-May 2021.

3.4.1 WORKER SCHEDULES

Upon mobilizing to the Cardinal site, each contractor's crew generally worked five days per week at eight hours per workday. Holiday and weekend work were and will be kept to a minimum.

3.4.2 PILING

Having been awarded the piling contract in mid-January 2020, the piling contractor mobilized to the project site by the end of January 2020. Upon fully mobilizing to the site, the piling contractor installed test piles and conducted the necessary load tests to qualify the piles specified for the project. After performing successful load tests, the piling contractor started boring holes for the piles the Unit 1 silo site, installed the rebar cage in each hole, and finally cast the piles. The same work was performed at Units 2 and 3 around the same time. Overall, the piling for all three units' new fly ash storage silos was installed by early March 2020, after which the piling contractor demobilized from the project site.

3.4.3 SUBSTRUCTURES / FOUNDATIONS

Upon mobilizing to the project site in March 2020, the substructure contractor started the civil grading work. In addition, since the piling had been installed at all three units by this time, the contractor also started installing the storage silo foundation as well as the foundations for the vacuum exhauster shelter, for the PDC building and transformer, and for the utility racks. The grounding grids, underground piping, and duct banks were also installed at this time.

Like the other construction work for this project, the substructure contractor started installing the foundations and underground utilities at Unit 1 first, followed by Unit 2 and finally by Unit 3. Although the start times were staggered, foundation work was generally ongoing at each unit throughout the spring and summer of 2020. The foundations for Unit 1's dry fly ash system were completed by the end of May 2020, after which the project site was turned over to the mechanical general work contractor. The Unit 2 and 3 foundations were fully installed by the end of July 2020, finishing within two weeks of each other. After turning over the Unit 3 site to the mechanical general work contractor, the substructure contractor demobilized from Cardinal.

3.4.4 MECHANICAL EQUIPMENT INSTALLATION

3.4.4.1 DRY FLY ASH SYSTEM EQUIPMENT & COMPONENTS

Once the foundations are installed at a given unit, the mechanical general work contractor can start installing and erecting the mechanical equipment and components of the dry fly ash system. In general, each system will be installed in accordance with the dry fly ash system vendor's component delivery schedule. Thus, the storage silo and support steel will be erected first since these components will be delivered to the site first. The erection of each storage silo will include the installation of the silo's rings, roof, piping, and roof enclosure, in that order. As the mechanical general work contractor erects each storage silo, the major system components – bin vent filter, vacuum exhausters, silo unloading chutes, vent fans, fluidizing and compressed air systems, pin mixers, and filter separators – will arrive to the project site in approximately two-week intervals. These components will be installed concurrent with the erection of each storage silo. As each component is installed, the mechanical general work contractor will turn the component over to the electrical general work contractor to be tied into the auxiliary power system.

Overall, each unit's storage silo is expected to take approximately six to eight months to erect, depending on the dry fly ash system's components delivery schedule and the time required to install each component.

The mechanical general work contractor mobilized to the project site at Unit 1 in late May 2020, just as the substructure contractor was finishing the foundation work at that unit. A few weeks thereafter, in mid-June 2020, the bin vent filter, vacuum exhausters, and fly ash storage silo (with supporting steel) were delivered to the site by the dry fly ash system vendor. Upon delivery of the silo components, the mechanical general work contractor began erecting the silo on the foundation installed by the substructure contractor. A similar sequence was followed at Units 2 and 3, with the mechanical general work contractor ultimately starting the erection of the fly ash storage silos at those units in late July 2020 and early September 2020, respectively. The equipment and components furnished by the dry fly ash system vendor are expected to be fully installed by late December 2020, early February 2021, and early May 2021 for Units 1, 2, and 3, respectively.

3.4.4.2 BALANCE-OF-PLANT EQUIPMENT & COMPONENTS

As the mechanical general work contractor is erecting the fly ash storage silo and installing the other equipment and components furnished by the dry fly ash system vendor, the contractor will also be installing the BOP equipment and piping. Equipment piping will be installed first followed by the piping to/from each unit's precipitator, which will be installed as the utility racks are erected. Erection of the utility racks will commence about one week after the structural steel fabricator has delivered all of the steel to the given unit. Thus, erection of the Unit 1 utility racks and installation of the corresponding piping commenced in early July 2020. The corresponding Unit 2 and 3 work began in early August and the end of October 2020, respectively.

Overall, it is expected to take approximately six to seven months to install the piping to each unit's precipitator and erect the utility racks to support the piping. After all of the BOP equipment and piping has been installed, the mechanical general work contractor will finally tie the new system into the existing fly ash system remaining in service. At this point, the mechanical general work contractors work at a given unit will conclude. Based on the preceding timeframes, the tie-ins at Units 1, 2, and 3 are currently anticipated to occur in mid-January 2021, late March 2021, and early May 2021, respectively.

Finally, while the mechanical general work contractor is still erecting the fly ash storage silo at each unit, the PDC buildings were delivered to the project site. As they arrived, the contractor installed them on the corresponding foundations previously installed by the substructure contractor. The PDC enclosures were erected on September 25, 2020 (Unit 1) and on October 15 (Units 2 and 3).

3.4.5 ELECTRICAL EQUIPMENT INSTALLATION & TIE-IN

The electrical general work contractor will start installing the electrical equipment for each unit's dry fly ash system once the PDC and transformer for that unit are delivered to the project site and the corresponding enclosure has been erected. Accordingly, it is expected that the electrical installation work will have started at all three units by October 2020. As the contractor installs the electrical equipment in the PDCs, the contractor will also install the wires and cables (including cable tray), lighting, and other electrical components at all three units. Like the rest of the construction activities for this project, the start dates for installing the units' electrical equipment will be staggered and will start at Unit 1.

While the electrical general work contractor is installing the electrical equipment and components at the dry fly ash system site for a given unit, the contractor will also be installing the DCS hardware prepared by the DCS modifications vendor and transferring the unit's PLC valve wiring to the station DCS. The new electrical lines will be tied into the existing unit systems during the unit's scheduled outage in the fall of 2020. Thus, this work occurred between September 29 and October 8, 2020 for Unit 1 and between September 26 and October 4, 2020 for Unit 2. This work is ongoing for Unit 3, which is scheduled to have its outage end on November 2, 2020.

Overall, the electrical installation work for the project is expected to take approximately six months to complete. Consequently, the electrical work is scheduled to be finished by mid-January 2021, late March 2021, and early May 2021 for Units 1, 2, and 3, respectively, just as the mechanical work is being finished.

3.5 START-UP & IMPLEMENTATION (COMMISSIONING)

As each major component of the new dry fly ash-handling system is installed, it will be turned over for commissioning to ensure it operates as intended. To ensure its reliability, each component will be tested over a period of one to two weeks. In addition to the overall vacuum-pneumatic conveying system, the following

major components will be individually tested and commissioned as the necessary mechanical and electrical work is completed: pin mixers, air fluidizing and compressor systems, vacuum exhausters, filter separators, bin vent filters, silo conditioned and dry unloading systems. To commission each of the preceding dry fly ash system components and the dry fly ash system as a whole, field service engineers from the fly ash system vendor will use a prepared start-up plan to ensure each piece of equipment is operational and functional. Based on the projected installation schedule at each unit, it is expected that the commissioning process will take approximately two months per unit. Thus, the dry fly ash systems at Units 1, 2, and 3 are expected to be fully commissioned by mid-January 2021, late March 2021, and mid-May 2021.

In addition to commissioning the dry fly ash system and its components, the precipitator valves in the DCS and the auxiliary power subsystems will also be commissioned. This commissioning work will occur during each unit's scheduled outage in the fall of 2020.

After the dry fly ash system is commissioned, the fly ash system vendor will work with the optimize and tune the system in accordance with the Cardinal Operating Company's acceptance criteria. In general, this process will ensure the system operates at maximum efficiency and in accordance with the project design criteria and all permits (*i.e.*, Ohio EPA air pollution PTI). During this "performance guarantees" period, a third-party testing agency hired by the fly ash system vendor will observe the system tests and document the results. For each individual guarantee that is not met, modifications will be made as necessary in order to meet the guaranteed performance.

In accordance with the dry fly ash system contract, the "performance guarantees" period within 30 days after the dry fly ash system is operational. Thus, the fly ash-handling system at Cardinal is expected to be fully converted into a dry system approximately 30 days after the Unit 3 system has been commissioned in May of 2021. Therefore, per the visual timeline representation of the project schedule in Section 2.0, the Cardinal Operating Company expects Cardinal to have access to alternative disposal capacity to replace FAR II by June 7, 2021.

4.0 PROJECT SCHEDULE: PROGRESS TO DATE

4.1 ENGINEERING & DESIGN

4.1.1 BALANCE-OF-PLANT COMPONENTS

The engineering and design for the BOP components of the dry fly ash-handling system mostly took place between March 2019 and July 2020. The following subsections summarize the BOP engineering that has been completed for this project as it pertained to each discipline.

4.1.1.1 **GENERAL**

The general engineering and design tasks for this project primarily included activities that otherwise did not fall under a specific discipline. Most of these general tasks were initiated at the beginning of the project in March 2019 and completed a few months thereafter. These activities included the development of the project design basis, the creation of a three-dimensional computer model to aid in engineering and design tasks, and any necessary engineering support during the permitting process. The remaining general engineering and design activities, primarily construction support and drawing closeout, will be completed after the new vacuum-pneumatic handling system and auxiliary components are commissioned, optimized, and tuned.

4.1.1.2 CIVIL & STRUCTURAL

The engineering and design work for the civil and structural aspects of the project began in March 2019 and was completed by March 2020. This work included preparing and/or designing:

- · Geotechnical investigations and evaluations;
- Temporary erosion control;
- Topographic surveys and underground utility locations;
- Civil sitework and grading plans;
- Roads and paving for the updated site layout;
- Foundations for:
 - Fly ash storage silos;
 - PDCs and transformers,
 - Utility racks supporting new piping, cable tray, etc.,
 - Exhauster blowers, and
 - Miscellaneous equipment pads and pipe supports; and
- Structural steel for:
 - Utility racks supporting new piping, cable tray, etc.,
 - Shelters for the exhauster blowers, and
 - Miscellaneous stairs, platforms, and walkways.

The only anticipated civil and structural engineering work remaining for the project is to support the ongoing construction of the new dry fly ash-handling system.

4.1.1.3 MECHANICAL

The mechanical engineering and design work began in March 2019 and was completed by April 2020. This work included preparing and/or designing:

- · General arrangement drawings;
- Equipment location drawings;
- Service water, fly ash, and underground piping, including:
 - o Pipe and instrumentation diagrams (P&IDs),
 - Isometrics, and
 - o Supports, including auxiliary steel; and
- Demolition work for existing electrostatic precipitator piping.

The only anticipated mechanical engineering and design work remaining for the project is preparation of asbuilt drawings for the project's mechanical components. This activity will be performed near the end of construction.

4.1.1.4 ELECTRICAL

The electrical engineering and design work began in March 2019 and was completed by July 2020. The work completed to date included preparing and/or designing:

- Project electrical load list,
- Single line and phasing diagrams,
- Auxiliary power and arc flash studies,
- Grounding,
- · Underground duct banks,
- Lighting,
- Electrical installation drawings, and
- Cable and cable tray routing.
- Updating the project electrical load list as necessary,
- · Performing relay setting calculations, and
- Preparing electrical schematic diagrams, wiring drawings, and cable tabulations.

Finally, as-built drawings for the project's electrical components will be prepared shortly after these components have been installed. This work is currently expected to be finished by the beginning of November 2020.

4.1.1.5 INSTRUMENTATION & CONTROLS

The engineering and design work for the project's instrumentation and controls began in May 2019 and was completed by March 2020. This work included preparing and/or designing:

- Distributed control system (DCS) network architecture drawings;
- Control logic; and
- Instrument lists, data sheets, installation details, and location drawings.

The only anticipated instrumentation and control work remaining for the project is preparation of corresponding as-built drawings. Similar to the mechanical as-builts, this activity will be performed near the end of construction.

4.1.2 VACUUM-PNEUMATIC CONVEYING SYSTEM

About one month following the start of the BOP engineering and design work, Cardinal Operating Company solicited bids from potential vendors for the vacuum-pneumatic conveying systems to be installed at each unit. This contract was ultimately awarded to a vendor in August 2019. Upon receiving the notice to proceed, this vendor began the engineering and design work for each unit's vacuum-pneumatic conveying system. These activities included, among other tasks, the design of the required process and the associated equipment including fly ash storage silos; vacuum exhausters; pin mixers; filter separators; and the corresponding piping, electrical wiring, and instrumentation for the conveying system. The vendor has completed these design tasks to date and finished its design by the end of July 2020.

4.2 PERMITTING

The Cardinal Operating Company submitted the air pollution PTI application for the project to the Ohio EPA in late June 2019. Approximately two months later, on August 22, 2019, the Ohio EPA issued the final PTI authorizing the Cardinal Operating Company to construct and operate the new dry fly ash-handling system at Cardinal.

In addition, the Cardinal Operating Company submitted a PTI application to close FAR II on October 18, 2019 and received an approved PTI on February 20, 2020. The Cardinal Operating Company is currently finalizing the appropriate state and federal permits for storm water pollution prevention, sediment and erosion control, wetland and endangered species evaluations, landfill leachate waste water treatment system, and modifications to the station's NPDES permit (by Ohio EPA) and FAR II dam permit (by Ohio Department of

Natural Resources). These permit applications are expected to be submitted to the appropriate agencies by the end of 2020.

4.3 PROCUREMENT

Including the vacuum-pneumatic conveying system, Cardinal Operating Company has awarded all of the contacts for obtaining the necessary components for the new dry fly ash-handling system. These procurement contracts were awarded between November 2019 and March 2020 and cover the structural steel for the utility racks, auxiliary mechanical equipment and piping, the required DCS modifications, and the pre-manufactured electrical PDC buildings with electrical equipment and transformers.

The installation of the new dry fly ash-handling system is being performed under four separate contracts: a piling contract, a substructure contract, a mechanical general work contract, and an electrical general work contract. All installation contracts have been awarded to date.

4.4 CONSTRUCTION

Construction started at Unit 1 in January 2020 when the piling contractor mobilized to the project site and started installing test piles. Since then, the various installation contractors for this project have installed the piling and foundations for the fly ash storage silos and the foundations for the PDCs, transformers, and utility racks; the underground utilities; and the underground ductbanks. The mechanical general work contractor also recently finished erecting the support steel for the fly ash storage silos and the PDCs at all three units.

Given that construction of each component of the dry fly ash system started at Unit 1, the system at that unit is further along in construction. To date, the following components have also been installed at Unit 1 in addition to those stated earlier:

- · Silo rings,
- Silo roof.
- Pin mixers,
- Vacuum exhausters,
- Fluidizing air blowers, and
- Air compressor.

Due to the staggered construction schedule, Unit 2 is also ahead of Unit 3 in construction. To date, the following components have also been installed at Unit 2 in addition to those stated earlier:

- Pin mixers,
- Vacuum exhausters,
- Fluidizing air blowers, and
- Air compressor.

5.0 REFERENCES

- 40 CFR Part 257 Subpart D, "Standards for the Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments."
- 2. United States Court of Appeals, District of Columbia Circuit, *Utility Solid Waste Activities Group et al. v. Environmental Protection Agency*, No. 15-1219, 08/21/2018.
- Ohio Environmental Protection Agency, "Fact Sheet Regarding an NPDES Permit To Discharge to Waters of the State of Ohio for Cardinal Operating Company," Public Notice No. 18-05-061, Ohio EPA Permit No. 0IB00009*WD, Application No. OH0012581, 05/21/2018.
- 4. Buckeye Power, "CCR Rule Compliance Data and Information," https://ohioec.org/buckeye-power/ccr-rule-compliance-data-information/, Accessed 10/20/2020.
- Environmental Protection Agency, "Hazardous and Solid Waste Management System: Disposal of Coal Combustion Residuals from Electric Utilities, Part VI (Development of Final Rule – Technical Requirements)," 80 Fed. Reg. 74, p. 21423, 04/17/2015.
- 6. 55 Ohio Revised Code 5577, "Load Limits on Highways," https://codes.ohio.gov/orc/5577, Accessed 10/20/2020.
- 7. Ohio Department of Transportation, "Transportation Data Management System, Location ID 541, LRS ID SJEFSR00007**C," http://www.ms2soft.com/tcds/?loc=Odot&mod=tcds&local_id=541, Accessed 10/20/2020.
- 8. Environmental Protection Agency, "Effluent Limitations Guidelines and Standards for the Steam Electric Power Generating Point Source Category," 80 Fed. Reg. 212, pp. 67838–67903, 11/03/2015.
- Environmental Protection Agency, "Hazardous and Solid Waste Management System: Disposal of Coal Combustion Residuals From Electric Utilities; A Holistic Approach to Closure Part A: Deadline to Initiate Closure," 85 Fed. Reg. 168, pp. 53516–53566, 08/28/2020.
- 10. Ohio Administrative Code, 3745-29-19, "Operational Criteria for an Industrial Solid Waste Landfill Facility," Effective 09/23/2014.
- 11. Geosyntec Consultants, "Location Restriction Evaluation, Cardinal FAR II, Brilliant, Ohio," August 2018.



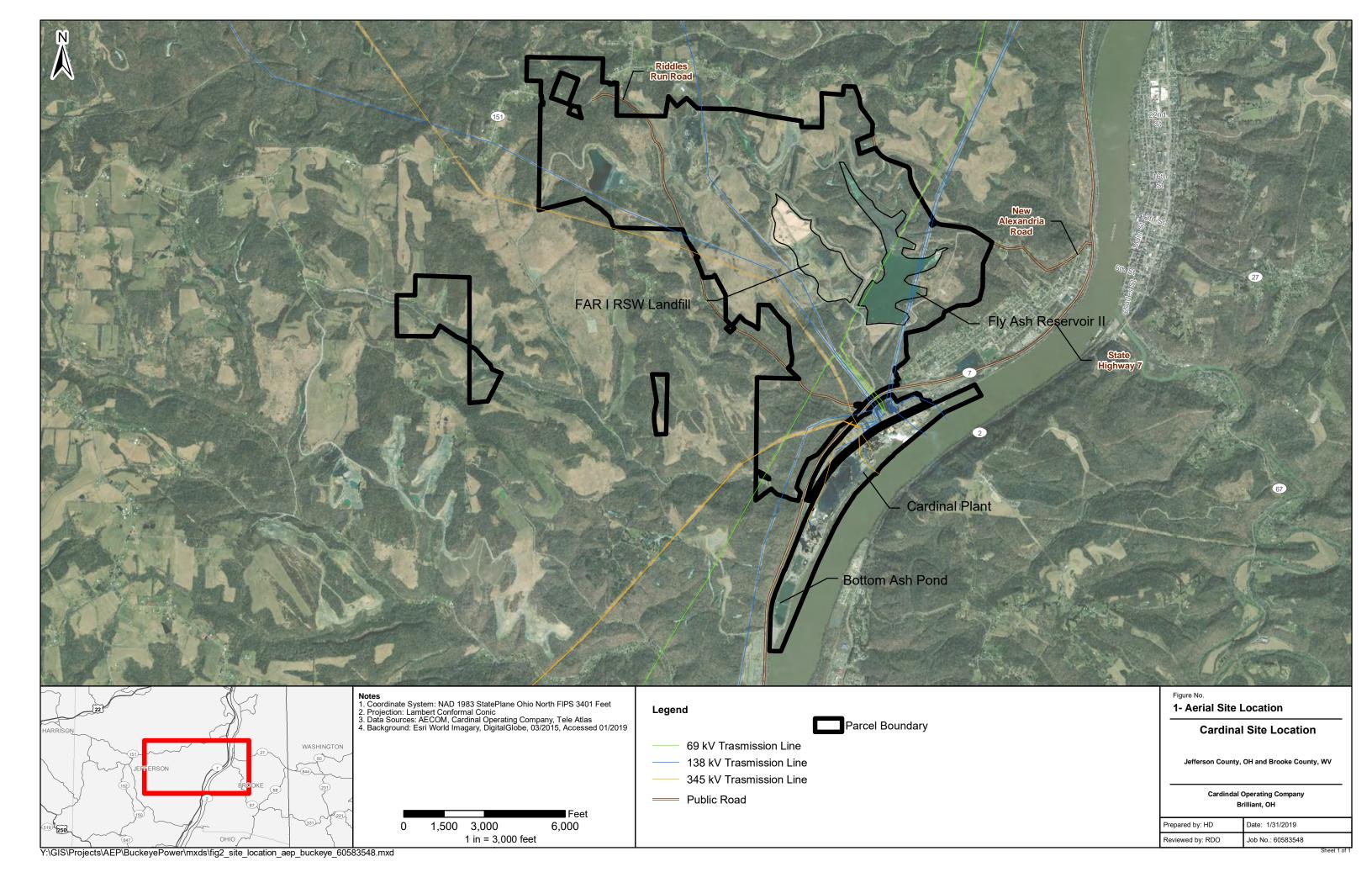
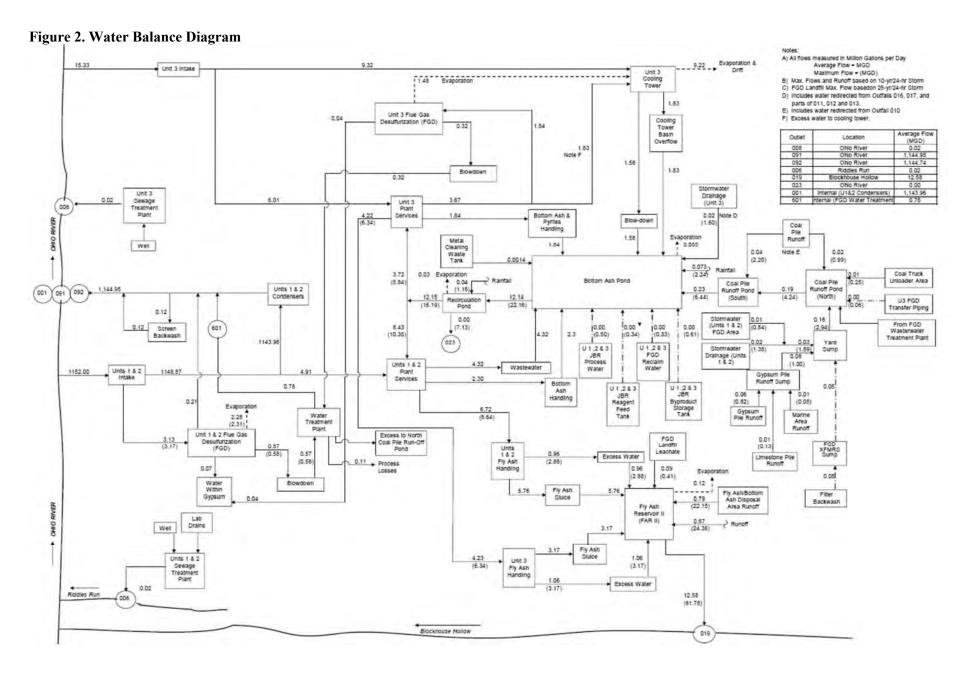


Figure 2- Water Balance from Cardinal NPDES Permit





7.0 Certification of Compliance

To demonstrate that the criteria in 40 CFR § 257.103(f)(1)(iii) has been met, the following information is provided pursuant to 40 CFR § 257.103(f)(1)(iv)(B) to demonstrate that the Facility is in compliance with the CCR Rule.

7.1 Owners Certification of Compliance

In accordance with 40 CFR § 257.103(f)(1)(iv)(B)(1), I hereby certify that, based on my inquiry of those persons who are immediately responsible for compliance with environmental regulations at Cardinal Power Plant, the Facility CCR Units are in compliance with all requirements contained in 40 CFR § 257 Subpart D.

(SPMT NK

Thomas M. Alban

TC M. alban

Vice President

November 30, 2020

7.2 Compliance Documents

Compliance documentation is outlined below for Cardinal's three CCR Units, Fly Ash Reservoir II, Bottom Ash Pond and FAR I RSW Landfill.

Fly Ash Reservoir II

In accordance with 40 CFR§ 257.103(f)(1)(iv)(B)(2) through (B)(8), the following documents are provided below in Attachment 1 for Fly Ash Reservoir II:

§257.103(f)(1)(iv)(B)(2)(i)-(iii) – Maps of groundwater monitoring wells relative to CCR Unit, well construction and drilling logs, and seasonal groundwater flow maps.

§257.103(f)(1)(iv)(B)(3) – Groundwater monitoring results through first 2020 Semi-Annual monitoring period. In addition, the most recent Annual Groundwater Report (January 2020) is also provided for reference.

§257.103(f)(1)(iv)(B)(4) – Description of site geology and stratigraphic cross sections. Text is provided from the Groundwater Monitoring Network Report in accordance with §257.91.

§257.103(f)(1)(iv)(B)(5) – Corrective Measures Assessment – Completed in July 2019 and subsequent public meeting was held on September 4th, 2019.

§257.103(f)(1)(iv)(B)(6) – Remedy Selection Report – Provided are two corrective measure selection progress reports (March & September 2020) and completed Remedy Selection Report, uploaded to Operating Record on October 27, 2020.

§257.103(f)(1)(iv)(B)(7) – Structural Stability Assessment pursuant to §257.73(d) was completed in October 2016. The next Stability Assessment will be completed prior to October 2021.

§257.103(f)(1)(iv)(B)(8) – Safety Factor Assessment pursuant to §257.73(e) was completed in October 2016. The next Safety Factor Assessment will be completed prior to October 2021.

Bottom Ash Pond

In accordance with 40 CFR§ 257.103(f)(1)(iv)(B)(2) through (B)(8), the following documents are provided below in Attachment 2 for the Bottom Ash Pond:

§257.103(f)(1)(iv)(B)(2)(i)-(iii) – Maps of groundwater monitoring wells relative to CCR Unit, well construction and drilling logs, and seasonal groundwater flow maps.

§257.103(f)(1)(iv)(B)(3) – Groundwater monitoring results through first 2020 Semi-Annual monitoring period. In addition, the most recent Annual Groundwater Report (January 2020) is also provided for reference.

§257.103(f)(1)(iv)(B)(4) – Description of site geology and stratigraphic cross sections. Text is provided from the Groundwater Monitoring Network Report in accordance with §257.91.

§257.103(f)(1)(iv)(B)(5) – Corrective Measures Assessment – Not applicable, the Bottom Ash Pond is currently in Assessment Monitoring and does not exceed Groundwater Protection Standards.

§257.103(f)(1)(iv)(B)(6) – Remedy Selection Report – Not applicable, the Bottom Ash Pond is currently in Assessment Monitoring and no remedy selection report is required.

§257.103(f)(1)(iv)(B)(7) – Structural Stability Assessment pursuant to §257.73(d) was completed in October 2016. The next Stability Assessment will be completed prior to October 2021.

§257.103(f)(1)(iv)(B)(8) – Safety Factor Assessment pursuant to §257.73(e) was completed in October 2016. The next Safety Factor Assessment will be completed prior to October 2021.

FAR I RSW Landfill

In accordance with 40 CFR§ 257.103(f)(1)(iv)(B)(2) through (B)(8), the following documents are provided below in Attachment 3 for the FAR I RSW Landfill:

§257.103(f)(1)(iv)(B)(2)(i)-(iii) – Maps of groundwater monitoring wells relative to CCR Unit, well construction and drilling logs, and seasonal groundwater flow maps.

§257.103(f)(1)(iv)(B)(3) – Groundwater monitoring results through first 2020 Semi-Annual monitoring period. In addition, the most recent Annual Groundwater Report (January 2020) is also provided for reference.

§257.103(f)(1)(iv)(B)(4) – Description of site geology and stratigraphic cross sections. Text is provided from the Groundwater Monitoring Network Report in accordance with §257.91.

§257.103(f)(1)(iv)(B)(5) – Corrective Measures Assessment – Not applicable, the FAR I RSW Landfill is currently in Detection Monitoring and does not exceed Groundwater Protection Standards.

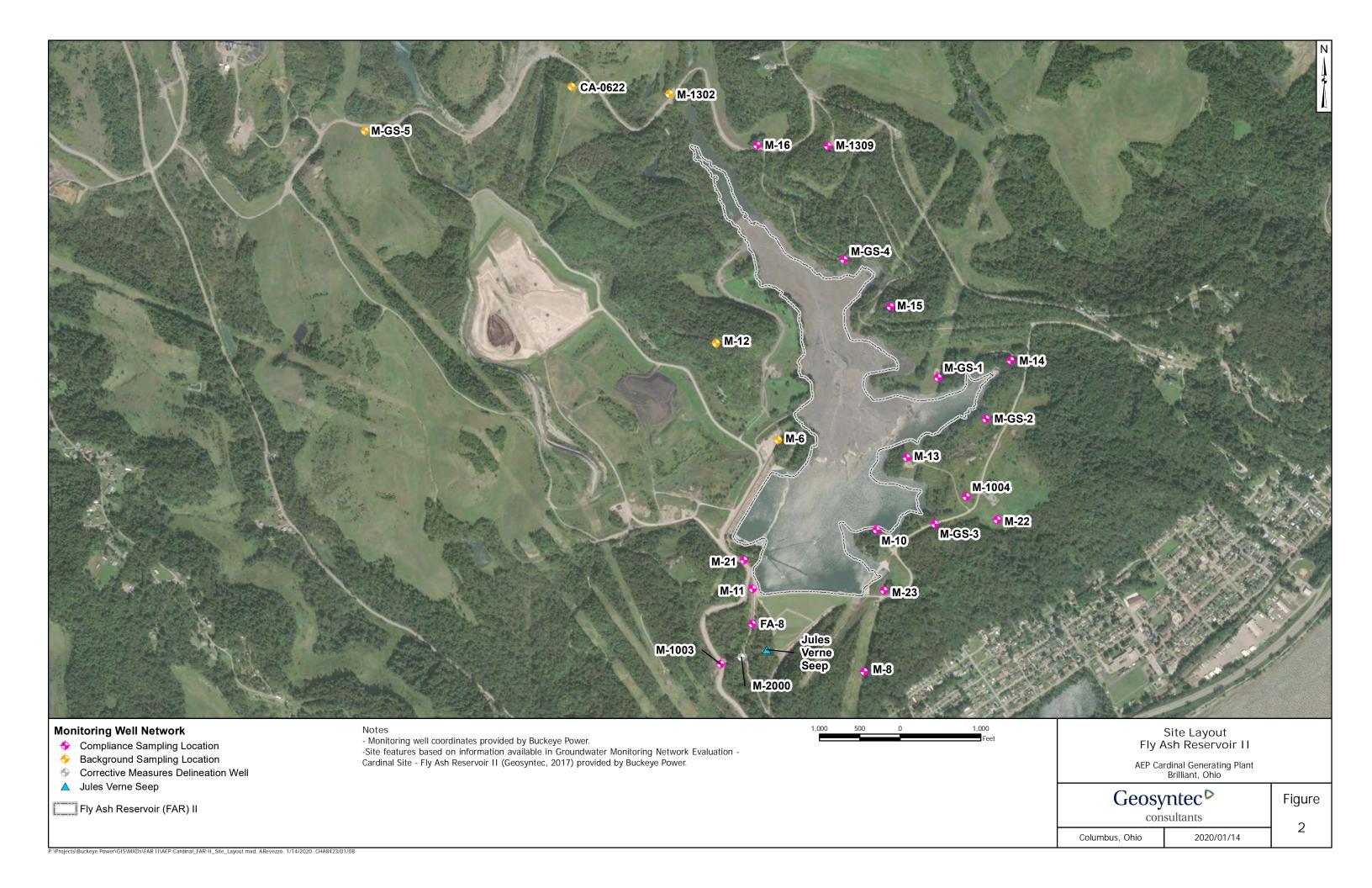
§257.103(f)(1)(iv)(B)(6) – Remedy Selection Report – Not applicable, the FAR I RSW Landfill is currently in Detection Monitoring and no remedy selection report is required.

§257.103(f)(1)(iv)(B)(7) – Not applicable, The CCR Unit is a Landfill and Structural Stability Assessments pursuant to §257.73(d) do not apply.

§257.103(f)(1)(iv)(B)(8) – Not applicable, The CCR Unit is a Landfill and Safety Factor Assessments pursuant to §257.73(e) do not apply.

Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(2)(i)

Maps of Groundwater monitoring well locations in relation to CCR Unit



Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(2)(ii)

Well construction diagrams and drilling logs for all groundwater monitoring wells



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								-			Loose, black, COAL; dry.			
	7	SS	12.0					-	3					
	'	55	12.0						7		Loose, orange, silty SAND (SM); d	ry, fine		
								_			grained; over 6" of grayish brown,	clayey silt.		
		00	14.0					-			Lacco gravich brown to grange si	thy SAND		
	8	SS	14.0								Loose, grayish brown to orange, si (SM); dry; non-plastic; micaceous.	ity SAND		
								15 -						
		60	10.0					_	<u> </u>					
	9	SS	16.0						·					
								-	<u> </u>					
/15								_						
7/17	10	SS	18.0											
GD.								-	<u> </u>					
AEF											Moderately hard, greenish gray, SA	ANDSTONE;		
FGD_LANDFILL.GPJ AEP.GDT 7/17/15			TYPE	OF C	ASING USE	D					Continued Next Pa	age		
IPFI-			NQ-2 R	OCK CO	RE			PIEZOMI	ETER	TYPI	E PT = OPEN TUBE POR	OUS TIP. SS =	OPEN	N TUBE
¥.			6" x 3.25 9" x 6.25				-				CREEN, G = GEONOR, P =		J!	
			HW CAS	SING AD	VANCER	4"		WELL TY	/PE:	O۱	V = OPEN TUBE SLOTTED	SCREEN. GM	= GE(OMON
8			NW CAS			3" 6"					RECORDER	,		

AIR HAMMER

LOG OF BORING

JOB NUMBER

FGD LANDFILL.GPJ AEP.GDT 7/17/15

8

Monitoring Well: M-21



__ DATE <u>7/17/15</u> SHEET <u>2</u> OF COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0620 PROJECT **CARDINAL LANDFILL** 8/25/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE SAMPLE NUMBER DEPTH SAMPLE GRAPHIC S **DEPTH** LOG USC SOIL / ROCK DRILLER'S WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO SS 20.0 fine grained; rust color along fractures; massive. 22.0 30.0 1 RC Hard, light gray, LIMESTONE; fine grained; rust color along fractures; vertical fracture at 24.0 ft.; Soft, greenish gray (GLEY1-6/1-5GY), SANDSTONE; micaceous; massive. 30 RC 30.0 40.0 Soft, gray, SAND and SHALE; micaceous. Soft, gray to greenish gray, SANDSTONE; medium grained; slight shale like foliations; 3" sandy shale at 33 ft.; 5 horizontal and vertical fractures. 35 Very soft, gray to greenish gray, CLAYSHALE; massive. Hard, gray to greenish gray, SANDSTONE. 40 RC 40.0 50.0 Very soft, gray to greenish gray, CLAYSHALE; high sand content; massive to foliated.

LOG OF BORING

Monitoring Well: M-21

## PROJECT CARDINAL LANDFILL ## PRO			NUMI PAN)		ERICA	AN ELECTRIC	POW	- /ER			ВС	RING NO. <u>CA-0620</u> DATE <u>7/17/15</u> S	HEET	3 OF 12
Moderately hard, greenish gray, SANDSTONE: fine grained, rust color along cracks; massive. 50 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
Moderately hard, greenish gray, SANDSTONE: fine grained, rust color along cracks; massive. 50 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.	_					1								
fine grained, rust color along cracks; massive. 50 – 55 – 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.	RAMPI	NUMBER	SAMPLE	DEF IN F	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs		WELL	
fine grained, rust color along cracks; massive. 50 – 55 – 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
fine grained, rust color along cracks; massive. 50 – 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
fine grained, rust color along cracks; massive. 50 – 2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.												Moderately hard, greenish gray, SANDSTONE		
2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. 5 RC 60.0 70.0 Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
2 horizontal fractures near 55.0 ft. Soft, gray to black, CLAYSHALE; massive to foliated. 5 RC 60.0 70.0 Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.		4	RC	50.0	60.0				50 -					
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.										-				
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.										-				
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black, CLAYSHALE; massive to foliated. Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
5 RC 60.0 70.0 Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.									55 -			2 horizontal fractures near 55.0 ft.		
5 RC 60.0 70.0 Hard, light gray (GLEY2-7/1-10B), LIMESTONE; massive. Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black; CLAYSHALE; massive.												foliated.		
Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black; CLAYSHALE; massive.														
Soft, gray to black; CLAYSHALE; massive.		5	RC	60.0	70.0				60 -					
										毌		massive.		
												Soft, gray to black; CLAYSHALE ; massive.		
6 RC 70.0 80.0 70 = 10 = 10 = 10 = 10 = 10 = 10 = 10									65 -					
Figure 1														
Feb Part P	15													
6 RC 70.0 80.0 70.0 80.0 70.0 80.0 70.0 80.0 70.0 80.0 70.0 80.0 70.0 80.0 70.0 80.0 8	. 7/17/													
99 6 RC 70.0 80.0 70 10 10 10 10 10 10 1	P.GDT									Ħ				
70	PJ AE													
PED LAND	FILL.G	6	RC	70.0	80.0				70 -					
	LAND	-												
	FGD													

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21



				N ELECTRIC	POW	/EK				RING NO. <u>CA-0620</u> DATE <u>7/17/15</u> SHEET <u>4</u> OF <u>12</u>
PR	DJECT	CAF	KDINA	L LANDFILL					ВО	RING START 8/25/06 BORING FINISH 6/1/06
SAMPLE	SAMPLE	DE	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION ☐ DRILLER'S NOTES
										Moderately hard, greenish gray, SANDSTONE ; fine grained; massive. Vertical fracture at 73.0 ft. Horizontal fracture at 74.5 ft.
7	RC	80.0	90.0				75 - - - - 80			Soft, dark gray, CLAYSHALE; massive.
							85 -			Light gray, LIMESTONE; iron staining. Soft, dark gray, CLAYSHALE; massive.
							- - 90 –			
EP.GDT 7/17/15	RC	90.0	100.0				-			Light gray, LIMESTONE. Soft, dark gray, CLAYSHALE; massive.
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15							95 –			Hard, gray (GLEY1-6/2-N), LIMESTONE ; massive.
PGD FGD										Soft, dark gray, CLAYSHALE ; massive.
VEP C										Continued Next Page

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21



			ERICA	N ELECTRIC L LANDFILL	POV	/ER				RING NO. <u>CA-0620</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>12</u> RING START <u>8/25/06</u> BORING FINISH <u>6/1/06</u>
SAMPLE		SAM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	도 이	SOSO	SOIL / ROCK IDENTIFICATION BONNET INITIAL OTHER OTHER DRILLER'S NOTES
9	RC	100.0	110.0				100 -			Hard, gray, LIMESTONE ; massive. Soft, gray (GLEY1-4/1-N), CLAYSTONE ; massive.
							105 -			
10	RC	110.0	120.0				- 110 – -			
							115 - -			
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	RC	120.0	130.0				- 120 - -			Light gray, LIMESTONE.
FGD_LA							-			Dark gray, CLAYSHALE .
Ē C										Continued Next Page

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21

			AN ELECTRIC L LANDFILL	POV	VER				RING NO. <u>CA-0620</u> DATE <u>7/17/15</u> RING START <u>8/25/06</u> BORING FIN		
SAMPLE NUMBER SAMPLE	DE IN F	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
	FROM	ТО	BLOWS / 6"	~		125 -			Light gray, LIMESTONE .		
12 RC	C 130.0	140.0				130 -			Dark gray to black to red brown (10R-3/2-/2), CLAYSHALE; dry; massive.		
						135 -					
13 RC	C 140.0	150.0				140 -			Hard, dark gray (GLEY2-3/1-5PB), CLAYSTONE; vertical fractures refilled with calcite; massive.		
						145 -					
									Continued Next Page		

LOG OF BORING

Monitoring Well M-21



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0620</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL LANDFILL 8/25/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE SAMPLE NUMBER DEPTH SAMPLE S DEPTH LOG SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES** FEET FROM TO RC 150.0 160.0 Hard, gray, **SANDSTONE**; medium grained. Hard, grayish brown (10YR 5/3), SANDSTONE; medium grained. 155 160 15 RC 160.0 170.0 165 Hard, gray (GLEY2-5/1-10B), **SANDSTONE**; medium grained. 170 16 RC 170.0 180.0 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 Vertical fracture at 172.0 ft. 175

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21



COMPANY AMERICAN ELECTRIC POWER ___ DATE <u>7/17/15</u> SHEET <u>8</u> OF _ BORING NO. CA-0620 PROJECT CARDINAL LANDFILL **BORING START** 8/25/06 BORING FINISH 6/1/06 STANDARD
PENETRATION PENETRATI SAMPLE SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO 180 RC 180.0 190.0 185 Thin (1mm) clay hair line seams between 185.5 and 187.0 ft. and through sandstone to 225.5 ft. 190 18 RC 190.0 200.0 Pebbly subrounded limestone clasts. 195 CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15 200 RC 200.0 210.0

AMERICAN ELECTRIC POWER SERVICE CORPORATION

					AE	EP C	IVILE			ERING LABORATORY F BORING Monitor	ing Well	: M-21
	NUM					_		LO		F BORING Women	ing wen	. 141 21
				N ELECTRIC	POV	VER				RING NO. <u>CA-0620</u> DATE <u>7/17/15</u>		
PRC	JECT	_CAF	<u> </u>	L LANDFILL					ВО	RING START <u>8/25/06</u> BORING FIN	ISH <u>6/</u>	1/06
SAMPLE	SAMPLE	SAM DEF IN F FROM		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							205 - -					
20	RC	210.0	220.0				210 - - -	- : : : : : : : : : : : : : : : : : : :		Sandstone conglomerate between 210.0 and 211.0 ft. Sandstone conglomerate between 212.0 and		
							215 -			213.5 ft. Sandstone conglomerate between 214.0 and 215.0 ft.		
21	RC	220.0	230.0				- - 220 -					
							- - - 225 -			Sandstone conglomerae between 221.5 and 221.7 ft. Sandstone conglomerate between 225.0 and 225.5 ft.		

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: M-21

JOB NUMBER DATE 7/17/15 SHEET 10 OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0620 PROJECT CARDINAL LANDFILL 8/25/06 BORING FINISH 6/1/06 **BORING START** SAMPLE STANDARD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S LOG USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO Hard, dark gray, **SANDSTONE**; fine grained. 230 RC 230.0 240.0 Moderately hard to hard, dark gray to black, CLAYSHALE; contains brown, angular, coarse to fine gravel inclusions (>5%); massive. Sandstone conglomerate between 233.0 and 233.3 ft. 235 Sandstone conglomerate between 237.6 and 238.0 ft. 240 RC 240.0 250.0 245 Hard, light gray, **SANDSTONE**; medium grained; thin coal streaks (1mm) throughout. 250 RC 250.0 260.0 Dark gray claystone between 250.0 and 250.5 ft.

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21

H H H	SAM								RING START <u>8/25/06</u> BORING FINIS	,	1700
SAMPLE NUMBER SAMPLE	DEF IN F		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	문의	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
						255 -					
25 RC	260.0	270.0				260 -			Gray, SANDSTONE ; conglomerate. Soft to moderately hard, greenish gray	-	
						-			(GLEY2-4/1-5B), CLAYSHALE ; slightly foliated.		
26 RC	270.0	280.0							Hard, gray (GLEY1-4/1-5B), LIMESTONE ; contains fossils, fractured and broken glass at 273.0 ft.		
									Dark gray to black, CLAYSHALE ; slicken slides throughout.		

LOG OF BORING

JOB NUMBER

Monitoring Well: M-21

AEP

			ERICA	N ELECTRIC	POV	VER							HEET	12 OF 12
PROJ	ECT	CAF	RDINA	LANDFILL					ВО	RING START	8/25/06	BORING FINIS	H <u>6/</u>	1/06
SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL / ROCK IDENTIFICATION		WELL	DRILLER'S NOTES
27	RC	280.0	290.0	BLOW3/6				= =						
	RC	290.0	300.0											

LOG OF BORING

JOB NUMBER

Monitoring Well: M-22

AEP

				AN ELECTRIC						RING NO. <u>CA-0702</u> DATE <u>7/17/15</u> SHEET <u>1</u> OF <u>9</u>
				L LANDFILL						RING START <u>5/1/07</u> BORING FINISH <u>5/21/07</u>
				0,925.1 E 2,5						EZOMETER TYPE N/A WELL TYPE OW
GRC	UND			1005.7 SY	STEM			_		ST. RISER ABOVE GROUND 2.359 DIA 2"
Wate	er Lev	el, ft	$\overline{\Delta}$	¥		Ā				PTH TO TOP OF WELL SCREEN
TIME	Ε									ELL DEVELOPMENT YES BACKFILL QUICK GROUT
DAT	E								FIE	ELD PARTY MCR / ZLR RIG D-120
		SVIV	IPLE	STANDARD	>	POD				
SAMPLE	SAMPLE		PTH		ATT. ER,	RQD	DEPTH	GRAPHIC LOG	C S	SOIL / ROCK
AMF	AMF	IN F	EET	PENETRATION RESISTANCE	SENT	%	IN	RAP	OS O	SOIL / ROCK ☐ DRILLER'S IDENTIFICATION NOTES
ωz	o)	FROM	TO	BLOWS / 6"	LE		FEET	9		
							_			
							-	-		
							_			
							_			
							5	-		
							_			
							_			
							_	-		
							_			
							10 —			
							10			
							_	-		
							_	-		
							_			
							_	-		
							15 —			
1	NQ	15.5	17.9		1.2	0		Щ		HARD N4 MEDIUM GRAY LIMESTONE
							_	Ħ		SOFT N4 MEDIUM GRAY CLAY
							_	F		HARD N4 MEDIUM GRAY LIMESTONE
2										
2	NQ	17.9	24.2		4.0	55	_	\square		HARD N4 MEDIUM GRAY LIMESTONE
)T 7.								H		all fractured
P.G							_			HARD 5B 7/1 MEDIUM LIGHT BLUISH GRAY
A PE										LIMEY FINE-GRAIN SANDSTONE
FGD_LANDFILL.GPJ AEP.GDT 7/17/15				ASING USED						Continued Next Page
X X		NQ-2 R0		RE			PIEZOME			
٦		9" x 6.25	HSA				SLC) I I E		SCREEN, G = GEONOR, P = PNEUMATIC
		HW CAS		VANCER	4" 3"		WELL TY	PE:	O	N = OPEN TUBE SLOTTED SCREEN, GM = GEOMON
8		SW CAS			6"					RECORDER
₩ X		AIR HAN			8"					NEOONDEN

LOG OF BORING

JOB NUMBER

Monitoring Well: M-22

COM	PAN	AM	ERIC	N ELECTRIC	POV	VER			ВС	RING NO. <u>CA-0702</u> DATE <u>7/17/15</u> SHEET <u>2</u> OF <u>9</u>
PRO	JECT	CAF	RDINA	L LANDFILL					ВС	RING START BORING FINISH
SAMPLE	SAMPLE	SAM DEF IN F	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK ☐ DRILLER'S IDENTIFICATION NOTES
3	NQ	24.2	34.2		9.2	54	25 -			HARD 5B 7/1 MEDIUM LIGHT BLUISH GRAY LIMEY FINE-GRAIN SANDSTONE
										SOFT N5 MEDIUM GRAY CLAY SHALE HARD LIMESTONE
							-	\blacksquare		SOFT N5 MEDIUM GRAY CLAY SHALE
							-			
							30 -	-		HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE / LIMESTONE NODULE
4	NQ	34.2	40.7		6.4	0	35 -			SOFT 5YR 4/1 BROWNISH GRAY CLAY SHALE W/5G 6/1 greenish gray clay shale, w/high angle fracture
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	40.7	49.2		8.2	28	45 -			HARD TO MEDIUM 5G 6/1 GREENISH GRAY CLAY SHALE
ල ['] ස			l	I	I				l	Continued Next Page

LOG OF BORING

Monitoring Well: M-22

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**___ SHEET **_3**__ OF BORING NO. CA-0702 PROJECT CARDINAL LANDFILL 5/1/07 BORING FINISH 5/21/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S LOG USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 10.0 NQ 49.2 59.2 42 50 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 55 SOFT 5G 6/1 GEEENISH GRAY CLAY SHALE HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/traces of limestone 59.2 69.2 7.7 HARD 5G 6/1 GREENISH GRAY LIMESTONE 7 NQ 43 60 HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/fracture 64.0' and 65.0 high angles 65 NQ 69.2 75.2 6.0 33 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 70

Continued Next Page

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER

Monitoring Well: M-22



BORING NO. <u>CA-0702</u> DATE <u>7/17/15</u> SHEET <u>4</u> OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL **BORING START** 5/1/07 BORING FINISH 5/21/07 STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD N5 MEDIUM GRAY LIMESTONE 75 NQ 75.2 8.4 HARD N7 LIGHT GRAY LIMESTONE 84.2 49 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 80 HARD 5G 6/1 GREENISH GRAY CLAY SHALE NQ 84.2 90.7 4.7 0 w/limestone nodules 85 90 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 11 NQ 90.7 97.7 5.4 61 w/limestone nodules CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 95

LOG OF BORING

Monitoring Well: M-22

	OB I	NUME	BER _				_		LO	<i>-</i>	January Child
(ОМ	PANY	AM	ERIC	N ELECTRIC	POW	/ER			ВС	DRING NO. <u>CA-0702</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>9</u>
F	PRO.	JECT	CAF	RDINA	LANDFILL					ВС	DRING START BORING FINISH
-	SAMPLE	SAMPLE		PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION DRILLER'S NOTES
	12	NQ	97.7	TO 104.2	BLOWS / 6"	6.4	44				HARD 5G 6/1 GREENISH GRAY CLAY SHALE
								-			HARD 5G 6/1 GREENISH GRAY LIMESTONE
-								100 –			HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/limestone nodules
								-			Willinestone nodules
								-			
								-			
								_			HARD 5G 6/1 GREENISH GRAY LIMESTONE
	13	NQ	104.2	114.2		8.8	70	105 -			MEDIUM TO HARD 5G 6/1 GREENISH GRAY CLAY SHALE
								105 -			
								-			
								-			
								-			
								-			
								110 –			
								-			
								-			
								-			
								-			
	14	NQ	114.2	121.2		6.2	19	115 -			HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE
								-			HARD 5B 5/1 MEDIUM BLUISH GRAY LIMESTONE
								-			W/fracture throughout HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY
								-			SHALE
7/17/15								-			HARD 5B 5/1 MEDIUM BLUISH GRAY LIMESTONE W/fracture throughout
VEP.GDT 7.								120 -			HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE
CD_FGD_LANDFILL.GPJ AEP.GDT	15	NQ	121.2	129.2		8.3	51	-			HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE
LAND								-			
75 G											

LOG OF BORING

JOB NUMBER

Monitoring Well: M-22

AEP

BORING NO. $\underline{\text{CA-0702}}$ DATE $\underline{\text{7/17/15}}$ SHEET $\underline{\text{6}}$ OF $\underline{\text{}}$ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 5/1/07 BORING FINISH 5/21/07 **BORING START** STANDARD
PENETRATION PLOUS
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK WELL DRILLER'S SCS IN FEET **IDENTIFICATION NOTES FEET** FROM TO 125 -HARD N5 MEDIUM GRAY CLAY SHALE 16 NQ 129.2 139.2 10.0 33 w/fractures 130 135 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY NQ 139.2 149.2 10.0 17 41 SHALE 140 FGD LANDFILL.GPJ AEP.GDT 7/17/15 145 -COAL HARD N3 DARK GRAY SHALE 148.0' - 215.0' **N5 MEDIUM GRAY WELL CEMENTED** MORGANTOWN **MEDIUM GRAIN SANDSTONE** SANDSTONE / 18 NQ 149.2 159.2 10.0 87 **N5 MEDIUM GRAY WELL CEMENTED** SHALLOW WELL? 8

LOG OF BORING

Monitoring Well: M-22

JOB	NUMI	BER _				_		LO	0 0	BOITING				,			
COM	PAN	/ <u>AM</u>	ERICA	AN ELECTRIC	POV	VER			во	RING NO. CA-	-0702	DATE 7/17/1	5 SH	HEET _	7 C)F	9
PRO	JECT	CAF	RDINA	L LANDFILL					ВО	RING START	5/1/07	BORII	NG FINISH	<u> 5/2</u>	21/07		
SAMPLE NUMBER	SAMPLE	DEI	IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	uscs		SOIL / I			WELL		LLER'S	 3
		FROIVI	10	BLOWS / 6						MEDIUM GR	AIN SANDS	TONE					
							- - - 155 —										
19	NQ	159.2	169.2		10.0	93	- - - 160 —			N5 MEDIUM MEDIUM GR		L CEMENTED TONE					
							-										
							165 - - -										
20	NQ	169.2	179.2		10.0	71	170 - -			N5 MEDIUM MEDIUM GR		L CEMENTED TONE					
							175 -										

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: M-22

SAMPLE	SAMPLE	SAM DEF IN F	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL ENGTH ECOVERY	RQD %	DEPTH IN	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
		FROM	ТО	BLOWS / 6"			FEET -					
21	NQ	179.2	189.2		10.0	52	- 180 - -			N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		SWL 117.8' 05/21/0 NQ HOLE TO 179.2 THIS IS A 408 HR READING
							- -			N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
							185 -			N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE W/coal		
22	NQ	189.2	199.2		10.0 85	85				N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE N5 MEDIUM GRAY MEDIUM GRAIN	_	
							190 — - -			SANDSTONE W/coal seams N5 MEDIUM GRAY WELL CEMENTED MEDIUM GRAIN SANDSTONE		
							- 195 –					
							- -					
23	NQ	199.2	209.2		9.2	87	200 -			N5 MEDIUM GRAY WELL CEMENTED MEDIUM GRAIN SANDSTONE		

O ...

LOG OF BORING

JOB NUMBER

Monitoring Well: M-22

AEP

ROJECT CARDINAL LANDFILL		BORING NO. <u>CA-0702</u> DATE <u>7/17/15</u> SHEET <u>9</u> OF <u>9</u>					
OF UNDINAL PRIME		BORING START <u>5/1/07</u> BORING FINISH <u>5/21/07</u>					
SAMPLE STANDARD DEPTH PENETRATION RESISTANCE FROM TO BLOWS / 6" RQD DEPTH PENETRATION RESISTANCE FROM TO BLOWS / 6"	ZAPH LOG	SOIL / ROCK D IDENTIFICATION DRILLER'S NOTES					
205	-	COAL SEAM N5 MEDIUM GRAY SANDY COARSE STONE					
		N5 MEDIUM GRAY MEDIUM GRAIN WELL CEMENTED SANDSTONE					
24 NQ 209.2 219.2 10.0 95 210		N5 MEDIUM GRAY WELL CEMENTED MEDIUM GRAIN SANDSTONE					
215		HARD N5 MEDIUM GRAY SILTY SHALE					
		STOPPED BORING @ 219.2' 05/21/07; INSTALLED 2" PVC MONITORING WELL; SWL 90.8' 05/22/07; NQ HOLE TO 219.2'; 14 HR READING					

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16

I	7	\mathbf{E}

COMPANY AMERICAN ELECTRIC POWER									BORING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>1</u> OF <u>11</u>				
PROJECT CARDINAL LANDFILL									BORING START BORING FINISH				
COORDINATES N 835,565.0 E 2,516,519.0										PIEZOMETER TYPE N/A WELL TYPE OW			
GROUND ELEVATION 1065.8 SYSTEM									HGT. RISER ABOVE GROUND 2.798 DIA 2"				
Water Level, ft $\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$									DEPTH TO TOP OF WELL SCREEN 201.3BOTTOM 250.3				
TIME									WI	ELL DEVELOPMENT YES BACKFILL	_ Q	UICK GROUT	
DATE									FIELD PARTY MCR / ZLR RIG D-120				
				1					1			T	
SAMPLE	SAMPLE	SAM DEF IN FI FROM	TH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	NSCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES	
FGD_LANDFILL.GPJ AEP.GDT 7/17/15 X X				BLOWER			5 — 10 — 15 —					GROUNDING PROCEDURES NOT IN USE; DECONNED TOOLS 01/08/07; WATER TO DRILL AND DECON FROM FIRE PROTECTION SYSTEM @ CARDINAL PLANT; BLIND DRILL HW 4" CASING FROM GRADE TO BEDROCK @ 78' THROUGH MINE SPOIL; BLIND DRILLED 4" ROLLER BIT FROM 78' TO 82.8'	
L.GPJ AE	TYPE OF CASING USED								Continued Next Page				
X	X NQ-2 ROCK CORE PIEZOMETER 6" x 3.25 HSA SI OTT									TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE ED SCREEN, G = GEONOR, P = PNEUMATIC			
<u> </u>	9" x 6.25 HSA HW CASING ADVANCER 4" WELL TYPE:												
8	NW CASING 3"								OW = OPEN TUBE SLOTTED SCREEN, GM = GEOMON				
AEP (RECORDER						

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16

COMPANY AMERIC PROJECT CARDINA	AN ELECTRIC POWER L LANDFILL	BORING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>2</u> OF <u>11</u> BORING START <u>1/18/07</u> BORING FINISH <u>1/24/07</u>
SAMPLE DEPTH IN FEET FROM TO	STANDARD PENETRATION RESISTANCE BLOWS / 6" STANDARD PENETRATION RESISTANCE PLOY PENETRATION FEET RQD DEPTH OF PROPERTY OF PR	SOIL / ROCK OSOIL / ROCK IDENTIFICATION DRILLER'S NOTES
	25 —	
	30 -	
	35 -	
3DT 7/17/15	40 -	
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	45 —	Continued Next Page

LOG OF BORING

JOB NUMBER

Monitorign Well: M-16



				AN ELECTRIC	POV	VER				PRING NO. <u>CA-0616</u> DATE_		
PR	DJECT	_CAF	RDINA	L LANDFILL					ВО	ORING START	BORING FINISH 1	<u>/24/07</u>
SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							50 -					
							55 -	_				
							60 -	_				
01 7/17/15							65 -	_				
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15							70 -	_		Condinue of Marie 7		
AEF										Continued Next P	ay c	

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16



			<u>IN ELECTRIC</u> L LANDFILL	POW	VER				RING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SI RING START <u>1/18/07</u> BORING FINISI		
PROJECT	· CAI	CDIIVAL	L LANDI ILL					ЬС	RING START THOU	1 <u>1/</u>	24/07
SAMPLE NUMBER SAMPLE	SAM DEF IN F FROM	PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
						75 -	-				
						80 -	_				
1 NQ	82.8	89.6		6.6	12	85 -			MEDIUM HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		STARTED CORING @ 82.8'
						-					
2 NQ.	89.6	97.6		7.4	36	90 -			MEDIUM HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15 © © NAME	97.6	104.6		6.8	56	95 -			SOFT 5YR 3/4 MODERATE BROWN SHALE Continued Next Page		

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16

				N ELECTRIC	POV	VER				RING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>11</u>
PRO	JECT	CAF	RDINA	L LANDFILL						RING START BORING FINISH
SAMPLE	SAMPLE	SAM DEF IN F FROM	EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION □ DRILLER'S NOTES
							100 -			HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE
							-	-		
4	NQ	104.6	114.6		10.0	67	105 - -	-		SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE
							110 – -			HARD SILTY FINE 5B 5/1 MEDIUM BLUISH GRAY SANDSTONE w/limestone nodules
							-			HARD 5B 5/1 MEDIUM BLUISH GRAY FINE TO MEDIUM GRAIN SANDSTONE
	NQ	114.6	124.6		10.0	82	115 - - - -			HARD 5B 5/1 MEDIUM BLUISH GRAY FINE TO MEDIUM GRAIN SANDSTONE
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15							120 - - -			LOST ALL DRILL
AEP CD_FG										Continued Next Page

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16



COMPANY AMERICAN ELECTRIC POWER ___ DATE <u>7/17/15</u> SHEET <u>6</u> OF __ BORING NO. CA-0616 PROJECT CARDINAL LANDFILL 1/18/07 BORING FINISH 1/24/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK DRILLER'S P00 WELL SC IN IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 123.0'; NO VISABLE SIGNS OF SOFT TO HARD 5B 5/1 MEDIUM BLUISH 6 NQ 124.6 129.6 4.6 13 125 FRACTURES OR **GRAY CLAY SHALE IRON STAINING** HARD N5 MEDIUM GRAY SHALEY LIMESTONE 7 NQ 129.6 139.6 9.6 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY 39 SWL 1,122' ON 130 SHALE 01/22/07 (80 HR w/limestone nodules throughout READING) NQ HOLE TO 129.6' 135 HARD N5 MEDIUM GRAY LIMESTONE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 8 NQ 139.6 146.1 5.7 SOFT TO HARD 5B 5/1 MEDIUM BLUISH 32 140 **GRAY CLAY SHALE** HARD N5 MEDIUM GRAY LIMEY SHALE HARD N5 MEDIUM GRAY LIMESTONE 145 FGD LANDFILL.GPJ AEP.GDT 7/17/15 NQ 146.1 154.6 8.4 31 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 8

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16

AEP

BORING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF ___ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 1/18/07 BORING FINISH 1/24/07 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S TOTAL LENGT ECOVE WELL SC IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 10 NQ 154.6 159.6 5.0 22 SOFT TO HARD 5B 5/1 MEDIUM BLUISH SWL 109.4' ON 155 **GRAY CLAY SHALE** 01/23/07 (~15 HR READING) NQ HOLE TO 159.6' HARD N5 MEDIUM GRAY LIMESTONE 11 NQ 159.6 169.6 5.8 72 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY FROM 159.6' - 169.6' 160 SHALE INNER TUBE DID w/limestone nodules throughout NOT LATCH IN CORE BARREL; **PULLED TOOLS &** RECOVERED 5.8' OF CORE FROM **INSIDE CORE** BARREL; CURE COULD BE MISPLACED IN BOX; **RESET TOOLS &** 165 STARTED CORING @ 169.6', CORED 5.0' - 174.6'; PICKED UP 2.1' OF CORE FROM RUN #11 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY 12 NQ 169.6 174.6 7.1 75 170 SHALE w/limestone nodules LANDFILL.GPJ AEP.GDT 7/17/15 HARD 5B 7/1 LIGHT BLUISH GRAY SILTY FINE GRAIN WELL CEMENTED SANDSTONE HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY 13 NQ 174.6 184.6 10.0 64 175 FGD FINE GRAIN WELL CEMENTED SANDSTONE 8

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16



	SAMPLE	SAM		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TAL IGTH OVERY	RQD	DEPTH	— —	ВО	RING START <u>1/18/07</u> BORING FINIS	н <u>1</u>	/24/07
SAMPLE	SAMPLE	DEI IN F	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE	TAL IGTH OVERY	RQD	DEDTU					
				BLOWS / 6	RECE	%	IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							180 -					
								× × × × × × × × × × × × × × × × × × ×		HARD N4 MEDIUM DARK GRAY SILTY SILTSTONE		
14	NQ	184.6	194.6		10.0	25	185 -			HARD 5B 7/1 LIGHT BLUISH GRAY MEDIUM SANDSTONE HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY FINE SANDSTONE		
							190 - - -			HARD N7 LIGHT GRAY MEDIUM SANDSTONE HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY FINE SANDSTONE	-	
	NQ	194.6	199.6		5.1	90	195 -	-		HARD N7 LIGHT GRAY MEDIUM SANDSTONE HARD N7 LIGHT GRAY MEDIUM TO COARSE SANDSTONE	-	SWL 117.6' ON 01/24/07 (18 HR READING) NQ HOLE TO 199.6'
CD FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	199.6	209.6		10.1	94	200 -			HARD N6 MEDIUM LIGHT GRAY WELL CEMENTED MEDIUM TO COARSE SANDSTONE	_	

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16

				N ELECTRIC L LANDFILL	POV	VER				RING NO. <u>CA-0616</u> DATE <u>7/17/15</u> S RING START <u>1/18/07</u> BORING FINIS		
SAMPLE NUMBER	SAMPLE	SAMI DEP IN FE FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							- 205 -					
_17	NQ	209.6	219.6		9.9	97	210 - -			HARD N6 MEDIUM LIGHT GRAY WELL CEMENTED MEDIUM TO COARSE SANDSTONE W/small 1" seams of coal		
							215 -					
18	NQ	219.6	229.6		10.0	86	- 220 - -			HARD N6 MEDIUM LIGHT GRAY WELL CEMENTED MEDIUM TO COARSE SANDSTONE		
CD_FGD_LANDFILL.GFJ AEF.GD1 //1/15							- - 225 -	× × × × × × × × × × × × × × × × × × ×		HARD N4 MEDIUM DARK GREY SILTSTONE HARD N6 MEDIUM LIGHT GRAY WELL		
AEP CD_LANDI							-			CEMENTED MEDIUM TO COARSE SANDSTONE Continued Next Page		

LOG OF BORING

JOB NUMBER

Monitoring Well: M-16



COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>10</u> OF __ PROJECT CARDINAL LANDFILL 1/18/07 BORING FINISH 1/24/07 **BORING START** STANDAKL
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S **DEPTH** SOIL / ROCK DRILLER'S FOG WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 19 NQ 229.6 239.6 10.0 HARD N6 MEDIUM LIGHT GRAY WELL 230 **CEMENTED MEDIUM TO COARSE** SANDSTONE HARD N4 MEDIUM DARK GRAY SILTSTONE 235 HARD N6 MEDIUM LIGHT GRAY WELL **CEMENTED MEDIATE TO COARSE** SANDSTONE HARD N4 MEDIUM DARK GRAY SILTSTONE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY 20 NQ 239.6 249.6 9.9 72 240 SHALE w/limestone nodules HARD N6 MEDIUM LIGHT GRAY WELL **CEMENTED MEDIUM TO COARSE** SANDSTONE COAL SEAM HARD N6 MEDIUM LIGHT GRAY WELL **CEMENTED MEDIUM TO COARSE SANDSTONE** 245 HARD N4 MEDIUM DARK GRAY SILTSTONE FGD LANDFILL.GPJ AEP.GDT 7/17/15 21 NQ 249.6 254.6 5.1 HARD N5 MEDIUM GRAY WELL CEMENTED 41 250 MEDIUM TO COARSE SANDSTONE w/siltstone crossbedded throughout HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE w/limestone nodules 8

LOG OF BORING

Monitoring Well: M-16

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0616</u> DATE <u>7/17/15</u> SHEET <u>11</u> OF __ PROJECT CARDINAL LANDFILL 1/18/07 BORING FINISH 1/24/07 **BORING START** PENETRATION RESISTANCE BLOWS / 6" RQD W SAMPLE SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO STOPPED BORING @ 254.6 ON 01/24/07; BUILD 2" MONITORING WELL; 111.0' - 130.0' IS CONNELLSVILLE; 194.6' - 249.7' IS MORGANTOWN

CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER _

Monitoring Well: M-15

AEP

				AN ELECTRIC	POW	VER				DRING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHE		11
				L LANDFILL	10 17	2 2				PRING START 7/18/07 BORING FINISH		
				3,569.0 E 2,5 1071.8 s						EZOMETER TYPE N/A WELL TYPE GT. RISER ABOVE GROUND 2.45 DIA		
				<u>107 1.0</u> 3	IOILIVI					EPTH TO TOP OF WELL SCREEN 214.0BOTTOM		
		/el, ft	$\overline{\Delta}$	<u> </u>		1 ×	•			ELL DEVELOPMENT YES BACKFILL		DUT
TIMI											D-120	
DAT	<u> </u>											
SAMPLE	SAMPLE	DE	MPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	DRILLE NOTE	ES
1	NQ	8.6	14.4		3.7	11	5			SOFT N6 MEDIUM LIGHT GRAY BROKEN SILTY CLAYSHALE	PROCEDUR IN USE ON TO BORING; DECON WAY USED FROM CARDINAL FOR PROTECTIC SYSTEM; DECONNED & DRILL 07/ BLIND DRILL HW CASING START COR 8.6'	ES NOT THIS RILL AND TER A FIRE DN TOOLS 18/07; LED 4"
2	NQ	14.4	24.4		6.3	30	- - 15 - -			N5 MEDIUM GRAY BROKEN SILTSTONE		
FGD_LANDFILL.GPJ AEP.GDT 7/17/15							-	××		HARD N8 VERY LIGHT GRAY LIMESTONE w/heavy iron staining throughout		
L.GP.		TYP	E OF C	ASING USED)					Continued Next Page		
GD_LANDFIL		6" x 3.25 HSA 9" x 6.25 HSA SLOT						ETER TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE OTTED SCREEN, G = GEONOR, P = PNEUMATIC				
8		HW CA NW CA		VANCER	<u>4"</u> 3"		WELL T	YPE:	0	W = OPEN TUBE SLOTTED SCREEN, GM =	= GEOMON	
AEP C		SW CA	SING		6"					RECORDER		
٩l		AIR HA	WINER		8"							

LOG OF BORING

JOB NUMBER

Monitoring Well: M-15



COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>2</u> OF _ PROJECT CARDINAL LANDFILL 7/18/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER GRAPHIC LOG DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK WELL DRILLER'S SCS IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO HARD N7 LIGHT GRAY SILTY CLAYSHALE NQ 24.4 29.9 4.3 47 w/iron staining 30 NQ 29.9 31 HARD N5 MEDIUM GRAY WELL CEMENTED 39.4 9.1 **FINE SANDY SILTSTONE** w/iron staining throughout; high angle fracture @ 35.2' 35 NQ HARD N3 DARK GRAY FINE SANDY 5 39.4 49.9 10 22 40 SILTSTONE Well Cemented FGD LANDFILL.GPJ AEP.GDT 7/17/15 45 8

LOG OF BORING

Monitoring Well: M-15

JOB NUMBER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>3</u> OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 7/18/07 BORING FINISH 7/25/07 **BORING START** STANDARD PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** LOG SOIL / ROCK WELL DRILLER'S SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO N1 BLACK COAL NQ 49.9 54.9 68 **N6 MEDIUM LIGHT GRAY LIMESTONE** N5 MEDIUM GRAY LIMEY SILTSTONE 55 **N5 MEDIUM GRAY BROKEN LIMEY** NQ 54.9 59.9 4.6 43 SILTSTONE HARD 5Y 6/4 DUSKY YELLOW FINE GRAIN WELL CEMENTED SANDSTONE w/heavy iron staining; vertical fracture @ 56.5' 60 HARD N5 MEDIUM GRAY WELL CEMENTED 8 NQ 59.9 69.9 7.1 61 LIMESTONE 65 **N5 MEDIUM GRAY BROKEN CLAYSHALE** w/fractures @ 61' and 64.0' FGD LANDFILL.GPJ AEP.GDT 7/17/15 70 43 HARD N5 MEDIUM GRAY LIMESTONE NQ 69.9 79.9 5.4 SOFT N7 LIGHT GRAY CLAYSHALE 8

LOG OF BORING

Monitoring Well: M-15

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>4</u> OF _ PROJECT CARDINAL LANDFILL **BORING START** 7/18/07 BORING FINISH 7/25/07 STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES** FEET FROM TO 75 N1 BLACK COAL 10 NQ 79.9 89.9 6.6 23 **N5 MEDIUM GRAY SILTSTONE** w/high angle fracture 85 90 N6 MEDIUM LIGHT GRAY FINE GRAIN WELL 11 NQ 89.9 99.9 10 12 **CEMENTED SANDY CLAYSHALE** CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 95

LOG OF BORING

Monitoring Well: M-15

JOB	NUMI	BER _				_			o or bording
				N ELECTRIC	POV	VER			BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>1</u>
PRO	JECT	CAF	RDINA	L LANDFILL					BORING START <u>7/18/07</u> BORING FINISH <u>7/25/07</u>
SAMPLE	SAMPLE		PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	SOIL / ROCK SOIL /
12	NQ	99.9	106.9	BLOWER	5.8	53	- 100 — - -		N6 MEDIUM LIGHT GRAY FINE GRAIN SANDY CLAYSHALE
13	NQ	106.9	114.9		5.6	0	105 - - -		N6 MEDIUM LIGHT GRAY WELL CEMENTED FINE GRAIN SANDSTONE Whigh angle fracture throughout whole piece N6 MEDIUM LIGHT GRAY WELL CEMENTED FINE GRAIN SANDSTONE HARD N4 MEDIUM GRAY SHALE W/machine break
							110 - - -		SOFT N4 MEDIUM GRAY CLAYSHALE
14	NQ	114.9	120.9		5.2	8	115 - - -		HARD N5 MEDIUM GRAY CLAYSHALE N6 MEDIUM LIGHT GRAY LIMESTONE w/ high angle fracture from 117' - 118.4'
15	NQ	120.9	129.9		4.8	38	- 120 - - -		SOFT N5 MEDIUM GRAY CLAYSHALE HARD N5 MEDIUM GRAY CLAYSHALE
							-	릠	N6 MEDIUM LIGHT GRAY LIMESTONE

Continued Next Page

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

Monitoring Well: M-15

				N ELECTRIC L LANDFILL					BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>6</u> OF <u>11</u> BORING START <u>7/18/07</u> BORING FINISH <u>7/25/07</u>
SAMPLE NUMBER	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	SOIL / ROCK
							125 - - -		HARD N5 MEDIUM GRAY CLAYSHALE
16	NQ	129.9	134.9		3.9	10	130 — - - -		HARD N6 MEDIUM LIGHT GRAY SILTSTONE w/high angle fracture @ 130.9' SOFT N6 MEDIUM LIGHT GRAY CLAYSHALE
17	NQ	134.9	138.4		2.3	17	135 -		HARD N5 MEDIUM GRAY LIMESTONE N5 MEDIUM GRAY SILTSTONE HARD N5 MEDIUM GRAY CLAYSHALE
18	NQ	138.4	143.9		6.5	0	- - 140 —		HARD N5 MEDIUM GRAY CLAYSHALE
19	NQ	144.4	149.4		4.0	18			SOFT N4 MEDIUM DARK GRAY CLAYSHALE N5 MEDIUM GRAY LIMEY SILTSTONE
20	NQ	149.4	154.4		3.9	0	-	× × × × × × × × = = =	SOFT N5 MEDIUM GRAY CLAYSHALE N4 MEDIUM DARK GRAY CLAYSHALE

LOG OF BORING

Monitoring Well: M-15

AEP M-15

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL LANDFILL 7/18/07 BORING FINISH 7/25/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION LOG SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO NQ 154.4 158.4 3.5 11 HARD N6 MEDIUM LIGHT GRAY SILTSTONE 155 HARD N5 MEDIUM GRAY CLAYSHALE NQ 158.4 164.9 5.7 16 160 165 23 NQ 164.9 168.9 3.4 0 SOFT N6 MEDIUM LIGHT GRAY CLAYSHALE w/high angle fracture @ 168.7' NQ 168.9 174.9 5.7 0 HARD N5 MEDIUM GRAY CLAYSHALE 24 170 175 NQ 174.9 179.9 10 HARD N5 MEDIUM GRAY SILTSTONE

Continued Next Page

FGD LANDFILL.GPJ AEP.GDT 7/17/15

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LOG OF BORING

JOB NUMBER

Monitoring Well: M-15



COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>8</u> OF _ PROJECT CARDINAL LANDFILL 7/18/07 BORING FINISH 7/25/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S **DEPTH** PENETRATION F0G SOIL / ROCK WELL DRILLER'S TOTAL LENGT ECOVE SC IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 180 NQ 179.9 182.4 HARD N4 MEDIUM DARK GRAY LIMESTONE 1.9 0 N4 MEDIUM DARK GRAY SILTSTONE 27 NQ 182.4 183.9 1 0 N4 MEDIUM DARK GRAY SILTY LIMESTONE SWL 67.4' - 14 HR READING / NQ HOLE TO 182.9' NQ 183.9 187.9 HARD N5 MEDIUM GRAY LIMEY SILTSTONE 28 3.4 50 185 SOFT N5 MEDIUM GRAY SILTSTONE w/high angle fracture @ 186.7' SOFT N5 MEDIUM GRAY LIMEY SILTSTONE HARD N5 MEDIUM GRAY LIMEY SILTSTONE 29 NQ 187.9 189.9 2.4 0 190 HARD N6 MEDIUM LIGHT GRAY LIMEY 30 NQ 189.9 194.9 4.9 0 SILTSTONE SOFT N6 MEDIUM LIGHT GRAY CLAYSHALE 195 31 NQ 194.9 32 **N5 MEDIUM GRAY CLAYSHALE** 199 9 5 **N5 MEDIUM GRAY FINE GRAIN WELL CEMENTED SANDSTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 **N6 MEDIUM LIGHT GRAY SILTSTONE** 200 **N6 MEDIUM LIGHT GRAY SANDY FINE** 32 NQ 199.9 204.9 36 5 **GRAIN WELL CEMENTED SILTSTONE** w/crossbeddings in sandstone 8

LOG OF BORING

Monitoring Well: M-15

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET _**9**__ OF __ BORING NO. CA-0614 PROJECT CARDINAL LANDFILL 7/18/07 BORING FINISH 7/25/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S **DEPTH** PENETRATION F0G SOIL / ROCK DRILLER'S OTAL WELL SC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **N6 MEDIUM LIGHT GRAY FINE GRAIN WELL CEMENTED SANDSTONE** badly broken by machine 205 33 NQ 204.9 10.2 28 **N6 MEDIUM LIGHT GRAY FINE GRAIN** 214.9 SANDSTONE N4 MEDIUM DARK SANDY CLAYSHALE **N6 MEDIUM LIGHT GRAY SILTY SANDSTONE** w/crossbeddings 210 -SOFT N4 MEDIUM DARK GRAY SANDY CLAYSHALE **N6 MEDIUM LIGHT GRAY WELL CEMENTED** FINE GRAIN SANDSTONE **N4 LIGHT GRAY WELL CEMENTED FINE SANDY SILTSTONE** w/sandstone lenses 215 34 NQ 214.9 224.9 10 76 **N5 MEDIUM GRAY FINE GRAIN SILTSTONE** w/sandstone lenses **N5 MEDIUM GRAY FINE SANDSTONE** w/crossbedding throughout **N5 MEDIUM GRAY FINE GRAIN SILTSTONE** w/sandstone lenses 220 **N5 MEDIUM GRAY COARSE SANDSTONE** well cemented throughout 225 **N6 MEDIUM LIGHT GRAY COARSE** 35 NQ 224.9 229.9 5 86 **SANDSTONE** crossbedded w/siltstone N5 MEDIUM GRAY SILTSTONE **N6 MEDIUM LIGHT GRAY COARSE** SANDSTONE

VEP CD_FGD_LANDFILL.GPJ

AEP.GDT 7/17/15

Monitoring Well: M-15 LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>10</u> OF <u>11</u> PROJECT CARDINAL LANDFILL

NUMBER	SAMPLE	DEF IN F	PLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	%	DEPTH IN FEET	GRAPHIC	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
36	NQ	229.9	234.9		5	90	230 - - -			N5 MEDIUM GRAY SILTSTONE N6 MEDIUM LIGHT GRAY COARSE SANDSTONE W/coal lenses HARD N7 LIGHT GRAY COARSE SANDSTONE		
							-	2		COAL PARTINGS HARD WELL CEMENTED COAL LENSES		
37	NQ	234.9	244.9		10	90	235 -	-		N7 LIGHT GRAY MEDIUM GRAIN SANDSTONE w/1" cross of clayshale		
							240 -			HARD WELL CEMENTED CLAYSHALE crossbedded w/fine grain sandstone		
38	NQ	244.9	254.9		9.2	91	245 -			N6 MEDIUM LIGHT GRAY MEDIUM GRAIN SANDSTONE W/clayshale crossbedding		
							250 –			N2 GRAYISH BLACK CLAYSHALE crossbedded w/fine grain sandstone N6 MEDIUM LIGHT GRAY MEDIUM GRAIN SANDSTONE w/clayshale crossbedding		
							-			N2 GRAYISH BLACK CLAYSHALE crossbedded w/fine grain sandstone HARD N7 MEDIUM LIGHT GRAY MEDIUM		

Monitoring Well: M-15



LOG OF BORING JOB NUMBER BORING NO. <u>CA-0614</u> DATE <u>7/17/15</u> SHEET <u>11</u> OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 7/18/07 BORING FINISH 7/25/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO crossbedded w/clayshale 255 39 NQ 254.9 264.1 9.2 100 N7 LIGHT GRAY COURSE GRAIN SANDSTONE w/lenses 260 NQ 264.1 269.6 6.2 89 HARD N6 MEDIUM LIGHT GRAY WELL **CEMENTED COARSE SANDSTONE** 265 w/coal parting @ 266.0' 270 41 NQ 269.9 277.4 65 HARD N6 MEDIUM LIGHT GRAY WELL 7.5 **CEMENTED COARSE SANDSTONE** w/limestone nodules @ 273.9' - 274.9' CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 275 HARD N5 MEDIUM GRAY WELL CEMENTED SILTSTONE STOPPED BORING @ 277.4' 07/25/07

LOG OF BORING

JOB NUMBER

Monitoring Well: M-14



COM	1PAN	Y _ AM	IERIC/	AN ELECTRIC	POV	VER			BC	DRING NO. <u>C</u>	\-0612	DATE_	7/17/15 SH	HEET	_1 OF8
PRO	JECT	CAF	RDINA	L LANDFILL									BORING FINISH		
COC	RDIN	IATES _	N 832	2,901.9 E 2,5	19,66	1.8			PII	EZOMETER TY	/PE		WELL TYPE	<u> </u>)W
GRO	UND	ELEVAT	TON _	984.9 SY	'STEM				HC	GT. RISER ABO	OVE GROU	UND 3.30	01 DI	A _2	
Wate	er Lev	el, ft	∇	T		A	-		DE	EPTH TO TOP	OF WELL	SCREEN	127.3 BOTTON	1 <u>1</u>	84.3
TIME	Ε	•				-			WI	ELL DEVELOP	MENT _	YES	BACKFIL		QUICK GROUT
DAT									FIE	ELD PARTY _	MCR/	ZLR	RIC	₃ <u>_</u> _)-120
	_														
SAMPLE	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	SOSO			L / ROCK TIFICATION		WELL	DRILLER'S NOTES
1	NQ	14.0	19.3		2.2	18	5 - 5 - 10 - 15 - 15 - 15 - 15 - 15 - 15			HARD N6 LI SOFT 5B 5/ SHALE					GROUNDING PROCEDURES NOT IN USE; DECONNED TOOLS & DRILL 03/01/07; DRILL WATER USED COMING FROM FIRE PROTECTION SYSTEM @ CARDINAL; BLIND DRILLED 325 HSA'S TO TOP OF BEDROCK @ 14.0'; STARTED CORING AT 14.0'
LANDFILL.GPJ AEP.GDT 7/17/15	NQ	19.3	24.7		2.7 30				SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY						
.GPJ		TYPE	OF C	ASING USED	USED					C	Continue	ed Next Pa	age		
<u> </u>	Т		2 ROCK CORE PIEZOMETER					Continued Next Page TER TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE				DENITI IRE			
P		6" x 3.25	5 HSA	-											LINTUDE
- GD		9" x 6.25 HSA						OTTED SCREEN, G = GEONOR, P = PNEUMATIC YPE: OW = OPEN TUBE SLOTTED SCREEN, GM = GEOMON				SECMON			
8		NW CAS		VANUEN	3"		WELL T	YPE:	0	vv = OPEN	I UBE S	LOTTED	SCREEN, GN	/I = C	SEUMUN
<u>а</u> —		SW CAS	SING		6"]				RECORDE	R ZLI	R			

LOG OF BORING

JOB NUMBER

Monitoring Well: M-14



BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>2</u> OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S DEPTH LOG SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO SHALE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY 3 NQ 24.7 9.9 23 34.7 25 SHALE w/vertical fractures 30 HARD N5 MEDIUM GRAY CLAY SHALE NQ 34.7 4.6 417 35 w/limestone nodules throughout, w/fractures 40 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N5 MEDIUM GRAY CLAY SHALE NQ 41.7 44.7 1.5 27 w/limestone nodules throughout NQ 44.7 54.7 10.0 69 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY 45 SHALE

Monitoring Well: M-14 LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>3</u> OF _ PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH DEPTH LOG SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 50 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 7 NQ 54.7 64.7 9.6 49 SOFT N7 LIGHT GRAY CLAY SHALE 55 HARD N7 LIGHT GRAY CLAY SHALE 60 -SOFT N7 LIGHT GRAY CLAY SHALE 8 NQ 64.7 72.7 7.9 28 SOFT 5G 6/1 GREENISH GRAY CLAY SHALE 65 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 70 w/limestone nodules

FGD LANDFILL.GPJ AEP.GDT 7/17/15

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LOG OF BORING

JOB NUMBER

Monitoring Well: M-14



				AN ELECTRIC L LANDFILL	POV	/ER			BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>4</u> OF <u>5</u> BORING START <u>3/6/07</u> BORING FINISH <u>3/21/07</u>
SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	SOIL / ROCK DRILLER'S IDENTIFICATION SOIL / ROCK NOTES
9	NQ	72.7	79.7		7.0	27	75 -	-	HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/limestone nodules
10	NO	70.7	90.7		10.0	67			SOFT 5G 6/1 GREENISH GRAY CLAY SHALE
10	NQ	79.7	89.7		10.0	67	80 -	× × × × × × × × × × × × × × × × × × ×	HARD 5G 6/1 GREENISH GRAY LIMESTONE HARD WELL CEMENTED SILTSTONE W/limestone nodules
							85 -	- X X X X X X X X X X X X X X X X X X X	
11	NQ	89.7	99.7		10.0	40	90 -	× × × × × × × × × × × × × × × × × × ×	HARD 5G 6/1 GREENISH GRAY WELL CEMENTED SILTSTONE
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15							95 –	X X X X X X X X X X X X X X X X X X X	SOFT 5G 6/1 GREENISH GRAY SHALE
D_FGD_LANDF							-		SOFT N7 LIGHT GRAY SHALE
AEP CI									Continued Next Page

LOG OF BORING

JOB NUMBER

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COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF _ PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PLOUS
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH GRAPHIC S **DEPTH** LOG SOIL / ROCK WELL DRILLER'S SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD N7 LIGHT GRAY LIMESTONE 12 NQ 99.7 109.7 10.0 60 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 100 HARD 5G 6/1 GREENISH GRAY WELL **CEMENTED SILTSTONE** 105 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 13 NQ 109.7 119.7 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 9.6 66 110 115 HARD 5G 6/1 GREENISH GRAY WELL **CEMENTED SILTSTONE** CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 14 NQ 119.7 129.7 10.0 82 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 120 N3 DARK GRAY COAL HARD N5 MEDIUM GRAY CLAY SHALE

Monitoring Well: M-14 LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>6</u> OF _ PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PLOUS
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO w/traces of sandstone HARD N7 LIGHT GRAY WELL CEMENTED 125 -MEDIUM TO COARSE SANDSTONE w/cross bedding throughout 15 NQ 129.7 139.7 10.0 96 MEDIUM TO COARSE N6 MEDIUM LIGHT 130 **GRAY WELL CEMENTED SANDSTONE** 135 16 NQ 139.7 149.7 10.0 90 MEDIUM TO COARSE N6 MEDIUM LIGHT 140 **GRAY WELL CEMENTED SANDSTONE** 145 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

Continued Next Page

HARD 5G 6/1 GREENISH GRAY SILTSTONE

LOG OF BORING

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Monitoring Well: M-14

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COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S DEPTH SOIL / ROCK DRILLER'S SCS WELL IN FEET % **IDENTIFICATION NOTES FEET** FROM TO NQ 149.7 159.7 10.0 56 MEDIUM TO COARSE N6 MEDIUM LIGHT **GRAY WELL CEMENTED SANDSTONE** MEDIUM TO COARSE N6 MEDIUM LIGHT GRAY WELL CEMENTED SILTSTONE FINE TO MEDIUM N6 MEDIUM LIGHT GRAY WELL CEMENTED SANDSTONE w/cross bedding silt stone 155 18 NQ 159.7 169.7 FINE TO MEDIUM N6 MEDIUM LIGHT GRAY 10.0 86 160 WELL CEMENTED SANDSTONE w/trace siltstone 165 FINE TO MEDIUM N7 LIGHT GRAY WELL 19 NQ 169.7 179.7 9.8 83 170 **CEMENTED SANDSTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 175 8

LOG OF BORING

JOB NUMBER

Monitoring Well: M-14



COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0612</u> DATE <u>7/17/15</u> SHEET <u>8</u> OF _ PROJECT CARDINAL LANDFILL 3/6/07 BORING FINISH 3/21/07 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S **DEPTH** SOIL / ROCK WELL DRILLER'S SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO FINE TO MEDIUM N7 LIGHT GRAY WELL **CEMENTED SANDSTONE** 20 NQ 179.7 189.7 9.8 93 180 w/limestone nodules MEDIUM TO COARSE N7 LIGHT GRAY WELL **CEMENTED SANDSTONE** w/limestone nodules 185 **5G 6/1 GREENISH GRAY WELL CEMENTED** SILSTONE 21 NQ 189.7 194.7 **5G 6/1 GREENISH GRAY WELL CEMENTED** 4.6 93 190 SILTSTONE STOPPED BORING @ 194.7'; SWL @ 44.2' 03/23/07; NQ HOLE TO 194.7' CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

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nitoring Well: M1309			•

IOB	NII IM	RER						LO	G C	F BORING	Monitoring \	Wel	l: M1309
				AN ELECTRIC		- VER			ВС	DRING NO. <u>B-1309D</u> DATE <u>7/1</u>	7/15 SHE	ET	1 OF 15
				L FLY ASH DA						DRING START 5/2/13 BG			
COC	ORDIN	NATES _	N 83	5,558.0 E 2,5	17,39	6.3				EZOMETER TYPE			
GRO	DUND	ELEVAT	TION'	1170.2 SY	'STEM	Stat	te Plane usin D27/29	ıg		ST. RISER ABOVE GROUND _ 1.85			
			∇	▼					DE	PTH TO TOP OF WELL SCREEN _ 3	07.9 воттом	34	47.5
TIM		701, 11	-			- -			W	ELL DEVELOPMENT YES	BACKFILL	Q	UICK GROUT & I
DA										ELD PARTY ZLR / TAS			
D/ (
ще	{ щ		IPLE DTU	STANDARD	_±%	RQD	DEPTH	2	ဟ	2011 / 12001/			DDII I EDIO
SAMPLE	SAMPLE		PTH EET	PENETRATION RESISTANCE	NG S	%	IN	GRAPHIC LOG	SC	SOIL / ROCK IDENTIFICATION		WELL	DRILLER'S NOTES
S Z	S	FROM				70	FEET	GR _	\supset	IDENTIFICATION		>	NOTES
1	SPT		1.5				_			STONE PAD			STONE PAD
,	CDT	1.5	2.0	4 7 44				-		VEDV STIFF MODERATE VEH OWIG	211		
2	SPT	1.5	3.0	4-7-11	.9			-		VERY STIFF MODERATE YELLOWIS BROWN 10YR 6/2 CLAY	DΠ		
										tsf 2.0			
3	SPT	3.0	4.5	8-11-16	1.0					VERY STIFF DARK YELLOWISH BRO	OWN		
										10YR 4/2 CLAY AND SHALE tsf 2.0			
4	SPT	4.5	4.7	50/.2	.9		_			HARD PALE BROWN 5YR 5/2 SHALE	EY CLAY		
							5 -	†==		tsf 4.5			
5	SPT	6.0	6.4	50/.4	.4					HARD PALE BROWN 5YR 5/2 SHALE tsf 0	EY CLAY		
								+					STOPPED
													SAMPLING / AUGER
1	NQ	8.2	14.1		5.9	22				HARD LIGHT OLIVE GRAY 5Y 5/2			REFUSAL @ 7.0' / SET 4" CASING
								-		CLAYSHALE			027 7 07101110
							4.0						
							10 -						
								-					
	NO	444	04.4		40.0			==					
2	NQ	14.1	24.1		10.0	9	4.5						
							15 -						
								-==					
								==					
ω								= =					
11/1								==					
7 10													
AEP.GDI //1//15		TYPE	OF C	ASING USED			1		1	Continued Next Page)		
X X		NQ-2 R		RE			PIEZOM					OP	EN TUBE
		6" x 3.25 9" x 6.25					SLO	OTTE	ED S	SCREEN, $G = GEONOR$, $P = PN$	NEUMATIC		
≰		HW CAS	SING AE	OVANCER	4"		WELL T	YPE:	O'	W = OPEN TUBE SLOTTED SC	CREEN, GM	= G	EOMON
8		NW CAS			3" 6"					RECORDER TAS			
AIR HAMMER 8"										NEOCROER IAU			

LOG OF BORING

Monitoring Well: M1309

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1309D** DATE **7/17/15** SHEET **2** OF ___ PROJECT CARDINAL FLY ASH DAM BORING START 5/2/13 BORING FINISH 5/30/13

FNO				LILIAGIIDA					DOMING START		_ BORING FINISI		30/13
SAMPLE NUMBER	SAMPLE	SAM DEF IN FI FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPH LOG	S O S O	SOIL / ROCK	I	WELL	DRILLER'S NOTES
							-						
3	NQ	24.1	26.6		2.5	0	25 -						
4	NQ	26.6	34.1		7.5	17	-						
							30 -						
5	NQ	34.1	39.1		5.0	40	35 -						
							- - -						
6	NQ	39.1	44.1		5	53	40 -		HARD GREI	ENISH GRAY 5G 6/1	CLAYSHALE		
							- - -		HARD DAR	K GRAY N3 CLAYSH,	ALE		
7	NQ	44.1	54.1		10	36	45 -		CLAYSHALI	WNISH GRAY 5YR 4/ E e fractures @ 1.8', 6.0'			
										Continued Next Pa	000		

LOG OF BORING

Monitoring Well: M1309

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1309D** DATE **7/17/15** SHEET **3** OF _ PROJECT CARDINAL FLY ASH DAM 5/2/13 BORING FINISH 5/30/13 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO 50 VERY HARD MEDIUM LIGHT GRAY N6 LIMEY NQ 54.1 64.1 10 46 SHALE 55 w/limestone nodules @ 4.5' 60 -VERY HARD MEDIUM LIGHT GRAY N6 LIMEY NQ 64.1 69.1 5 16 SHALE 65 CD FA DAM.GPJ AEP.GDT 7/17/15 10 NQ HARD MEDIUM DARK GRAY N4 CLAYSHALE 69.1 74.1 5 20 70

LOG OF BORING

Monitoring Well: M1309

	MPAN		IERIC <i>A</i>	N ELECTRIC	POV	VER			ВС	PRING NO. B-1309D DATE 7/17/15 S	HEET	4 OF 15
				L FLY ASH DA						PRING START 5/2/13 BORING FINIS		
SAMPLE	SAMPLE	DE	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
1-	NQ	74.1	84.1		10	52	75 -			VERY HARD LIGHT OLIVE GRAY 5Y 5/2 LIMESTONE w/high angle fractures @ .8', 1.3', 3.0', & 4.0'		
							80 -			HARD MEDIUM BLUISH GRAY 5B 5/1 CLAYSHALE		
12	P. NQ	84.1	94.1		10	98	85 -					
GDT 7/17/15	3 NQ	94.1	104.1		10	72	90 -			HARD MEDIUM DARK GRAY N4 CLAYSHALE		
CD_FA_DAM.GPJ AEP.GDT 7/17/15							-			HARD LIGHT GRAY N7 CLAYSHALE w/limestone nodules, high angle fractures @ 4.9' & 5.4' of recovery		

LOG OF BORING

JOB NUMBER

Monitoring Well: M1309

			ERIC <i>A</i> RDINA	AN ELECTRIC L FLY ASH DA	POV AM	VER_				RING NO. <u>B-1309D</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>15</u> RING START <u>5/2/13</u> BORING FINISH <u>5/30/13</u>
SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	N S C S	SOIL / ROCK ☐ DRILLER'S IDENTIFICATION NOTES
							100 -			HARD MEDIUM BLUISH GRAY 5B 5/1 CLAYSHALE w/limestone nodules throughout
14	NQ	104.1	114.1		10	79	105 -			
							110 - -			
15	NQ	114.1	124.1		10	76	115 -			HARD MEDIUM DARK GRAY N4 SILTSTONE
							120 -	× × × × × × × × × × × × × × × × × × ×		
							-			HARD BLACK N1 COAL Continued Next Page

LOG OF BORING

Monitoring Well: M1309

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1309D** DATE **7/17/15** SHEET **6** OF _ PROJECT CARDINAL FLY ASH DAM 5/2/13 BORING FINISH 5/30/13 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE **RQD** GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE **DEPTH** SOIL / ROCK DRILLER'S USC WELL IN IN FEET % **IDENTIFICATION NOTES FEET** FROM TO 10 NQ 124.1 134.1 65 125 VERY HARD VERY LIGHT GRAY N8 LIMESTONE w/high angle fracture @ 6.8' 130 HARD VERY LIGHT GRAY N8 LIMESTONE NQ 134.1 144.1 10 67 w/ high angle fracture @ 1.1' 135 140 HARD GREENISH GRAY 5G 6/1 LIMEY **CLAYSHALE** HARD GREENISH GRAY 5G 6/1 CLAYSHALE NQ 144.1 154.1 10 26 145 CD FA DAM.GPJ AEP.GDT 7/17/15 MEDIUM HARD BLACK N1 COAL

LOG OF BORING

Monitoring Well: M1309

	NUMI IPANY		IERIC <i>A</i>	N ELECTRIC	POV	/ER			BORING NO. B-1:	309D D.	ATE 7/17/15	SHEET _	7 OF1
PROJECT CARDINAL FLY ASH DAM										5/2/13			
SAMPLE NUMBER	SAMPLE	SAMPLE STANDARD DEPTH PENETRATION RESISTANCE FROM TO BLOWS / 6"		RQD %	DEPTH IN FEET	LOG	8 O S O S O O	SOIL / RO		WELL	DRILLER'S NOTES		
19	NQ	154.1	164.1		10	69	155 —		HARD MEDIL	JM LIGHT GRA	NY N6 CLAYSHALE		
							160 —						
20	NQ	164.1	174.1		10	89	165 —						
							170						
21	NQ	174.1	184.1		10	77	175 —		CLAYSHALE	BLUISH GRA			
									Co	ontinued Ne	xt Page		

Monitoring Well: M1309

JOB NUMBER _______ BORING NO. B-1309D DATE 7/17/15 SHEET 8 OF 15
PROJECT CARDINAL FLY ASH DAM BORING START 5/2/13 BORING FINISH 5/30/13

SAMPLE NUMBER SAMPLE	DE	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
						- - 180 — -					
22 NQ	184.1	194.1		10	95	- 185 - - -			HARD MEDIUM BLUISH GRAY 5B 5/1 CLAYSHALE		
						190 – -					
23 NQ	194.1	204.1		10	62	- 195 – -					
						200 –					

LOG OF BORING

Monitoring Well: M1309

JOB NUMBER BORING NO. **B-1309D** DATE **7/17/15** SHEET **9** OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL FLY ASH DAM 5/2/13 BORING FINISH 5/30/13 **BORING START** STANDARD
PENETRATION FENETRATION FOR STANCE SAMPLE **RQD** SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S **DEPTH** SOIL / ROCK WELL DRILLER'S SCS IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD DARK REDDISH BROWN 10R 3/4 MULTICOLORED CLAYSHALE NQ 204.1 214.1 10 75 HARD MEDIUM BLUISH GRAY 5B 5/1 **CLAYSHALE** 205 210 HARD DARK REDDISH BROWN 10R 3/4 **CLAYSHALE** NQ 214.1 224.1 10 90 HARD MEDIUM BLUISH GRAY 5B 5/1 SHALE w/limestone nodules 215 HARD MEDIUM BLUISH GRAY 5B 5/1 FINE SANDY SHALE 220 FA DAM.GPJ AEP.GDT 7/17/15 NQ 224.1 234.1 10 76 225 8

LOG OF BORING

JOB NUMBER

Monitoring Well: M1309

			ERICA RDINA	N ELECTRIC L FLY ASH DA	POV AM	VER				RING NO. B-1309D DATE 7/17/15 RING START 5/2/13 BORING FINI		
SAMPLE	SAMPLE		IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	문 의	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							230 -					
							-			HARD DARK GREENISH GRAY 5G 4/1 CLAYSHALE		
27	NQ	234.1	244.1		10	88	235 -			HARD LIGHT BLUISH GRAY 5B 7/1 CLAYSHALE w/limestone nodules		
							-					
							-					
							240 -					
28	NQ	244.1	254.1		10	54	-					
							245 -					
							-					
P.GDI 7/1/15							250 -					
AEP CD_FA_DAM.GPJ AEP.GDT 7/17/15							-					
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LOG OF BORING

JOB NUMBER

Monitoring Well: M1309

				N ELECTRIC		VER			ВО	RING NO. B-1309D			
PRO	JECT	CAF	RDINA	L FLY ASH DA	AM_				ВО	RING START	BORING FIN	IISH <u>5/</u> 3	30/13
SAMPLE NUMBER	SAMPLE	DE	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / RO		WELL	DRILLER'S NOTES
29	NQ	254.1	264.1	BLOWS / 0	10	65							
							255 -						
							260 -						
30	NQ	264.1	274.1		10	77	265 -			HARD GREENISH GRAY 5	G 6/1 CLAYSHALE		
							-						
							-			HARD DARK REDDISH BR MULTICOLORED CLAYSH			
							270 -			HARD GREENISH GRAY 5 w/limestone nodules throug			
<u>5</u>	NQ	274.1	284.1		10	89	275 -			HARD GREENISH GRAY 5 HARD DARK REDDISH BR SHALE			
AEP CD_FA_DAM.GPJ AEP.GDT 7/17/15										HARD GREENISH GRAY 5 w/limestone nodules	G 6/1 SHALE		
면 인		1	l	I	l					Continued Ne	ext Page		

LOG OF BORING

Monitoring Well: M1309

	NUM		IEDIC/	AN ELECTRIC	DOM:	VED		LO		DOMENO BA	300D	DATE 7/47/46	5 CUE	ET '	12 OF <u>15</u>
				L FLY ASH DA		VER	_			RING NO. <u>B-1</u> RING START			IG FINISH		
SAMPLE	SAMPLE	DE	MPLE PTH FEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL / F		- -	WELL	DRILLER'S NOTES
							-			HARD GREE SHALE	NISH GRAY	5G 6/1 SANDY			
32	NQ	284.1	294.1		10	88	-				JM LIGHT G	RAY N6 SANDY			
							285 -			SHALE					
							290 -								
								-							
33	NQ	294.1	304.1		10	97	295 -			HARD MEDII CEMENTED		RAY N4 WELL STONE			
AEP CD_FA_DAM.GPJ AEP.GDT 7/17/15							300 -								
LA DAM.GPJ	NQ	304.1	314.1		10	100	305 -			HARD MEDII CEMENTED		RAY N6 WELL STONE			
AEP CI					•	•				C	ontinued N	lext Page			

Monitoring Well: M1309 LOG OF BORING JOB NUMBER BORING NO. **B-1309D** DATE **7/17/15** SHEET **13** OF ___ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL FLY ASH DAM 5/2/13 BORING FINISH 5/30/13 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE RQD SAMPLE SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO 310 NQ 314.1 324.1 10 100 315 320 MEDIUM HARD BLACK N1 COAL HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED FINE SANDSTONE NQ 324.1 334.1 10 97 325 CD FA DAM.GPJ AEP.GDT 7/17/15 HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED FINE SANDSTONE 330 w/limestone fragments

LOG OF BORING

JOB NUMBER

Monitoring Well: M1309

				N ELECTRIC L FLY ASH DA					во	ORING NO. B-1309D DATE 7/17/15 SORING START 5/2/13 BORING FINIS		
SAMPLE	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
37	NQ				10	93	335 -			HARD MEDIUM GRAY N5 SHALEY SANDSTONE HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED MEDIUM SANDSTONE		
							340 -					
38	NQ	344.1	354.1		10	95	345 -					
							350 -			HARD LIGHT BLUISH GRAY 5B 7/1 CLAYSHALE w/limestone nodules		
AEP CD_FA_DAM.GPJ AEP.GDT 7/17/15	NQ	354.1	364.1		10	100	355 -					
EP CD_FA_[Continued Next Page		

Monitoring Well: M1309

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1309D** DATE **7/17/15** SHEET **15** OF ___ PROJECT CARDINAL FLY ASH DAM **BORING START 5/2/13** BORING FINISH **5/30/13** STANDARD
PENETRATION ZESISTANCE HQD RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH USCS DEPTH SOIL / ROCK WELL DRILLER'S IN FEET **IDENTIFICATION NOTES** FEET FROM TO 360 CD_FA_DAM.GPJ AEP.GDT 7/17/15

Monitoring Well: M-1302

JOB	NUME	BER .				_		LO	G C	OF BORING		
PRO	JECT ORDIN	CA I	RDINA N 836	AN ELECTRIC L FLY ASH DA 5,201.9 E 2,5	AM 15,43	2.0	e Plane usinç		B(DRING NO. B-1302M DATE 7/17/15 SHORING START 3/7/13 BORING FINISH EZOMETER TYPE SS WELL TYPE	5 C	//30/13 DW
GRO	DUND			1028.9 SY	STEM	NAD	27/29			GT. RISER ABOVE GROUND 1.8 DIA		
Wat	er Lev	el, ft	$\overline{\Delta}$	Ţ		Ā				EPTH TO TOP OF WELL SCREEN 168.4BOTTOM		
TIM	E									ELL DEVELOPMENT YES BACKFILI		
DAT	Έ								FI	ELD PARTY ZLR / TAS RIG	<u> </u>)-120
SAMPLE	SAMPLE	DE IN F	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE	TOTAL LENGTH ECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
		FROM		BLOWS / 6"	<u>~</u>					STONE DAD #4 LIMESTONE		STONE DAD OFF
	SPT	1.5	3.0	5-13-13 22-20-10	1.3		-	<u> </u>		VERY STIFF DUSKY BROWN 5YR 2/2 MINE SPOIL VERY STIFF MEDIUM LIGHT GRAY N6 SHALE		STONE PAD OFF HAUL ROAD
4	SPT	4.5	6.0	4-5-7	1.2		5 -			STIFF DUSKY BROWN 5YR 2/2 MINE SPOIL		
5	SPT	6.0	7.5	4-5-7	.7		-	5-5		STIFF GRAYISH BROWN 5YR 3/2 MINE SPOIL		
6	SPT	7.5	9.0	7-4-4	1.1		-			STIFF DARK YELLOWISH BROWN 10YR 5/4 MINE SPOIL tsf 1.5		
7	SPT	9.0	10.5	9-6-6	.6		10 -	<u></u>		STIFF DARK YELLOWISH BROWN 10YR 4/2 MINE SPOIL		
8	SPT	10.5	12.0	6-8-8	.1		-			VERY STIFF LIGHT GRAY N7 MINE SPOIL		
9	SPT	12.0	13.5	7-5-5	.5		-			STIFF MODERATE YELLOWISH BROWN 10YR 5/4 MINE SPOIL		
10	SPT	13.5	15.0	6-5-4	.7		-	5-7		STIFF MODERATE YELLOWISH BROWN 10YR 5/4 MINE SPOIL tsf 2.0		
11	SPT	15.0	16.5	4-5-7	.8		15 -))))		STIFF MODERATE YELLOWISH BROWN 10YR 5/4 MINE SPOIL		
12	SPT	16.5	18.0	5-5-9	1.5		-	0,				
13	SPT	18.0	19.5	27-7-6	.6		-	<u> </u>		STIFF LIGHT BROWN 5YR 5/6 MINE SPOIL		
년 14	SPT	19.5	21.0	23-12-15	.2					VERY STIFF LIGHT GRAY N7 MINE SPOIL		
AEP.GDT		TYP	E OF C	ASING USED					_	Continued Next Page		
X X X X	!	6" x 3.2 9" x 6.2	5 HSA	OVANCER	4"		PIEZOMI SLO WELL TO	OTTE	ED :			
8		NW CA	SING		3"		VVELL I	IFE:		·	ı – C	JEOIVIOIN
AEP		SW CA AIR HA			<u>6"</u> 8"	\equiv				RECORDER		

AIR HAMMER

AEP CIVIL ENGINEERING LABORATORY Monitoring Well: M-1302 LOG OF BORING

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1302M** DATE **7/17/15** SHEET **2** OF _ PROJECT CARDINAL FLY ASH DAM BORING START <u>3/7/13</u> BORING FINISH <u>5/30/13</u>

SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPH LOG USC	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
15	SPT	21.0	21.1	50/.1	.1				HARD LIGHT GRAY N7 LIMESTONE		
16	SPT	22.5	22.8	50/.3	1.5				HARD LIGHT GRAY N7 LIMEY CLAYSHALE		
17	SPT	24.0	24.3	50/.3	.2		25 -		HARD DUSKY BROWN 5YR 2/2 LIMEY CLAYSHALE		
1	NQ	25.5	34.0		8.5	27	30 -		MEDIUM HARD MEDIUM BLUISH GRAY 5B 5/1 SANDY CLAYSHALE		
2	NQ	34.0	44.0		5.3	28	35 -		MEDIUM HARD MEDIUM GRAY N5 CLAYSHALE HARD MEDIUM GRAY N5 LIMESTONE		
							40 -		MEDIUM HARD MEDIUM GRAY N5 CLAYSHALE badly broken w/iron stains throughout		Lost water return @ 36.0'
3	NQ	44.0	54.0		3.9	51	45 -		MEDIUM HARD LIGHT BLUISH GRAY 5B 7/1 SANDY CLAYSHALE w/iron stains throughout		

AEP CD_FA_DAM.GPJ AEP.GDT 7/17/15

AEP CIVIL ENGINEERING LABORATORY Monitoring Well: M-1302 LOG OF BORING

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. **B-1302M** DATE **7/17/15** SHEET **3** OF PROJECT CARDINAL FLY ASH DAM BORING START 3/7/13 BORING FINISH 5/30/13

NUMBER	DE	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
						- 50 -					
4 NQ	54.0	64.0		6.4	80				MEDIUM HARD LIGHT BLUISH GRAY 5B 7/1		
						55 -			TO GRAYISH PURPLE 5P 4/2 CLAYSHALE		
						60 -					
5 NQ	64.0	74.0		7.7	62	- 65 -			HARD LIGHT BLUISH 5B 7/1 WELL CEMENTED FINE GRAIN SANDSTONE		
						-					

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-1302

INOULO	· _	<u> </u>		L FLT ASH DA	-1141				ьс	RING START 3/1/13 BORING FINE	511 <u>O</u>	700/10
SAMPLE NUMBER SAMPLE		SAMF DEP IN FE	PLE TH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
6 NC		74.0	84.0		9.3	63	- 75 - -					
							- - 80 – -					
7 NG	Q 8	34.0	94.0		9.8	68	- 85 -					
							- 90 – -			MEDIUM HARD MEDIUM BLUISH GRAY 5B		
8 NC	Q 9	94.0	104.0		1.3	0	- - - 95 –			5/1 CLAYSHALE		
							-			Continued Next Page		

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-1302

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. B-1302M __ DATE <u>7/17/15</u> SHEET <u>5</u> OF PROJECT CARDINAL FLY ASH DAM 3/7/13 BORING FINISH 5/30/13 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH SAMPLE S DEPTH PENETRATION LOG SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO 100 NQ 104.0 MEDIUM HARD LIGHT BLUISH GRAY 5B 7/1 114.0 3.5 23 CLAYSHALE 105 110 NQ 114.0 124.0 6.5 HARD MEDIUM BLUISH GRAY 5B 5/1 10 48 CLAYSHALE 115 CD FA DAM.GPJ AEP.GDT 7/17/15 HARD MEDIUM GRAY N5 LIMESTONE 120 HARD MEDIUM GRAY N5 CLAYSHALE

LOG OF BORING

Monitoring Well: M-1302

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. B-1302M __ DATE <u>7/17/15</u> SHEET <u>6</u> OF PROJECT CARDINAL FLY ASH DAM 3/7/13 BORING FINISH 5/30/13 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER GRAPHIC LOG DEPTH SAMPLE S TOTAL LENGTH RECOVER DEPTH PENETRATION SOIL / ROCK WELL DRILLER'S USC IN IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO NQ 124.0 134.0 10 73 HARD MEDIUM GRAY 5B 5/1 CLAYSHALE w/limestone nodules, high angle fracture @ 2.8' 125 (126.8')130 HARD MEDIUM GRAY N5 CLAYSHALE 12 NQ 134.0 144.0 10 46 135 140 7.85 HARD MEDIUM GRAY N5 CLAYSHALE NQ 144.0 154.0 38 w/limestone nodules 145

VEP CD_FA_DAM.GPJ_AEP.GDT_7/17/15

AEP CIVIL ENGINEERING LABORATORY

Monitoring Well: M-1302

LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. B-1302M __ DATE <u>7/17/15</u> SHEET <u>7</u> OF PROJECT CARDINAL FLY ASH DAM 3/7/13 BORING FINISH 5/30/13 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVE LOG SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO NQ 154.0 159.0 HARD MEDIUM GRAY N5 CLAYSHALE 14 5.1 59 w/high fractures @ 1.7', 3.6', & 4.1' 155 NQ 159.0 164.0 2.25 53 HARD MEDIUM GRAY N5 CLAYSHALE 15 w/high angle fracture @ .4' 160 HARD MEDIUM GRAY N5 SANDY NQ 164.0 174.0 10 16 CLAYSHALE 165 HARD MEDIUM GRAY N5 WELL CEMENTED FINE SANDSTONE 170 DAM.GPJ AEP.GDT 7/17/15 NQ 174.0 184.0 10.1 94 175

NEP CD_FA_

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING



JOB NUMBER ______ BORING NO. B-1302M DATE 7/17/15 SHEET 8 OF 9

BORING START 3/7/13 BORING FINISH 5/30/13

PROJECT CARDINAL FLY ASH DAM

SAMPLE		SAMPLE DEPTH IN FEET	- O	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							180 —					
18 N	Q 184	4.0 19	4.0		9.6	81	- 185 - -			HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED FINE SANDSTONE w/shale lenses, limestone nodules @ 6.8'		
							- 190 — -					
19 NO	Q 19	4.0 20	4.0		10	69	- 195 — -			HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED FINE SANDSTONE w/shale lenses, pyrite and limestone nodules @ 7.8' and 8.3'		
							- 200 -					

LOG OF BORING

Monitoring Well: M-1302

RO	JECT	CAF	RDINA	L FLY ASH DA	AM.				ВО	RING START 3/7/1	BORING FINI	ISH <u>5/3</u>	30/13
NUMBER	SAMPLE	SAM DEF IN F FROM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS		/ ROCK FICATION	WELL	DRILLER'S
20	NQ	204.0	209.0		5.0	66	205 -			HARD MEDIUM LIGHT CEMENTED FINE GRA			
21	NQ	209.0	219.0		10	56				HARD MEDIUM GRAY w/limestone nodules thr HARD MEDIUM DARK	oughout GRAY N4 CLAYSHALE	_	
							210 -			w/limestone nodules thr	oughout		
							215 -						
							-						

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-13

		IBER _					_				DOMINE						
						POW	/ER										_1_ OF8_
				L LAND											BORING FINISH		
CO	ORDII	NATES _	N 83	1,697.9	E 2,5	18,37	4.3								WELL TYPE		
GR	OUNE	ELEVAT	TON _	988.4	SY	STEM									4 DIA		
Wa	ter Le	vel, ft	∇		Ī		$ar{oldsymbol{\Lambda}}$	-		DE	PTH TO TOP	OF V	VELL SCR	EEN _	130.3 BOTTON	1 <u>1</u>	87.3
TIN	1E															C	UICK GROUT
DA	TE									FIE	ELD PARTY	MC	R / ZLR		RIC	<u>D</u>)-120
								1									
SAMPLE			1PLE PTH	PENET	DARD	TER.	RQD	DEPTH	GRAPHIC LOG	S			SOIL / RO)CK		بـ	DRILLER'S
AMP	SAMPLE	1	EET	RESIS	RATION TANCE /S / 6"	15 NO S	%	IN	ZAPI COC	SC		ı	DENTIFICA			WELL	NOTES
S Z	ž Š	FROM	ТО	BLOV	/S / 6"		,0	FEET	9	\supset			<i>D</i> _111111107				110120
								5 10 15									GROUNDING PROCEDURES NOT IN USE ON THIS BORING; DECONNED RIG & TOOL 04/02/07; ALL WATER USED COMING FROM FIRE PROTECTION SYSTEM @ CARDINAL PLANT; BLIND DRILLED 3.25" HSA'S TO 19.0'; STARTED CORING @ 19.0'
FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	19.0	24.4			5.4	20	-			SOFT N7 L	LIGHT	GRAY SA	NDY CL	_AY SHALE		
PD A		TVD		ACINIO	Herr							Ca:-	tinuad NI	ovt De	~~		
JILL.G	,			ASING	USED								inued Ne				
ADNA X		NQ-2 R 6" x 3.25		RE				PIEZOMI							DUS TIP, SS :	= OP	PEN TUBE
3D_L		9" x 6.25	5 HSA	N /A N / C = -		4"		SLC	ווע						PNEUMATIC		
95 P		HW CAS		VANCER	(4" 3"		WELL TY	YPE:	0	W = OPEN	TUI	BE SLOT	TED	SCREEN, GN	1 = G	SEOMON
AEP C		SW CAS	SING			6"					RECORD	ER	RACER	2			
₹		AIR HAI	иMER			8"						_					

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

JOB NUMBER

Monitoring Well: M-13

COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET **_2**__ OF _ BORING NO. CA-0610 PROJECT CARDINAL LANDFILL 4/3/07 BORING FINISH 4/3/07 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** F0G SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO LOST ALL DRILL RETURN WATER @ +/-22.0' HARD FINE SANDY LIMESTONE NQ 24.4 34.4 7.3 7 HARD N7 LIGHT GRAY FINE SANDY LIMESTONE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE w/trace of iron staining throughout 30 NQ 34.4 42.4 2.4 0 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY 35 SHALE HARD N7 LIGHT GRAY LIMESTONE 40 FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N6 MEDIUM LIGHT GRAY CLAY NQ 42.4 49.4 3.4 0 SHALE 45 SOFT N6 MEDIUM LIGHT GRAY CLAY SHALE 8

LOG OF BORING

Monitoring Well: M-13

AEP

TOMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0610 DATE 7/17/15 SHEET 3 OF 8

PROJECT CARDINAL LANDFILL BORING START 4/3/07 BORING FINISH 4/3/07

FRO	JECI	_CAI	LDINA	L LANDFILL						RING START 4/3/01 BURING FINISH		
SAMPLE	SAMPLE		IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
5	NQ	49.4	57.9		6.0	0	50 — - - - - - 55 —			MEDIUM HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		
6	NQ	57.9	64.4		6.5	17	- - - 60 —			SOFT TO MEDIUM 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		SWL @ 13.8' 04/04/07; NQ HOLE TO 64.4' - 14 HOUR READING
7	NQ	64.4	69.4		1.4	0	- - 65 -			HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	69.4	76.4		5.9	0	- - 70 -			SOFT N5 MEDIUM GRAY CLAY SHALE		REASON FOR POOR RECOVERY - HSA'S NOT SEATED @ ROCK & SOIL INTERFACE; PULLED NQ'S RODS
ਹ ਹ										Continued Next Page		

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-13

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0610</u> DATE <u>7/17/15</u> SHEET <u>4</u> OF _ PROJECT CARDINAL LANDFILL 4/3/07 BORING FINISH 4/3/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** F0G SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO AND HSA'S; DRILLED 4" CASING TO 24.0' FOR GOOD **SEAL** 75 NQ 76.4 79.4 39 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SWL @ 22.5' SHALE 04/09/07; NQ HOLE TO 79.4' - 130 HOUR READING MEDIUM TO HARD 5B 5/1 MEDIUM BLUISH NQ 79.4 89.4 10.0 10 4 80 **GRAY CLAY SHALE** 86.0 to 89.4 has iron staining throughout 85 HIGH ANGLE FRACTURE @ 88.4' 89.4 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY 11 NQ 99.4 10.0 15 90 SHALE CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 95 HARD N5 MEDIUM GRAY LIMESTONE

AEP CIVIL ENGINEERING LABORATORY

Monitoring Well: M-13

LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET _**5**__ OF _ BORING NO. CA-0610 PROJECT CARDINAL LANDFILL 4/3/07 BORING FINISH 4/3/07 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH **DEPTH** PENETRATION SOIL / ROCK DRILLER'S WELL LOG SC IN FEET RESISTANCE **IDENTIFICATION** NOTES **FEET** FROM BLOWS / 6" TO HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 12 NQ 99.4 106.4 6.7 HARD N5 MEDIUM GRAY LIMESTONE 45 100 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE high angle fracture @ 103.9 105 MEDIUM HARD 5B 5/1 MEDIUM BLUISH 13 NQ 106.4 114.4 6.5 74 **GRAY SILTY CLAY SHALE** broken, possibly machine breaks MEDIUM TO HARD N5 MEDIUM GRAY SILTY **CLAY SHALE** 110 HARD 5YR 6/1 LIGHT BROWNISH GRAY **CLAY SHALE** NQ 114.4 124.4 10.0 68 115 HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE FGD LANDFILL.GPJ AEP.GDT 7/17/15 120

8

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-13

SAMPLE NUMBER SAMPLE	SAM DEF IN F	IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH ECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
15 NQ	124.4	TO 134.4	BLOWS / 6"	10.0	69	125 -			SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE		
						-	× × × × × × × × × × × × × × × × × × ×		HARD N3 DARK GRAY SILTSTONE		
						-			FINE TO MEDIUM N5 MEDIUM GRAY SANDSTONE well cemented		
						130 –	-		MEDIUM N5 MEDIUM GRAY SANDSTONE well cemented		
						-					
						-					
16 NQ	134.4	144.4		10.0	91	135 -			N5 MEDIUM GRAY LARGE GRAIN WELL CEMENTED SANDSTONE		
						-					
						-					
						140 -					
						-					
17 NQ	144.4	154.4		10.0	62	-			N5 MEDIUM GRAY LARGE GRAIN WELL		
						145			CEMENTED SANDSTONE		
						-					
						-					

AEP CIVIL ENGINEERING LABORATORY

Monitoring Well: M-13

LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0610</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL LANDFILL 4/3/07 BORING FINISH 4/3/07 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **N5 MEDIUM GRAY LARGE GRAIN WELL** 18 NQ 154.4 164.4 10.0 75 155 **CEMENTED SANDSTONE** 160 HARD 5GY 4/1 DARK GREENISH GRAY SILTSTONE HARD N5 MEDIUM GRAY SILTSTONE 19 NQ 164.4 169.4 5.0 20 165 **N5 MEDIUM GRAY LARGE GRAIN WELL CEMENTED SANDSTONE N5 MEDIUM GRAY LARGE GRAIN WELL** 20 NQ 169.4 179.4 10.0 90 170 **CEMENTED SANDSTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 175

8

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING Monitoring Well: M-13

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0610</u> DATE <u>7/17/15</u> SHEET <u>8</u> OF _ PROJECT CARDINAL LANDFILL 4/3/07 BORING FINISH 4/3/07 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S DEPTH SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 21 NQ 179.4 189.4 10.0 90 **N5 MEDIUM GRAY LARGE GRAIN WELL** 180 **CEMENTED SANDSTONE** 185 HARD N5 MEDIUM GRAY SHALEY LIMESTONE 22 NQ 189.4 194.4 5.0 58 HARD N5 MEDIUM GRAY SHALEY 190 LIMESTONE SWL @ 49.8' 04/11/07; NQ HOLE FINISHED @ 194.4'; 18 HR READING; STOPPED BORING @ 194.4 04/10/07; CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 **INSTALLED 2"** MONITORING WELL

AFP CIVIL ENGINEERING LABORATORY

10		IBER			, _			LO	G O	F BORING Monitoring Well: M-12	ı			
		_	IERIC/	AN ELECTRIC	POW	ER			ВС	ORING NO. CA-0608 DATE 7/17/15 SHEET 1 OF 16	3			
				L LANDFILL						DRING START 12/13/06 BORING FINISH 12/13/06				
CC	ORDII	NATES	N 833	3,112.2 E 2,5	16,01	3.2			PII	EZOMETER TYPE WELL TYPE				
GR	OUND	ELEVA ⁻	TION	1187.7 SY	STEM				НС	ST. RISER ABOVE GROUND <u>3.009</u> DIA <u>1.5</u>				
Wa	ater Le	vel, ft	∇	Ţ		Ī			DE	PTH TO TOP OF WELL SCREEN 393.0BOTTOM 398.0				
TIT	ΛE								WI	ELL DEVELOPMENT BACKFILL				
DA	TE								FIE	ELD PARTY MCR / ZLR RIG D-120				
		SAN	ИPLE	STANDARD	>	RQD								
SAMPLE	NUMBER		PTH		AFRY WERY	INQD	DEPTH	GRAPHIC LOG	S	SOIL / ROCK				
SAM	SAM	IN F	FEET	PENETRATION RESISTANCE		%	IN	SRAF LO	s n	SOIL / ROCK ☐ DRILLER'S IDENTIFICATION NOTES				
0,		FROM		BLOWS / 6"	, 그띪		FEET	0						
		0.0	10.0				- - 5 -	-		Deconed rig & tools 08/29/06 using fire protection water from Cardinal U3. Grounding procedures not in use on this boring. Drilling water used from cardinal u3 fire protection. Blind drilled 4" roller bit from grade to 10.0'.				
1	NQ	10.0	14.5		3.1	16	10 -			HARD 10YR/5/4 MODERATE YELLOWISH BROWN CLAY SHALES w/fractured and soft areas from 10.0' to 12.0'				
2	NQ	14.5	24.5		4.05	69	- 15 - -			10YR 5/4 MODERATE YELLOWISH BROWN MEDIUM CLAY SHALE W/fractures and soft areas HARD N6 MEDIUM LIGHT GRAY LIMESTONE 10YR 5/4 MODERATE YELLOWISH BROWN				
FGD_LANDFILL.GPJ AEP.GDT 7/17/15							-			MEDIUM CLAY SHALE w/fractures				
L.GP.		TYP	E OF C	ASING USED)					Continued Next Page				
NDFI			OCK CO	RE			PIEZOM				_			
4		6" x 3.2 9" x 6.2					SLC	OTTE	ED S	SCREEN, G = GEONOR, P = PNEUMATIC				
57	+			VANCER	4" 3"	\dashv	WELL T	YPE:	_O	W = OPEN TUBE SLOTTED SCREEN, GM = GEOMON				
8		NW CA			6"		RECORDER							

RECORDER

AIR HAMMER

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-12

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH **DEPTH** PENETRATION SOIL / ROCK DRILLER'S WELL LOG SC IN FEET RESISTANCE **IDENTIFICATION** NOTES **FEET** FROM BLOWS / 6" TO NQ 10.0 | 100 SOFT 10Y 4/2 GRAYISH OLIVE CLAY SHALE 24.5 34.5 25 MEDIUM HARD 10YR 6/6 DARK YELLOWISH **ORANGE CLAY SHALE** HARD N7 LIGHT GRAY LIMESTONE **5G 6/1 GREENISH GRAY MEDIUM CLAY** HARD N7 LIGHT GRAY LIMESTONE MEDIUM HARD 5G 6/1 GREENISH GRAY **CLAY SHALE** NQ 34.5 44.5 10.0 100 35 MEDIUM HARD 5GY 3/2 GRAYISH OLIVE **GREEN and 5GY 6/1 GREENISH GRAY CLAY** w/fractures and iron staining @ 34.5 - 35.4, 35.7 -36.5, 36.7 - 40.0, 40.4, & 40.9 - 44.5 40 FGD LANDFILL.GPJ AEP.GDT 7/17/15 NQ 44.5 9.8 71 HARD 5B 5/1 MEDIUM BLUISH GRAY 54.5 **SHALEY LIMESTONE** w/fractures and iron staining throughout

8

LOG OF BORING

Monitoring Well: M-12

OB NUM	_				_		LO		Monitoring		
			AN ELECTRIC	POV	VER				RING NO. <u>CA-0608</u> DATE <u>7/17/15</u> S		
ROJECT	CAF	RDINA	L LANDFILL					ВО	RING START 12/13/06 BORING FINIS	:н <u>1</u>	2/13/06
SAMPLE NUMBER SAMPLE	DEI IN F	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
	FROM	ТО	BLOWS / 6"	~		- 50 -					
6 NQ	54.5	59.5		5.0	44	- 55 -	- 1		HARD N6 MEDIUM LIGHT GRAY LIMESTONE w/fractures and iron staining throughout		
7 NQ	59.5	69.5		4.5	100	60 -			MEDIUM HARD 5G 6/1 GREENISH GRAY CLAY SHALE W/fractures and iron staining throughout HARD N7 LIGHT GRAY LIMESTONE W/fractures and iron staining throughout		
						- 65 -					Lost all drill retur water @ ~61.5'
0 100	60.5	70.5			60		- 1		MEDILIM HADD FOV 6/4 ODEENIGH ODAY		
8 NQ	69.5	79.5		9.1	66	70 -			MEDIUM HARD 5GY 6/1 GREENISH GRAY CLAY SHALE w/fractures throughout		
		1	1	1		1			Continued Next Page		1

LOG OF BORING

JOB NUMBER

Monitoring Well: M-12



COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>4</u> OF _ BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** LOG SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION** NOTES **FEET** FROM BLOWS / 6" TO HARD N5 MEDIUM GRAY LIMESTONE MEDIUM HARD 5GY 6/1 GREENISH GRAY **CLAY SHALE** HARD N5 MEDIUM GRAY LIMESTONE Air hammer to 77.0' HARD N5 MEDIUM GRAY SHALEY LIMESTONE NQ 79.5 89.5 9.7 82 HARD N7 LIGHT GRAY LIMESTONE SOFT N5 MEDIUM GRAY FRACTURED CLAY HARD N7 LIGHT GRAY LIMESTONE 10 NQ 89.5 99.5 9.2 HARD N5 MEDIUM GRAY CLAY SHALE 43 Pumped 70 gals quick 90 w/fracture grout into bore hole & let set all weekend to try to seal fractures in limestone. SWL DRY 09/05/06; this is 96 hr reading SOFT N5 MEDIUM GRAY CLAY SHALE FGD LANDFILL.GPJ AEP.GDT 7/17/15 N1 BLACK COAL All coal placed in sepatate box. SOFT N5 MEDIUM GRAY CLAY SHALE w/fractures & iron staining throughout 8

LOG OF BORING

Monitoring Well: M-12

		NUMI			N EL ECTRIC	DOV	_ /FD			D.C	
					<u>IN ELECTRIC</u> _ LANDFILL	PUV	VER				ORING NO. <u>CA-0608</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>16</u> ORING START <u>12/13/06</u> BORING FINISH <u>12/13/06</u>
	-RO	JECT	CAI	VDINA	LANDI ILL					ЬС	NING START 12/13/00 BORING FINISH 12/13/00
	SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM	PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION □ DRILLER'S NOTES
	11	NQ	99.5	109.5		10.0	55	- 100 – -			HARD N5 MEDIUM GRAY CLAY SHALE w/fractures and iron staining throughout HARD N5 MEDIUM GRAY CLAY SHALE
								- 105			
	12	NQ	109.5	119.5		10.0	62	-			HARD N7 LIGHT GRAY LIMESTONE w/fractures HARD N7 LIGHT GRAY LIMESTONE
			100.0	710.0		10.0		110 -			w/fractures SOFT N5 MEDIUM GRAY CLAY SHALE
·								- - 115 –			HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/fractures & fine grain sandstone lenses throughout
7/15								-			
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	13	NQ	119.5	122.0		2.5	0	120 -			HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/fractures throughout
CD_FGD_LANDFILL.	14	NQ	122.0	129.5		6.5	42	-			HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/fractures throughout 9/6/06 - SWL = 123.6' (16 hr reading)

LOG OF BORING

JOB NUMBER

Monitoring Well: M-12



COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET _**6**__ OF __ BORING NO. CA-0608 PROJECT **CARDINAL LANDFILL** 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK DRILLER'S LOG SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO SOFT N5 MEDIUM GRAY CLAY SHALE w/ NQ rods @ 149.5' HARD N4 MEDIUM DARK GRAY CLAY SHALE 125 w/fractures HARD N7 LIGHT GRAY LIMESTONE w/fractures 15 NQ 129.5 139.5 10.0 72 HARD N7 LIGHT GRAY LIMESTONE 130 w/fractures & iron staining throughout 135 HARD 5BG 5/2 GRAYISH BLUE GREEN CLAY SHALE w/fractures SOFT 5BG 5/2 GRAYISH BLUE GREEN CLAY SHALE HARD 5BG 5/2 GRAYISH BLUE GREEN CLAY SHALE 139.0' - 153.6' HARD 5G 6/1 GREENISH GRAY FINE GRAIN Possible Connellsville 16 NQ 139.5 149.5 10.0 74 140 SILTY SANDSTONE w/crossbedding throughout 145 FGD LANDFILL.GPJ AEP.GDT 7/17/15 NQ 149.5 156.5 5.5 40 HARD 5G 6/1 GREENISH GRAY FINE GRAIN 8

LOG OF BORING

JOB NUMBER

Monitoring Well: M-12

				N ELECTRIC L LANDFILL	POW	/ER				RING NO. <u>CA-0608</u> DATE <u>7/17/15</u> SHEET <u>7</u> OF <u>16</u> RING START <u>12/13/06</u> BORING FINISH <u>12/13/06</u>
SAMPLE	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	문 의	nscs	SOIL / ROCK ☐ DRILLER'S IDENTIFICATION NOTES
										SILTY SANDSTONE w/crossbedding throughout
							155 -	8		COAL
18	NQ	156.5	159.5		2.45	22				HARD N5 MEDIUM GRAY SILTY CLAY SHALE w/limestone modules & fractures
19	NQ	159.5	169.5		10.0	90	160 -			HARD N7 LIGHT GRAY LIMESTONE w/fractures throughout
							165 -			
00	10	100.5	470.5		5.4					
20	NQ	169.5	179.5		5.4	52	170 -			HARD N7 LIGHT GRAY LIMESTONE
AEP.GDT 7/1										SOFT N5 MEDIUM GRAY CLAY SHALE
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15										HARD N7 LIGHT GRAY LIMESTONE w/fractures
FGD_LA							175 -			SOFT N5 MEDIUM GRAY CLAY SHALE
AEP CD			ı	I	I					Continued Next Page

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

JOB NUMBER

Monitoring Well: M-12

AEP

COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **8** OF BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 **BORING FINISH** 12/13/06 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION F0G SOIL / ROCK DRILLER'S WELL SC IN FEET RESISTANCE **IDENTIFICATION** NOTES **FEET** FROM BLOWS / 6" TO MINE VOID Lost water pressure @ 176.0'. Stopped rotation @ 176.5'. Using no rotation & water presssure, moved NQ rods from 176.5' to 186.0'. Mine void of abandoned mine from 176.5' to 186.0. SWL at this time - DRY 09/09/06 NQ Rods @ 180 179.5; SWL Dry @ 32 hr reading; Bottom of mine floor w/ air hammer 186.6' SWL @ 182.7 on 12/11/06; 80 hr reading with NQ hole to 289.8'. HW casing seated on bottom of 185 mine floor NQ 186.6 194.8 7.3 MEDIUM HARD N5 MEDIUM GRAY SILTY 10/6/06 Pulled air 21 56 **FINE SANDSTONE** hammer & rods. Set HW casing to 186.6'; resumed NQ rock coring SWL @ 187.6 on 12/12/06: 14 hr 190 reading with NQ hole to 312.8'. HW casing seated on bottom of mine floor 195 HARD N7 LIGHT GRAY LIMESTONE 22 NQ 194.8 204.8 9.3 73 MEDIUM HARD N5 MEDIUM GRAY SILTY **CLAY SHALE** HARD 5G 6/1 GREENISH GRAY SILTY CLAY FGD LANDFILL.GPJ AEP.GDT 7/17/15 SHALE w/limestone nodules throughout 200 8

LOG OF BORING

Monitoring Well: M-12



JOB NUMBER BORING NO. <u>CA-0608</u> DATE <u>7/17/15</u> SHEET <u>9</u> OF __ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 9.8 61 205 HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY 23 NQ 204.8 214.8 **CLAY SHALE** 210 SOFT 5GY 6/1 GREENISH GRAY CLAY SHALE 215 24 NQ 214.8 224.8 10.0 HARD 5GY 6/1 GREENISH GRAY FINE 53 **SANDY CLAY SHALE** 220 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 25 NQ 224.8 234.8 9.9 225 HARD 5GY 6/1 GREENISH GRAY FINE 41 **SANDY CLAY SHALE**

LOG OF BORING

JOB NUMBER

Monitoring Well: M-12

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BORING NO. <u>CA-0608</u> DATE <u>7/17/15</u> SHEET <u>10</u> OF <u>16</u> COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD N7 LIGHT GRAY LIMESTONE 230 HARD 5GY 6/1 GREENISH GRAY CLAY SHALE w/limestone nodules throughout 235 26 NQ 234.8 243.8 7.9 20 SOFT 5G 6/1 GREENISH GRAY CLAY SHALE 240 HARD 5B 5/1 MEDIUM BLUISH GRAY FINE **SANDY CLAY SHALE** HARD 5B 5/1 MEDIUM BLUISH GRAY FINE NQ 243.8 249.8 6.0 75 SANDY CLAY SHALE 245 w/limestone nodules throughout FGD LANDFILL.GPJ AEP.GDT 7/17/15 250 HARD 5B 5/1 MEDIUM BLUISH GRAY FINE 28 NQ 249.8 259.8 9.8 79 **GRAIN SANDY CLAY SHALE** 8

LOG OF BORING

Monitoring Well: M-12



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER _ DATE_<u>7/17/15</u>__ SHEET <u>11</u>_ OF _ BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** SAMPLE STANDARD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S FOG SCS WELL IN FEET RESISTANCE **IDENTIFICATION** NOTES **FEET** BLOWS / 6" FROM TO 255 29 NQ 259.8 269.8 260 MEDIUM HARD 5G 6/1 GREENISH GRAY **CLAY SHALE** HARD 5B 7/1 LIGHT BLUISH GRAY SHALEY LIMESTONE HARD 5G 6/1 GREENISH GRAY CLAY SHALE w/limestone nodules throughout HARD 5B 7/1 LIGHT BLUISH GRAY SHALEY LIMESTONE HARD 5G 6/1 GREENISH GRAY CLAY SHALE 265 w/limestone nodules throughout MEDIUM TO HARD 5G 6/1 GREENISH GRAY 270 30 NQ 269.8 275.8 55 **CLAY SHALE** 275 FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD 5G 6/1 GREENISH GRAY CLAY SHALE 31 NQ 275.8 284.8 9.0 60 w/limestone nodules throughout 8

LOG OF BORING LABORA

Monitoring Well: M-12

SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-	-				
32	NQ	284.4	289.4		5.0	28	285 -			HARD GRAY SHALE N6 MEDIUM LIGHT GRAY HARD GRAY SHALE W/limestone nodules HARD N6 MEDIUM LIGHT GRAY SHALE		
							-			HARD N6 MEDIUM LIGHT GRAY SHALE w/limestone nodules HARD N6 MEDIUM LIGHT GRAY SHALE		
33	NQ	289.8	-299.8		10.0	63	290 - -			5YR 4/1 BROWNISH GRAY SHALE SOFT GRAY SHALE wet HARD 5YR 4/1 BROWNISH GRAY SHALE		
							-			SOFT GRAY SHALE 5YR 4/1 BROWNISH GRAY SHALE w/brownish red shale		
							295 - - -			HARD GRAY / RED SHALE		
34	NQ	-299.8 -	309 .8		10.0	62	300 - - -			RED GRAY SHALE		
							305 -					
										Continued Next Page		

EP CIVIL ENGINEERING LABORATO

LOG OF BORING

Monitoring Well: M-12

JOB NUMBER ______ BORING NO. CA-0608 DATE 7/17/15 SHEET 13 OF 16

PROJECT CARDINAL LANDFILL BORING START 12/13/06 BORING FINISH 12/13/06

NUMBER	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
		FROM	10	BLOWS / 6			-			GRAY SILTY SHALE		
							-			GRAY HARD LIMESTONE		
35	NQ	309.8	312.8		3.0	93	310 –	× × × × × × × × × × × × × × × × × × ×		HARD 5B 7/1 LIGHT BLUISH GRAY FINE GRAIN SANDY SILTSTONE w/limestone lenses throughout		
36	NQ	312.8	319.8		7.0	93	-	× × × × × × × ×		HARD N4 MEDIUM DARK GRAY FINE GRAIN STILTY SANDSTONE		
							315 -					
							-					
37 -	NQ	319.8	329.8		10.0	100	320 -			HARD N4 MEDIUM DARK GRAY FINE GRAIN SILTY SANDSTONE		
							325 -					
							-					

AEP CIVIL ENGINEERING LABORATORY

JOB NUMBER

FGD LANDFILL.GPJ AEP.GDT 7/17/15

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Continued Next Page

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COMPANY AMERICAN ELECTRIC POWER __ DATE <u>7/17/15</u> SHEET <u>14</u> OF <u>16</u> BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO MEDIUM HARD N5 MEDIUM DARK GRAY SANDSTONE HARD N5 MEDIUM DARK GRAY FINE SILTY 335 SANDSTONE MEDIUM HARD N5 MEDIUM DARK GRAY **SANDSTONE** 39 NQ 339.8 349.8 10.0 100 340 HARD N7 LIGHT GRAY WELL CEMENTED MEDIUM to COARSE GRAIN SANDSTONE w/some crossbedding 345 350 40 NQ 349.8 359.8 10.0 100 HARD MEDIUM DARK GRAY WELL **CEMENTED MEDIUM to COARSE GRAIN SANDSTONE** w/some crossbedding 355

LOG OF BORING

Monitoring Well: M-12

JOB NUMBER BORING NO. <u>CA-0608</u> DATE <u>7/17/15</u> SHEET <u>15</u> OF <u>16</u> COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 360 41 NQ 359.8 369.8 HARD MEDIUM DARK GRAY WELL 10.0 100 **CEMENTED MEDIUM to COARSE GRAIN** SANDSTONE w/some crossbeddings 365 370 HARD MEDIUM DARK GRAY WELL 42 NQ 369.8 379.8 10.0 97 **CEMENTED MEDIUM to COARSE GRAIN SANDSTONE** w/some crossbeddings 375 HARD N4 MEDIUM DARK GRAY SHALE FGD LANDFILL.GPJ AEP.GDT 7/17/15 380 43 NQ 379.8 389.8 10.0 95 HARD N4 MEDIUM DARK GRAY SILTY SHALE HARD N4 MEDIUM DARK GRAY WELL **CEMENTED MEDIUM to COARSE GRAIN** SANDSTONE w/some crossbedding 8

LOG OF BORING

Monitoring Well: M-12

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE <u>7/17/15</u> SHEET <u>16</u> OF __ BORING NO. CA-0608 PROJECT CARDINAL LANDFILL 12/13/06 BORING FINISH 12/13/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S WELL SC IN IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 385 **COAL PARTING IN SANDSTONE** HARD N4 MEDIUM DARK GRAY WELL **CEMENTED MEDIUM to COARSE GRAIN** SANDSTONE w/some crossbedding 390 44 NQ 389.8 399.8 10.0 88 **N5 MEDIUM GRAY FINE GRAIN SANDSTONE** COAL LENSE **N5 MEDIUM GRAY FINE GRAIN SANDSTONE** w/ coal lenses 395 Bottom of HARD N4 MEDIUM DARK GRAY FINE GRAIN Morgantown **SANDY CLAY SHALE** Sandstone @ 398.4' 400 45 NQ 399.8 809.6 5.0 34 FINE GRAIN SILTY SANDSTONE w/limestone nodules FINE GRAIN CLAY SHALE FGD LANDFILL.GPJ AEP.GDT 7/17/15 Stopped boring @ 404.8' on 12/13/06. Flushed w/~700 gals water; installed 1" geomon type well w/ 5' screen. 8

LOG OF BORING

Monitoring Well: M-11	A	

JOB NUM	MBER _				_		LO	GO	FBORING		,	
COMPAN	Y AM	IERIC <i>A</i>	AN ELECTRIC	POV	VER			ВС	RING NO. <u>MW-5</u> DATE <u>7/</u>	20/15 SHI	EET _	1 OF 12
PROJEC [*]	T _ CAF	RDINA	L FLY ASH D	AM				ВС	RING START E	BORING FINISH	5/4	4/99
COORDII	NATES _	N 830	0,072.4 E 2,5	516,46	55.1			PII	ZOMETER TYPE GEO-MON	_ WELL TYPE	GI	M
GROUNE	ELEVAT	TON _	977.8 s	YSTEM				НС	T. RISER ABOVE GROUND 2.39	DIA	3	
Water Le		$\overline{\mathbb{V}}$	<u> </u>			, -			PTH TO TOP OF WELL SCREEN			
TIME	701, 10							WI	ELL DEVELOPMENT	BACKFILL	10	0 gallons of Qui
DATE									LD PARTY TJH-REB			ME-75
DATE												
SAMPLE NUMBER SAMPLE	DEI	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION		WELL	DRILLER'S NOTES
						5 -			NO SAMPLE - RUN 3"CASING TO 7	7.3'		Decon drill with potable water & alconox prior to setup.
1 NQ-	2 7.3	9.6		2.3	22				5GY 6/1 GREENISH GRAY SANDST	ONE		Started coring at 7.3' Note: No water return.
2 NQ-	9.6	13.3		3.0	0	10 -			5GY 6/1 GREENISH GRAY SANDY S Badly broken.	SHALE		
3 NQ-	2 13.3	14.6		1.1	0	-						
4 NO-	2 14.6	16.5		2.1	0	4-						
5 NQ-	2 16.5	19.6		2.3	0	15 -						
7/20/15									5GY 6/1 GREENISH GRAY CLAY SI	HALE		
6 NQ-	2 19.6	22.1		2.4	0				N6 MEDIUM LIGHT GRAY LIMESTO)NE /		
AEP. G			ASING USED)					Continued Next Pag	e		
X X	NQ-2 R0 6" x 3.25 9" x 6.25	OCK CO 5 HSA				PIEZOM SL(JS TIP, SS =	OPE	EN TUBE
4	HW CAS	SING AD	VANCER	4"		WELL T	YPE:	O'	W = OPEN TUBE SLOTTED S	CREEN, GM	= GI	EOMON
3	NW CAS			3" 6"					RECORDER REB	•		
Ė	AIR HAI			8"					RECURDER RED			

LOG OF BORING

Monitoring Well: M-11

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>2</u> OF _ PROJECT CARDINAL FLY ASH DAM BORING FINISH 5/4/99 **BORING START** SAMPLE STANDARD RQD PENETRATION PENETRATION RESISTANCE SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK DRILLER'S FOG WELL SC IN FEET **IDENTIFICATION** NOTES **FEET** FROM TO BLOWS / 6" With iron stain N5 MEDIUM GRAY CLAY SHALE N6 MEDIUM LIGHT GRAY LIMESTONE Broken up; iron stain **N5 MEDIUM GRAY CLAY SHALE** 7 NQ-2 22.1 24.6 0 Broken up 8 NQ-2 24.6 29.2 4.0 30 **N6 MEDIUM LIGHT GRAY CLAY SHALE** 25 9 NQ-2 29.2 34.6 5.4 33 30 **5GY 6/1 GREENISH GRAY SANDY SHALE N6 MEDIUM LIGHT GRAY CLAY SHALE** 10 NQ-2 34.6 5GY 6/1 GREENISH GRAY SANDSTONE 34.7 0.1 35 11 NQ-2 34.7 39.6 5.0 32 **N5 MEDIUM GRAY CLAY SHALE** 12 NQ-2 39.6 43.8 3.0 20 **RED, BROWN & GRAY CLAY SHALE N6 MEDIUM LIGHT GRAY LIMESTONE** Oxidized above & below DAM.GPJ AEP.GDT 7/20/15 **N5 MEDIUM GRAY CLAY SHALE** N5 MEDIUM GRAY CLAY SHALE 13 NQ-2 43.8 49.1 3.8 0 45 **RED & GREENISH GRAY CLAY SHALE** Ā 8

LOG OF BORING

Monitoring Well: M-11

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>3</u> OF _ PROJECT CARDINAL FLY ASH DAM BORING FINISH 5/4/99 **BORING START** STANDARD
PENETRATION ZEDA RQD SAMPLE SAMPLE NUMBER SAMPLE DEPTH DEPTH LOG SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO **N6 MEDIUM LIGHT GRAY LIMESTONE** 14 NQ-2 49.1 54.6 2.5 0 **N6 MEDIUM LIGHT GRAY CLAY SHALE** 50 **N6 MEDIUM LIGHT GRAY LIMESTONE N6 MEDIUM LIGHT GRAY CLAY SHALE** Iron stain at 56.8' 15 NQ-2 54.6 59.6 4.6 30 55 **N6 MEDIUM LIGHT GRAY LIMESTONE** Iron stain; broken up 16 NQ-2 59.6 64.6 3.3 0 60 **N5 MEDIUM GRAY CLAY SHALE** 10R 4/2 GRAYISH RED CLAY SHALE **N6 MEDIUM LIGHT GRAY CLAY SHALE** 17 NQ-2 64.6 69.3 4.7 60 65 Soft from 64.6'-67.2' AEP.GDT 7/20/15 **N5 MEDIUM GRAY CLAY SHALE** 18 NQ-2 69.3 74.6 4.6 26 70 FA DAM.GPJ Note: At approx. 70.0', the rock became more competent. 8

LOG OF BORING

Monitoring Well: M-11

JO	ΒN	NUMI	BER _				_		LO	00	FBORING	WOITE	oring wei	. 101-11
CC	DMI	PANY	AM	ERICA	N ELECTRIC	POV	VER			ВС	RING NO. MW-5	DATE 7/20/15	_ SHEET _	4 OF 12
PF	ROJ	ECT	CAF	RDINA	L FLY ASH DA	AM.				ВС	RING START	BORING F	INISH _ 5/ 4	l/99
SAMPLE	NUMBER	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS		./ROCK IFICATION	WELL	DRILLER'S NOTES
_1	9 1	NQ-2	74.6	84.6		10.0	30	- 75 - - -			5GY 6/1 GREENISH G Iron stain; fractures N5 MEDIUM GRAY CL	AY SHALE		
								80 -			N5 MEDIUM GRAY CL			
_22	0 1	NQ-2	84.6	94.6		10.0	53	85			N5 MEDIUM GRAY LIF N5 MEDIUM GRAY CL LIMESTONE LENSES	AY SHALE with		
CD_FA_DAM.GPJ_AEP.GI	1 1	NQ-2	94.6	104.6		9.9	84	90			N5 MEDIUM GRAY CL			
AEP											Continue	d Next Page		

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Monitoring Well: M-11

С	ОМ		AM		N ELECTRIC		VER				PRING NO. <u>MW-5</u> DATE <u>7/20/15 S</u>		
Р	RO	JECT	CAF	RDINA	L FLY ASH DA	AM				ВО	PRING START BORING FINIS	H <u>5</u>	/4/99
ZAMDI E	NUMBER	SAMPLE	DEF	EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
								100 -					
	22	NQ-2	104.6	114.6		9.6	85	105 -			N5 MEDIUM GRAY SANDY CLAY SHALE	_	
								110 -			10YR 5/4 MODERATE YELLOWISH BROWN SANDSTONE		
	23	NQ-2	114.6	124.2		10.0	96	115 -					
AEP CD_FA_DAM.GPJ AEP.GDT 7/20/15								120 -	-				
AEP CI											Continued Next Page		

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-11

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>6</u> OF _ BORING START _____ BORING FINISH _______ PROJECT CARDINAL FLY ASH DAM STANDARD
PENETRATION FENETRATION FOR STANCE RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK WELL DRILLER'S SCS IN FEET **IDENTIFICATION NOTES** FEET FROM TO 24 NQ-2 124.2 129.6 5.4 100 125 -**N5 MEDIUM GRAY SANDSTONE** 10YR 5/4 MODERATE YELLOWISH BROWN **SANDSTONE N5 MEDIUM GRAY SANDSTONE** 25 NQ-2 129.6 134.6 5.0 100 130 -10.0 100 26 NQ-2 134.6 144.6 135 140 0.1' limestone at 141.0' 27 NQ-2 144.6 154.6 9.6 76 145 -10YR 5/4 MODERATE YELLOWISH BROWN FA DAM.GPJ AEP.GDT 7/20/15 **SANDSTONE** 8

Monitoring Well: M-11

LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL FLY ASH DAM BORING FINISH 5/4/99 **BORING START** STANDARD
PENETRATION ZEDA SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE **DEPTH** SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **N5 MEDIUM GRAY SANDSTONE** 10YR 5/4 MODERATE YELLOWISH BROWN **SANDSTONE** 28 NQ-2 154.6 164.6 10.0 80 155 Lost drill water at 155'; geared rig down from 5th to 3rd gear. **N5 MEDIUM GRAY SANDSTONE** 10YR 5/4 MODERATE YELLOWISH BROWN SANDSTONE **N5 MEDIUM GRAY SANDSTONE** 10YR 5/4 MODERATE YELLOWISH BROWN **SANDSTONE** 160 29 NQ-2 164.6 174.6 10.0 68 165 **N5 MEDIUM GRAY SANDSTONE with COAL** STREAKS 10YR 5/4 MODERATE YELLOWISH BROWN **SANDSTONE N5 MEDIUM GRAY SANDSTONE with COAL STREAKS** 170 10YR 5/4 MODERATE YELLOWISH BROWN Mud seam at 169.8' **SANDSTONE** Mud seam at 169.8' DAM.GPJ AEP.GDT 7/20/15 **N5 MEDIUM GRAY SANDSTONE with COAL STREAKS** 30 NQ-2 174.6 184.6 10.0 64 175 Ā 8

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING Monitoring Well: M-11

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>8</u> OF _ BORING START _____ BORING FINISH _______ PROJECT CARDINAL FLY ASH DAM STANDARD
PENETRATION ZEDO
RESISTANCE ON THE PENETRATION AND THE PE RQD RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES** FEET FROM TO **N5 MEDIUM GRAY SANDSTONE CONGLOMERATE N5 MEDIUM GRAY SANDSTONE with COAL STREAKS** 180 -(Morgantown) Vertical crack at 189.3' 31 NQ-2 184.6 194.6 10.0 90 185 190 -32 NQ-2 194.6 204.6 10.0 89 195 CD FA DAM.GPJ AEP.GDT 7/20/15 200 **N5 MEDIUM GRAY CLAY SHALE** Mud seam at 200.2'

Monitoring Well: M-11 LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>MW-5</u> DATE <u>7/20/15</u> SHEET <u>9</u> OF _ BORING START BORING FINISH 5/4/99 PROJECT CARDINAL FLY ASH DAM RQD RQD SAMPLE STANDARD GRAPHIC LOG PENETRATION PENETRATION RESISTANCE SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES** FEET FROM TO BLOWS / 6" N4 MEDIUM DARK GRAY SANDY SHALE 10.0 96 33 NQ-2 204.6 214.6 205 Rock fracture at 210 -209.7' 34 NQ-2 214.6 224.6 10.0 80 215 -**N6 MEDIUM LIGHT GRAY SHALEY** SANDSTONE 220 N6 MEDIUM LIGHT GRAY LIMESTONE Vertical crack at 220.1'-220.7' (fossils) FA DAM.GPJ AEP.GDT 7/20/15 **N6 MEDIUM LIGHT GRAY SHALEY** 10.0 83 LIMESTONE 35 NQ-2 224.6 234.6 225 **N5 MEDIUM GRAY CLAY SHALE N5 MEDIUM GRAY SANDY SHALE**

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Monitoring Well: M-11

LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. MW-5 DATE 7/20/15 SHEET 10 OF 12 PROJECT CARDINAL FLY ASH DAM BORING START BORING FINISH 5/4/99 STANDARD
PENETRATION ZEDA RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES** FEET FROM BLOWS / 6" TO 230 Mud seam at 230.0' **N6 MEDIUM LIGHT GRAY LIMESTONE** Shale streaks. **N5 MEDIUM GRAY SHALEY SANDSTONE** 36 NQ-2 234.6 235.2 .6 100 235 With calcite. 37 NQ-2 235.2 237.6 2.4 0 Mud seam at 235.2' **N5 MEDIUM GRAY CLAY SHALE** Broken up **N3 DARK GRAY CLAY SHALE** Broken up. 38 NQ-2 237.6 244.6 6.3 33 **N5 MEDIUM GRAY SANDSTONE** 240 Mud seam at 239.9' **N5 MEDIUM GRAY CLAY SHALE** Broken up 39 NQ-2 244.6 249.2 0 245 40 NQ-2 249.2 254.6 2.3 0 Note: Run 3" casing to 83.6' 250

FA DAM.GPJ AEP.GDT 7/20/15

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LOG OF BORING

Monitoring Well: M-11

	PAN	′ <u>AM</u>		N ELECTRIC		/ER		LO	ВС	ORING NO. <u>MW-5</u> DATE <u>7/20/15</u> S	SHEET	11 OF 12
PROJ	ECT	CAF	RDINA	L FLY ASH DA	ΔM				BC	PRING START BORING FINIS	SH <u>5/</u>	4/99
SAMPLE	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
41 N	IQ-2	254.6	264.6		9.8	51	255 -			N3 DARK GRAY CLAY SHALE		
							-	4		N1 BLACK COAL		
							-			N4 MEDIUM DARK GRAY CLAY SHALE Limestone nodules.		
							260 -					
							-					
							-					
		264.6 264.8			9.8	<u>0</u> 70	265 -					
							-					
							-					
							270 -			10Y 6/2 PALE OLIVE LIMESTONE		
							-			N5 MEDIUM GRAY SHALEY LIMESTONE		
	IQ-2	274.6	284.6		10.0	38	275 -			N5 MEDIUM GRAY CLAY SHALE Limestone nodules		
										Continued Next Page		

LOG OF BORING

Monitoring Well: M-11

JOB NUM	BER						LO	00	F BURING	World	3	
			N ELECTRIC	POW	- VER			ВС	RING NO. MW-5	DATE 7/20/15	SHEET	12 OF 12
			L FLY ASH DA							BORING F		
SAMPLE NUMBER SAMPLE	SAM DEF IN FI FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS		IL / ROCK TIFICATION	WELL	DRILLER'S NOTES

Job	NUME	BER _						LO	G O	F BORING	3			Monitor	ing v	veii:	M-1004
CON	(PAN	AN	IERIC <i>A</i>	N ELECTRIC	POW	/ER			ВС	RING NO. I	W-1004	ID.	DATE	7/17/15	_ SH	IEET	1 OF 9
PRC	JECT	CAF	RDINA	L LANDFILL					ВС	RING STAR	т _	3/23/1	0	BORING F	INISH	_3/	31/10
COC	RDIN	ATES _	N 831	1,215.4 E 2,5	19,11	2.4			PIE	ZOMETER	TYPE	N/A		WELL	TYPE	0	W
GRO	DUND	ELEVAT	TON	1005.6 SY	STEM				HG	T. RISER A	BOVE	GROUN	D 2.6	5	_ DIA	<u>2'</u>	•
Wat	er Lev	el, ft	$\overline{\nabla}$	Y		T			DE	РТН ТО ТО	P OF V	VELL SO	CREEN	148.4 BO	TTOM	19	98.4
TIM									WI	ELL DEVELO	PMEN	ΙΤ <u>ΥΙ</u>	ES	BAC	KFILL	_ V	OLCLAY
DAT	E								FIE	LD PARTY	MC	R/ZLF	₹		RIG	<u>D</u>	-120
SAMPLE	SAMPLE	DE IN F	IPLE PTH EEET	STANDARD PENETRATION RESISTANCE	TOTAL LENGTH ECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS		11		ROCK ICATION			WELL	DRILLER'S NOTES
1	SPT	3.0	5.0	7-6-5-9			- - -			SPLIT SP FROM CE SPT'S TAI SPOIL PL	RE PUN S IN US OON / O FIRE KEN FF ACED	MP / NO SE / DRI DRILL & PROTE ROM 0' - FOR DF	GROUNI LL 4" CA DECON CTION S' - 3.0' DUE RILL PAD	DING SING THEN WATER YSTEM / NC E TO MINE)		
2	SPT		6.2 7.1	5-23-50/.2 50/1			5 - -			VERY HAI moist				CLAYSHALI	E		
1	NQ	8.1	14.4		5.9	52	-			REFUSAL ON 03/24/ CASING 1 HARD ME	. @ 8.1 /10 / S\ [O 8.1' EDIUM AIN SAI	' / STAF WL DRY BLUISH NDSTO	RTED COI ON 03/2 GRAY 5I	RING @ 8.1 ¹ 4/10 / HW B 5/1 SILTY			
2	NQ	14.4	24.4		10	70	10 - - - -			w/high and				B 5/1 SILTY			

HARD MEDIUM GRAY N5 LIMEY SILTSTONE

HARD LIMESTONE

AEP.GDT 7/17/15									
LL.GPJ		TYPE	OF C	ASING USE	כ			Continued Next Page	
J_LANDFI	X	NQ-2 R0 6" x 3.25 9" x 6.25	5 HSA	RE				E: PT = OPEN TUBE POROUS TIP, SS = OPEN CCREEN, G = GEONOR, P = PNEUMATIC	I TUBE
FG		HW CAS		VANCER	4" 3"	WELL TYPE:	OW	N = OPEN TUBE SLOTTED SCREEN, GM = GEO	OMON
EP C		SW CAS	SING		6" 8"			RECORDER	
⋖ .।	l .		v11v1∟1 \						

JOB	NUMI	BER _				_		LOC	5 O	F BORING Monitoring Well: M-1004
COM	IPAN'	AM	ERICA	N ELECTRIC	POW	/ER			во	RING NO. M-1004D DATE 7/17/15 SHEET 2 OF 9
PRO	JECT	CAF	RDINA	L LANDFILL					во	RING START 3/23/10 BORING FINISH 3/31/10
SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	E S	USCS	SOIL / ROCK IDENTIFICATION □ DRILLER'S NOTES
		11.0111		BEOWER						SOFT TO MEDIUM CLAYSHALE
							-			
							-			HARD GREENISH GRAY 5GY 4/1 CLAYSHALE
3	NQ	24.4	34.4		10	52	-			MEDIUM LIGHT GRAY N6 SILTY FINE GRAIN
							25 -			SANDSTONE
							-			HARD MEDIUM GRAY N5 CLAYSHALE HARD MEDUIM LIGHT GRAY N6 LIMESTONE
							-			HARD GREENISH GRAY 5GY 6/1
							-			CLAYSHALE w/limestone nodules throughout; w/high angle fracture @ 29.5'
							30 -			
							-			SOFT LIGHT GRAY N7 CLAYSHALE
							-			HARD LIGHT GRAY N7 LIMESTONE
4	NQ	34.4	41.8		3.8	24	35 -			HARD GREENISH GRAY 5GY 6/1 CLAYSHALE badly broken
							- - -			
61//1//							40 -			
AET CUTTGO LANDPILL.GF3 AET.GD1 (717.13)	NQ	41.8	49.4		7.5	35	-			HARD MEDIUM LIGHT GRAY N6 CLAYSHALE 45.0' - 49.4' badly broken machine break
P. P							45 -			
ቻ 3							_			Continued Next Page

JOB	NUMI	BFR						LO	GΟ	F BORING	Monitoring Wel	I: M-1004
			ERIC/	N ELECTRIC	POW	/ER			во	RING NO. M-1004D DATE	7/17/15 SHEET	г <u>3</u> оғ <u>9</u>
PRO	JECT	CAF	RDINA	L LANDFILL						RING START 3/23/10		
SAMPLE	SAMPLE	SAM DEF IN F	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
6	NQ	49.4	59.4	BLOWOTO	8.7	49	50 - -			HARD MEDIUM BLUISH GRAY S CLAYSHALE	5B 5/1	
							55 - - -			HARD MEDIUM LIGHT GRAY Not willimestone nodules throughout; s 5.6'		
7	NQ	59.4	67.4		5.5	9	60 -	* * X		MEDIUM HARD TO SOFT DARK GRAY 5GY 4/1 CLAYSHALE w/limestone nodule @ 4.8' to 5.5'		
AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	67.4	74.4		7.0	24	65 - - - - 70			HARD GREENISH GRAY 5G 6/1	CLAYSHALE	
EP CD_FGI										Continued Next P	Page	

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

1 1			
Monitoring Well: M-1004	$ar{\Lambda}$	14	
Monitoring Well: M-1004			

					-			, ,				1	-	
NUMBER	SAMPLE	SAM DEF IN F	IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	OTAL NGTH OVERY	RQD 0/	DEPTH IN	GRAPHIC LOG	SCS		SOIL / ROCK		WELL	DRILLER'S
Ŋ	SA	FROM	TO	BLOWS / 6"	드빌잂	70	FEET	GR	n		IDENTIFICATION	N	>	NOTES
							-							
							-							
							-	Ħ		HARD MEDILI	M LIGHT GRAY N	6 LIMESTONE		
9	NQ	74.4	84.4		9.9	59					IISH GRAY 5G 6/1	$\overline{}$		
							75 –			w/limestone no				
							-							
							-							
							-							
							-	E						
							80 –							
							-							
											OFT MODERATE	OLIVE		
							-				4 CLAYSHALE M LIGHT GRAY N	E LIMESTONE		
											OFT MODERATE			
							-				4 CLAYSHALE			
							-			HARD MEDIU	M LIGHT GRAY N	6 LIMESTONE		
10	NQ	84.4	93.4		6.1	33				HARD DARK	GREENISH GRAY	5G 4/1		
							85 -			CLAYSHALE				
							-							
							-							
							-							
							-							
							90 –							
								H		HARD MEDIU	M DARK GRAY N	LIMESTONE		
							-							
11	NO	00.4	00.4		_	4.4				HADD MED	M DI LIIOLI ODAY	ED E/4		
11	NQ	93.4	99.4		5.6	41	-			CLAYSHALE	M BLUISH GRAY	5/1 do		
											odules throughout			
							95 –	Ħ						
								耳						

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LOG OF BORING Monitoring Well: M-1004 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **5** OF BORING NO. M-1004D PROJECT **CARDINAL LANDFILL** 3/23/10 BORING FINISH 3/31/10 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVE F0G SOIL / ROCK DRILLER'S WELL SC IN IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO NQ 99.4 109.4 9.9 HARD MEDIUM BLUISH GRAY 5B 5/1 12 98 100 **CLAYEY SILTSTONE** w/limestone nodules 99.4' - 99.6' 105 NQ 109.4 119.4 9.4 HARD MEDIUM BLUISH GRAY 5B 5/1 13 66 110 **CLAYEY SILTSTONE** 115 HARD MEDIUM GRAY N5 LIMESTONE HARD MEDIUM BLUISH GRAY 5B 5/1 **CLAYEY SILTSTONE** HARD MEDIUM GRAY N5 LIMESTONE **SOFT TO MEDIUM GREENISH GRAY 5G 6/1 CLAYSHALE** HARD MEDIUM BLUISH GRAY 5B 5/1 **CLAYEY SILTSTONE** w/limestone nodules 14 NQ 119.4 129.4 10 62 HARD MEDIUM BLUISH GRAY 5B 5/1 120 **CLAYSHALE** w/limestone nodules throughout @ 119.4' -124.6'; Hard Very Dark Red 5R 2/6 Clayshale mixed w/Hard Medium Bluish Gray 5B 5/1 Clayshale from 125.6' - 126.6'

EP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: M-1004

JOB NUMBER ______ BORING NO. M-1004D DATE 7/17/15 SHEET 6 OF 9

PROJECT CARDINAL LANDFILL BORING START 3/23/10 BORING FINISH 3/31/10

ין אַר	щ	1	1PLE	STANDARD PENETRATION RESISTANCE BLOWS / 6"	'ΞĶ	RQD	DEPTH	೦	S				
NUMBER	SAMPLE		PTH EET	PENETRATION RESISTANCE	NGT OVE	0/	IN	GRAPHIC LOG	SCS	SOIL / ROCK		WELL	DRILLER'S
$\frac{2}{2}$	SA	FROM	TO	BLOWS / 6"	REA	%	FEET	GR I	Π	IDENTIFICATION		>	NOTES
								= =					
							125 -						
							-						
							=						
								= =					
							-						
15	NQ	129.4	139.4		9.5	64	130 -			MEDIUM HARD VERY DARK RED W/MEDIUM GRAY N4 CLAYSHAI			
										HARD MEDIUM GRAY N4 CLAYS	HALE		
							-						
							=			VERY HARD MEDIUM BLUISH GI	RAY 5B 5/1		
							-			CLAYSHALE			
							-						
							135 -						
							-						
							-						
16	NQ	139.4	149.4		10	57	-			HARD MEDIUM BLUISH GRAY 58	R 5/1		
10	ING	139.4	149.4		10	31	140 -			CLAYSHALE	5 3/1		
							-						
							-						
										HARD DARK GRAY N3 CLAYSHA	AI F		
							-			w/coal seams @ 144.2' - 144.3', 14			
							145 -						
							-						
							-			HARD MEDIUM BLUISH GRAY 58 FINE GRAIN SANDSTONE	3 5/1 SILTY		
							-			HARD MEDIUM BLUISH GRAY 58 CEMENTED FINE GRAIN SANDS			
17	NQ	149.4	159.4		10	100				HARD MEDIUM BLUISH GRAY 58			

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: M-1004

NUMBER	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	nscs	SOIL / ROCK IDENTIFICATION CEMENTED FINE GRAIN SANDSTONE	WELL	DRILLER'S NOTES
							- - - 155 —					
118	NQ	159.4	169.4		9.9	92	160 - - -			HARD MEDIUM BLUISH GRAY 5B 5/1 WELL CEMENTED MEDIUM TO FINE GRAIN SANDSTONE		
							165 - - -					
19	NQ	169.4	179.4		10	98	- 170 - - -					
							175 -					

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Monitoring Well: M-1004

JOB NUMBER BORING NO. <u>M-1004D</u> DATE <u>7/17/15</u> SHEET <u>8</u> OF _ COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL LANDFILL 3/23/10 BORING FINISH 3/31/10 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S DEPTH SOIL / ROCK DRILLER'S LOG SC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO HARD MEDIUM LIGHT GRAY N6 FINE SANDY **CLAYSHALE** HARD MEDIUM BLUISH GRAY 5B 5/1 WELL **CEMENTED FINE GRAIN SANDSTONE** NQ 179.4 189.4 HARD MEDIUM LIGHT GRAY N6 WELL 9.9 95 20 180 **CEMENTED MEDIUM TO FINE GRAIN** SANDSTONE HARD LIGHT GRAY N7 WELL CEMENTED FINE GRAIN SANDSTONE HARD MEDIUM LIGHT GRAY N6 WELL **CEMENTED MEDIUM GRAIN SANDSTONE** w/Hard Black N1 Clayshale streaks 185 21 NQ 189.4 199.4 9.9 99 HARD MEDIUM LIGHT GRAY N6 WELL 190 **CEMENTED MEDIUM GRAIN SANDSTONE** w/gravel in bed @ 189.4' - 189.7' 195 HARD MEDIUM LIGHT GRAY N6 SANDY LIMESTONE AEP.GDT 7/17/15 HARD MEDIUM LIGHT GRAY N6 FINE SANDY CLAYSHALE HARD MEDIUM LIGHT GRAY N6 SILTY FINE **GRAIN SANDSTONE** FGD LANDFILL.GPJ 22 NQ 199.4 209.4 HARD MEDIUM LIGHT GRAY N6 FINE GRAIN 10 99 200 **SANDSTONE** HARD MEDIUM LIGHT GRAY N6 WELL **CEMENTED FINE GRAIN SANDY SILTSTONE** 8

LOG OF BORING

Monitoring Well: M-1004

JOB NUMBER __ DATE <u>7/17/15</u> SHEET <u>9</u> OF __ COMPANY AMERICAN ELECTRIC POWER BORING NO. M-1004D PROJECT CARDINAL LANDFILL 3/23/10 BORING FINISH 3/31/10 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S TOTAL LENGTH RECOVER **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S SCS IN IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 205 HARD MEDIUM LIGHT GRAY N6 WELL NQ 209.4 23 214.4 5 100 210 CEMENTED FINE GRAIN SANDY SILTSTONE HARD MEDIUM LIGHT GRAY N6 WELL CEMENTED FINE GRAIN SANDSTONE HARD MEDIUM LIGHT GRAY N6 SILTSTONE w/slickenslide @ 212.1' STOPPED BORING @ 214.4' ON 03/30/10 / SWL 13.1' ON 03/31/10 - 18 HR READING / NQ HOLE TO 214.4' / BUILT 2" PVC MONITORING WELL

CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

AEP

LOG OF BORING

Monitoring Well: M-1003

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03	11	

	NUM		IERIC <i>A</i>	N ELECTRIC	POW	- /ER			BC	DRING NO. <u>M-1003</u> DATE 7/17/15 SH	HEET	1 OF 7
				L LANDFILL						DRING START 4/7/10 BORING FINISH		
COC	RDIN	IATES _	N 829	9,139.1 E 2,5	16,07					EZOMETER TYPE N/A WELL TYPE	<u>C</u>)W
GRO	UND	ELEVAT	ON _	933.6 SY	STEM				HG	GT. RISER ABOVE GROUND 2.33 DIA	"	
Wate	er Lev	el, ft	$\overline{\nabla}$	Ţ		Ā			DE	PTH TO TOP OF WELL SCREEN	1 <u>1</u>	39.3
TIME	Ξ									ELL DEVELOPMENT <u>YES</u> BACKFILI		
DAT	E								FIE	ELD PARTY ZLR/DLF RIC	<u> </u>)-120
		SAN	1PLE	STANDARD		POD						
SAMPLE	SAMPLE		PTH			RQD	DEPTH	GRAPHIC LOG	S	SOIL / ROCK	Ⅎ	DRILLER'S
SAME	AME	IN F	EET	PENETRATION RESISTANCE		%	IN	RA O	O S O	IDENTIFICATION	WELL	NOTES
0, 2	0,	FROM	ТО	BLOWS / 6"	- JĀ		FEET	Ю				
										BLIND DRILLED TO 3.7'		NO SPT'S TAKEN DUE TO STARTING
												DRILLING ON BEDROCK /
								-				ELEVATION
												LOWERED FOR DRILL PAD /
												DECONED 04/07/10 / LIQUI-NOX HIGH
1	NQ	3.7	9.0		2.0	0				HARD LIGHT GRAY N7 LIMESTONE		PRESSURE WASH /
							5 -			HARD MODERATE YELLOWISH BROWN		NO GROUNDING PROCEDURE IN
										10YR 5/4 CLAYSHALE		USE / 4" CASING
												BLIND DRILLED TO 3.7'
								-==				
2	NQ	9.0	14.4		4.7	88				SOFT DARK REDDISH BROWN 10R 3/4		
							10 -			CLAYSHALE		
							10					
								Ħ				
3	NQ	14.4	24.4		2.7	0				HARD LIGHT BLUISH GRAY 5B 7/1		
							15 -			CLAYSHALE badly broken		
										bedry broken		
17/15												
DT 7/												
AEP.G												
FGD_LANDFILL.GPJ_AEP.GDT_7/17/15	TYPE OF CASING USED									Continued Next Page		
N X		NQ-2 R		RE			PIEZOM	ETER	TYP	E: PT = OPEN TUBE POROUS TIP, SS :	= OF	PEN TUBE
<u> </u>		6" x 3.25 9" x 6.25					SLO	OTTE	D S	SCREEN, G = GEONOR, P = PNEUMÂTIC		
								YPE:	0	W = OPEN TUBE SLOTTED SCREEN, GN	1 = C	SEOMON
AEP CD		SW CAS	SING		6"					RECORDER		
₹∟		AIR HAI	иMER		8"							

JOB NUMBER

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

AEP CIVIL ENGINEERING LABORATORY Monitoring Well: M-1003 LOG OF BORING

Continued Next Page

COMPANY AMERICAN ELECTRIC POWER __ DATE <u>7/17/15</u> SHEET <u>2</u> OF _ BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH GRAPHIC **DEPTH** PENETRATION LOG SOIL / ROCK WELL DRILLER'S SCS IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 4 NQ 24.4 29.4 3.9 46 HARD TO SOFT LIGHT BLUISH GRAY 5B 7/1 25 **CLAYSHALE** HARD LIGHT GRAY N7 LIMESTONE NQ 29.4 34.4 1.6 38 30 w/iron staining and badly broken SOFT GREENISH GRAY 5G 6/1 CLAYSHALE 6 NQ 34.4 39.4 4.6 33 HARD GRAYISH RED 10R 4/2 CLAYSHALE 35 SOFT MODERATE REDDISH BROWN 10R 4/6 7 NQ 39.4 44.4 3.1 32 40 **CLAYSHALE** HARD DARK GREENISH GRAY 5G 4/1 NQ 44.4 49.4 5.0 48 45 **CLAYSHALE** w/limestone nodules

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Monitoring Well: M-1003 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>3</u> OF _ BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER DEPTH GRAPHIC SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVER LOG SOIL / ROCK WELL DRILLER'S SCS IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO NQ 49.4 54.4 4.9 78 HARD DARK GREENISH 5G 4/1 CLAYSHALE 50 w/limestone nodules @ 52.0' to 54.4'; w/iron staining & calcite HARD DARK GREENISH GRAY 5G 4/1 10 NQ 54.4 64.4 9.9 42 55 **CLAYSHALE** w/iron staining throughout HARD MEDIUM BLUISH GRAY 5B 5/1 WELL **CEMENTED FINE TO MEDIUM GRAIN** SANDSTONE 60 w/high angle fracture @ 58.6' and iron staining throughout NQ 64.4 74.4 10 100 HARD MEDIUM BLUISH GRAY 5B 5/1 WELL 11 65 **CEMENTED MEDIUM GRAIN SANDSTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 70

Continued Next Page

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AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

Monitoring Well: M-1003

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE <u>7/17/15</u> SHEET <u>4</u> OF _ BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S **DEPTH** SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD MEDIUM BLUISH GRAY 5B 5/1 WELL 12 NQ 74.4 84.4 10 100 75 **CEMENTED MEDIUM TO FINE GRAIN** SANDSTONE w/black shale streak @ 99.4' and 100.2' 80 13 NQ 84.4 94.4 10 100 85 90 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 104.4 14 NQ 94.4 10 100 95

AEP CIVIL ENGINEERING LABORATORY



LOG OF BORING Monitoring Well: M-1003 JOB NUMBER __ DATE <u>7/17/15</u> SHEET <u>5</u> OF __ COMPANY AMERICAN ELECTRIC POWER BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE GRAPHIC LOG DEPTH S DEPTH SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 100 15 NQ 104.4 114.4 HARD LIGHT GRAY N7 WELL CEMENTED 9.8 94 105 **SANDSTONE** w/black shale streaks from 104.7' to 107.0', 107.3', 107.4', 109.1', & 111.0' 110 16 NQ 114.4 124.4 10 83 HARD LIGHT GRAY N7 WELL CEMENTED 115 FINE TO MEDIUM GRAIN SANDSTONE w/black N1 shale streaks @ 115.0', 115.2' -115.8', 116.5', 117.2' - 117.6'; black N1 coal lens @ 116.8' - 116.9' & 121.6'; high angle fracture @ 119.6' 120

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

JOB NUMBER

Monitoring Well: M-1003

COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET **_6**__ OF _ BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER GRAPHIC LOG DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK DRILLER'S SCS WELL IN FEET RESISTANCE % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO HARD LIGHT GRAY N7 WELL CEMENTED 17 NQ 10 124.4 134.4 98 125 FINE GRAIN SANDSTONE w/gravel nodules 124.9' - 125.3'; w/black N1 shale streaks throughout 130 HARD MEDIUM BLUISH GRAY 5B 5/1 WELL 18 NQ 134.4 144.4 10 100 135 **CEMENTED FINE TO MEDIUM GRAIN** SANDSTONE 140 HARD MEDIUM BLUISH GRAY 5B 5/1 19 NQ 144.4 154.4 10 86 CLAYSHALE CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD MEDIUM BLUISH GRAY 5B 5/1 **CLAYSHALE** w/limestone nodules throughout

AEP CIVIL ENGINEERING LABORATORY



Monitoring Well: M-1003

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE_<u>7/17/15</u> SHEET <u>7</u> OF __ BORING NO. M-1003 PROJECT CARDINAL LANDFILL 4/7/10 BORING FINISH 4/7/10 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD MEDIUM BLUISH GRAY 5B 5/1 FINE SANDY CLAYSHALE HARD MEDIUM BLUISH GRAY 5B 5/1 FINE **GRAIN SANDSTONE** STOPPED DRILLING @ 154.4' ON 04/13/10 / INSTALLED 2" PVC MONITORING WELL

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: FA-8



JOB N	NUME	BER _					_			00	I BOKING	wormorning	•••	. 17. 0
PROJECT CARDINAL FLY ASH DAM											RING NO. <u>FA-8</u> DATE <u>7/2</u>			
											RING START B			
		_		9,635.1			04-4	- Di		PII	EZOMETER TYPE SS	_ WELL TYPE	0)W
GRO	JND	ELEVAT	TON _	918.2	SY	STEM	NAI	te Plane usin 027/29	g 	HC	T. RISER ABOVE GROUND 2.8	DIA	·	
Wate	r Lev	el, ft	∇		Ī		Ā			DE	PTH TO TOP OF WELL SCREEN _4	0 BOTTOM	<u> 5</u>	0
TIME										WI	ELL DEVELOPMENT	BACKFILL	_ Q	UICK GROUT
DATE										FIE	ELD PARTY REB / DLB	RIG	<u>C</u>	ME-75
								1						T
비유	Ш		IPLE PTH	STAN	DARD	그돈监	RQD	DEPTH	₽ .n	ဟ	SOIL / ROCK		_	DRILLER'S
SAMPLE	SAMPLE		EET	RESIS	RATION TANCE	TON S	%	IN	GRAPHIC LOG	SC	IDENTIFICATION		WELL	NOTES
S Z	S)	FROM	TO	BLOW	/S / 6"	L H H	,0	FEET	5	\supset	is a result of the state of the			110120
Al		₹ 0.0	15.8								AUGERED TO 15.8'			Deconned with
														alconox and steam
														ginny before drilling.
								-	-					
								-						
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								5 -	-					
								-						
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								10 -						
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								-						
								-						
								15 -						
								13						
1	NQ2	15.8	20.0			1.8	23	-			N6 LIGHT GRAY to 5G 6/1 GREENIS	SH GRAY		
											FRACTURED LIMESTONE			
								-			High angle fractures			
Ω.														
/20/1								-	士					
7 70														
AEP.GDT 7/20/15		TYPE	OF C	ASING	USED						Continued Next Page)		
DAM.GPJ			OCK CO	RE				PIEZOM					OP	PEN TUBE
X X		<u>6" x 3.25</u> 9" x 6.25					\dashv	SLC	OTTE	ED S	SCREEN, $G = GEONOR$, $P = PI$	NEUMATIC		
⊈		HW CAS	SING AD	VANCER	}	4"		WELL T	YPE:	0'	N = OPEN TUBE SLOTTED SO	CREEN, GM	1 = G	SEOMON
8 X		NW CAS SW CAS				3" 6"	_					,		
A A		AIR HAI				8"					RECORDER DLB			

LOG OF BORING

Monitoring Well: FA-8

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ DATE **7/20/15** SHEET **2** OF _ BORING NO. FA-8 PROJECT **CARDINAL FLY ASH DAM** 3/8/04 BORING FINISH 3/23/04 **BORING START** STANDARD
PENETRATION ZEONO
PEN SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK DRILLER'S WELL LOG SC IN FEET % **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO NQ2 20.0 25.0 4.7 10YR 4/6 DARK YELLOWISH BROWN 85 **CLAYEY SHALE** 5YR 5/2 REDDISH GRAY SHALE 25 NQ2 25.0 35.0 87 **5YR 3/4 DARK REDDISH GRAY CLAYEY** SHALE 5B 5/1 MEDIUM BLUISH GRAY CLAYEY SHALE w/ angle fractures @ 27' (120 deg.), 27.5' (60 deg.), & 28.0' (140 deg.) 5B 5/1 MEDIUM BLUISH GRAY HARD SHALE w/ large limestone nodules and cross beds, w/ angle fractures @ 31.8' (80 deg.), & 32.2' (80 deg.) 30 35 4 NQ2 35.0 45.0 **5B 5/1 MEDIUM BLUISH GRAY SANDY** SHALE 40 10YR 5/4 YELLOWISH BROWN SANDY Lost water @ 42.5' SHALE 10YR 5/4 YELLOWISH BROWN MEDIUM GRAIN SANDSTONE **5B 7/1 LIGHT BLUISH GRAY MEDIUM GRAIN** SANDSTONE 45 NQ2 45.0 55.0 10 96 10YR 5/4 YELLOWISH BROWN MEDIUM **GRAIN SANDSTONE**

Continued Next Page

DAM.GPJ AEP.GDT 7/20/15

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LOG OF BORING

Monitoring Well: FA-8

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ DATE <u>7/20/15</u> SHEET <u>3</u> OF _ BORING NO. FA-8 PROJECT CARDINAL FLY ASH DAM 3/8/04 BORING FINISH 3/23/04 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO w/ angle fractures @ 47.0' (110 deg.) and limonitic vugs @ 46.4' 5B 5/1 MEDIUM BLUISH GRAY SHALE w/ limestone cross beds 10YR 4/3 BROWN MEDIUM GRAIN **SANDSTONE** 50 w/ angle fractures @ 49.0' (115 deg.) **5B 5/1 MEDIUM BLUISH GRAY MEDIUM GRAIN SANDSTONE** Well cemented 55 NQ2 55.0 65.0 10 100 10YR 5/6 GRAYISH BROWN MEDIUM GRAIN **SANDSTONE** 5B 5/1 MEDIUM BLUISH GRAY MEDIUM **GRAIN SANDSTONE** Well cemented 60 65 5B 5/1 MEDIUM BLUISH GRAY MEDIUM 7 NQ2 65.0 75.0 9.8 100 **GRAIN SANDSTONE** w/ black shale streaks throughout FA DAM.GPJ AEP.GDT 7/20/15 70

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LOG OF BORING

Monitoring Well: FA-8

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ DATE_**7/20/15** SHEET **4** OF _ BORING NO. FA-8 PROJECT CARDINAL FLY ASH DAM 3/8/04 BORING FINISH 3/23/04 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 75 NQ2 75.0 85.0 10 100 5B 5/1 MEDIUM BLUISH GRAY MEDIUM to Some water return @ **COARSE GRAIN SANDSTONE** 75.0' w/ coal lenses throughout 80 85 5B 5/1 MEDIUM BLUISH GRAY MEDIUM 9 NQ2 85.0 95.0 10 100 **GRAIN SANDSTONE** w/ coal lenses throughout, 2" bands of 10YR 4/4 BROWN SANDSTONE in bottom 2.0' 90 CD FA DAM.GPJ AEP.GDT 7/20/15 95 10YR 4/4 DARK YELLOWISH BROWN 10 NQ2 95.0 105.0 10 100 Lost water @ 95.0' **COARSE GRAIN SANDSTONE** w/ limonitic vugs throughout, well cemented

LOG OF BORING

Monitoring Well: FA-8

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ DATE <u>7/20/15</u> SHEET <u>5</u> OF _ BORING NO. FA-8 PROJECT CARDINAL FLY ASH DAM BORING FINISH 3/23/04 STANDARD
PENETRATION ZEDA SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 100 **N5 MEDIUM GRAY MEDIUM to COARSE GRAIN SANDSTONE** w/ coal streaks N1 BLACK COAL **N5 MEDIUM GRAY MEDIUM GRAIN** 105 11 NQ2 105.0 115.0 10 92 SANDSTONE **N5 MEDIUM GRAY MEDIUM to COARSE GRAIN SANDSTONE** w/ coal streaks N1 BLACK COAL **N5 MEDIUM GRAY MEDIUM GRAIN** SANDSTONE w/ black shale streaks N1 BLACK COAL **N5 MEDIUM GRAY MEDIUM GRAIN** 110 **SANDSTONE** w/ coal streaks and limestone nodules **5B 5/1 MEDIUM BLUISH GRAY MEDIUM GRAIN SANDSTONE** w/ black coal streaks 115 12 NQ2 115.0 100 5B 5/1 MEDIUM BLUISH GRAY MEDIUM 125.0 10 **GRAIN SANDSTONE** w/ limestone nodules (1 1/2") @ 116.8' to 117.4' 120 10YR 4/4 DARK YELLOWISH BROWN **MEDIUM GRAIN SANDSTONE** w/ limonitic vugs 5B 5/1 MEDIUM BLUISH GRAY MEDIUM

FA DAM.GPJ AEP.GDT 7/20/15

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LOG OF BORING

Monitoring Well: FA-8

	NUM IPAN'		IERIC <i>A</i>	AN ELECTRIC	POV	_ VER				DRING NO. FA-8 DATE 7/20/15 SI	HFFT	6 OF 7
				L FLY ASH DA						DRING START 3/8/04 BORING FINIS		
SAMPLE NUMBER	SAMPLE	DEI	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	SOSO	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
		FROIVI	10	BLOWS / 6	-					GRAIN SANDSTONE		
13	NQ2	125.0	135.0		10	100	125 -			w/ coal streaks 5B 5/1 MEDIUM BLUISH GRAY MEDIUM		
										GRAIN SANDSTONE 10YR 4/4 DARK YELLOWISH BROWN MEDIUM to COARSE GRAIN SANDSTONE		
							130 –			N5 MEDIUM GRAY MEDIUM to COARSE GRAIN SANDSTONE w/ coal streaks and limestone nodules in bottom 3.0'		
14	NQ2	135.0	145.0		10	97	- 135 - -			N6 MEDIUM LIGHT GRAY COARSE to MEDIUM GRAIN SANDSTONE w/ coal streaks and limestone nodules in bottom 1.5'		
							140 -					
							-		-	N4 MEDIUM DARK GRAY FINE GRAIN SHALEY SANDSTONE		
15	NQ2	145.0	155.0		10	100	145 -			N5 MEDIUM GRAY SANDY SHALE		
							-			N5 MEDIUM GRAY SHALEY FINE GRAIN SANDSTONE		
									<u> </u>	Continued Next Page		

LOG OF BORING

Monitoring Well: FA-8

	(IPAN	Y AM		AN ELECTRIC		VER				RING NO. FA-8 DATE 7/20/15 DRING START 3/8/04 BORING		
SAMPLE	SAMPLE	SAM DEF IN F	PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
	SAI	FROM		BLOWS / 6"	TO LEY	%	155 —			N5 MEDIUM GRAY SHALE		NOTE: Had to set 31.6' of 6" casing before using roller bit in hole.
AEP CD_FA_DAM.GPJ AEP.GDT 7/20/15												

AEP CIVIL ENGINEERING LABORATORY Monitoring Well: CA-0622





		IBER _							00	- BOKING		
				AN ELECTI						PRING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SH		
				L LANDFIL						PRING START 4/10/06 BORING FINISH		
				6,291.1 E						EZOMETER TYPE WELL TYPE		
GR	OUNE			1159.2						T. RISER ABOVE GROUND 2.281 DIA		
Wa	ater Le	vel, ft	$\overline{\Delta}$	Ţ		Ā				PTH TO TOP OF WELL SCREEN 354.9BOTTOM		
TIN	ΛE									ELL DEVELOPMENT BACKFILI		
DA	TE								FIE	ELD PARTY DLB / MCR / MWJ RIG	<u> </u>	-120
SAMPLE	SAMPLE	DE IN F	MPLE PTH EET	STANDAR PENETRATI RESISTANO	TOTAL ECOVER	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
		FROM		BLOWS / 6	6" ~			_				GPOLINDING
		0.0	10.0				5 -	-				GROUNDING PROCEDURES NOT IN USE ON THIS BORING. BLIND DRILLED FROM GRADE TO 10' WITH 3 7/8" ROLLER BIT & SET 3" PVC CASING. STARTED CORING AT 10.0'
1	NQ	10.0	13.9		3.3		10 -			HARD N8 VERY LIGHT GRAY LIMESTONE w/ 1/2" clay bands in bottom 0.3'		
2	NQ	13.9	18.9		5.0		15 -			HARD N8 VERY LIGHT GRAY LIMESTONE		
FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NIC	18.9	23.9		4.7					SOFT 5G 6/1 GREENISH GRAY SHALE HARD 5R 4/2 GRAYISH RED SHALE		
JEP.G	NQ	10.9	23.9		4.7					FIAND OR 4/2 GRATION RED SHALE		
ILL.GPJ A		TYPE	E OF C	ASING US	ED					Continued Next Page		
NDFI	+	NQ-2 R 6" x 3.25		RE		\dashv	PIEZOM				OP	EN TUBE
200		9" x 6.2	5 HSA				SLO	JΙΤΕ	בט צ	SCREEN, G = GEONOR, P = PNEUMATIC		
CD_FG	+	HW CA		VANCER	<u>4"</u> 3"	\dashv	WELL T	YPE:	O)	N = OPEN TUBE SLOTTED SCREEN, GM	1 = G	SEOMON
AEP C		SW CAS	SING		6"					RECORDER		
₩		AIR HAI	MMER		8"							

LOG OF BORING

Monitoring Well: CA-0622

JOB	NUM	BER _				_		LOC	30	1 BOILING
CON	/IPAN	_ AM	IERIC/	N ELECTRIC	POW	/ER			ВС	ORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEET <u>2</u> OF <u>16</u>
PRC	JECT	CAF	RDINA	L LANDFILL					ВС	ORING START 4/10/06 BORING FINISH 6/1/06
				1						
ще	Щ		1PLE	STANDARD PENETRATION RESISTANCE BLOWS / 6"	그મ쏪	RQD	DEPTH	2	S	DDILLEDIO
SAMPLE	SAMPLE		PTH EET	RESISTANCE	F F E E E	07	IN	GRAPHIC LOG	SC	SOIL / ROCK ☐ ☐ DRILLER'S IDENTIFICATION > NOTES
S S	S S	FROM		BLOWS / 6"		%	FEET	GR	\supset	IDENTIFICATION ≥ NOTES
		1 IXOIVI		BLOWS/0						5G 6/1 GREENISH GRAY LIMESTONE
										fractured throughout
							-	+		
							-			5GY 6/1 GREENISH GRAY SHALE
1	NQ	23.9	33.9		9.7		-			5B 5/1 MEDIUM BLUISH GRAY SHALE
4	INQ	23.9	33.9		9.7					fractured
							25 -			
							_			N7 LIGHT GRAY LIMESTONE
							-			
								\Box		
							-	Ħ		5G 6/1 GREENISH GRAY SHALE 5G 6/1 GREENISH GRAY LIMESTONE
							-	H		fractured fractured
										5G 6/1 GREENISH GRAY SHALE
							30 –			
							-			
										HARD 5B 5/1 MEDIUM BLUISH GRAY SHALEY LIMESTONE
							-	+++		SHALET LIMESTONE
								量		
5	NQ	33.9	43.9		9.8		-			HARD 5B 5/1 MEDIUM BLUISH GRAY
							35 -			SHALEY LIMESTONE fractured in bottom 1.5'
							33	耳		nactured in bottom 1.5
							-			
								丰		
							-	ĒŢ		
							-	丰		
								丰		
							-			
							40 -			
							40	异		
7/15							-			
T 7/1								耳		
P.GD										
J AE										
E.GP.										
6	NQ	43.9	46.9		3.0		-			HARD 5B 5/1 MEDIUM BLUISH GRAY
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15 O	-						45 -			SHALEY LIMESTONE
된							-			
9 <u>'</u>	1			<u> </u>				1 — 1		

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LOG OF BORING

JOB NUMBER

Monitoring Well: CA-0622

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COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEET <u>3</u> OF <u>16</u> PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE SAMPLE NUMBER DEPTH SAMPLE DEPTH F0G SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO NQ 46.9 7.0 53.9 5B 5/1 MEDIUM BLUISH GRAY SHALE HARD 5B 5/1 MEDIUM BLUISH GRAY **SHALEY LIMESTONE** NQ 53.9 63.9 9.6 HARD N5 MEDIUM GRAY SHALEY LIMESTONE 10.0 HARD 5B 5/1 MEDIUM BLUISH GRAY to N6 NQ 63.9 73.9 **MEDIUM LIGHT GRAY SHALE** 65 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N4 MEDIUM DARK GRAY SHALE small coal band @ 73.8 70

AEP CIVIL ENGINEERING LABORATORY

JOB NUMBER

LOG OF BORING

Monitoring Well: CA-0622

__ DATE <u>7/17/15</u> SHEET <u>4</u> OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH **DEPTH** F0G SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 10.0 HARD N6 MEDIUM LIGHT GRAY SHALE 10 NQ 73.9 83.9 w/ coal band @ 74.4, angle fracture @ 75.7 75 SOFT N4 MEDIUM DARK GRAY SHALE 80 HARD N2 GRAYISH BLACK SHALE COAL HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE NQ 83.9 93.9 10.0 HARD N5 MEDIUM GRAY SHALE 11 85 90 HARD 5B 7/1 LIGHT BLUISH GRAY MIXED w/ **N6 MEDIUM LIGHT GRAY SHALE** w/ limestone nodules CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 12 NQ 93.9 103.9 10.0 HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE 95

LOG OF BORING

JOB NUMBER

Monitoring Well: CA-0622

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COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **5** OF **16** BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S FOG SCS WELL IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO HARD 5B 7/1 LIGHT BLUISH GRAY SHALE w/ sandstone streaks, angle fracture @ 98.5 100 HARD N6 MEDIUM LIGHT GRAY SHALE NQ 103.9 113.9 10.0 w/ sandstone streaks, bottom 0.5 carbonious 105 110 **N8 VERY LIGHT GRAY LIMESTONE** HARD N3 DARK GRAY SHALE **N7 LIGHT GRAY LIMESTONE** w/ 0.2 5B 5/1 medium bluish gray shale band @ NQ 113.9 123.9 10.0 N7 LIGHT GRAY LIMESTONE HARD 5GY 4/1 DARK GREENISH GRAY 115 SHALE **5GY 4/1 DARK GREENISH GRAY SHALE** CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N6 MEDIUM LIGHT GRAY SHALE w/ sandstone streaks 120

LOG OF BORING

Monitoring Well: CA-0622

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>6</u> OF _ BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK DRILLER'S LOG WELL SC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO NQ 123.9 133.9 10.0 HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE bottom 0.8 N3 dark gray carbonious 125 130 **N5 MEDIUM GRAY FINE GRAIN SANDSTONE** w/ shale band NQ 133.9 143.9 10.0 HARD N5 MEDIUM GRAY SHALE 16 135 COAL w/ hard shale bands N4 MEDIUM DARK GRAY SHALE w/ 0.5 of carbonious shale at 142.0, bottom 1.9 hard 17 NQ 143.9 153.9 10.0 HARD N6 MEDIUM LIGHT GRAY SHALE **N8 VERY LIGHT GRAY LIMESTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 145 HARD N6 MEDIUM LIGHT GRAY SHALE **N8 VERY LIGHT GRAY LIMESTONE** w/ 0.3 shale bands @ 147.8 & 152.4 8

Monitoring Well: CA-0622 LOG OF BORING JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **7** OF BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL SOIL / ROCK DRILLER'S WELL LOG SC IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO NQ 153.9 163.9 6.2 68 HARD N6 MEDIUM LIGHT GRAY LIMESTONE SWL 21.4' on 18 04/17/06 w/ NQ 155 HOLE TO 153.9'. USED ±4.000 GALS. WATER TO THIS **POINT** HARD N6 MEDIUM LIGHT GRAY FRACTURED LIMESTONE HARD N5 MEDIUM GRAY SHALE/LIMESTONE SOFT N5 MEDIUM GRAY SHALE/LIMESTONE **LOST ALL WATER RETURN AT 157.8'.** HARD N5 MEDIUM GRAY SHALE/LIMESTONE HYD. PUSH - NO **ROTATION FROM** 163.9' - 165.9' 160 (VOID) NQ 163.9 168.9 VOID 19 1.9 84 165 SOFT 5B 5/1 MEDIUM BLUISH GRAY SHALE 20 NQ 168.9 170.9 1.3 0 SOFT N5 MEDIUM GRAY SHALE wet 170 Stopped after going through mine void. NQ HARD N6 MEDIUM LIGHT GRAY SHALE Started drilling HW 21 170.9 178.9 7.9 67 casing and cleaning SOFT N4 MEDIUM DARK GRAY SHALE inside of casing w/ 4" fractures throughout roller bit. At 155', roller bit broke off inside casing. It was decided to abandon HARD N6 MEDIUM LIGHT GRAY SHALE and grout this boring. fractured Moved east +/- 5" 175 and started drilling new boring w/6" air

LANDFILL.GPJ AEP.GDT 7/17/15

FGD

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LOG OF BORING

Monitoring Well: CA-0622

SAMPLE NUMBER SAMPLE	SAMPLE DEPTH IN FEET FROM TO	TH PENETRAT	D _≿	RQD %	DEPTH IN FEET	GRAPHIC		RING START 4/10/06 BORING FINIS SOIL / ROCK IDENTIFICATION HARD N7 LIGHT GRAY SHALE	MEIL WELL	DRILLER'S NOTES hammer and inserted
F	DEPTH IN FEET FROM TO	PTH PENETRATI EET RESISTAN TO BLOWS /	RECOVER		DEPTH IN FEET	GRAPHIC	USCS	IDENTIFICATION	WELL	NOTES
22 NQ	178.9 186	186.9	6.6	56				HARD N7 LIGHT GRAY SHALE		hammer and inserted
					180 -			SOFT N7 LIGHT GRAY SHALE W/ fracture SOFT N6 MEDIUM LIGHT GRAY SHALE		HW casing to bottom old mine floor @ 173.3'. This boring was drilled through mine piller; no came work done on this boring. Coal seam estimated @ +/- 165.0'-17
					-			SOFT N6 MEDIUM LIGHT GRAY SHALE w/ fracture, wet HARD N7 LIGHT GRAY SHALE dry N7 LIGHT GRAY CLAY SHALE dry		
					185 -			N4 MEDIUM DARK GRAY SHALE		
23 NQ	186.9 189	189.4	2.5	88	-			VERY HARD N6 MEDIUM LIGHT GRAY SHALE w/ trace of fine limestone		Resumed coring and logging core @ 186.9'
24 NQ	189.4 194	194.4	5.0	40	190 -			N5 MEDIUM GRAY SHALE fracture, wet N6 MEDIUM LIGHT GRAY SHALE/LIMESTONE SOFT MEDIUM GRAY SHALE wet MEDIUM LIGHT GRAY SHALE		
25 NQ	194.4 204	204.4	10.0	83	195 –			SOFT N5 MEDIUM GRAY SHALE moist 5B 5/1 MEDIUM BLUISH GRAY SHALE HARD N5 MEDIUM GRAY SHALE fracture		
					200 -			HARD N5 MEDIUM GRAY SHALE		
					-					

LOG OF BORING

Monitoring Well: CA-0622

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>9</u> OF _ BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S WELL LOG SC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **N5 MEDIUM GRAY SHALE** \fracture, wet HARD N5 MEDIUM GRAY SHALE 26 NQ 204.4 214.4 8.7 64 HARD N4 MEDIUM DARK GRAY SHALE 205 5G 6/1 GREENISH GRAY SHALE w/trace of fine imestone, wet **N2 GRAYISH BLACK SHALE** \fractured SOFT N4 MEDIUM DARK GRAY SHALE **N2 GRAYISH BLACK SHALE** fracture **N5 MEDIUM GRAY SHALE** fracture, wet 210 **5G 6/1 GREENISH GRAY SHALE 5G 6/1 GREENISH GRAY SHALE** wet **5GY 6/1 GREENISH GRAY** NQ 214.4 219.4 5.0 66 215 SHALE/LIMESTONE N5 MEDIUM GRAY SHALE **SOFT 5YR 6/1 LIGHT BROWNISH GRAY SANDY SHALE** 28 NQ 219.4 HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE 229.4 9.9 81 220 w/limestone fractures 5B 5/1 MEDIUM BLUISH GRAY SHALE w/limestone FGD LANDFILL.GPJ AEP.GDT 7/17/15 225 N4 MEDIUM DARK GRAY SHALE fractured, wet 8

LOG OF BORING

Monitoring Well: CA-0622

JOB	NUM	BER _					LOC	DE BORIN	G	IVIOTIILOTI	ilg vvei	ii. CA	.0022
COM	IPAN'	/ <u>AM</u>	IERIC <i>A</i>	N ELECTRIC	POWER			ORING NO.	CA-0622	DATE 7/17/15	SHE	ET <u>1</u>	0_ OF10
PRO	JECT	CAF	RDINA	L LANDFILL				ORING STAF	RT <u>4/10/</u>	06 BORING	FINISH	6/1/0)6
SAMPLE	SAMPLE	DEI	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY MODAL	DEPTH IN FEET	GRAPHIC LOG			/ ROCK FICATION		WELL	DRILLER'S NOTES
29	NQ	229.4	238.8			230 -		fracture N4 MEDI fractured HARD MI	ONE UM DARK GR EDIUM DARK	SH GRAY SHALE/ EAY SHALE GRAY SHALE			
						235 -			DARK GRAY	LIMESTONE			
30	NQ	238.8	244.4			-			ARK GRAY LI	MESTONE RK GRAY SHALE			
						240 -		N2 GRAY	/ISH BLACK (COAL			
31	NQ	244.4	254.4			- 245		HARD N4 SHALE/L	4 MEDIUM DA IMESTONE	RK GRAY SHALE RK GRAY			
						-				H GRAY SHALE			
						250 -		SOFT 5G w/limesto	ne, wet	ISH GRAY SHALE SYR 4/1 BROWNISH			
						-		GRAY SH	HALE				
2						_		5B 5/1 M	EDIUM BLUIS	H GRAY SHALE			
į	I	ı	1	1				1	Continued	l Next Page			

LOG OF BORING

JOB NUMBER

Monitoring Well: CA-0622

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322		

COMPANY AMERICAN ELECTRIC POWER __ DATE_<u>7/17/15</u>__ SHEET_<u>11</u>_ OF __<u>16</u>_ BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH **DEPTH** LOG SOIL / ROCK WELL DRILLER'S SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO SOFT MEDIUM BLUISH GRAY SHALE 32 NQ 254.4 264.4 255 -HARD 5GY 6/1 GREENISH GRAY SHALE w/fractures of limestone 260 **5YR 4/1 BROWNISH GRAY RED SHALE** MEDIUM BLUISH GRAY SHALE w/fractures of limestone 33 NQ 264.4 274.4 N4 MEDIUM DARK GRAY SHALE 265 SOFT N4 MEDIUM DARK GRAY SHALE wet 270 34 NQ 274.4 284.4 SOFT N4 MEDIUM DARK GRAY SHALE 275 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 N7 LIGHT GRAY & N4 MEDIUM DARK GRAY SHALE w/trace of limestone

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: CA-0622

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>12</u> OF _ BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S DEPTH PENETRATION LOG SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO N4 MEDIUM DARK GRAY SHALE/LIMESTONE HARD SHALE NQ 284.4 294.4 N4 MEDIUM DARK GRAY SHALE 285 w/fractures of limestone HARD N3 DARK GRAY SHALE 290 HARD N4 MEDIUM DARK GRAY SHALE 36 NQ 294.4 304.4 295 300 CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 304.4 10.0 37 NQ 314.4 100 305

AEP CD

AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

Monitoring Well: CA-0622

SAMPLE	SAMPLE		IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	OTAL NGTH OVERY	DEPTH IN FEET	APHIC LOG	SCS	SOIL / ROCK	WELL	DRILLER'S
y N	SA	FROM	то	BLOWS / 6"	FEE 3	FEET	GR	\cap	IDENTIFICATION	>	NOTES
						310 -					
						310					
38	NQ	314.4	324.4		10.0	315 -			N4 MEDIUM DARK GRAY SHALE	1	
						313					
									N4 MEDIUM DARK GRAY & N6 MEDIUM	-	
									LIGHT GRAY SHALE w/fine sandstone		
									Willie Saliustolle		
						320 -					
						320			N4 MEDIUM DARK GRAY SHALE	-	
									w/traces of fine standstone lens N5 MEDIUM GRAY SHALE	-	
									w/trace of fine sandstone		
39	NQ	324.4	334.4		10.0	325 -			HARD MEDIUM GRAY & MEDIUM DARK	1	
						323			GRAY SHALE w/trace of coarse sandstone		
						330 -			N5 MEDIUM GRAY COARSE GRAIN SANDSTONE		
									HARD N3 DARK GRAY SHALE	1	
									w/trace of sandstone N5 MEDIUM GRAY COARSE GRAIN	1	MORGANTOWN

AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING Monitoring Well: CA-0622

AEP

JOB NUMBER _______ BORING NO. CA-0622 DATE 7/17/15 SHEET 14 OF 16

PROJECT CARDINAL LANDFILL BORING START 4/10/06 BORING FINISH 6/1/06

NUMBER	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-			SANDSTONE Morgantown sandstone starts @ 331.5'		SANDSTONE STARTS @ 331.5
40	NQ	334.4	344.4		10.0		005			N6 MEDIUM LIGHT GRAY SANSDSTONE		
							335 -			HARD N3 DARK GRAY SHALE w/trace of fine sandstone		
							-			N2 GRAYISH BLACK SHALE		
							-					
							340 -			N5 MEDIUM GRAY COARSE GRAIN		
							-			SANDSTONE HARD N2 GRAYISH BLACK SHALE w/trace of fine sandstone		
							-					
1	NQ	344.4	354.4		9.8	92	345 -			N5 MEDIUM GRAY COARSE GRAIN SANDSTONE W/trace of dark shale HARD N4 MEDIUM DARK GRAY SHALE		
							-			w/trace of fine sandstone		
							-					
							350 —					
							-					
							-			MEDIUM GRAY SANDSTONE w/dark shale fractures		
12	NQ	354.4	364.4		9.7	91	355 -			N6 MEDIUM LIGHT GRAY COARSE GRAIN SANDSTONE		
							-			∖GRAYISH BLACK COAL		
										fracture		

AEP CIVIL ENGINEERING LABORATORY Monitoring Well: CA-0622

-0622 **AFP**

JOB NUMBER _______

COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 DATE 7/17/15 SHEET 15 OF 16

PROJECT CARDINAL LANDFILL BORING START 4/10/06 BORING FINISH 6/1/06

NUMBER	SAMPLE	DEI IN F	IPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH ECOVERY	RQD	DEPTH IN		nscs	SOIL / ROCK IDENTIFICATION	DRILLER'S NOTES
_		FROM	ТО	BLOWS / 6"			FEET 			N6 MEDIUM LIGHT GRAY COARSE GRAIN SANDSTONE	
							360 -			N5 MEDIUM GRAY SHALE	
							-				
3	NQ	364.4	373.4		10.0	90	365 -	× × × × × × × × × × × × × × × × × × ×		N6 MEDIUM LIGHT GRAY SILTSTONE	
							-	× × × ×			
							-	× × × × × × × × × × × × × × × × × × ×			
							370 -	× × × × × × × × × × × × × × × × × × ×			
							-	× × × × × × × × × × × × × × × × × × ×		HARD N5 MEDIUM GRAY SHALE	
4	NQ	373.4	383.4		10.0	81	375 -			HARD N3 DARK GRAY CLAY SHALE	
							-			N2 GRAYISH BLACK CLAY SHALE SEAM	
							-	3		N1 BLACK COAL SEAM HARD N5 MEDIUM GRAY CLAY SHALE	
							380 -				
							-				
							-				
											STOPPED BORIN

LOG OF BORING

Monitoring Well: CA-0622

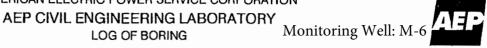
JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEET <u>16</u> OF _ PROJECT CARDINAL LANDFILL BORING START **4/10/06** BORING FINISH **6/1/06** STANDARD
PENETRATION ZEONO HAD ROD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO @ 383.4'. SET 1" **GEOMON WELL** CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: M-6

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		_	HIO BC		COMP	NIV	_				BODING NO COCA22 DATE		4 05 4			
PROJECT TIDD ASH POND SITE INVESTIGATION											BORING NO. 90CA22 DATE SHEET 1 OF 4 BORING START 07/23/90 BORING FINISH 08/09/90					
								JATION			PIEZOMETER TYPE BURING FINISH					
											HGT. RISER ABOVE GROUND 1.9 DIA 1.0					
											DEPTH TO TOP OF WELL SCREEN 220.6_ BOT					
-		.EVEL	<u>¥</u> 52	2.7	<u>¥</u>						WELL DEVELOPMENT BACKE					
TIMI							+				FIELD PARTY MCR-JD RIG	_				
DAT	Ε		7-3	0-90	L						TILLED FARTT WOTTOD RIV	J	<u>D-01</u>			
SAMPLE	SAMPLE	DE IN	MPLE EPTH FEET	PENET RESIS	IDARD RATION TANCE VS / 6"		%	DEPTH IN FEET	GRAPH	8 U S D	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES			
1	NQ		5.9	BLOV				10 -					WATER RETURNED AFTER SEATING CASING.			
2	NQ	25.6	30.4			4.8	59	25			GRAY SILTY CLAYSHALE Calcareous, vertical cracks 20.8-21.1, 21.6-21.8 GRAY SHALEY LIMESTONE Hard. GRAY SILTY SANDSTONE V-fine grain.					
3	NQ	30.4	40.4			10.0	77	30 -			GRAY LIMESTONE Hard, stain on joints and vertical cracks. GRAY TO BLACK CLAYSHALE GRAY SILTY SANDSTONE F-fine grain.					
								35 —			vertical cracks GRAY LIGHT GRAY CLAYSHALE Slightly sandy, calcareous.					
4	NQ	40.4	50.4			10.0	45	45			LIGHT GRAY SANDSTONE Silt crossbedding throughout, thin bedding at 43.1 GRAY TO LIGHT TO DARK GRAY CLAYSHALE Broken slightly calcareous. LIGHT GRAY LIMESTONE Vertical fracture from 46.0-46.9, calcite filled. GRAY SANDY CLAYSHALE Broken, silty,					
		TYPE	OF C	ASING	USED						Continued Next Page					
X		NQ-2 6" x 3. 9" x 6. HW C	ROCK (25 HSA 25 HSA ASING	CORE		4" 3"		PIEZOMI SLC WELL TY	TTE	D S	E: PT = OPEN TUBE POROUS TIP, SS = CREEN, G = GEONOR, P = PNEUMATIC N = OPEN TUBE SLOTTED SCREEN, GM					
_ X _	X NW CASING 3" SW CASING 6"										RECORDER JD					



JOB NUMBER		
COMPANY OHIO POWER COMPANY	BORING NO. 90CA22 DATE	_ SHEET <u>2</u> OF <u>4</u>
PROJECTTIDD ASH POND SITE INVESTIGATION	BORING START07/23/90 BORING F	INISH 08/09/90

SAMPLE NUMBER	THOW TO		PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	%	DEPTH IN FEET	GRAPH LOG	SCS	SOIL / ROCK 기 및	DRILLER'S NOTES
5	NQ	50.4	60.4		9.3	78	55 -			Slightly calcareous. DEEP MAROON PURPLE CLAYSHALE Blocky, slightly calcareous, slightly weathered. LIGHT GREEN TO LIGHT GRAY CLAYSHALE Slightly broken.	
										LIGHT TAN TO LIGHT GRAY SANDSTONE Fine grain, silt bedding throughout.	
6	NQ	60.4	65.4		4.7	37	60 -			RUST BROWN CLAYSHALE Iron precipitate	
7	NQ	65.4	70.4		5.0	27	65 -			staining throughout, broken, slightly sandy to very sandy, fine grained sand. LIGHT GRAY SANDSTONE Very fine grain, silt partings and cross bedding throughout.	
8	NQ	70.4	75.4		5.0	27	70 -			RUST BROWN CLAYSHALE Iron precipitate staining throughout, broken, slightly sandy to very sandy, fine grained sand. LIGHT GRAY SANDSTONE Very fine grain, silt partings and cross bedding throughout. LIGHT GRAY CLAYSHALE Slightly sandy, silty.	
9	NQ	75.4	80.4		4.7	25	75 - -			SILTY CLAYSHALE Soft, crack, appears to have been very plastic in the drill bit. LIGHT TO MEDIUM GRAY LIMESTONE Slightly sandy.	
10	NQ	80.4	90.4		9.9	79	80 -			MEDIUM GRAY LIMESTONE Slightly shaley. GRAY CLAYSHALE Some silt bedding.	
							85 — - -			GRAY SILTY CLAYSHALE Limestone nodules throughout, hard. GRAY CLAYSHALE Hard, with traces of	
11	NQ	90.4	100.4		10.0	84	90			GRAY CLAYSHALE Hard, with traces of limestone throughout, fine grain sand throughout.	
							-			VERY BROKEN 97.2-97.8	
12	NQ	100.4	110.4		10.0	66	100			VERY BROKEN 97.2-97.8	
			-				105				
13	NQ	110.4	120.4		9.5	52	110			LIGHT GRAY SANDSTONE Fine grain, silty, crossbedding.	
- 1					- 1				\rightarrow	LIGHT GRAY LIMESTONE Highly calcareous,	



144.2 TOP OF

SAND.

Monitoring Well: M-6 LOG OF BORING JOB NUMBER COMPANY OHIO POWER COMPANY BORING NO. 90CA22 DATE_____ SHEET 3 OF 4 BORING START 07/23/90 BORING FINISH 08/09/90 PROJECT TIDD ASH POND SITE INVESTIGATION STANDARD
PENETRATION HENCE OF THE PROPERTY OF RQD SAMPLE Ø SAMPLE NUMBER DEPTH SOIL / ROCK DEPTH Log WELL DRILLER'S ပ IN IN FEET Ø IDENTIFICATION NOTES % FEET FROM BLOWS / 6" TO very hard, some silty bedding. 120 14 NO 120.4 130.4 9.8 71 LIGHT GRAY SLIGHTLY SANDY SHALEY LIMESTONE SOME THIN SANDSTONE LENSES., VERY CALCAREOUS STREAKS THROUGHOUT, LIMESTONE BECOMING 125 V=VERY SANDY WITH DEPTH, LIMESTONE IS VERY SILTY. 130 10.0 83 NQ 130.4 140.4 135 -135.5 TOP OF SEAL

GRAY LIGHT GRAY SILTY SHALE Silt cross

LIGHT GRAY SANDSTONE Coarse grain,

some micaceous partings throughout.

siltstone lenses at 148.4-148.6 and 151.1-151.3

Continued Next Page

bedding throughout.

140

145

150

155

160

165

170

175

100

100

10.0 100

10.0

10.0 100

10.0

NQ

NQ

NQ

16

140,4

150.4

160.4

NQ 170.4

150.4

160.4

170.4

180.4

LOG OF BORING

Monitoring Well: M-6

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JOB NUMBE	R	_	
COMPANY	OHIO POWER COMPANY	BORING NO. 90CA22 DATE SHEET 4 OF	4_
PROJECT_	TIDD ASH POND SITE INVESTIGATION	BORING START07/23/90 BORING FINISH08/09/90	

SAMPLE	SAMPLE	DE IN F	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE	TOTAL ENGTH ECOVERY	RQD %	DEPTH IN FEET	GRAPH LOG	SCS	SOIL / ROCK IDENTIFICATION		DRILLER'S NOTES
20	NQ	180.4	190.4	BLOWS / 6"	9.5	95	_		ם			
							185					
21	NQ	190.4	200.4		10.0	100	190 —			SOME MICACEOUS PARTINGS		
							195 -					
23	NQ	200.4	210.4		8.7	87	200 -				· : · :	
							205 -			THIN COAL LENSES AT 205.1-205.5 BECOMING MORE BROKEN BELOW 205.5		
24	NQ	210.4	215.4		4.6	92	210			BOTTOM OF MORGANTOWN SANDSTONE SANDY LIMESTONE CONGLOMERATE		
25	NQ	215.7	225.7		10.0	100	215			SANDSTONE Fine grain, calcareous.		
							220					220.4 ÇHECL VALVE. 221.0 TOP OF SCREEN.
26	NQ	225.7	230.2		4.5	100	225			SANDSTONE V-fine grain, calcareous, silt crossbedding throughout.		223.0 BOTTOM OF SCREEN. 224.0 BOTTOM OF SAND.
							230	::::				230.0 BOTTOM OF SEAL.

LOG OF BORING

Monitoring Well: M-10

JOB N		_	·		OTDIO							- 100/4 -	
				AN ELE AL PLAN		POWI	EK				PRING NO. 85W-3 DATE_		
						18 683	2				PRING START 8/9/85 EZOMETER TYPE		
				1031.0							GT. RISER ABOVE GROUND		
							\ T				PTH TO TOP OF WELL SCREEN		
TIME		ei, it	<u> </u>	49.0	<u>¥</u> 11	7.0	- <u>-</u>				ELL DEVELOPMENT		
DATE			0	-9-85	0.1	0-85					ELD PARTY B. KGOLLIHUE		
DATE	-		0-	-9-05	0-1								
SAMPLE	SAMPLE	DE	MPLE PTH EEET TO	PENET RESIS	IDARD RATION TANCE VS / 6"	TOTAL LENGTH RECOVERY	rqd %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION		DRILLER'S NOTES
1		0.0	1.0			0.0					BROWN TOPSOIL		
2		1.0	40.0			0.0					COAL AND SHALE, FILL		
10								10 -					
7/1/2		TYPI	E OF	CASING	USED)		-	— <u>—</u> ——————————————————————————————————		Continued Next Pa		1 4/1
. X		NQ-2 R					\dashv	_					
		6" x 3.2	5 HSA	OI \L				PIEZOMETER TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE SLOTTED SCREEN, G = GEONOR, P = PNEUMATIC					
<u> </u>		9" x 6.2 HW CA		ADVANCER	₹	4"					W = OPEN TUBE SLOTTED		= GEOMON
X		NW CA	SING			3"	\dashv	WELL T	TPE:		VV - OFEIN TUDE SLUTTED	SUREEN, GIVI	- GEUIVIUN
<u> X</u>	1	SW CA	SING			6"	-	RECORDER					

LOG OF BORING

Monitoring Well: M-10

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ DATE <u>7/20/15</u> SHEET <u>2</u> OF _ BORING NO. 85W-3 PROJECT CARDINAL PLANT 8/9/85 BORING FINISH 8/13/85 **BORING START** SAMPLE STANDARD GRAPHIC LOG SAMPLE NUMBER DEPTH USCS SAMPLE **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S IN IN FEET RESISTANCE **IDENTIFICATION NOTES** FEET FROM BLOWS / 6" TO **BROWN WEATHERED SHALE** 40 NW 40.0 45.0 5.0 **BROWN AND GRAY SANDY SHALE, BROKEN** CD SI.GPJ AEP.GDT 7/20/15 45 NW 45.0 55.0 10.0

LOG OF BORING

Monitoring Well: M-10

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET <u>3</u> OF __ PROJECT CARDINAL PLANT 8/9/85 BORING FINISH 8/13/85 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **GRAY FINE SANDSTONE**, SHALE SEAMS, 50 **DARK GRAY SANDY SHALE, BROKEN** 55 NW 55.0 65.0 10.0 60 **GRAY FINE SANDSTONE, BROKEN** 65 NW 65.0 75.0 10.0 CD SI.GPJ AEP.GDT 7/20/15 70 **GRAY SANDY SHALE**, WITH SMALL CLAY **SEAMS**

LOG OF BORING

Monitoring Well: M-10

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET <u>4</u> OF _ PROJECT CARDINAL PLANT 8/9/85 BORING FINISH 8/13/85 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH USCS **DEPTH** SOIL / ROCK WELL DRILLER'S IN FEET **IDENTIFICATION NOTES FEET** FROM TO 75 NW 75.0 85.0 10.0 **LIGHT GRAY FINE SANDSTONE** BROKEN 80 DARK GRAY CLAYSTONE 85 NW 85.0 95.0 10.0 90 CD SI.GPJ AEP.GDT 7/20/15 95 NW 95.0 105.0 10.0

LOG OF BORING

Monitoring Well: M-10

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>85W-3</u> DATE <u>**7/20/15** SHEET <u>5</u> OF _</u> PROJECT CARDINAL PLANT **BORING START** 8/9/85 BORING FINISH 8/13/85 STANDARD
PENETRATION PENETRATI SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH USCS **DEPTH** SOIL / ROCK WELL DRILLER'S IN FEET **IDENTIFICATION NOTES FEET** FROM TO 100 **GRAY AND RED SILTY SHALE**, BROKEN 105 10 NW 105.0 115.0 10.0 **GRAY SILTY SHALE** 110 115 NW 115.0 125.0 10.0 **GRAY LIMESTONE**, HARD 120 CD SI.GPJ AEP.GDT 7/20/15

Monitoring Well: M-1

COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET <u>6</u> OF _ PROJECT CARDINAL PLANT **BORING START** 8/9/85 BORING FINISH 8/13/85 SAMPLE **STANDARD** GRAPHIC LOG SAMPLE NUMBER DEPTH USCS SAMPLE DEPTH PENETRATION SOIL / ROCK WELL DRILLER'S IN RESISTANCE IN FEET **IDENTIFICATION NOTES** FEET FROM BLOWS / 6" TO 125 12 NW 125.0 135.0 10.0 130 135 13 NW 135.0 10.0 145.0 140 145 14 NW 145.0 155.0 10.0

CD SI.GPJ AEP.GDT 7/20/15

JOB NUMBER

LOG OF BORING

Monitoring Well: M-10



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET <u>7</u> OF _ PROJECT CARDINAL PLANT 8/9/85 BORING FINISH 8/13/85 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH USCS **DEPTH** PENETRATION LOG SOIL / ROCK WELL DRILLER'S IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **GRAY AND RED SILTY SHALE** 155 15 NW 155.0 165.0 10.0 160 165 16 NW 165.0 175.0 10.0 170 **GRAY COARSE SANDSTONE** 171.5 TOP OF SEAL. CD SI.GPJ AEP.GDT 7/20/15 175 NW 175.0 185.0 10.0

LOG OF BORING

Monitoring Well: M-10

COI		AM		AN ELECTRIC L PLANT	POV	VER				BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET <u>8</u> BORING START <u>8/9/85</u> BORING FINISH <u>8/13/85</u>				
SAMPLE		SAM DEI	IPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL / ROCK		WELL	DRILLER'S NOTES
18	NW	185.0	195.0		10.0		180 - 185 -							178.0 TOP OF SAND.
							190 -							
AEP CD SI.GPJ AEP.GDT 7/20/15	NW	195.0	205.0		10.0		195 -							
EP CD SI.GF										Co	ontinued Next I	Page		

LOG OF BORING

Monitoring Well: M-10

JOB	NUM	BER _					LOO	OI BOITING		Worldoning W	Veli. IVI-10		
COM	IPAN'	/ <u>AM</u>	ERIC#	N ELECTRIC	POWE	iR	ı	BORING NO. <u>85</u> V	N-3 DATE	DATE <u>7/20/15</u> SHEET <u>9</u> OF <u>11</u>			
PRO	JECT	CAF	RDINA	L PLANT				BORING START	8/9/85	BORING FINISH _	8/13/85		
SAMPLE NUMBER	SAMPLE	DEF	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	QD DEPTH IN FEET	GRAPHIC LOG		SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES		
20	NW		215.0		10.0	205 -							
						210 -							
21	NW	215.0	225.0		10.0	215 -				1.*[]	. * [
						220 -							
22	NW	225.0	235.0		10.0	225 -							

AEP CD SI.GPJ AEP.GDT 7/20/15

LOG OF BORING

JOB NUMBER

CD SI.GPJ AEP.GDT 7/20/15

Monitoring Well: M-10

AEP

COMPANY AMERICAN ELECTRIC POWER ___ DATE **7/20/15** SHEET **10** OF __ BORING NO. 85W-3 PROJECT CARDINAL PLANT **BORING START** 8/9/85 BORING FINISH 8/13/85 STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER GRAPHIC LOG SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO 228.9 CHECK VALVE. 230 229.5 TOP OF SCREEN. 230.5 BOTTOM OF SCREEN. 235 23 NW 235.0 245.0 10.0 237.0 BOTTOM OF SAND. **GRAY LIMEY SHALE**, VERY HARD, BROKEN 240 245 NW 245.0 255.0 10.0 250

LOG OF BORING

Monitoring Well: M-10

COM	JOB NUMBER COMPANY _ AMERICAN ELECTRIC POWER PROJECT _ CARDINAL PLANT									BORING NO. <u>85W-3</u> DATE <u>7/20/15</u> SHEET _1 BORING START <u>8/9/85</u> BORING FINISH <u>8/1</u> 3				
PRO	JECT	CAF	KUINA	L PLAN I					BC	RING START	0/9/00	_ BORING FINISH	1 <u>0</u>	13/05
SAMPLE	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL / ROCK	1	WELL	DRILLER'S NOTES
25	NW	255.0	265.0		10.0		255 - -							
							260 - - -							
		265.0	265.0				265 -							
15														
AEP CD SI.GPJ AEP.GDT 7/20/15														
AEP CD SI.G														



Client: **AEP-Cardinal** Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-1

Page: 1 of 10

Drilling Start Date: 03/11/2016 14:00 Boring Depth (ft): 198 Well Depth (ft): 132 Boring Diameter (in): Drilling End Date: 03/17/2016 15:00 Well Diameter (in): 2 6 Sampling Method(s): Drilling Company: **Layne Drilling Rock Core** Screen Slot (in): 0.010

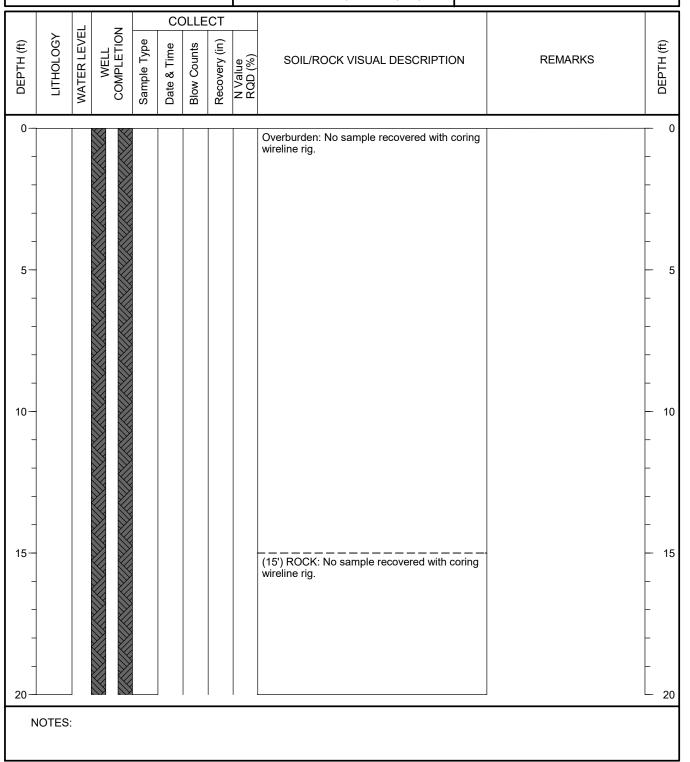
DTW During Drilling (ft): Drilling Method: **Rock Core**

Ground Surface Elev. (ft): 988.68 Drilling Equipment: CS1500 Wireline Rig Driller: **Bill Womack** Top of Casing Elev. (ft): 991.87

Logged By: **Doug Mateas** Location (X,Y): N 832,687.2 E 2,518,763.6

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC Seal Material(s): **Bentonite Pellets** Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-1

Page: 2 of 10

Drilling Start Date: 03/11/2016 14:00

Drilling End Date: 03/17/2016 15:00

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig
Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 198

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 988.68

Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

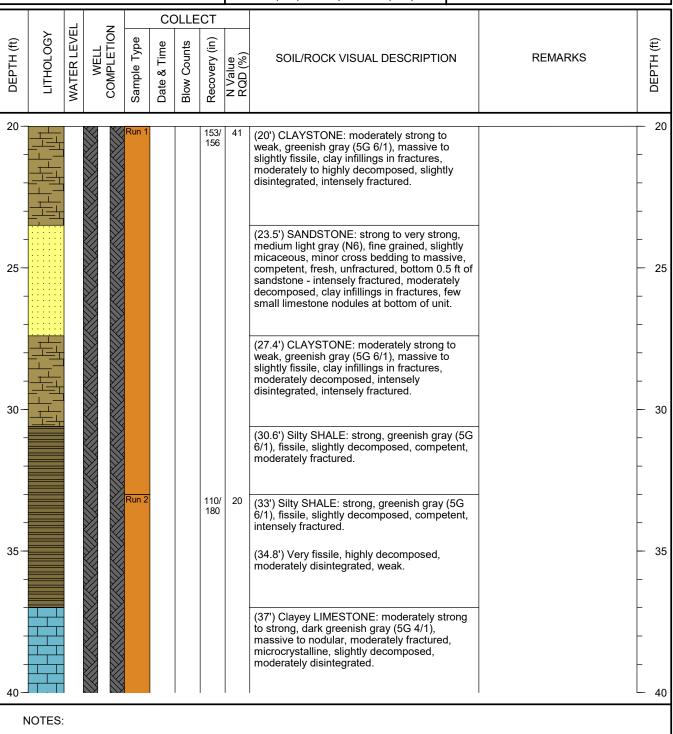
Well Depth (ft): 132

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-1

Page: 3 of 10

03/11/2016 14:00 Drilling Start Date: Drilling End Date: 03/17/2016 15:00 Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas** Boring Depth (ft): 198

Boring Diameter (in): 6

Sampling Method(s): **Rock Core** DTW During Drilling (ft):

Ground Surface Elev. (ft): 988.68 Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

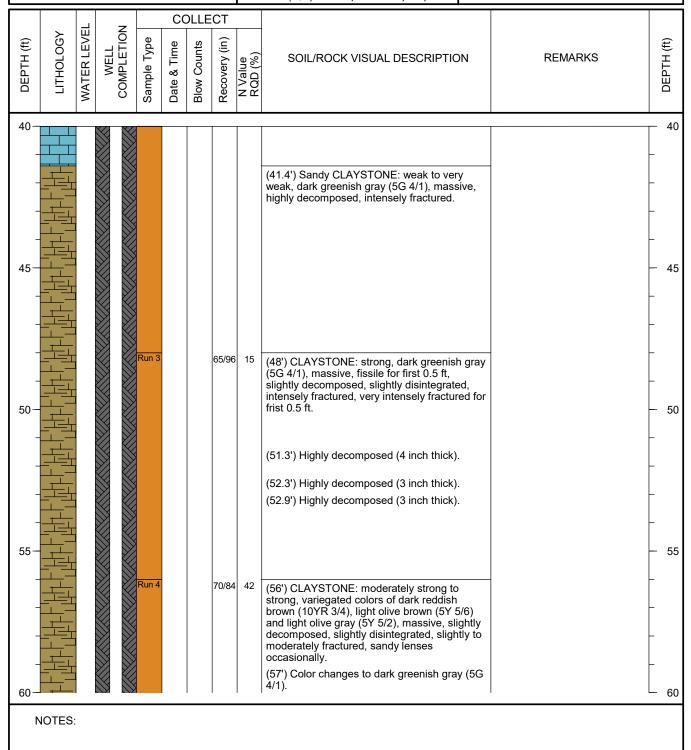
Well Depth (ft): 132

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material: Seal Material(s): **Bentonite Pellets**

Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-1

Page: 4 of 10

Well Depth (ft): **Drilling Start Date:** 03/11/2016 14:00 Boring Depth (ft): 132 198 Drilling End Date: 03/17/2016 15:00 Boring Diameter (in): Well Diameter (in): 2 6 Screen Slot (in): Drilling Company: 0.010 Layne Drilling Sampling Method(s): **Rock Core**

Drilling Method: Rock Core DTW During Drilling (ft):

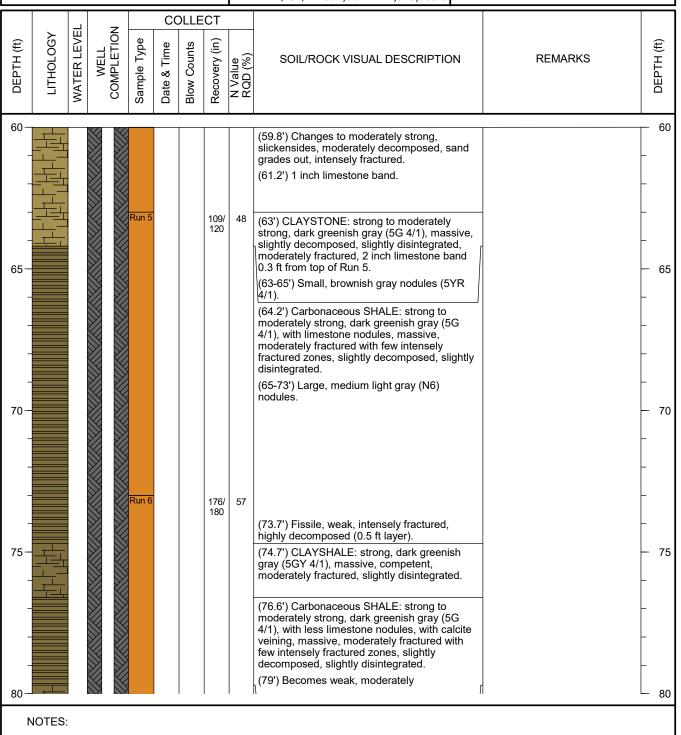
Drilling Equipment: CS1500 Wireline Rig

Driller: Ground Surface Elev. (ft): 988.68

Top of Casing Elev. (ft): 991.87

Logged By: Doug Mateas Location (X,Y): N 832,687.2 E 2,518,763.6

Riser Material: Sch 40 PVC
Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-1

Page: 5 of 10

Drilling Start Date: 03/11/2016 14:00
Drilling End Date: 03/17/2016 15:00

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Boring Depth (ft):

Ground Surface Elev. (ft): 988.68

Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

198

Well Depth (ft): 132
Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand

COLLECT WELL COMPLETION WATER LEVEL LITHOLOGY Sample Type Recovery (in) DEPTH (ft) DEPTH (ft) Date & Time **Blow Counts** N Value RQD (%) SOIL/ROCK VISUAL DESCRIPTION **REMARKS** 80 80 decomposed, fissile, clay infillings in fractures, intensely fractured. (79.7') Silty CLAYSHALE: strong, dark greenish gray (5G 4/1), massive, fresh to slightly decomposed, competent, slightly fractured. (82.7') 1 ft vertical fracture. 85 85 168/ (88') Sandy SHALE: strong, medium bluish 180 gray (5B 5/1), fissile, fresh, competent, intensely fractured. (89') Changes to massive. 90 90 (92.8') LIMESTONE: strong to very strong, medium bluish gray (5B 5/1), massive, microcrystalline to fine grained, some silty parts, moderately fractured to intensely fractured, fresh, slightly disintegrated. (may 95 be calcareous siltstone with interbedded \limestone) (95.3') MUDSTONE: very weak to weak, greenish gray (5GY 6/1), moderately to highly decomposed, very intensely fractured. (96.3') Calcareous SILTSTONE: 0.5 vertical fracture 15 ft from bottom of run, massive to nodular. (see previous limestone description) 100 100 NOTES:



Drilling Company:

Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

Rock Core

Sampling Method(s):

BORING LOG

Boring/Well No. M-GS-1 Page: 6 of 10

 Drilling Start Date:
 03/11/2016 14:00
 Boring Depth (ft):
 198
 W

 Drilling End Date:
 03/17/2016 15:00
 Boring Diameter (in):
 6
 W

Drilling Method: Rock Core DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 988.68

Layne Drilling

Driller: Bill Womack Top of Casing Elev. (ft): 991.87

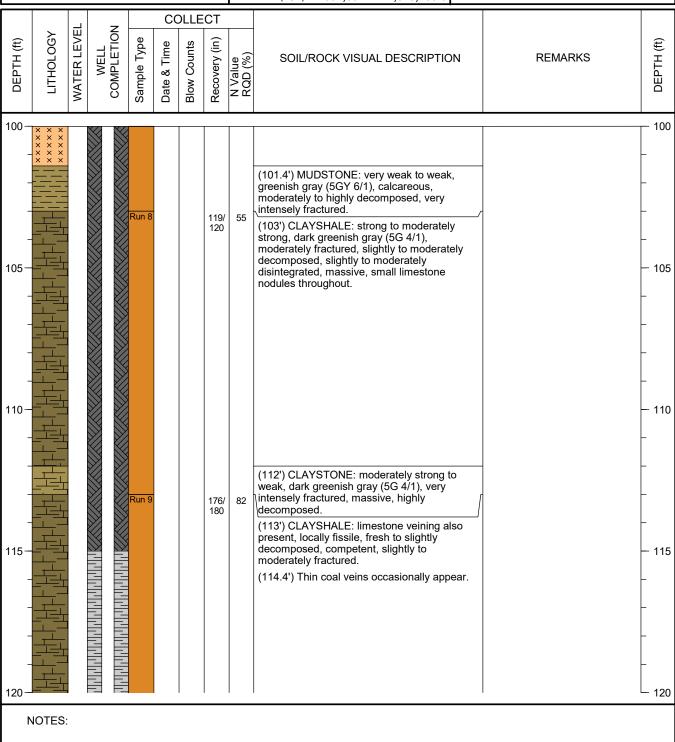
Logged By: Doug Mateas Location (X,Y): N 832,687.2 E 2,518,763.6

Well Depth (ft): 132

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-1

Page: 7 of 10

Drilling Start Date: 03/11/2016 14:00
Drilling End Date: 03/17/2016 15:00

Drilling Company: Layne Drilling
Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 198

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 988.68

Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

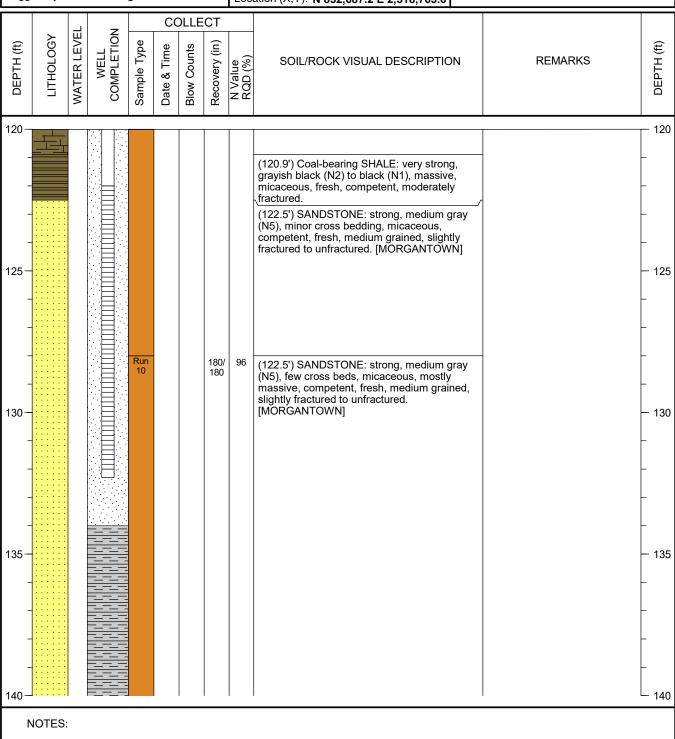
Well Depth (ft): 132
Well Diameter (in): 2

Screen Slot (in):

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand

0.010





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-1

Page: 8 of 10

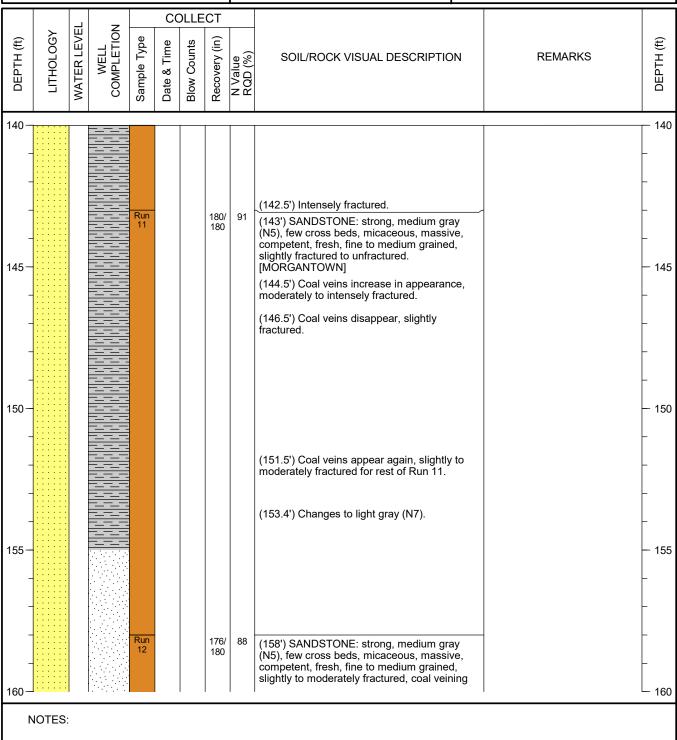
03/11/2016 14:00 Well Depth (ft): **Drilling Start Date:** Boring Depth (ft): 198 132 Drilling End Date: 03/17/2016 15:00 Boring Diameter (in): Well Diameter (in): 2 6 Drilling Company: **Layne Drilling** Screen Slot (in): 0.010 Sampling Method(s): **Rock Core** Riser Material: Sch 40 PVC

Drilling Method: **Rock Core** DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 988.68 Driller: **Bill Womack** Top of Casing Elev. (ft): 991.87

Logged By: **Doug Mateas** Location (X,Y): N 832,687.2 E 2,518,763.6

Pre-packed Sch 40 PVC Screen Material:





Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-1

Page: 9 of 10

Drilling Start Date: 03/11/2016 14:00

Drilling End Date: 03/17/2016 15:00 Drilling Company: Layne Drilling

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas**

DTW During Drilling (ft): Ground Surface Elev. (ft): 988.68 Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

198

Rock Core

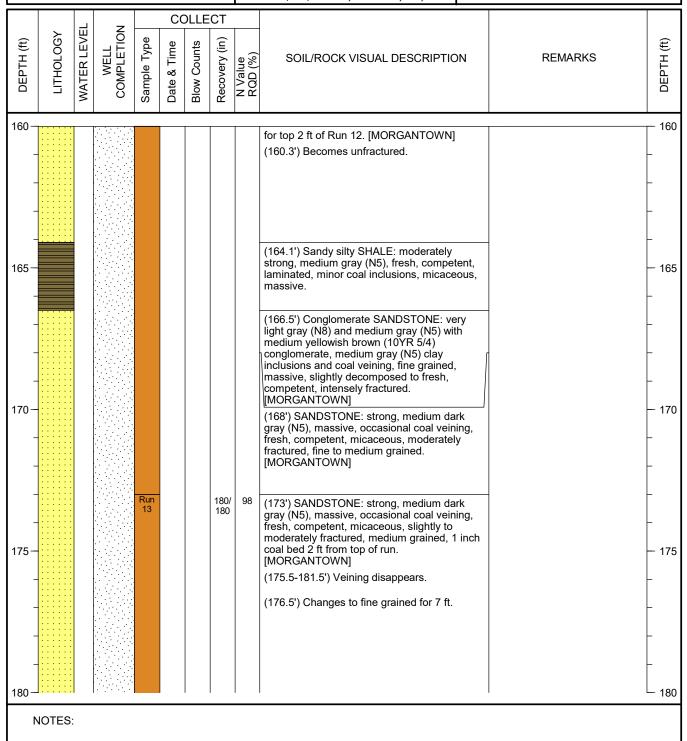
6

Well Depth (ft): 132

Well Diameter (in): 2

Screen Slot (in): 0.010 Sch 40 PVC Riser Material:

Pre-packed Sch 40 PVC Screen Material:





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-1

Page: 10 of 10

Drilling Start Date: 03/11/2016 14:00
Drilling End Date: 03/17/2016 15:00
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Sampling Method(s):
DTW During Drilling (ft):

Boring Diameter (in):

Boring Depth (ft):

Ground Surface Elev. (ft): 988.68

Top of Casing Elev. (ft): 991.87

Location (X,Y): N 832,687.2 E 2,518,763.6

198

Rock Core

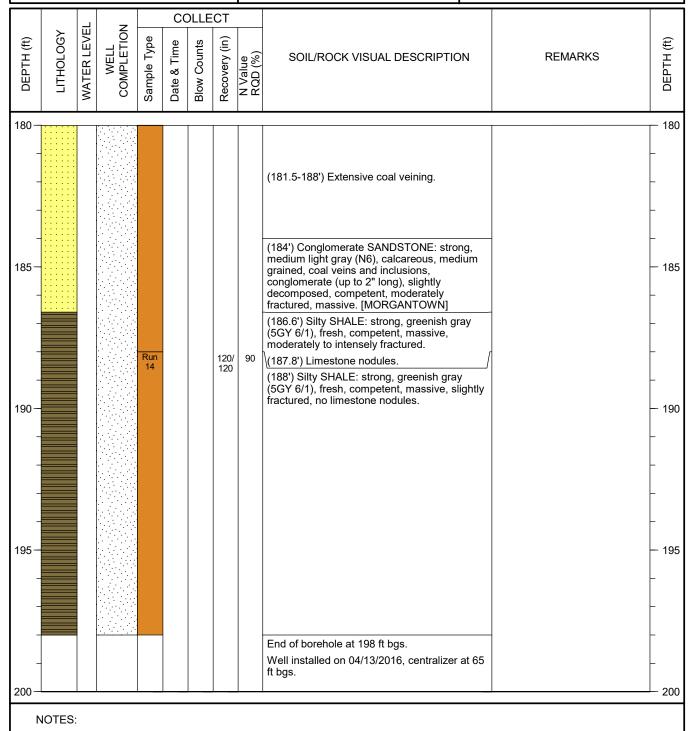
6

Well Depth (ft): 132

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-2

Page: 1 of 11

03/22/2016 08:30 Well Depth (ft): 140 Drilling Start Date: 209 Boring Depth (ft): Drilling End Date: 03/23/2016 09:45 Boring Diameter (in): Well Diameter (in): 2 6 Drilling Company: **Layne Drilling** Screen Slot (in): **Rock Core** Sampling Method(s):

Drilling Method: **Rock Core** DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 987.62

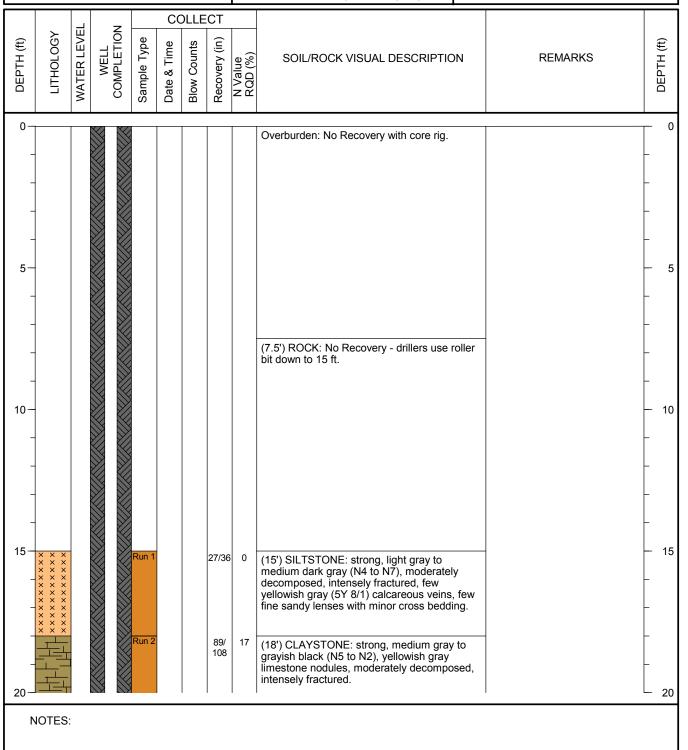
Driller: **Bill Womack** Top of Casing Elev. (ft): 990.81 Logged By: **Chad Gregory**

Location (X,Y): N 832,174.6 E 2,519,357.6

0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-2

Page: 2 of 11

Drilling Start Date: 03/22/2016 08:30

Drilling End Date: 03/23/2016 09:45
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): **987.62**Top of Casing Elev. (ft): **990.81**

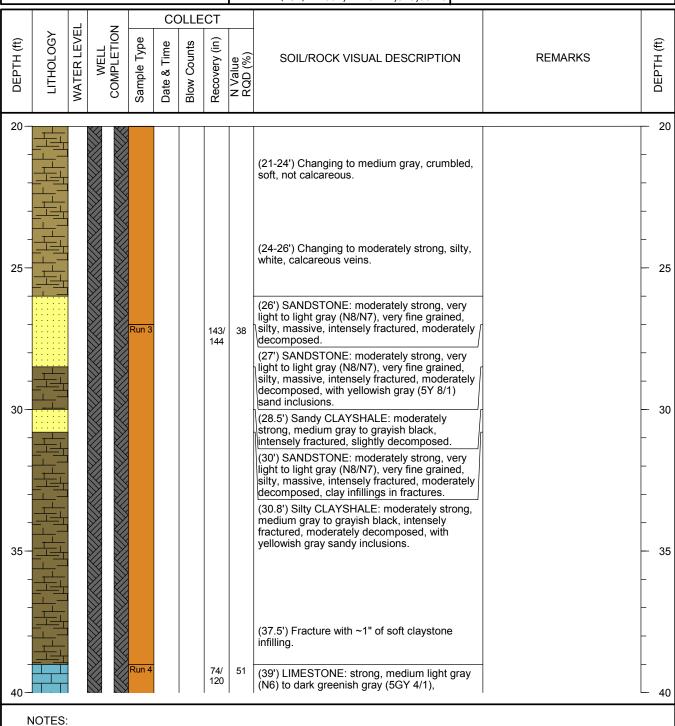
Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-2

Page: 3 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209
Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): **987.62**Top of Casing Elev. (ft): **990.81**

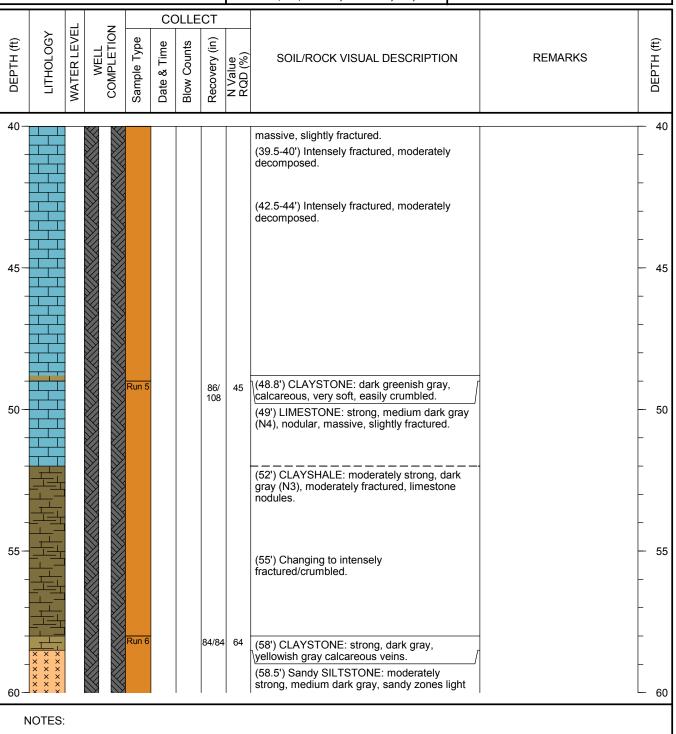
Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-2 Page: 4 of 11

Drilling Start Date: 03/22/2016 08:30

Drilling End Date: 03/23/2016 09:45
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Diameter (in): Sampling Method(s):

Boring Depth (ft):

Sampling Method(s): Rock Core
DTW During Drilling (ft):

Ground Surface Elev. (ft): 987.62

Top of Casing Elev. (ft): 990.81

Location (X,Y): N 832,174.6 E 2,519,357.6

209

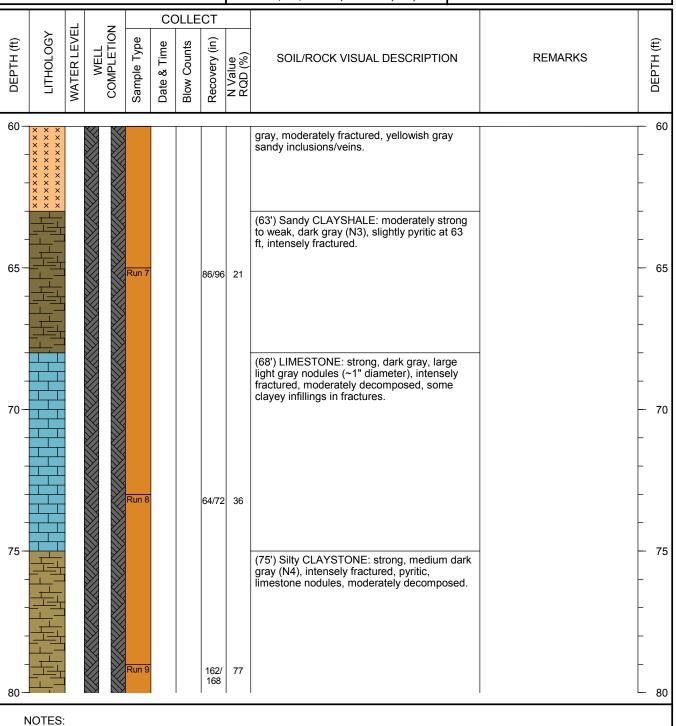
6

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-2

Page: 5 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209

Boring Diameter (in): 6

DTW During Drilling (ft):

Sampling Method(s):

Ground Surface Elev. (ft): **987.62**Top of Casing Elev. (ft): **990.81**

Location (X,Y): N 832,174.6 E 2,519,357.6

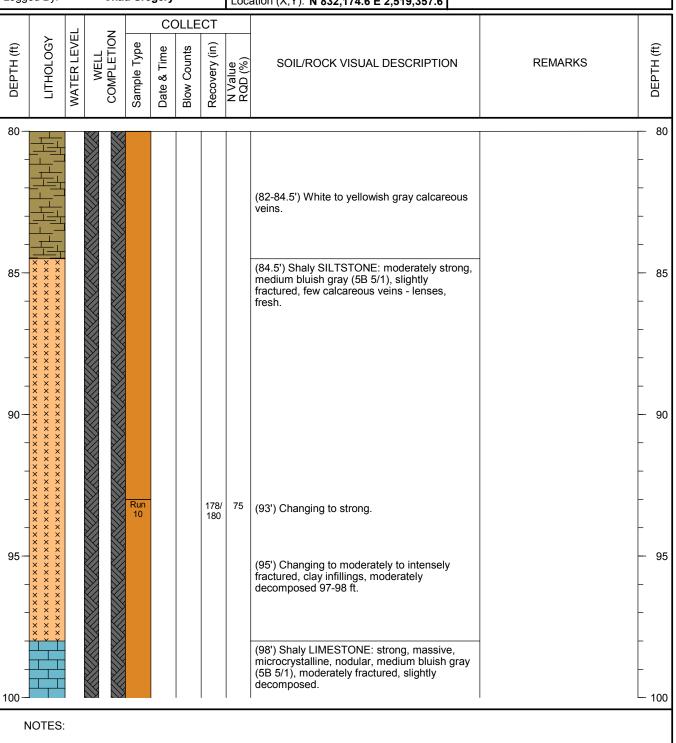
Rock Core

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

DTW During Drilling (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-2

Page: 6 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Ground Surface Elev. (ft): 987.62

Top of Casing Elev. (ft): 990.81

Location (X,Y): N 832,174.6 E 2,519,357.6

209

Rock Core

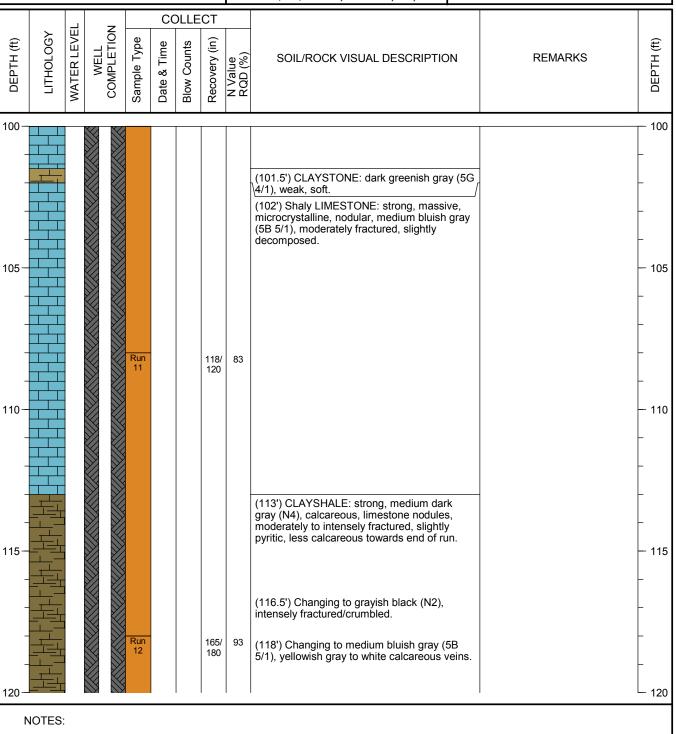
6

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-2

Page: 7 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209
Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): **987.62**Top of Casing Elev. (ft): **990.81**

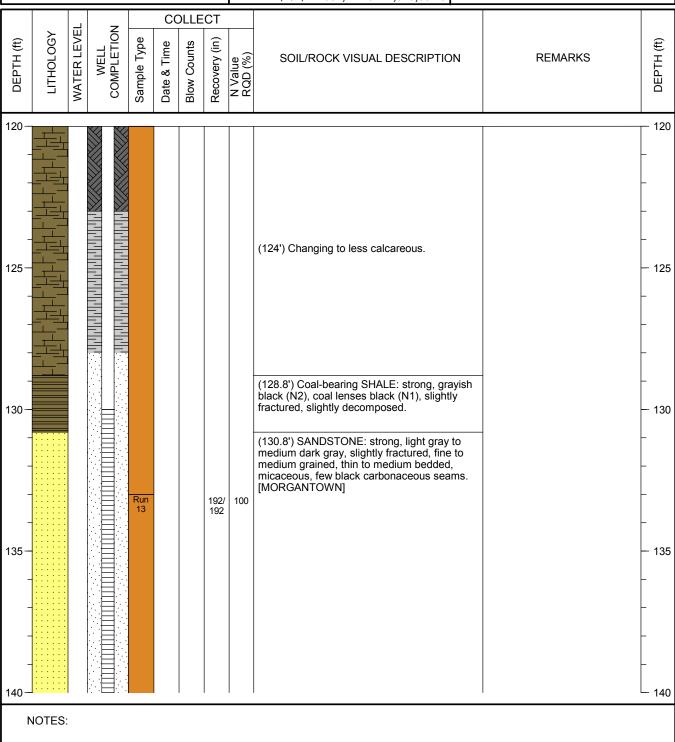
Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140
Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-2

Page: 8 of 11

Drilling Start Date: 03/22/2016 08:30 Boring Depth (ft): 209 Drilling End Date: 03/23/2016 09:45 Boring Diameter (in): 6 Drilling Company: **Layne Drilling Rock Core** Sampling Method(s):

Drilling Method: **Rock Core** DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 987.62

Driller: **Bill Womack** Top of Casing Elev. (ft): 990.81

Logged By: **Chad Gregory** Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC Seal Material(s): **Bentonite Pellets** Filter Pack:

#5 Medium Coarse Sand

COLLECT WELL COMPLETION WATER LEVEL LITHOLOGY Sample Type DEPTH (ft) Recovery (in) Date & Time Blow Counts DEPTH (ft) N Value RQD (%) SOIL/ROCK VISUAL DESCRIPTION REMARKS 140 140 145-145 180/ 180 98 150 - 150 155 - 155 (157-159') Mud inclusions. 160 160 NOTES:



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-2

Page: 9 of 11

Drilling Start Date: 03/22/2016 08:30

Drilling End Date: 03/23/2016 09:45
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209

Boring Diameter (in): 6

Sampling Method(s): Rock Core
DTW During Drilling (ft):

Ground Surface Elev. (ft): **987.62**

Top of Casing Elev. (ft): 990.81

Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140

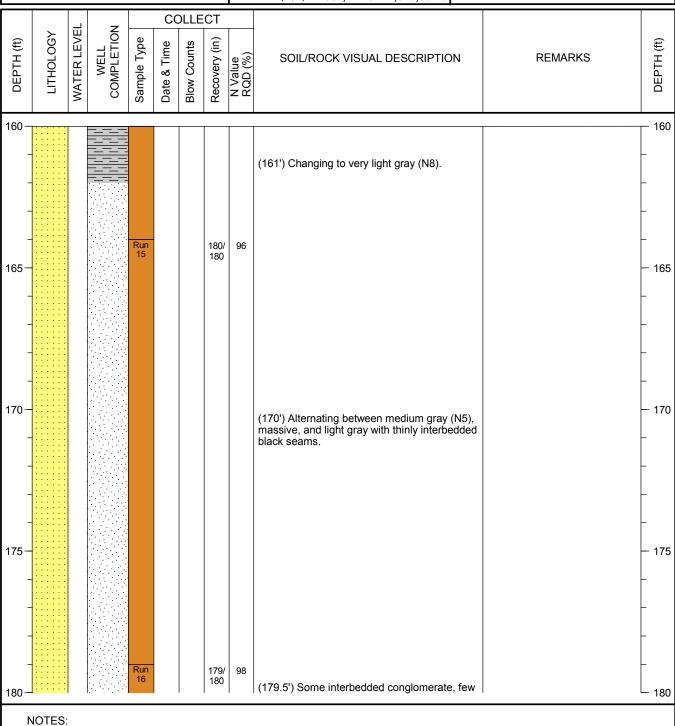
Well Diameter (in): 2

Screen Slot (in):

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

0.010





Drilling Method:

Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-2

Page: 10 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45

Drilling Company: Layne Drilling

Drilling Equipment: CS1500 Wireline Rig

Rock Core

Driller: Bill Womack
Logged By: Chad Gregory

Boring Depth (ft): 209

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): **987.62**Top of Casing Elev. (ft): **990.81**

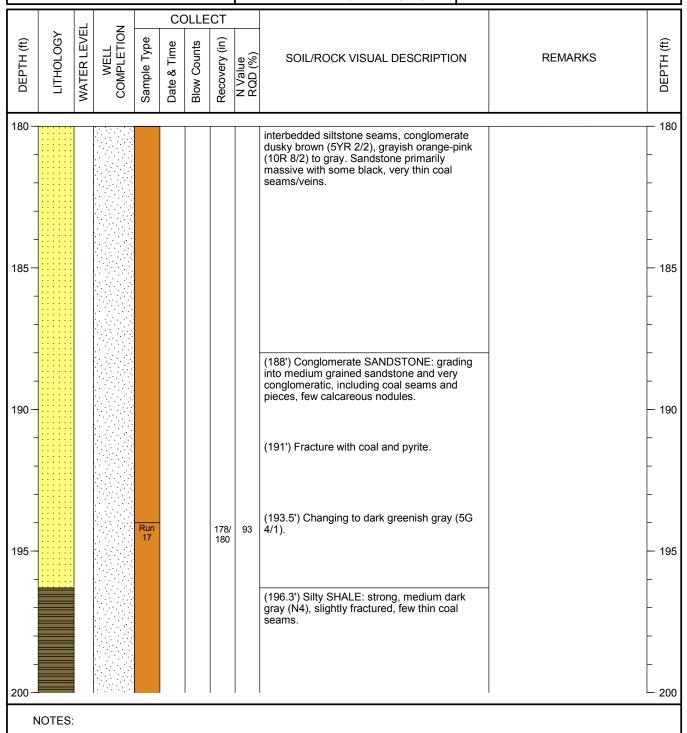
Location (X,Y): N 832,174.6 E 2,519,357.6

Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

DTW During Drilling (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-2

Page: 11 of 11

Drilling Start Date: 03/22/2016 08:30
Drilling End Date: 03/23/2016 09:45
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Chad Gregory

Ground Surface Elev. (ft): 987.62

Top of Casing Elev. (ft): 990.81

Location (X,Y): N 832,174.6 E 2,519,357.6

209

Rock Core

6

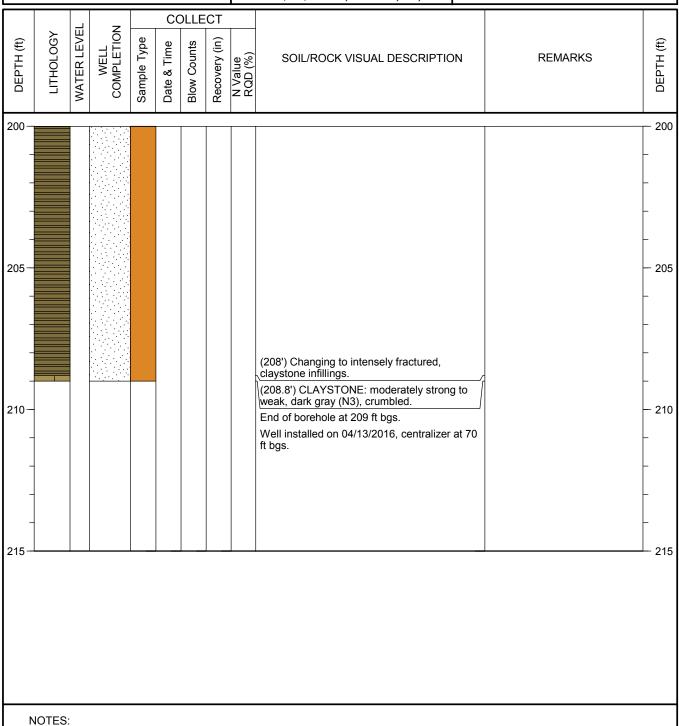
 Well Depth (ft):
 140

 Well Diameter (in):
 2

 Screen Slot (in):
 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-3

Page: 1 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

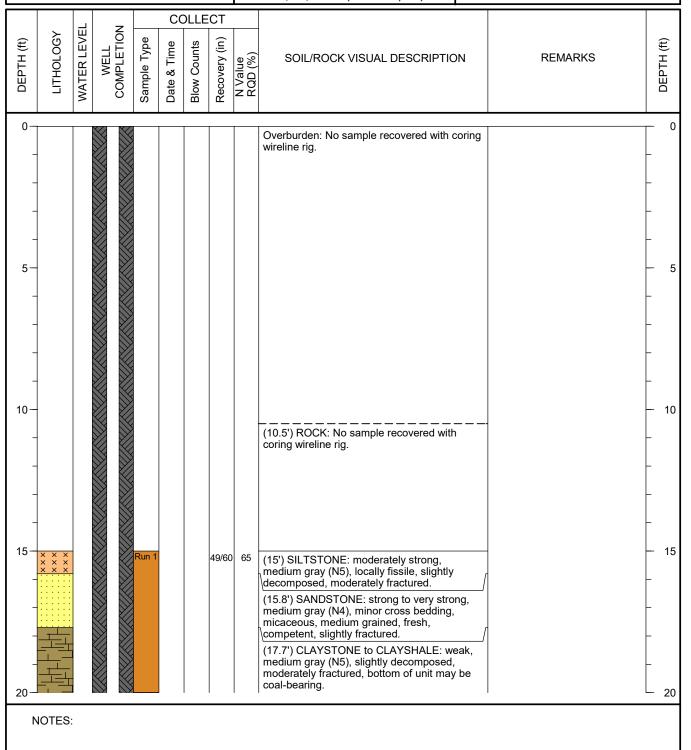
Location (X,Y): N 830,875.6 E 2,518,721.9

Well Depth (ft): 146

Well Diameter (in): 2

Screen Slot (in): 0.010
Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-3

Page: 2 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

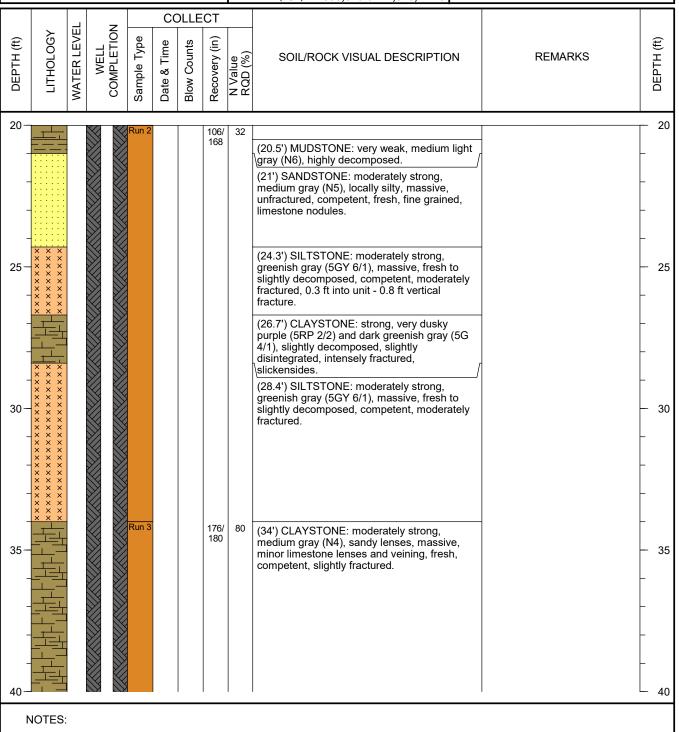
Location (X,Y): **N** 830,875.6 E 2,518,721.9

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-3

Page: 3 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

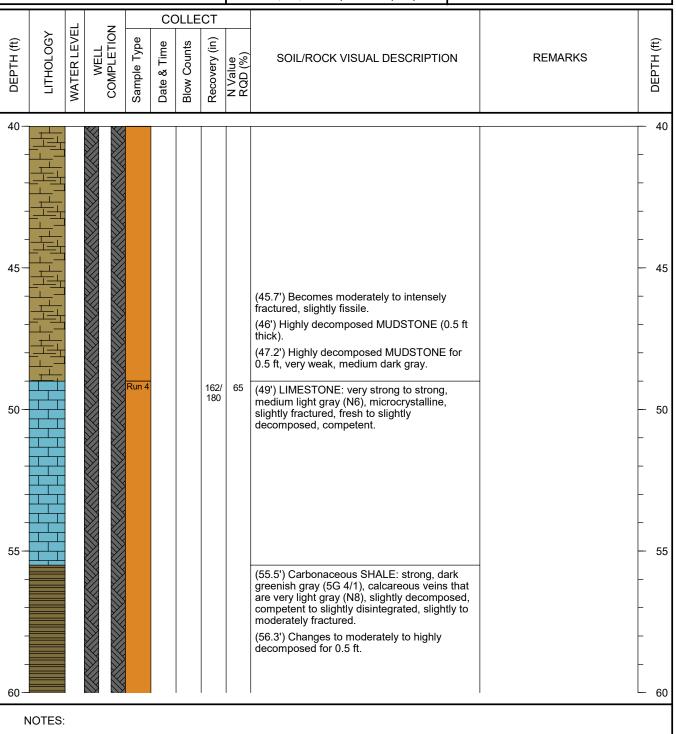
Location (X,Y): N 830,875.6 E 2,518,721.9

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-3

Page: 4 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6

Sampling Method(s): Rock Core

203.5

DTW During Drilling (ft):

Boring Depth (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

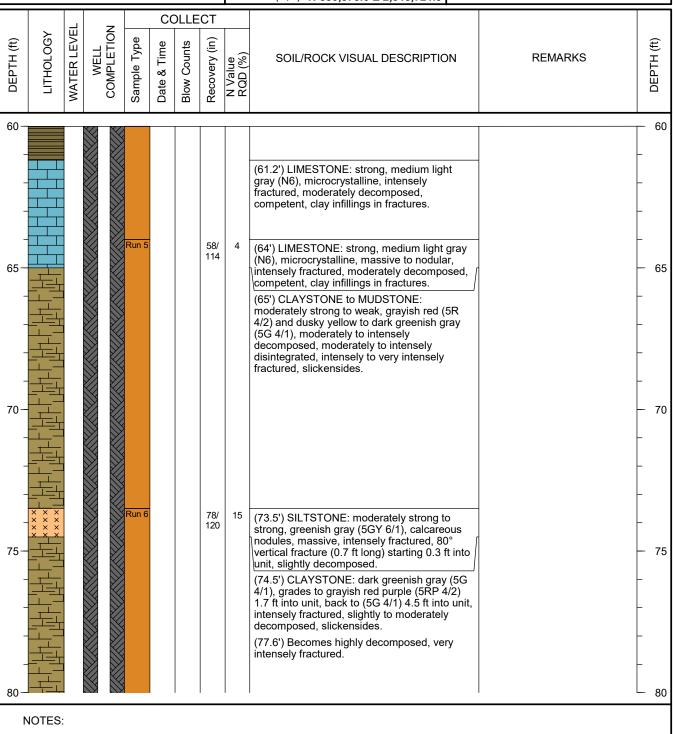
Location (X,Y): N 830,875.6 E 2,518,721.9

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-3

Page: 5 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

Location (X,Y): N 830,875.6 E 2,518,721.9

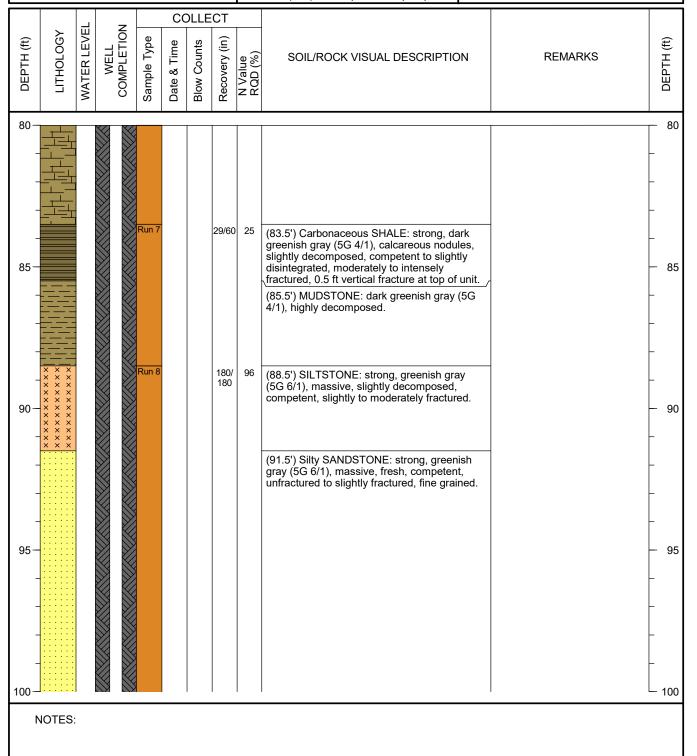
Rock Core

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-3

Page: 6 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

Location (X,Y): N 830,875.6 E 2,518,721.9

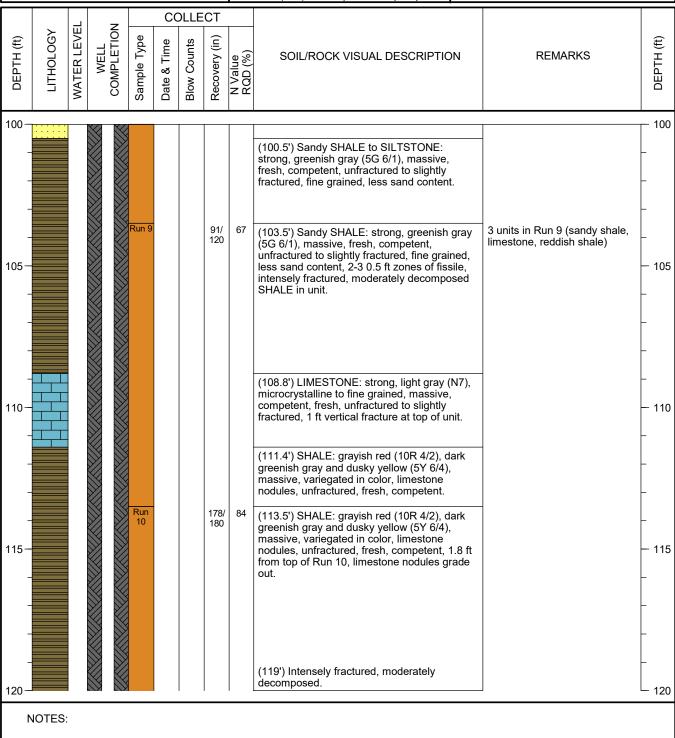
Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets

Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-3

Page: 7 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

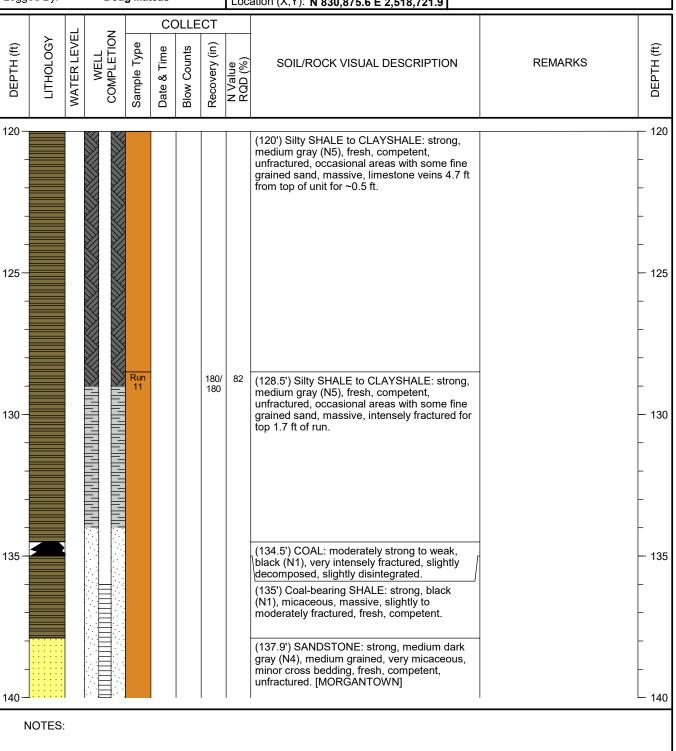
Location (X,Y): N 830,875.6 E 2,518,721.9

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-3

Page: 8 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

Location (X,Y): N 830,875.6 E 2,518,721.9

203.5

Rock Core

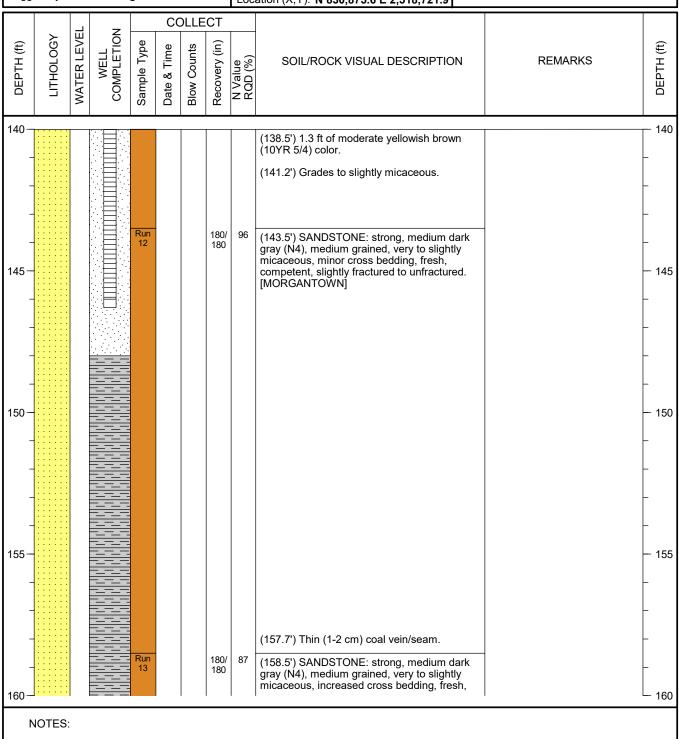
6

Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-3

Page: 9 of 11

03/10/2016 10:25 **Drilling Start Date:**

Drilling End Date: 03/11/2016 12:20 Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack**

203.5 Boring Depth (ft):

Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42

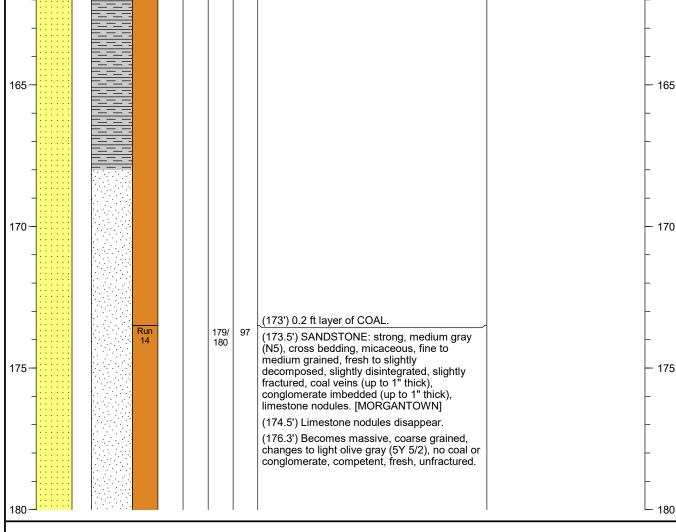
Well Depth (ft): 146

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material: **Bentonite Pellets**

Seal Material(s): Top of Casing Elev. (ft): 1,000.33 Logged By: **Doug Mateas** Filter Pack: #5 Medium Coarse Sand Location (X,Y): N 830,875.6 E 2,518,721.9 COLLECT WELL COMPLETION WATER LEVEL LITHOLOGY Sample Type Recovery (in) DEPTH (ft) DEPTH (ft) Date & Time **Blow Counts** N Value RQD (%) SOIL/ROCK VISUAL DESCRIPTION **REMARKS** 160 160 competent, slightly fractured, coal veins/seams (1-2 cm). [MORGANTOWN] 165



NOTES:



Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42

Address: 3202 Twp Rd 163, Brilliant, OH

203.5

Rock Core

6

BORING LOG Boring/Well No. M-GS-3

Page: 10 of 11

03/10/2016 10:25 **Drilling Start Date:**

Drilling End Date: 03/11/2016 12:20 Drilling Company: Layne Drilling

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

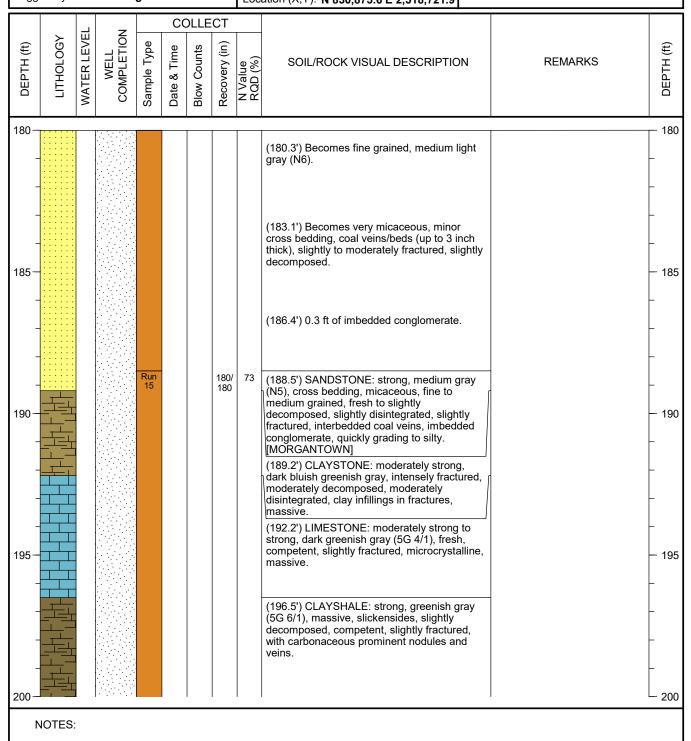
Driller: **Bill Womack** Logged By: **Doug Mateas**

Top of Casing Elev. (ft): 1,000.33 Location (X,Y): N 830,875.6 E 2,518,721.9 Well Depth (ft): 146 Well Diameter (in): 2

Screen Slot (in): 0.010

Sch 40 PVC Riser Material:

Pre-packed Sch 40 PVC Screen Material:





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-3

Page: 11 of 11

Drilling Start Date: 03/10/2016 10:25

Drilling End Date: 03/11/2016 12:20
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Daillia a Faraina and CO4500 Menals

Drilling Equipment: CS1500 Wireline Rig
Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 203.5

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 997.42
Top of Casing Elev. (ft): 1,000.33

Location (X,Y): N 830,875.6 E 2,518,721.9

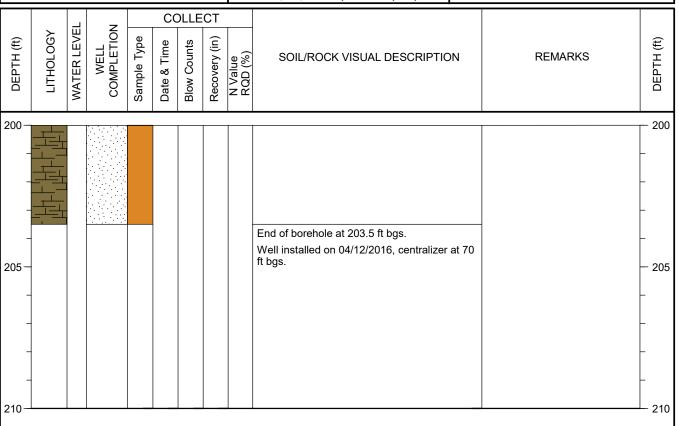
Well Depth (ft): 146

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets

Filter Pack: #5 Medium Coarse Sand



NOTES:



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-4

Page: 1 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

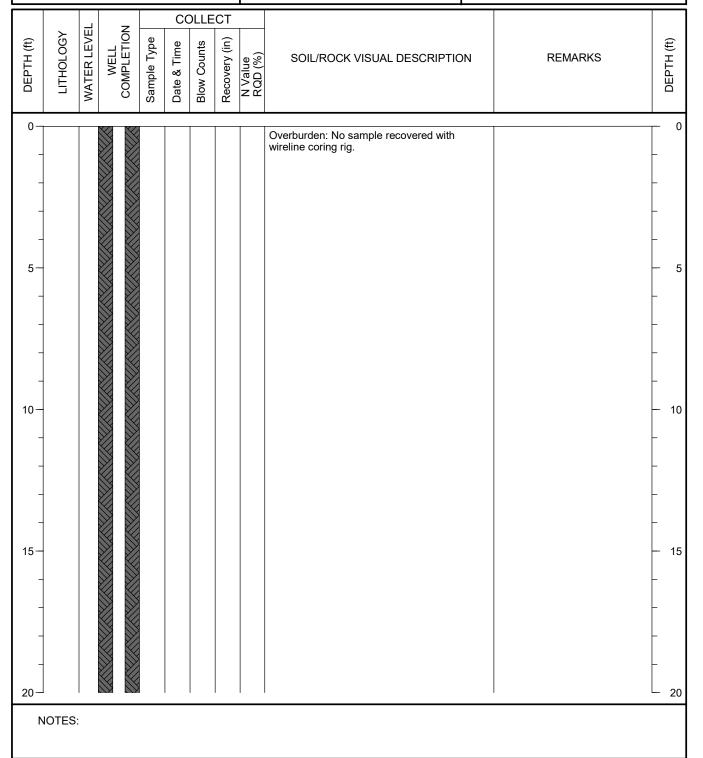
Location (X,Y): N 834,146.7 E 2,517,597.8

Well Depth (ft): 202

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4 Page: 2 of 12

03/13/2016 07:45 Well Depth (ft): 202 **Drilling Start Date:** Boring Depth (ft): 228 **Drilling End Date:** 03/14/2016 12:30 Well Diameter (in): 2 Boring Diameter (in): 6 Drilling Company: Layne Drilling

Sampling Method(s): **Rock Core** Drilling Method: **Rock Core** DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 1,025.65

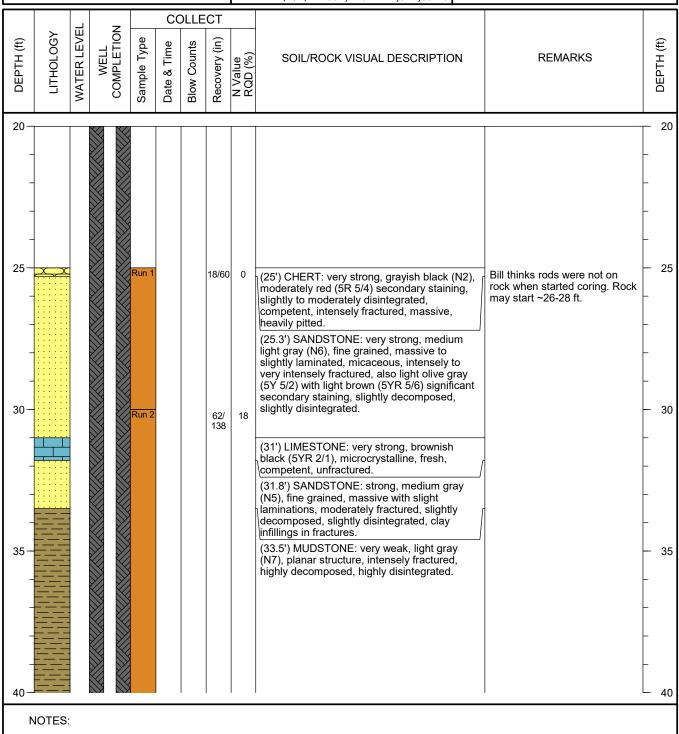
Driller: **Bill Womack** Top of Casing Elev. (ft): 1,028.73

Logged By: **Doug Mateas** Location (X,Y): N 834,146.7 E 2,517,597.8

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material: Seal Material(s): **Bentonite Pellets** Filter Pack: #5 Medium Coarse Sand





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4 Page: 3 of 12

Well Depth (ft): **Drilling Start Date:** 03/13/2016 07:45 202 228 Boring Depth (ft): Drilling End Date: 03/14/2016 12:30 Boring Diameter (in): Well Diameter (in): 2 6

Drilling Company: Layne Drilling Sampling Method(s): **Rock Core**

Drilling Method: Rock Core DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 1,025.65 Driller: **Bill Womack** Top of Casing Elev. (ft): 1,028.73

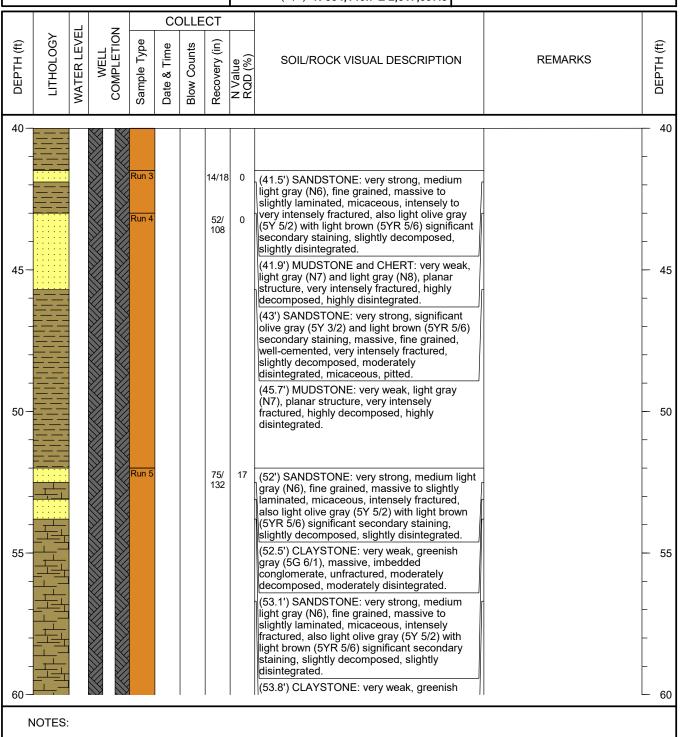
Logged By: **Doug Mateas** Location (X,Y): N 834,146.7 E 2,517,597.8

Screen Slot (in): 0.010

Sch 40 PVC Riser Material:

Screen Material: Pre-packed Sch 40 PVC **Bentonite Pellets**

Seal Material(s): Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4

Page: 4 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

Location (X,Y): N 834,146.7 E 2,517,597.8

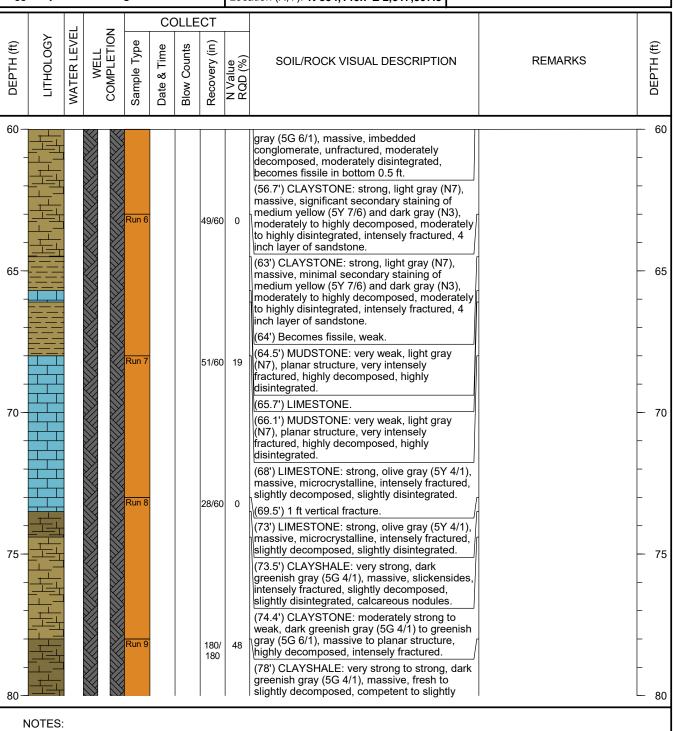
Well Depth (ft): 202

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-4

Page: 5 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6
Sampling Method(s): Rock Core

228

DTW During Drilling (ft):

Boring Depth (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

Location (X,Y): N 834,146.7 E 2,517,597.8

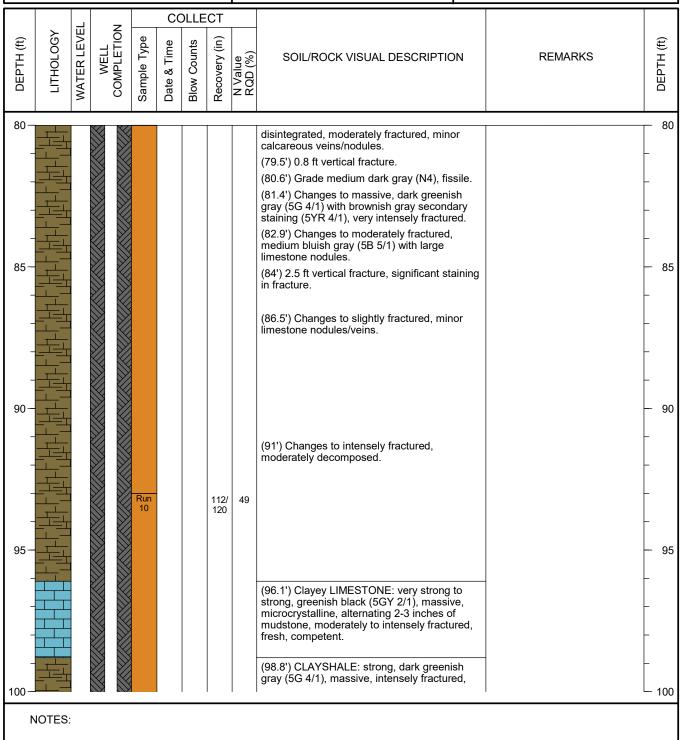
Well Depth (ft): 202

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-4

Page: 6 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65

Top of Casing Elev. (ft): 1,028.73

Location (X,Y): N 834,146.7 E 2,517,597.8

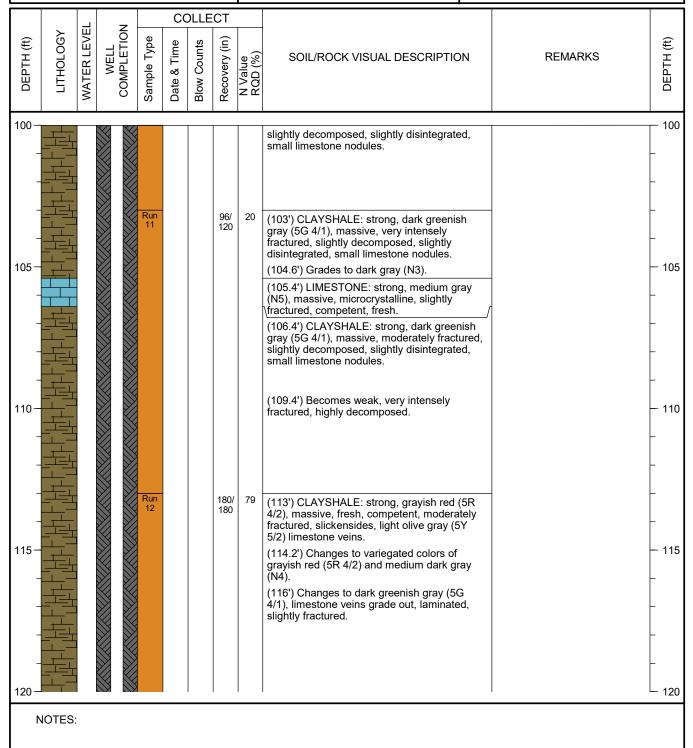
Well Depth (ft): 202

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4

Page: 7 of 12

03/13/2016 07:45 Drilling Start Date: Drilling End Date: 03/14/2016 12:30

Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas** Boring Depth (ft): 228

Boring Diameter (in): 6 Sampling Method(s):

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65 Top of Casing Elev. (ft): 1,028.73

Location (X,Y): N 834,146.7 E 2,517,597.8

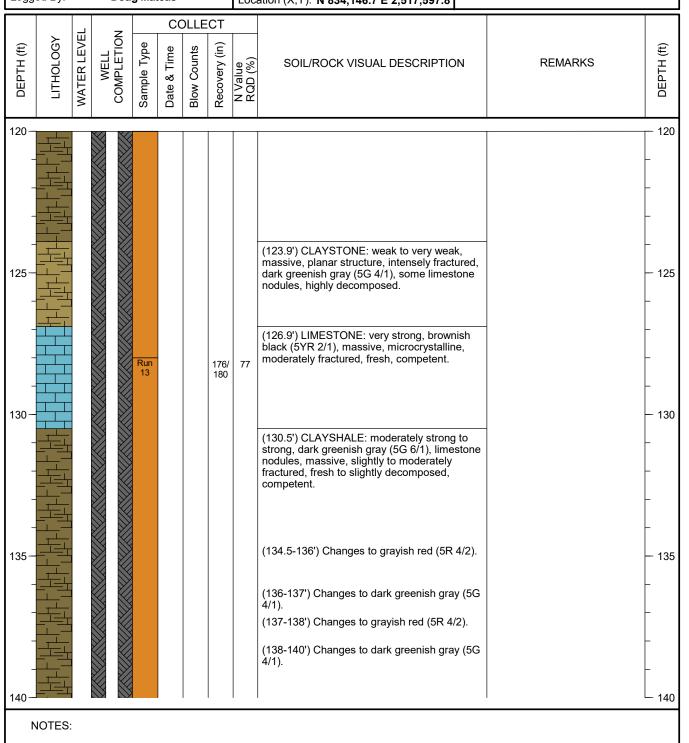
Rock Core

Well Depth (ft): 202

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material: Seal Material(s): **Bentonite Pellets** Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4 Page: 8 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6

228

Sampling Method(s): Rock Core
DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

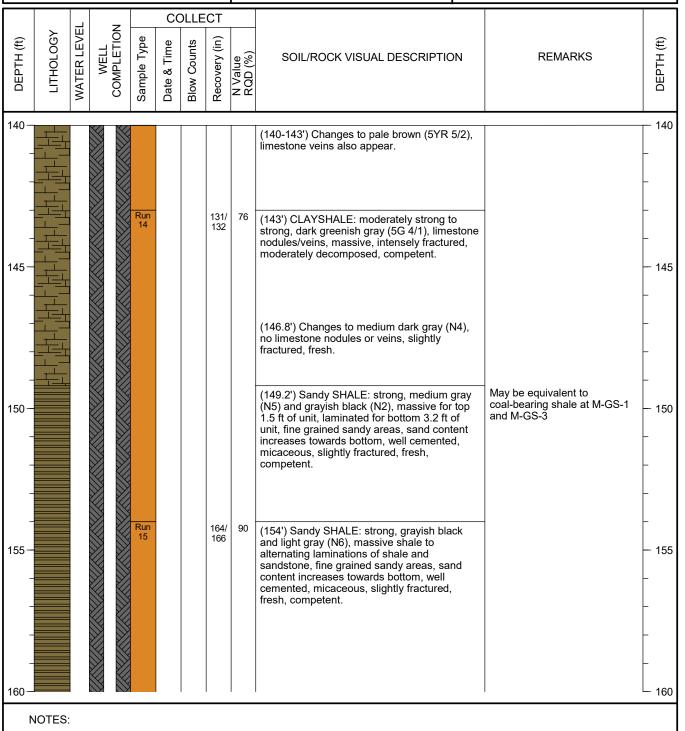
Location (X,Y): N 834,146.7 E 2,517,597.8

Well Depth (ft): 202

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-4

Page: 9 of 12

Drilling Start Date: 03/13/2016 07:45
Drilling End Date: 03/14/2016 12:30

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65

Top of Casing Elev. (ft): 1,028.73

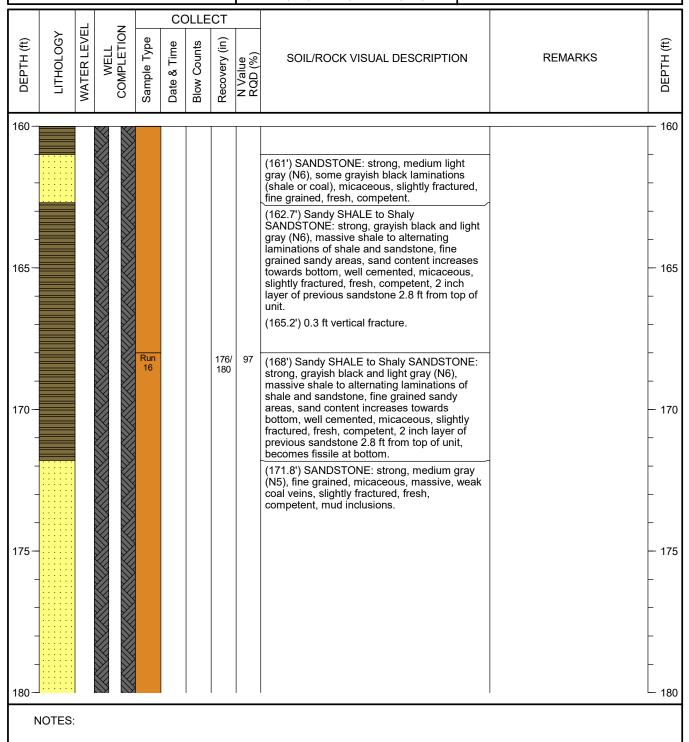
Location (X,Y): N 834,146.7 E 2,517,597.8

Well Depth (ft): 202

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC
Screen Material: Pre-packed Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-4

Page: 10 of 12

03/13/2016 07:45 Drilling Start Date:

Drilling End Date: 03/14/2016 12:30 Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas**

Well Depth (ft): 202 Boring Depth (ft): 228 Well Diameter (in): 2 Boring Diameter (in): 6 Sampling Method(s):

Screen Slot (in): 0.010 **Rock Core**

Riser Material: Sch 40 PVC DTW During Drilling (ft):

Pre-packed Sch 40 PVC Screen Material: Ground Surface Elev. (ft): 1,025.65 Seal Material(s): **Bentonite Pellets** Top of Casing Elev. (ft): 1,028.73 Filter Pack: #5 Medium Coarse Sand Location (X,Y): N 834,146.7 E 2,517,597.8

COLLECT WELL COMPLETION WATER LEVEL LITHOLOGY Sample Type Recovery (in) DEPTH (ft) DEPTH (ft) Date & Time **Blow Counts** N Value RQD (%) SOIL/ROCK VISUAL DESCRIPTION REMARKS 180 180 180/ (183') SANDSTONE: strong, medium gray 180 (N5), fine grained, micaceous, massive, weak coal veins, slightly fractured, fresh, competent. 185 185 (187.4') Becomes medium grained. (188.8') Becomes moderately fractured. 190 - 190 (192.5') Coal veins increase for rest of run. 195 - 195 (196.3') Mud inclusions begin to appear. Run 18 179/ 180 (198.3') Conglomerate SANDSTONE: strong, very light gray (N6), massive, micaceous, abundant imbedded conglomerates (up to 1.5 inch thick), mud inclusions, slightly fractured, 200 200 NOTES:



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-4

Page: 11 of 12

Drilling Start Date: 03/13/2016 07:45

Drilling End Date: 03/14/2016 12:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

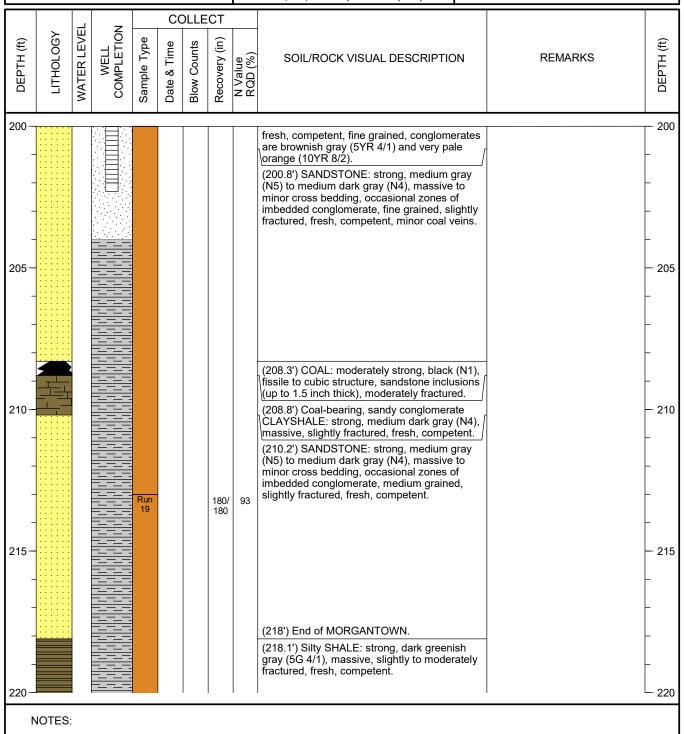
Location (X,Y): N 834,146.7 E 2,517,597.8

Well Depth (ft): 202

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC
Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-4

Page: 12 of 12

Drilling Start Date: 03/13/2016 07:45
Drilling End Date: 03/14/2016 12:30

Drilling Company: Layne Drilling
Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 228

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,025.65
Top of Casing Elev. (ft): 1,028.73

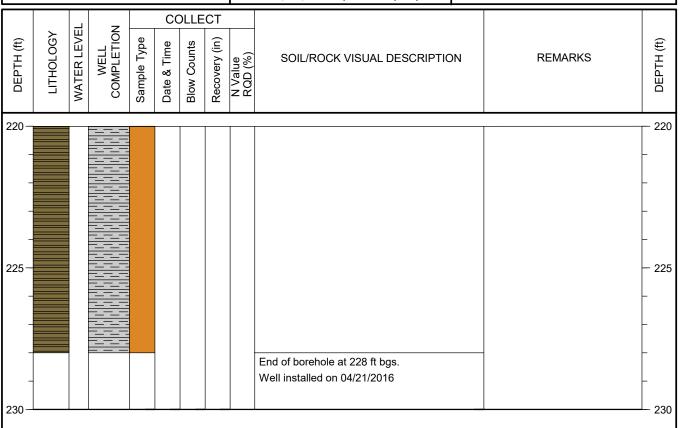
Location (X,Y): N 834,146.7 E 2,517,597.8

Well Depth (ft): 202
Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand



NOTES:



Driller:

Client: **AEP-Cardinal** Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

Top of Casing Elev. (ft): 1,039.54

BORING LOG

Boring/Well No. M-GS-5 Page: 1 of 12

Drilling Start Date: 03/14/2016 14:45 Well Depth (ft): 224 Boring Depth (ft): 233 Drilling End Date: 03/16/2016 09:30 Well Diameter (in): 2 Boring Diameter (in): 6 Drilling Company: **Layne Drilling**

Sampling Method(s): **Rock Core** Drilling Method: **Rock Core** DTW During Drilling (ft):

Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 1,036.92

Bill Womack

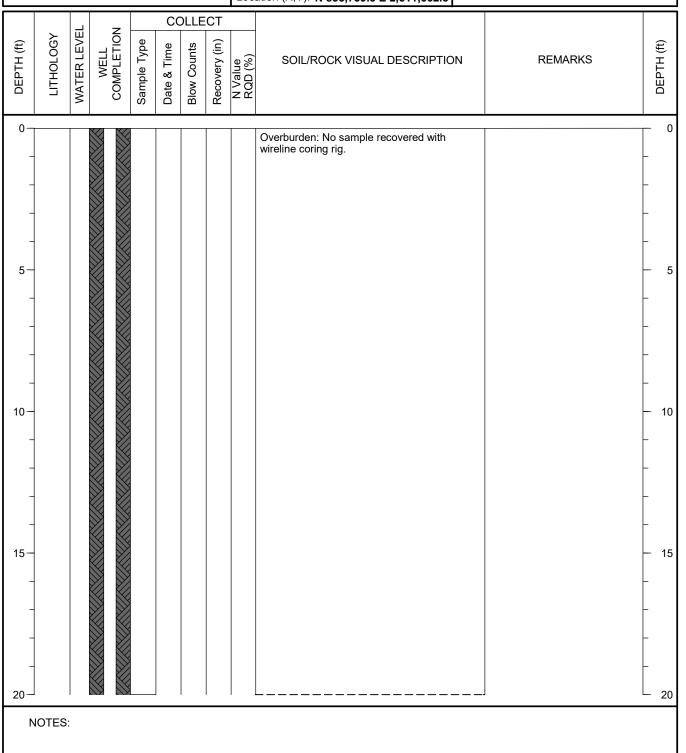
Logged By: **Doug Mateas** Location (X,Y): N 835,739.3 E 2,511,662.3

Screen Slot (in): 0.010

Riser Material:

Screen Material: Pre-packed Sch 40 PVC

Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5

Page: 2 of 12

03/14/2016 14:45 **Drilling Start Date:** Drilling End Date: 03/16/2016 09:30

Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core** Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack**

Logged By: **Doug Mateas** Boring Depth (ft): 233

Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92 Top of Casing Elev. (ft): 1,039.54

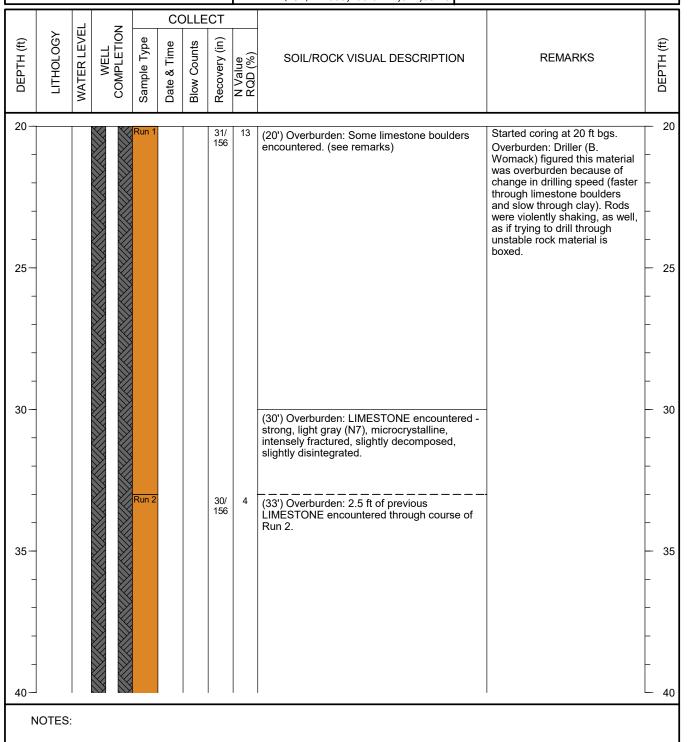
Location (X,Y): N 835,739.3 E 2,511,662.3

Well Depth (ft): 224

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-5

Page: 3 of 12

Drilling Start Date: 03/14/2016 14:45
Drilling End Date: 03/16/2016 09:30

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 233

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92
Top of Casing Elev. (ft): 1,039.54

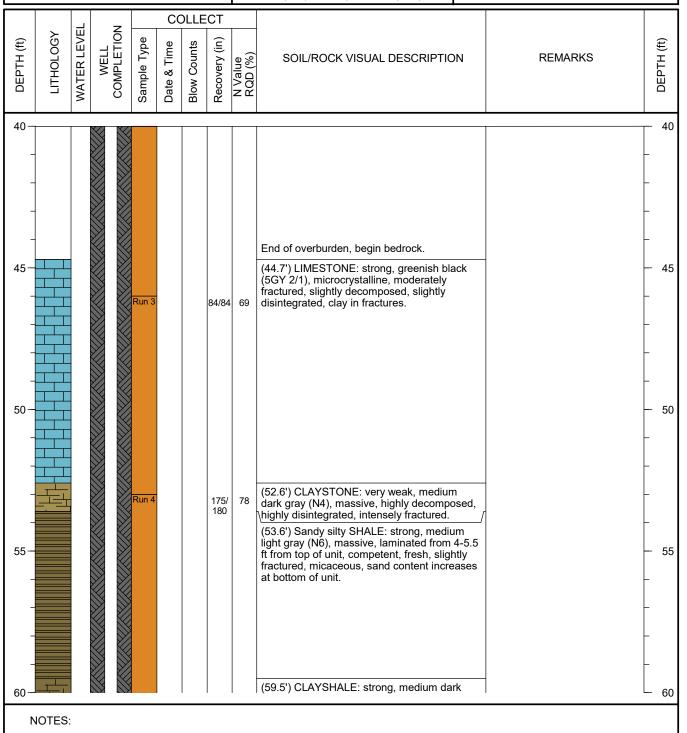
Location (X,Y): N 835,739.3 E 2,511,662.3

Well Depth (ft): 224

Well Diameter (in): 2

Screen Slot (in): 0.010
Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5

Page: 4 of 12

Drilling Start Date: 03/14/2016 14:45
Drilling End Date: 03/16/2016 09:30

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6

Sampling Method(s): Rock Core

233

DTW During Drilling (ft):

Boring Depth (ft):

Ground Surface Elev. (ft): 1,036.92

Top of Casing Elev. (ft): 1,039.54

Location (X,Y): N 835,739.3 E 2,511,662.3

Well Depth (ft): 224

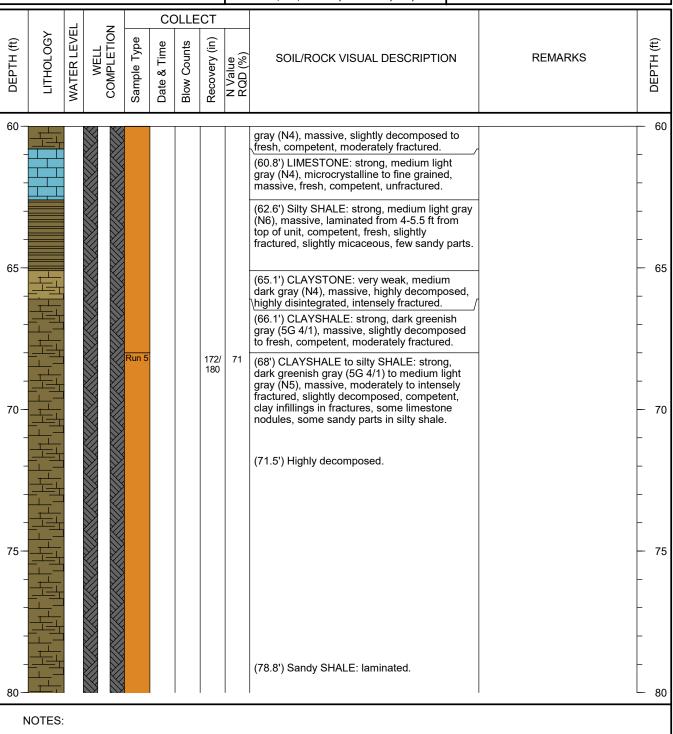
Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets

Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

DTW During Drilling (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5 Page: 5 of 12

Drilling Start Date: 03/14/2016 14:45

Drilling End Date: 03/16/2016 09:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

1500 Wireline Rig Ground Surface Elev. (ft): 1,036.92

Location (X,Y): N 835,739.3 E 2,511,662.3

Top of Casing Elev. (ft): 1,039.54

233

Rock Core

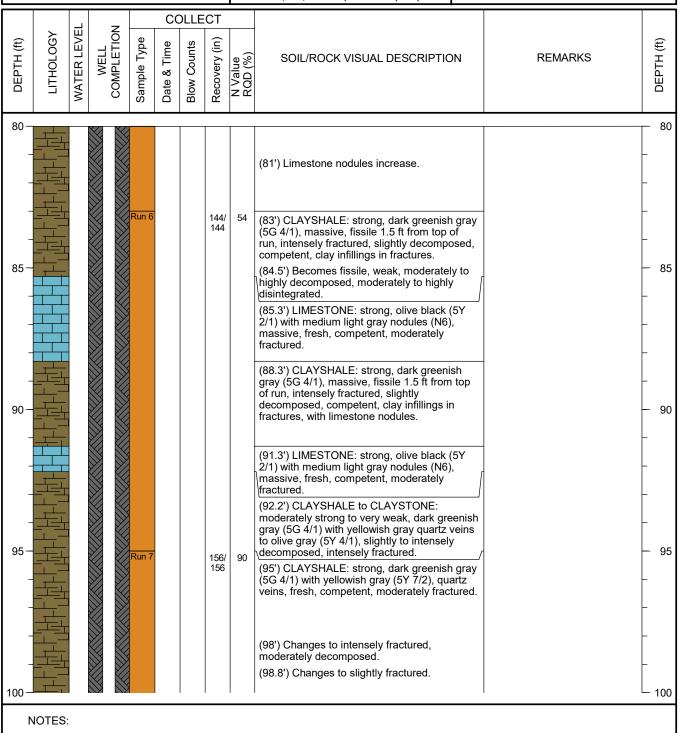
6

Well Depth (ft): 224
Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5 Page: 6 of 12

Drilling Start Date: 03/14/2016 14:45
Drilling End Date: 03/16/2016 09:30

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Diameter (in): 6

233

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92
Top of Casing Elev. (ft): 1,039.54

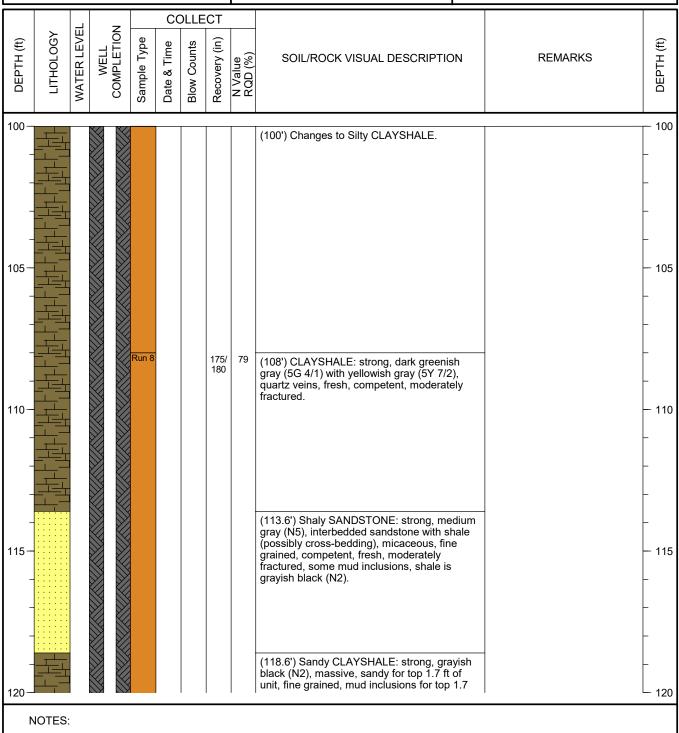
Location (X,Y): **N** 835,739.3 **E** 2,511,662.3

Well Depth (ft): 224

Well Diameter (in): 2

Screen Slot (in): 0.010
Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-5

Page: 7 of 12

Drilling Start Date: 03/14/2016 14:45

Drilling End Date: 03/16/2016 09:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 233 Well
Boring Diameter (in): 6 Well

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92

Top of Casing Elev. (ft): 1,039.54

Location (X,Y): N 835,739.3 E 2,511,662.3

Well Depth (ft): 224

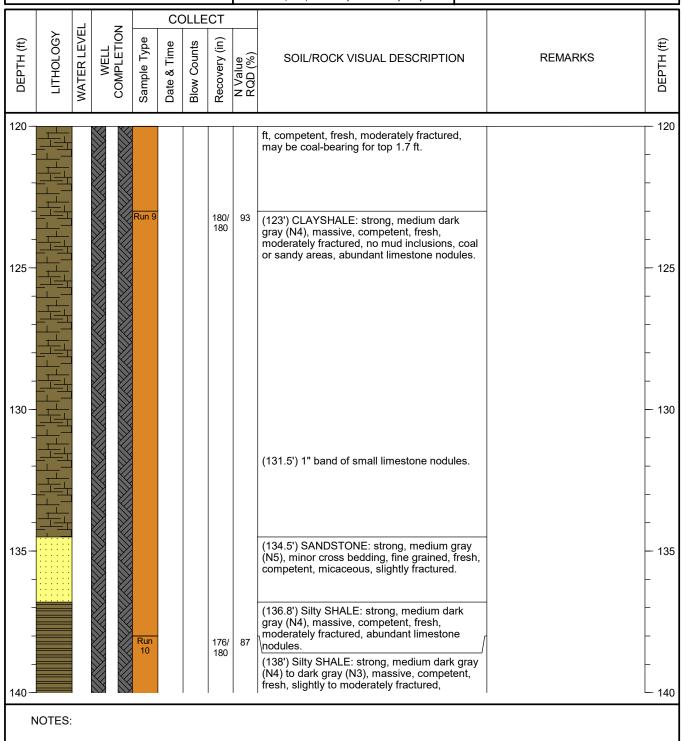
Well Diameter (in): 2

Screen Slot (in):

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

0.010





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-5

Page: 8 of 12

Drilling Start Date: 03/14/2016 14:45
Drilling End Date: 03/16/2016 09:30

Drilling End Date: 03/16/2016 09:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 233

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92
Top of Casing Elev. (ft): 1,039.54

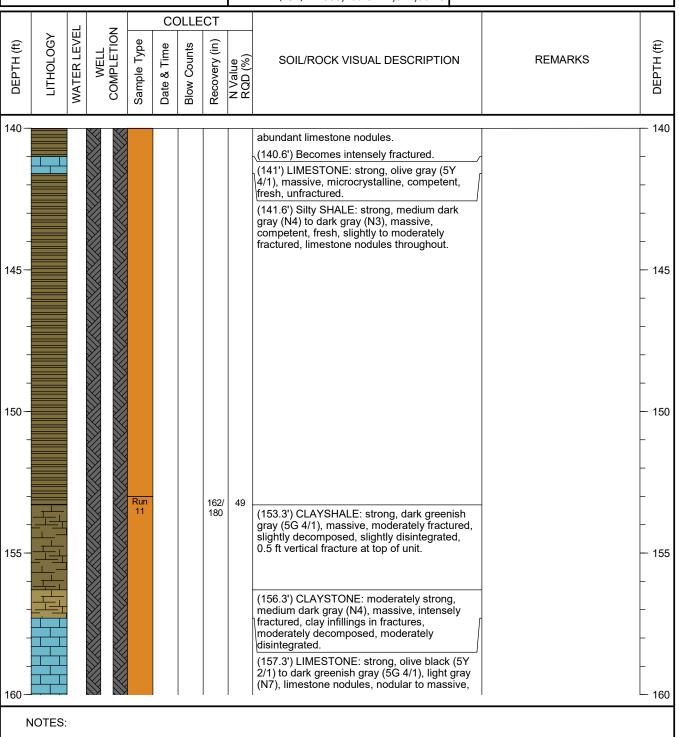
Location (X,Y): N 835,739.3 E 2,511,662.3

Well Depth (ft): 224

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5 Page: 9 of 12

Drilling Start Date: 03/14/2016 14:45 **Drilling End Date:** 03/16/2016 09:30

Drilling Company: Layne Drilling

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas**

233 Boring Depth (ft):

Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1.036.92

Top of Casing Elev. (ft): 1,039.54

Location (X,Y): N 835,739.3 E 2,511,662.3

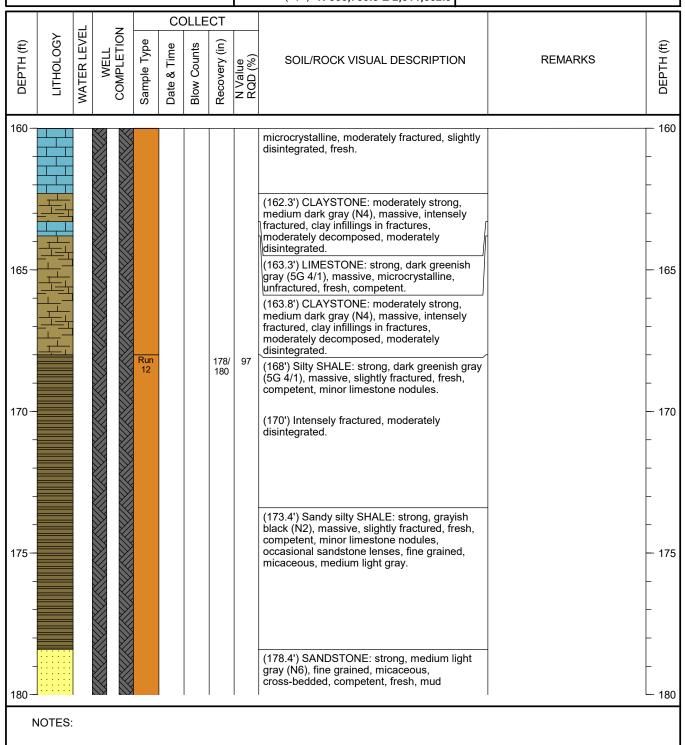
Well Depth (ft): 224

Well Diameter (in): 2

Screen Slot (in): 0.010

Sch 40 PVC Riser Material:

Pre-packed Sch 40 PVC Screen Material:





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. M-GS-5 Page: 10 of 12

03/14/2016 14:45 Well Depth (ft): Drilling Start Date: Boring Depth (ft): 233 **Drilling End Date:** 03/16/2016 09:30 Boring Diameter (in): Well Diameter (in): 6 Drilling Company: **Layne Drilling** Screen Slot (in): Sampling Method(s): **Rock Core**

Drilling Method: **Rock Core** DTW During Drilling (ft):

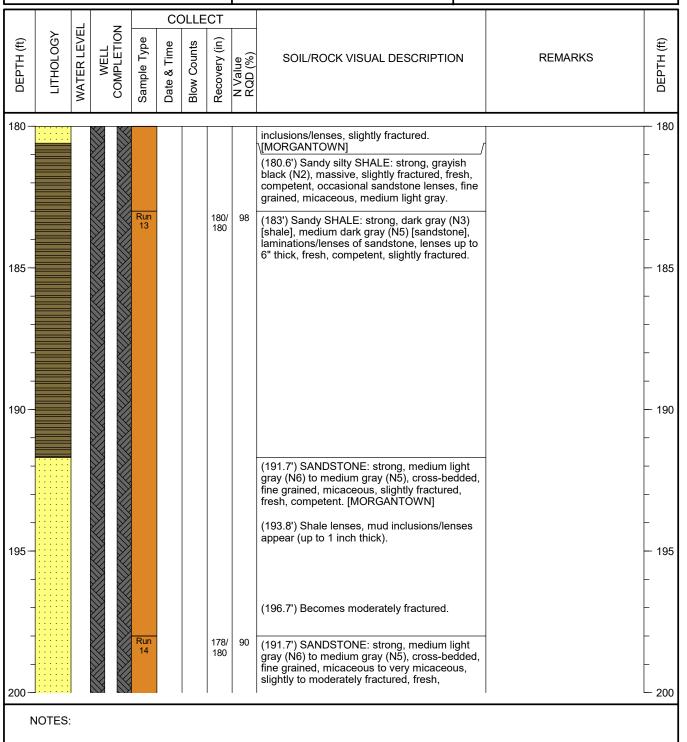
Drilling Equipment: CS1500 Wireline Rig Ground Surface Elev. (ft): 1,036.92 Driller: **Bill Womack** Top of Casing Elev. (ft): 1.039.54

Logged By: **Doug Mateas** Location (X,Y): N 835,739.3 E 2,511,662.3 224

2 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:





Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. M-GS-5

Page: 11 of 12

Drilling Start Date: 03/14/2016 14:45

Drilling End Date: 03/16/2016 09:30 Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack**

Logged By: **Doug Mateas** Boring Depth (ft): 233

Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92

Top of Casing Elev. (ft): 1,039.54

Well Depth (ft): 224

Well Diameter (in): 2

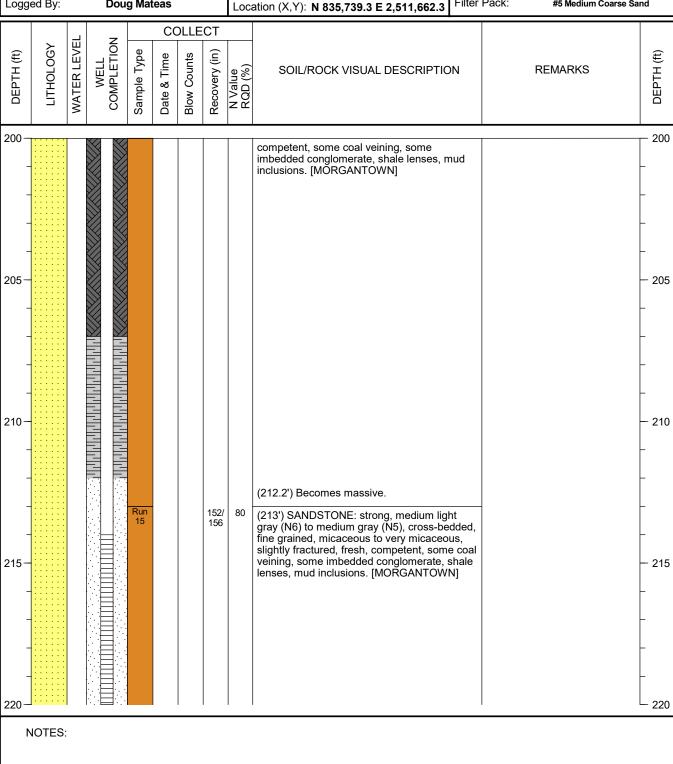
Screen Slot (in):

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:

0.010

Bentonite Pellets Seal Material(s): Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. M-GS-5

Page: 12 of 12

Drilling Start Date: 03/14/2016 14:45

Drilling End Date: 03/16/2016 09:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: Doug Mateas

Boring Depth (ft): 233

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.92

Top of Casing Elev. (ft): 1,039.54

Location (X,Y): N 835,739.3 E 2,511,662.3

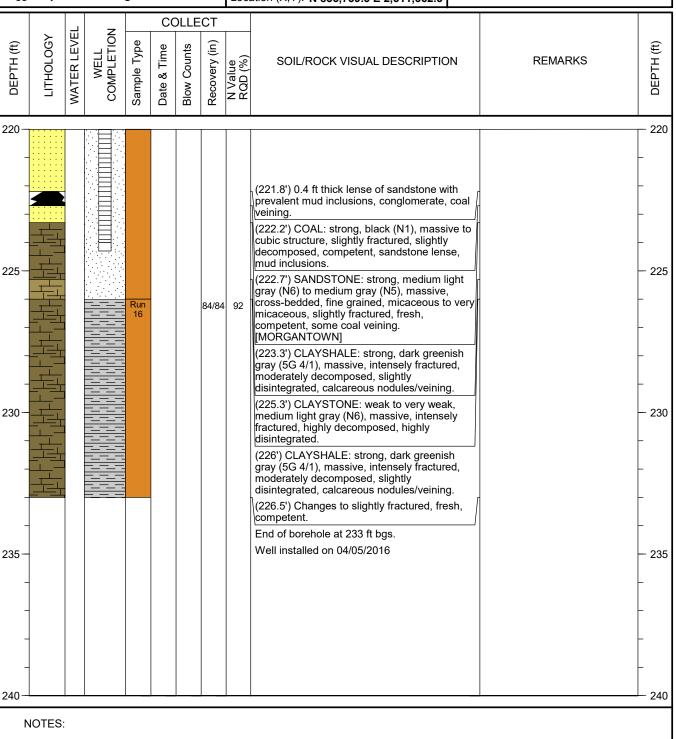
Well Depth (ft): 224

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Cardinal Operating Company 306 County Road 7E

Date Started : 3-6-2019 Date Completed : 3-8-2019 Logged by : Mielecki Reviewed by : J. Ardner **Drilling Contractor** : Terra Testing

Drilling Method

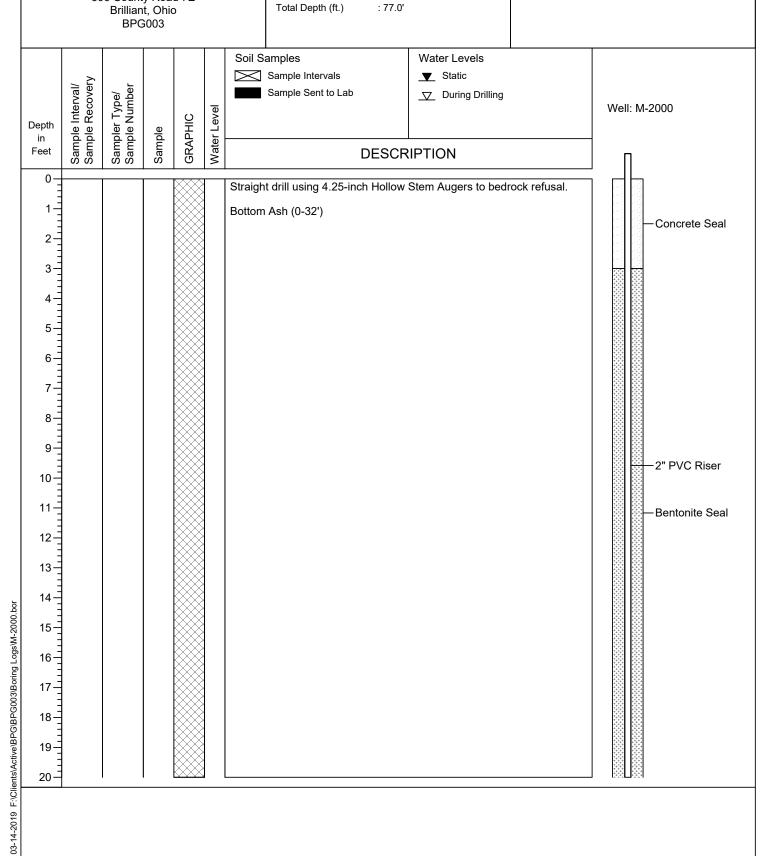
Sampling Method

: 4.25" HSA : 4" Core Barrel

: 77.0'

LOG OF BORING M-2000

(Page 1 of 4)





Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

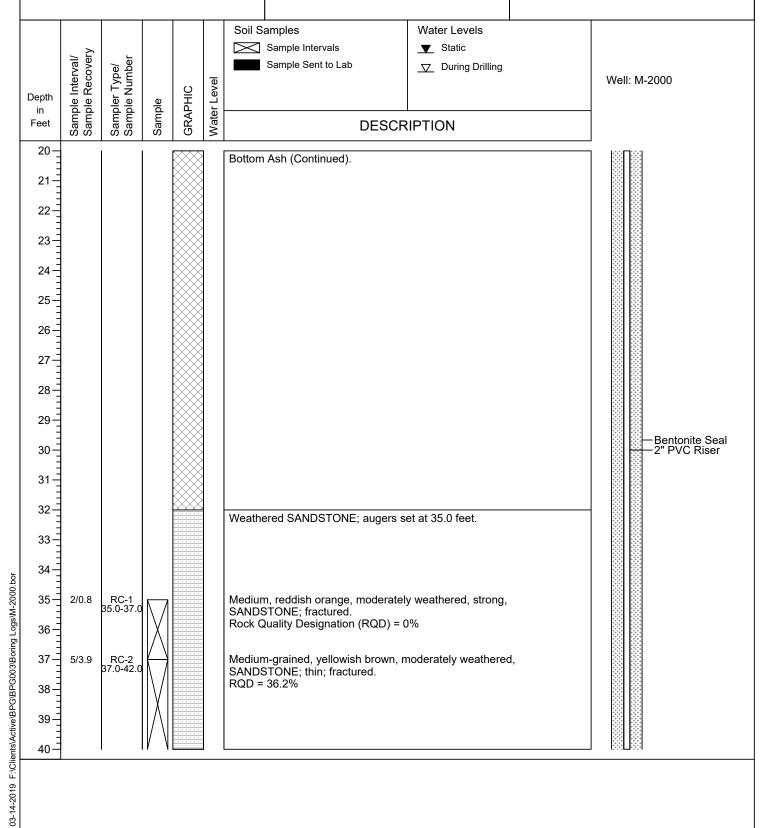
Total Depth (ft.)

Drilling Contractor : Terra Testing
Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel

: 77.0'

LOG OF BORING M-2000

(Page 2 of 4)





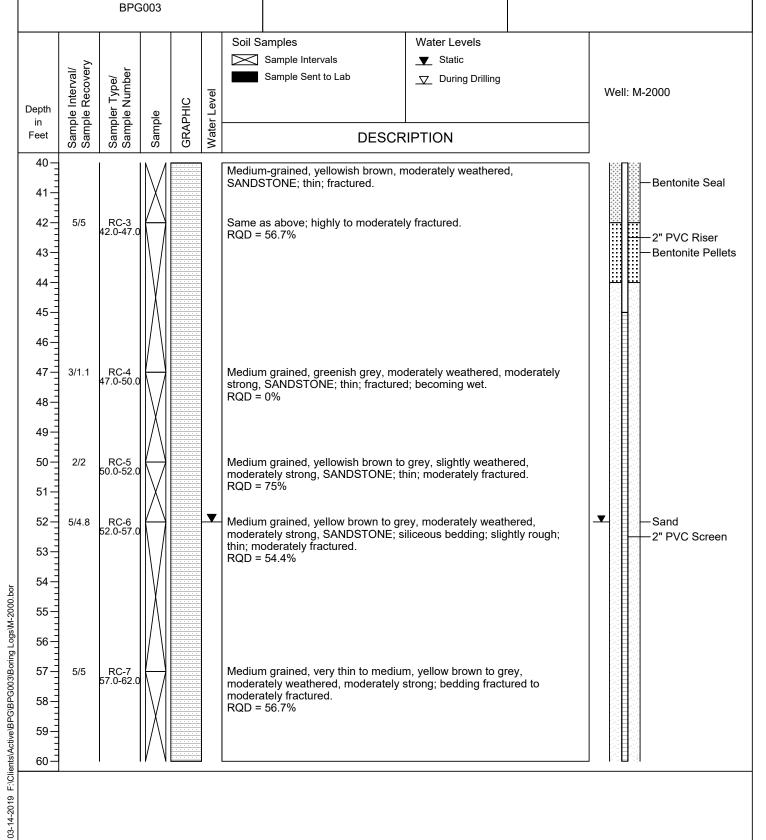
Cardinal Operating Company 306 County Road 7E Brilliant, Ohio Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 3 of 4)





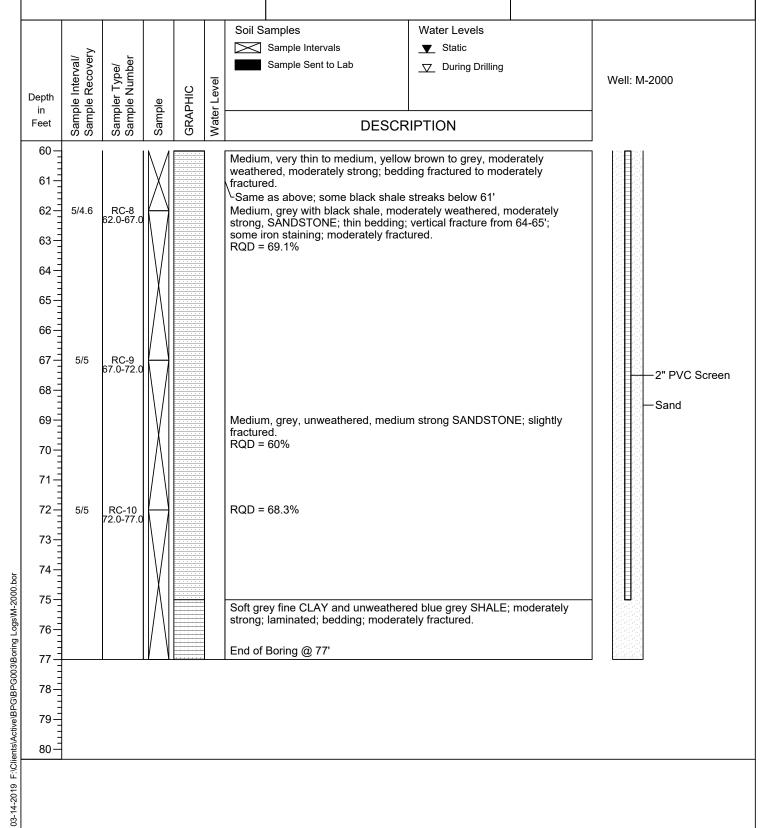
Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner

Drilling Contractor : Terra Testing
Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 4 of 4)



APPENDIX D WELL CONSTRUCTION LOGS



JOB NUMBER

COMPANY AMERICAN ELECTRIC POWER WELL No. <u>CA-0622A</u> BORING No. <u>CA-0622A</u> INSTALLED <u>8/16/16</u> PROJECT CARDINAL LANDFILL COORDINATES N 836,291.4 E 2,514,219.5 SYSTEM State Plane using NAD27/29 TOP RISER: 1162.28 FT. GROUND ELEVATION 1159.38 FT. GROUT SEAL: BENTONITE CHIPS TOP BENTONITE SEAL: 821.38 FT. BENTONITE SEAL: PELLETS SCREEN: 2" dia., U-PACK .10 SLOT, 10.0' GRAVEL PACK: TOP GRAVEL PACK: 816.38 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 813.38 FT. SPACERS, DEPTH: 20',80',140',200',250',325' GEOMCNST CD_FGD_LANDFILL BORINGS & WELLS.GPJ AEP.GDT 8/22/16 BOTTOM SCREEN: 803.38 FT. BOTTOM WELL: 803.38 FT. BOTTOM GRAVEL PACK: 803.38 FT. BOTTOM BORING: 803.38 FT.



JOB NUMBER

COMPANY AMERICAN ELECTRIC POWER WELL No. FA-8 BORING No. FA-8 INSTALLED 3/23/04 PROJECT CARDINAL FLY ASH DAM COORDINATES N 829,635.1 E 2,516,460.0 SYSTEM State Plane using NAD27/29 TOP RISER: 921.03 FT. GROUND ELEVATION 918.23 FT. GROUT SEAL: 90 GALLONS OF QUICK GROUT TOP BENTONITE SEAL: 883.03 FT. BENTONITE SEAL: 50 lbs 3/8" PELLETS SCREEN: 2" dia., 50 SLOT, 10' GRAVEL PACK: 225 lbs #4 QUARTZ TOP GRAVEL PACK: 880.43 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 878.23 FT. SPACERS, DEPTH: None Note: Backfilled hole from 156' to 52' with pea gravel GEOMCNST CD_FA_DAM.GPJ AEP.GDT 7/17/15 BOTTOM SCREEN: 868.23 FT. BOTTOM WELL: 867.73 FT. BOTTOM GRAVEL PACK: 866.23 FT. BOTTOM BORING: 763.23 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-1003 BORING No. M-1003 INSTALLED 4/7/10 PROJECT CARDINAL LANDFILL COORDINATES N 829,139.1 E 2,516,070.9 SYSTEM TOP RISER: 935.88 FT. GROUND ELEVATION 933.55 FT. GROUT SEAL: 100 GALS VOLCLAY BENTONITE TOP BENTONITE SEAL: 883.55 FT. BENTONITE SEAL: 3/8" COATED PELLETS SCREEN: 2" dia., 20 SLOT SCH 40, 80.0' GRAVEL PACK: 1550 LBS #4 QUARTZ SAND TOP GRAVEL PACK: 876.55 FT. RISER PIPE: 2", dia., SCH 40 TOP SCREEN: 874.25 FT. SPACERS, DEPTH: 50', 10' NOTES:
-Drill & decon water coming from CD Fire Protection System
-Decon rig & tools 04/13/10
-Drilled w/6" air hammer
-SWL @ 73.8' @ install BOTTOM SCREEN: 794.25 FT. BOTTOM WELL: 793.65 FT. BOTTOM GRAVEL PACK: 792.45 FT.

BOTTOM BORING: 792.45 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-1004 BORING No. M-1004D INSTALLED 3/31/10 PROJECT CARDINAL LANDFILL COORDINATES N 831,215.4 E 2,519,112.4 SYSTEM TOP RISER: 1008.29 FT. GROUND ELEVATION 1005.64 FT. GROUT SEAL: 250 GALS VOLCLAY TOP BENTONITE SEAL: 866.24 FT. BENTONITE SEAL: 100 LBS 3/8" COATED PELLETS SCREEN: 2" dia., 20 SLOT SCH 40, 50.0 GRAVEL PACK: 975 LBS #4 QUARTZ SAND TOP GRAVEL PACK: 859.44 FT. RISER PIPE: 2", dia., SCH 40 TOP SCREEN: 857.24 FT. SPACERS, DEPTH: 140', 80', 20' NOTES:
-Drill & decon water coming from CD Fire Protection System
-Decon rig & tools 3/31/10
-Drilled w/6" air hammer
-SWL @ 15.1' BOTTOM SCREEN: 807.24 FT. BOTTOM WELL: 806.64 FT. BOTTOM GRAVEL PACK: 805.44 FT.

BOTTOM BORING: 791.24 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-12 BORING No. CA-0608 INSTALLED 12/13/06 PROJECT CARDINAL LANDFILL COORDINATES N 833,112.2 E 2,516,013.2 SYSTEM TOP RISER: 1190.66 FT. GROUND ELEVATION 1187.65 FT. GROUT SEAL: 3,000 LBS HOLE PLUG TOP BENTONITE SEAL: 861.25 FT. BENTONITE SEAL: 30# BENTONITE PELLETS SCREEN: 1.5" dia., 0.20 SLOT, GEOMON, 5' GRAVEL PACK: #4 QUARTZ 250 LBS TOP GRAVEL PACK: 855.55 FT. RISER PIPE: 1", dia., TOP SCREEN: 794.65 FT. SPACERS, DEPTH: N/A -FLUSHED BORE HOLE W/700 GALS WATER
-6" AIR HAMMER TO 188.6' SET HW CASING THEN
NQ ROCK CORE TO 404.8'
-SWL @ INSTALL 186.0'
-TREMIED SAND & PELLETS INTO CORE HOLE W/1"
PIPE
-DECONNED TOOLS 11/7/06 BOTTOM SCREEN: 789.65 FT. BOTTOM WELL: 789.65 FT. BOTTOM GRAVEL PACK: 782.85 FT. BOTTOM BORING: 782.85 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-13 BORING No. CA-0610 INSTALLED 4/3/07 PROJECT CARDINAL LANDFILL COORDINATES N 831,697.9 E 2,518,374.3 SYSTEM TOP RISER: 991.14 FT. GROUND ELEVATION 988.42 FT. GROUT SEAL: ~200 Gals Quick Grout TOP BENTONITE SEAL: 871.52 FT. BENTONITE SEAL: 100 lbs 3/8" Pellets SCREEN: 2" dia., .020 Slot, 57.0' GRAVEL PACK: 1,050 lbs #4 Quartz TOP GRAVEL PACK: 864.12 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 858.12 FT. SPACERS, DEPTH: 150',100',50' NOTES: -Drilled w/6" Air Hammer -Deconned Tolls & Rig 04/05/07 -SWL @ Install 134.2 -3' SS Pump Type -Pump intake @ 185' BOTTOM SCREEN: 801.12 FT. BOTTOM WELL: 800.42 FT. BOTTOM GRAVEL PACK: 798.22 FT.

BOTTOM BORING: 794.02 FT.



MONITORING WELL CONSTRUCTION

COMPANY AMERICAN ELECTRIC POWER WELL No. M-1302 BORING No. B-1302M INSTALLED 5/30/13

PROJECT CARDINAL FLY ASH DAM

COORDINATES N 836,201.9 E 2,515,432.0

SYSTEM State Plane using NAD27/29

TOP RISER: 1030.72 FT. GROUND ELEVATION 1028.92 FT. GROUT SEAL: HOLE PLUG 600 LBS & QUICK GROUT 50 GALS TOP BENTONITE SEAL: 885.92 FT. BENTONITE SEAL: 3/8" COATED PELLETS 150 LBS SCREEN: 2" dia., SLOTTED .020, 39.6' GRAVEL PACK: #4 QUARTZ 1,100 LBS TOP GRAVEL PACK: 871.22 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 860.52 FT. SPACERS, DEPTH: 130 & 50 NOTES: -Pump installed BOTTOM SCREEN: 820.92 FT. BOTTOM WELL: 820.42 FT. BOTTOM GRAVEL PACK: 819.92 FT. BOTTOM BORING: 809.92 FT.



JOB NUMBER

COMPANY AMERICAN ELECTRIC POWER ___ BORING No. **B-1309D** INSTALLED **5/30/13** WELL No. M-1309 PROJECT CARDINAL FLY ASH DAM COORDINATES N 835,558.0 E 2,517,396.3 SYSTEM State Plane using NAD27/29 TOP RISER: 1172.09 FT. GROUND ELEVATION 1170.24 FT. GROUT SEAL: QUICK GROUT 100 GALS & HOLE PLUG 500 LBS TOP BENTONITE SEAL: 880.04 FT. BENTONITE SEAL: 3/8" COATED PELLETS 150 LBS SCREEN: 2" dia., SLOTTED .020, 39.6 GRAVEL PACK: #4 QUARTZ 750 LBS TOP GRAVEL PACK: 867.74 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 862.34 FT. SPACERS, DEPTH: 250',150',50' NOTES: -Hole plug from 209.2' to 130.0' -Pump installed BOTTOM SCREEN: 822.74 FT. BOTTOM WELL: 822.24 FT. BOTTOM GRAVEL PACK: 821.24 FT. BOTTOM BORING: 806.14 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING No. CA-0612 INSTALLED 3/21/07 WELL No. M-14 PROJECT CARDINAL LANDFILL COORDINATES N 832,901.9 E 2,519,661.8 SYSTEM TOP RISER: 988.21 FT. GROUND ELEVATION 984.91 FT. GROUT SEAL: ~150 Gals Quick Grout TOP BENTONITE SEAL: 866.01 FT. BENTONITE SEAL: 100 lbs 3/8" Pellets SCREEN: 2" dia., .020 Slot, 57.0' GRAVEL PACK: 1,150 lbs #4 Quartz TOP GRAVEL PACK: 859.11 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 857.61 FT. SPACERS, DEPTH: 150',100',50' NOTES: -Drilled w/6" Air Hammer -Deconned Tools & Drill 03/21/07 -SWL @ 43.8' -3' SS Pump Type -Pump intake @ 182' BOTTOM SCREEN: 800.61 FT. BOTTOM WELL: 799.91 FT. BOTTOM GRAVEL PACK: 797.71 FT.

BOTTOM BORING: 790.21 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-15 BORING No. CA-0614 INSTALLED 7/25/07 PROJECT CARDINAL LANDFILL COORDINATES N 833,569.0 E 2,518,172.3 SYSTEM TOP RISER: 1074.28 FT. GROUND ELEVATION 1071.83 FT. GROUT SEAL: ~600 Gals Quick Grout TOP BENTONITE SEAL: 868.13 FT. BENTONITE SEAL: 100 lbs 3/8" Pellets SCREEN: 2" dia., .020 Slot, 60' GRAVEL PACK: 1,275 lbs #4 Quartz TOP GRAVEL PACK: 860.83 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 857.83 FT. SPACERS, DEPTH: 250',200',150',100',50' NOTES: -Drilled w/6" Air Hammer -SWL @ Install 72.5' -Decon Tools 07/23/07 -3' SS Pump Type -Pump intake @ 273' BOTTOM SCREEN: 797.53 FT. BOTTOM WELL: 796.83 FT. BOTTOM GRAVEL PACK: 794.43 FT.

BOTTOM BORING: 794.43 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING No. CA-0616 INSTALLED 1/24/07 WELL No. **M-16** PROJECT CARDINAL LANDFILL COORDINATES N 835,565.0 E 2,516,519.0 SYSTEM TOP RISER: 1068.55 FT. GROUND ELEVATION 1065.75 FT. GROUT SEAL: ~850 Gals Quick Grout TOP BENTONITE SEAL: 878.25 FT. BENTONITE SEAL: 100 lbs 3/8" Pellets SCREEN: 2" dia., .020 Slot, 49.0' GRAVEL PACK: 1,950 lbs #4 Quartz TOP GRAVEL PACK: 871.85 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 864.45 FT. SPACERS, DEPTH: 250',200',150',100' NOTES: -Drilled w/6" Air Hammer -Deconned Tools & Drill 02/01/07 -SWL @ Install 101.4' -3' SS Pump Type -Pump intake @ 248' BOTTOM SCREEN: 815.45 FT. BOTTOM WELL: 814.75 FT. BOTTOM GRAVEL PACK: 813.65 FT.

BOTTOM BORING: 811.15 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER INSTALLED **6/1/06** WELL No. M-21 BORING No. CA-0620 PROJECT CARDINAL LANDFILL COORDINATES N 830,426.7 E 2,516,358.1 SYSTEM TOP RISER: 1018.61 FT. GROUND ELEVATION 1016.16 FT. GROUT SEAL: BENTONITE SLURRY TOP BENTONITE SEAL: 861.66 FT. BENTONITE SEAL: PELLETS SCREEN: 1" dia., .020 SLOT, 90.0' GRAVEL PACK: FILTER PRO TOP GRAVEL PACK: 856.16 FT. TOP SCREEN: 846.16 FT. RISER PIPE: 2", dia., PVC SPACERS, DEPTH: NOTES:
-Surface Seal: Cement
-Annular Sealant: Bentonite Slurry, Tremie Pipe
Installation, Overnight Setting Time
-Bentonite Seal: Poured Slowly, One Hr Setting Time
-Sand Pack: Poured Slowly -Original pump installed @ 258.5', then pulled. Grout in water sample.
-Well redeveloped with reclaimer.
-Pump reinstalled w/ intake @ 255.5' BOTTOM SCREEN: 756.16 FT. BOTTOM WELL: 756.16 FT. BOTTOM GRAVEL PACK: 753.06 FT.

BOTTOM BORING: 753.06 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY MONITORING WELL CONSTRUCTION



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING No. CA-0702 INSTALLED 5/21/07 WELL No. **M-22** PROJECT CARDINAL LANDFILL COORDINATES N 830,925.1 E 2,519,495.8 SYSTEM TOP RISER: 1008.04 FT. GROUND ELEVATION 1005.68 FT. GROUT SEAL: ~250 GALS QUICK GROUT TOP BENTONITE SEAL: 865.28 FT. BENTONITE SEAL: 100 LBS 3/8" PELLETS SCREEN: 2" dia., .020 SLOT, 65' GRAVEL PACK: 1,100 LBS #4 QUARTZ TOP GRAVEL PACK: 859.18 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 852.78 FT. SPACERS, DEPTH: 200',150',100',50' NOTES: -Decon 05/22/07 -Drilled w/6" Air Hammer -SWL @ Install 139.4' -3' SS Pump Type -Pump intake @ 214.5' BOTTOM SCREEN: 791.28 FT. BOTTOM WELL: 790.58 FT. BOTTOM GRAVEL PACK: 788.18 FT.

BOTTOM BORING: 786.48 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY MONITORING WELL CONSTRUCTION



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING No. CA-0703 INSTALLED 4/23/07 WELL No. **M-23** PROJECT CARDINAL LANDFILL COORDINATES N 830,051.2 E 2,518,092.0 SYSTEM TOP RISER: 985.90 FT. GROUND ELEVATION 983.44 FT. GROUT SEAL: 150 Gals Quick Grout TOP BENTONITE SEAL: 858.54 FT. BENTONITE SEAL: 100 lbs 3/8" Pellets SCREEN: 2" dia., .020 Slot, 45' GRAVEL PACK: 750 lbs #4 Quartz TOP GRAVEL PACK: 850.74 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 847.14 FT. SPACERS, DEPTH: 150', 100', 50' NOTES:
-Replacement well for 8501/1S
-Drilled w/6" Air Hammer
-Deconned Tools & Drill 04/18/07
-SWL @ Install 98.4' -3' SS Pump Type -Pump intake @ 175' BOTTOM SCREEN: 806.14 FT. BOTTOM WELL: 805.44 FT. BOTTOM GRAVEL PACK: 803.14 FT.

BOTTOM BORING: 803.14 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY MONITORING WELL CONSTRUCTION



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER INSTALLED 8/9/90 BORING No. 90CA22 WELL No. M-6 PROJECT CARDINAL PLANT COORDINATES N 831,918.6 E 2,156,681.5 SYSTEM STATE PLANE TOP RISER: 1010.46 FT. GROUND ELEVATION 1008.56 FT. GROUT SEAL: BENSEAL TOP BENTONITE SEAL: 873.06 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PVC SCH 40 20 SLOT, 2.0 GRAVEL PACK: #4 OHIO QUATZ TOP GRAVEL PACK: 864.36 FT. ELEV. CHECK VALVE: 788.56 FT. RISER PIPE: 1.0, dia., PVC SCH 80 TOP SCREEN: 787.96 FT. SPACERS, DEPTH: 20' OF CASING LOST (3" NW) IN HOLE BETWEEN 18' TO 50'? GEOMON A-36 STEEL CASING BOTTOM SCREEN: 785.96 FT. BOTTOM WELL: 785.56 FT. BOTTOM GRAVEL PACK: 784.56 FT.

BOTTOM BORING: 778.36 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. M-10 BORING No. 85W-3 INSTALLED 8/13/85 PROJECT CARDINAL PLANT COORDINATES N 829,994.0 E 2,518,683.2 SYSTEM STATE PLANE TOP RISER: 1033.42 FT. GROUND ELEVATION 1031.00 FT. GROUT SEAL: CEMENT\BENTONITE TOP BENTONITE SEAL: 859.50 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PORPUS POLYETHLENE, 1.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 853.00 FT. ELEV. CHECK VALVE: 802.10 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 801.50 FT. SPACERS, DEPTH: GEOMON 12"GEOMON BOTTOM SCREEN: 800.50 FT.

BOTTOM WELL: 800.50 FT.

BOTTOM BORING: 766.00 FT.

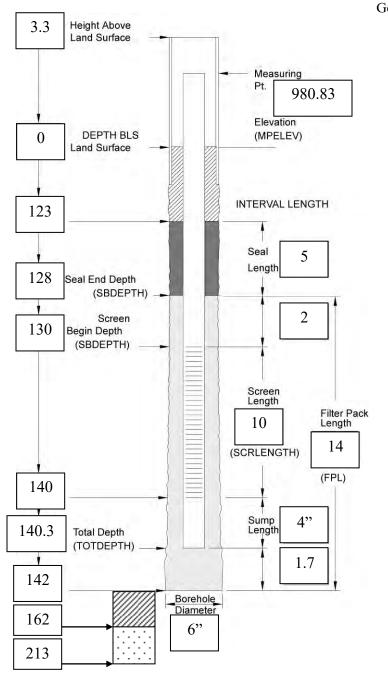
BOTTOM GRAVEL PACK: 794.00 FT.



Well I.D. (LOCID): M-GS-1	Site: AEP – Cardinal Project Number: CHE8126L
Drilling Company: Layne	Installation Method: HAS/Rotary
Drillers: Danny Allen	Casing Installation Date (INSDATE): 4/13/16
Geologist/Engineer: D. Mateas / M. Muenich	Well Type (WTCCODE): Monitoring Well
Signature:	Well Completion Method (WCMCODE): Above Grade
	Geologic Completion Zone (GZCODE):
3.6 Height Above	
Land Surface	
	Well Completion
Measuring Pt. 201.07	2 Guard Posts (Y / N) Date:
991.87	Surface Pad Size: 2 ft x 2 ft x 6"
1 - 0	Protective Casing or Cover
O DEPTH BLS Elevation (MPELEV)	Diameter/Type: 4" locking flip-top
Land Surface (Will ELEV)	Depth BGS: 2 Weep Hole (Y/N)
	Grout
	Composition/Proportions: 150 lbs Haliburton Bentonite
115 INTERVAL LENGTH	Quick Grout / 100 gal. H ₂ O
115 INTERVAL LENGTH	Placement Method: pressure tremie
Seal 5	Seal Date: <u>4/13/16</u>
Length	Type: 3/8" coated bentonite pellets
120 Seal End Depth	Source: Pel-Plug Western Bentonite
(SBDEPTH)	Set-up/Hydration Time: 30 mins
Screen 2	Placement Method: poured gravity
122 Begin Depth	Vol. Fluid Added: N/A - submerged
(SBDEPTH)	Filter Pack
	Type: #5 med. coarse sand
Screen	Source: Flat Rock, Sparta, MI
Length Filter Pack	Amount Used: 8 x 50 lb bags Placement Method: poured gravity
10 Filler Pack	Placement Method. poured gravity
(CORUGNOTION 14	Well Riser Pipe
(SCRLENGTH)	Casing Material (CMACODE): Sch. 40 PVC
(FPL)	Casing Inside Diameters (CASDIAM): 2.0 in
132	Screen
	Material: Pre-packed Sch. 40 PVC
132.3 Total Depth Sump Length 4"	Inside Diameter (SCRDIAM): 2.0 in.
(TOTDEPTH)	Screen Slot Size: (SOUA): 0.010 10-slot in.
1.7	Percent Open Area (PCTOPEN):
134	Sumpor Bottom Cap (Y) N)
Borehole	Type/Length: 4" Sch. 40 PVC
155 Diameter	Backfill Plug (Y) N)
6"	Material: 3/8" med. crushed bentonite chips
209	Placement Method: poured gravity
	Set-up/Hydration Time:
Comments	Total Water Volume During Construction
Total drilled depth = 209'; backfilled with sand and chips	Introduced (Gal): Recovered
to 134'; centralizer at 65'	(Gal):
	Reviewed By: J. Neil Couch Date: 4/22/2016



Well I.D. (LOCID): M-GS-2
Drilling Company: Layne
Drillers: Danny Allen
Geologist/Engineer: D. Mateas / M. Muenich
Signature:



Comments

Total drilled depth = 213'; backfilled to 142' with sand and chips (20' seal); 1 centralizer used at 70'

Site: AEP – Cardinal	Project Number: <u>CHE8126L</u>
Installation Method: HSA	
Casing Installation Date (INSD	
Well Type (WTCCODE): Mor	
Well Completion Method (WC	
Geologic Completion Zone (GZ	ZCODE):
Wall Consolidan	
Well Completion	N-4
2 Guard Posts (Y / N) D Surface Pad Size: 2 ft	
Protective Casing or Cove	
9	
Diameter/Type: 4" locking Depth BGS: 2 W	Voon Holo (V/N)
Grout	reep Hole (1/N)
	150 lbs Haliburton Bentonite
Quick Grout / 100 gal. H ₂ C	
Placement Method: pressu	
i lacement iviculou. <u>pressu</u>	ie treinie
Seal	Date: 4/13/16
Type: 3/8" coated bentonit	
Source: Pel-Plug Western	
Set-up/Hydration Time: 30	
Placement Method: poured	
Vol. Fluid Added: N/A - s	
Filter Pack	
Type: #5 med. coarse sand	
Source: Flat Rock, Sparta,	
Amount Used: 6 x 50 lb b	
Placement Method: poured	
Well Riser Pipe	
Casing Material (CMACOI	DE): Sch. 40 PVC
Casing Inside Diameters (C	CASDIAM): <u>2.0</u> in.
Screen	
Material: Pre-packed Sch.	40 PVC
Inside Diameter (SCRDIAN	·
Screen Slot Size: (SOUA):	
Percent Open Area (PCTO	
Sumpor Bottom Cap (Y)	
Type/Length: 4" Sch. 40	PVC_
Backfill Plug (Y) N)	
Material: 3/8" med. crushe	
Placement Method: poured	l gravity
Set-up/Hydration Time:	
Total Water Volume Duri	
Introduced (Gal): 0	Recovered
(Gal):	

Reviewed By: J. Neil Couch Date: 4/22/2016



Well I.D. (LOCID): M-GS-3	
Drilling Company: <u>Layne</u>	Installation Method: HSA/Rotary
Drillers: Danny Allen Geologist/Engineer: D. Mateas / M. Muenich Signature:	Casing Installation Date (INSDATE): 4/12/16 Well Type (WTCCODE): Monitoring Well Well Completion Method (WCMCODE): Above Grade Geologic Completion Zone (GZCODE):
3.1 Height Above Land Surface	
O DEPTH BLS Land Surface Measuring Pt. 1000.33 Elevation (MPELEV)	Well Completion 2 Guard Posts (Y / N) Date: Surface Pad Size: 2 ft x 2 ft x 6" Protective Casing or Cover Diameter/Type: 4" locking flip-top Depth BGS: 2 Weep Hole (Y / N) Grout Composition/Proportions: 150 lbs Haliburton Bentonite
129 INTERVAL LENGTH	Quick Grout / 100 gal. H ₂ O Placement Method: pressure tremie
Seal Seal End Depth (SBDEPTH) Screen Begin Depth (SBDEPTH)	Seal Type: 3/8" coated bentonite pellets Source: Pel-Plug Western Bentonite Set-up/Hydration Time: 30 mins Placement Method: poured gravity Vol. Fluid Added: N/A - submerged Filter Pack Type: #5 med. filter pack
Screen Length 10	Source: Flat Rock Bagging, Sparta, MI Amount Used:
146.3 Total Depth (TOTDEPTH) (SCRLENGTH) 14 (SCRLENGTH) Sump 4" Length 4"	Well Riser Pipe Casing Material (CMACODE): Sch. 40 PVC Casing Inside Diameters (CASDIAM): 2.0 in Screen Material: Pre-packed Sch. 40 PVC Inside Diameter (SCRDIAM): 2.0 in.
148	Screen Slot Size: (SOUA): 0.010 10-slot in. Percent Open Area (PCTOPEN): Sump or Bottom Cap (Y) N) Time/Length: 4" Sale 40 DVC
168 Borehole Diameter 6"	Type/Length: 4" Sch. 40 PVC Backfill Plug (Y) N) Material: 3/8" crushed bentonite hole plug Placement Method: poured gravity
Comments Total boring depth = 206'; backfilled with sand then chip To 148'; centralizer used at 70'	Set-up/Hydration Time: Total Water Volume During Construction Introduced (Gal): 0 Recovered (Gal): -
10 170, Centralizer used at /0	Reviewed By: J. Neil Couch Date: 4/22/2016



Well I.D. (LOCID): M-GS-4	Site: AEP – Cardinal Project Number: CHE8126L
Drilling Company: Layne	Installation Method: HAS/Rotary
Drillers: Danny Allen	Casing Installation Date (INSDATE): 04/21/2016
Geologist/Engineer: D. Mateas	Well Type (WTCCODE): Monitoring Well
Signature:	Well Completion Method (WCMCODE): Above Grade
	Geologic Completion Zone (GZCODE):
3.0 Height Above	• • • • • • • • • • • • • • • • • • • •
Land Surface	
	Well Completion
→ Measuring	2 Guard Posts (Y / N) Date:
Pt. 1028.73	Surface Pad Size: 2 ft x 2 ft
1020.73	Protective Casing or Cover
Elevation	Diameter/Type: 4" steel
0 DEPTH BLS (MPELEV)	Depth BGS: Weep Hole (Y/N)
Land Surface	Grout
	Composition/Proportions: 15 bags Bentonite grout
	Composition/1 toportions. 15 bags Bentonite grout
185 INTERVAL LENGTH	Placement Method: pressure tremie
163	riacement Method. <u>pressure trenne</u>
	Seal Date: <u>04/21/16</u>
Seal 5	
190 Seal End Depth	Type: 3/8" coated bentonite pellets
190 Seal End Depth (SBDEPTH)	Source: Pel-Plug Western Bentonite
	Set-up/Hydration Time: 30 mins
Screen 2	Placement Method: poured gravity
192 Begin Depth	Vol. Fluid Added: <u>N/A - submerged</u>
(SBDEPTH)	Filter Pack
	Type: #5 medium coarse sand
Screen	Source: Flat Rock, Sparta, MI
Length	Amount Used: 14 x 50 lb bags
Filter	Placement Method: poured gravity
	well Riser Pipe
(SCRLENGTH) 14	
	Casing Inside Diameters (CASDIAM): 2.0 in.
202 (FPL	
4"	Material: Pre-packed Sch. 40 PVC
Sump	Inside Diameter (SCRDIAM): 2.0 in.
202.3 Total Depth Length	Screen Slot Size: (SOUA): 0.010 10-slot in.
(TOTDEPTH)	Percent Open Area (PCTOPEN):
1.7	Sump or Bottom Cap (Y) N)
204	Type/Length: 4" Sch. 40 PVC
Borehole	Backfill Plug (Y) N)
232 Diameter	Material: 3/8" medium crushed bentonite chips
6"	Placement Method: poured gravity
	Set-up/Hydration Time:
Comments	Total Water Volume During Construction
Total drilled depth = 232'; backfilled with chips to 204'	Introduced (Gal): Recovered
	(Gal):
	Reviewed By: J. Neil Couch Date: 5/03/2016



Drilling Drillers:	D. (LOCID): <u>M-G</u> Company: <u>Layne</u> <u>Danny Allen</u> st/Engineer: <u>J. Ba</u> re:	e		
2.7	Height Above Land Surface			
0 207	DEPTH BLS Land Surface		Measu Pt. [Elevat (MPEL	1039.54 ion EV)
212	Seal End Depth (SBDEPTH) Screen Begin Depth (SBDEPTH)		Seal Length	2
224	Total Depth		Screen Length 10 (SCRLENG	TH) Filter Pack Length (FPL) 4"
226	(TOTBEPTH)	Borehole Diameter 6"		1.7
Commo Total dr	ents rilled depth = 233.	.3'; backf	illed with c	hips to 226'

Site: AEP – Cardinal Project Number: CHE8126L Installation Method: HSA/Rotary Casing Installation Date (INSDATE): 4/5/16 Well Type (WTCCODE): Monitoring Well Well Completion Method (WCMCODE): Above Grade Geologic Completion Zone (GZCODE):
Well Completion
Well Completion 2 Guard Posts (Y / N) Date:
Surface Pad Size: 2 ft x 2 ft x 6"
Protective Casing or Cover
Diameter/Type: 4" locking flip-top
Depth BGS: 2 Weep Hole (Y/N)
Grout
Composition/Proportions: <u>150 lbs Haliburton Bentonite</u>
Quick Grout / 100 gal. H ₂ O
Placement Method: <u>pressure tremie</u>
0.1
Seal Date: <u>4/5/16</u>
Type: 3/8" coated bentonite pellets
Source: Pel-Plug Western Bentonite
Set-up/Hydration Time: 30 mins
Placement Method: poured gravity
Vol. Fluid Added: N/A - submerged
Filter Pack
Type: #5 med. coarse sand
Source: Flat Rock, Sparta, MI
Amount Used: 8 x 50 lb bags
Placement Method: poured gravity
Well Riser Pipe Casing Material (CMACODE): Sch. 40 PVC
Casing Inside Diameters (CASDIAM):in.
Screen
Material: Pre-packed Sch. 40 PVC
Inside Diameter (SCRDIAM): 2.0 in.
Screen Slot Size: (SOUA): 0.010 10-slot in.
Percent Open Area (PCTOPEN):
Sumpor Bottom Cap (Y) N)
Type/Length: 4" Sch. 40 PVC_
Backfill Plug (Y) N)
Material: 3/8" med. crushed bentonite chips
Placement Method: poured gravity
Set-up/Hydration Time:
Total Water Volume During Construction
Introduced (Gal): Recovered
(Gal):
Reviewed By: J. Neil Couch Date: 5/3/2016

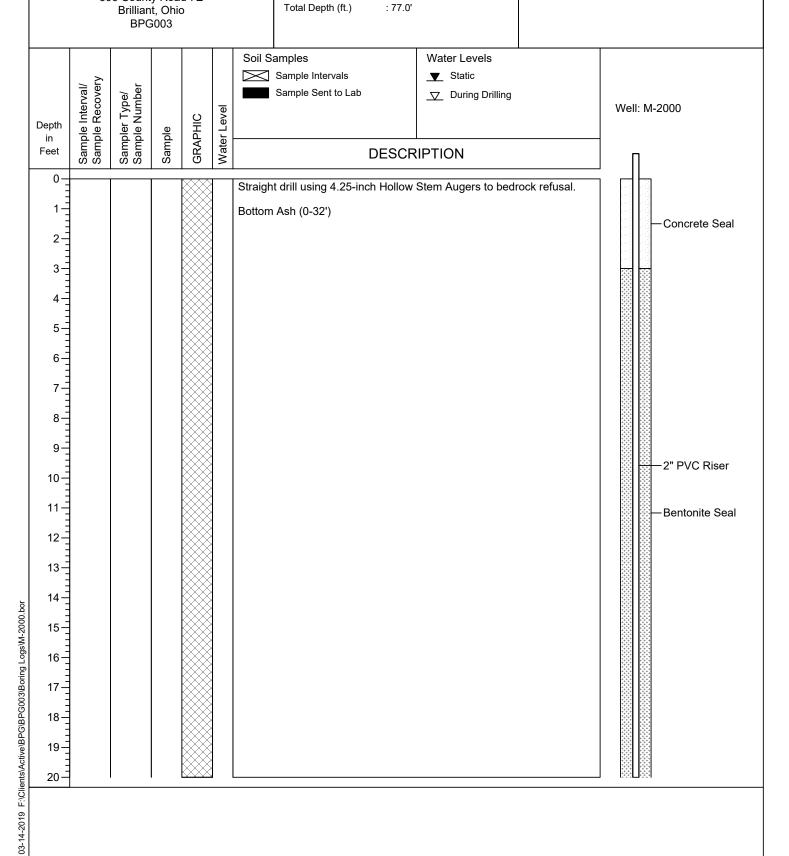


Cardinal Operating Company 306 County Road 7E Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel
Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 1 of 4)



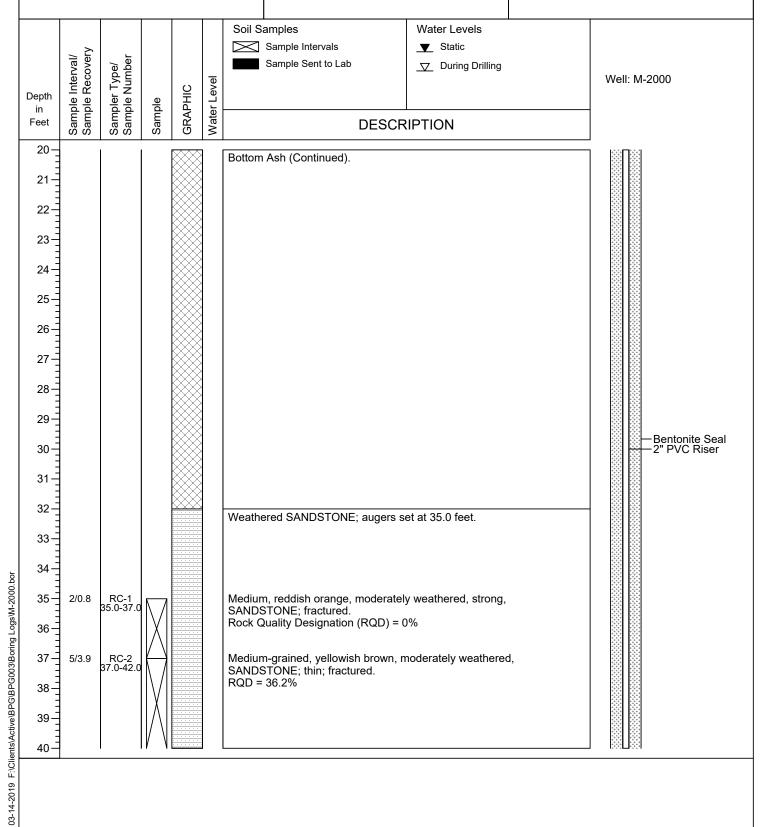


Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel
Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 2 of 4)





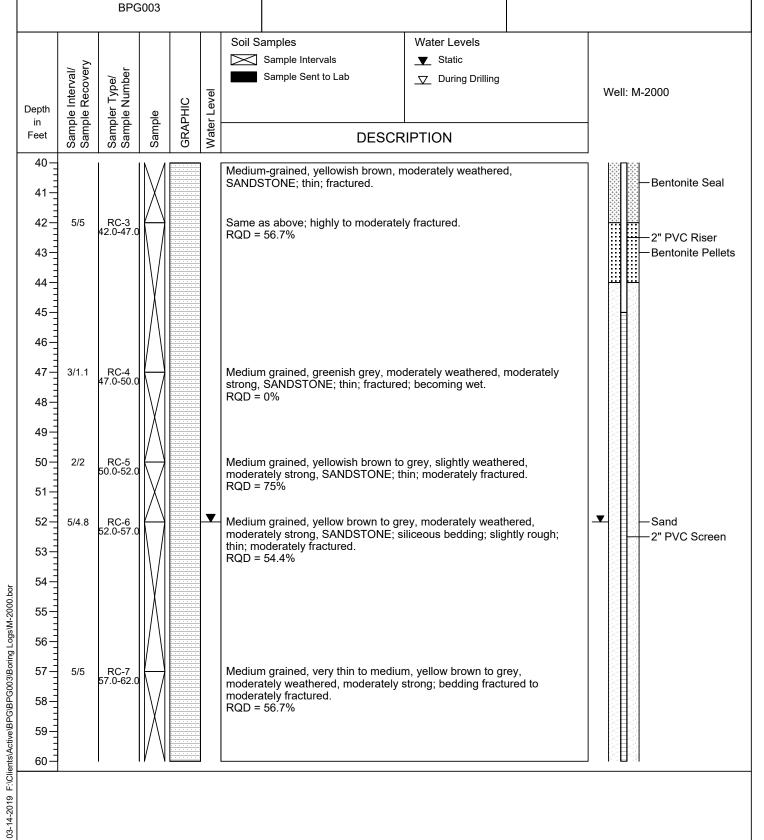
Cardinal Operating Company 306 County Road 7E Brilliant, Ohio Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Tes

Drilling Contractor : Terra Testing
Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 3 of 4)





Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner

Reviewed by : J. Ardner

Drilling Contractor : Terra Testing

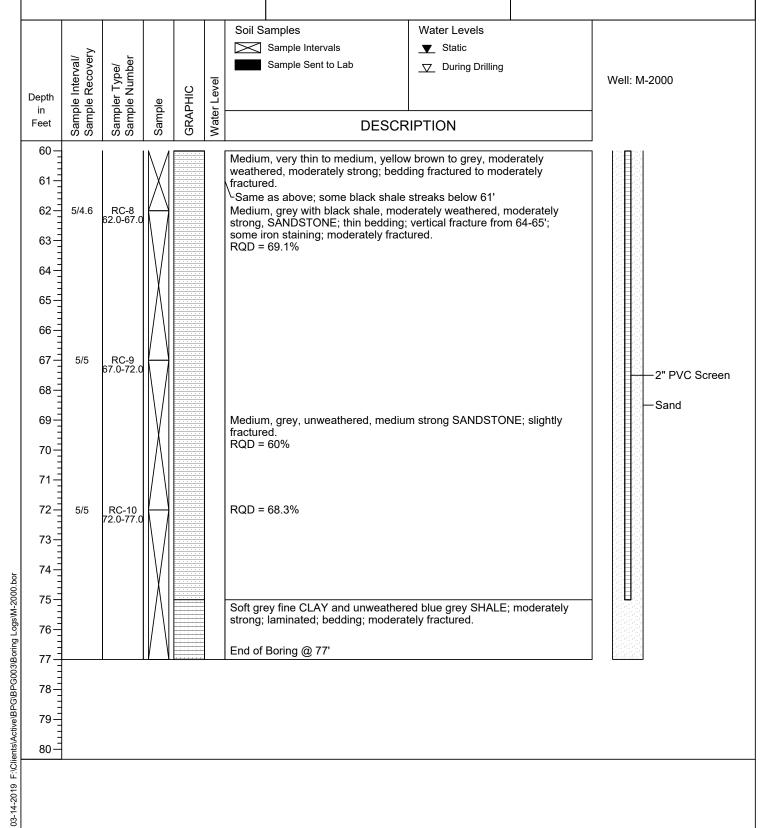
Drilling Method : 4.25" HSA

Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

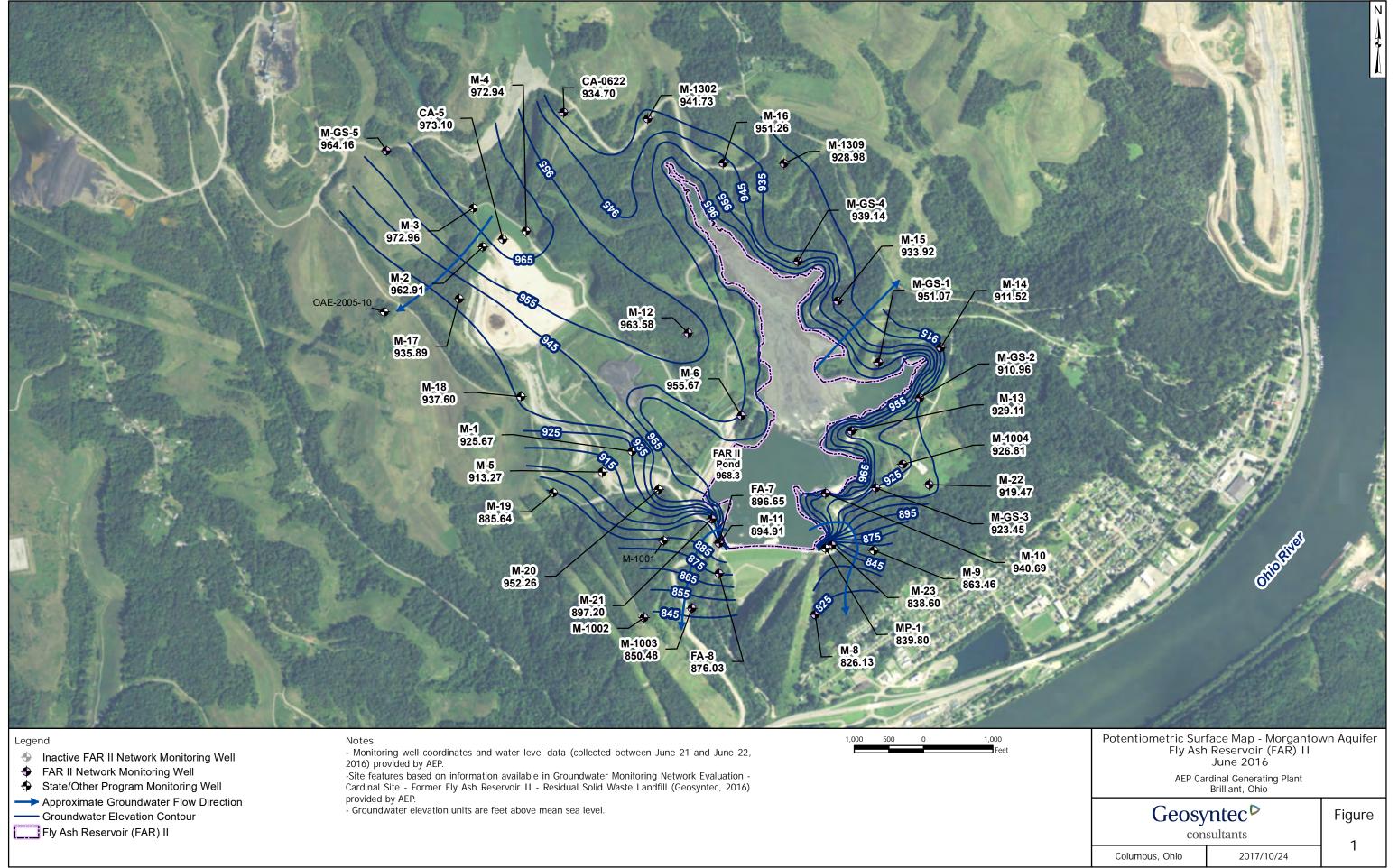
LOG OF BORING M-2000

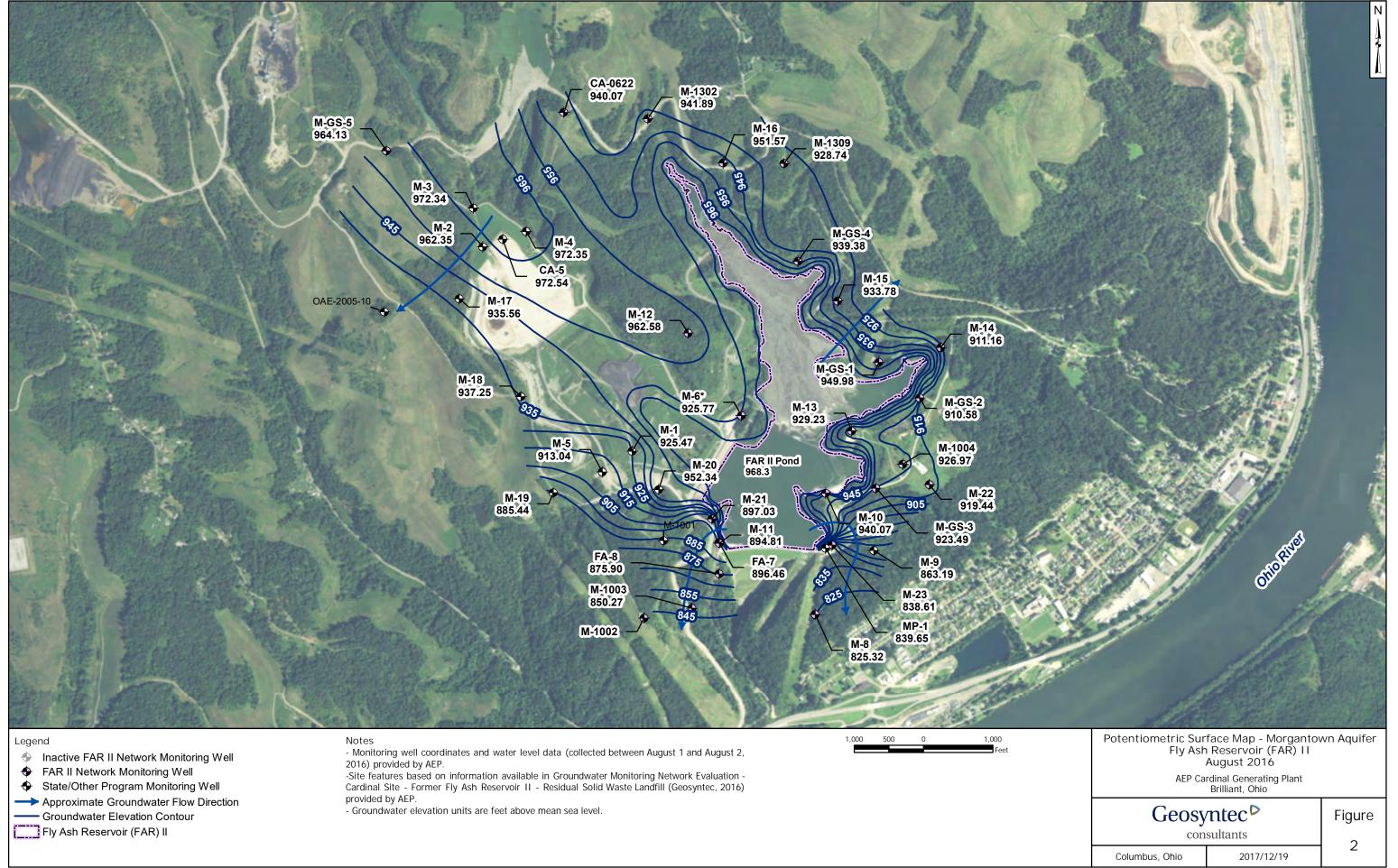
(Page 4 of 4)

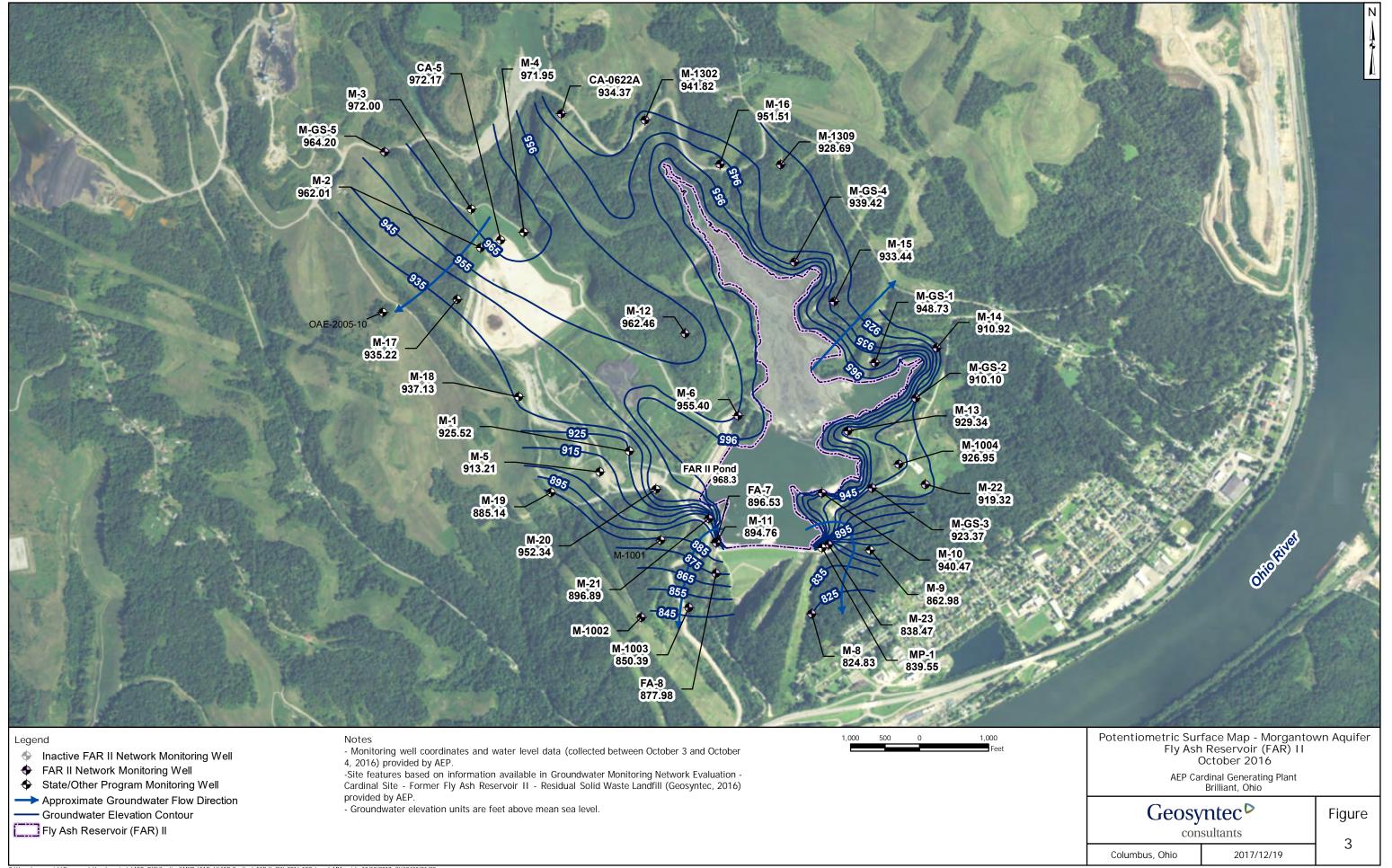


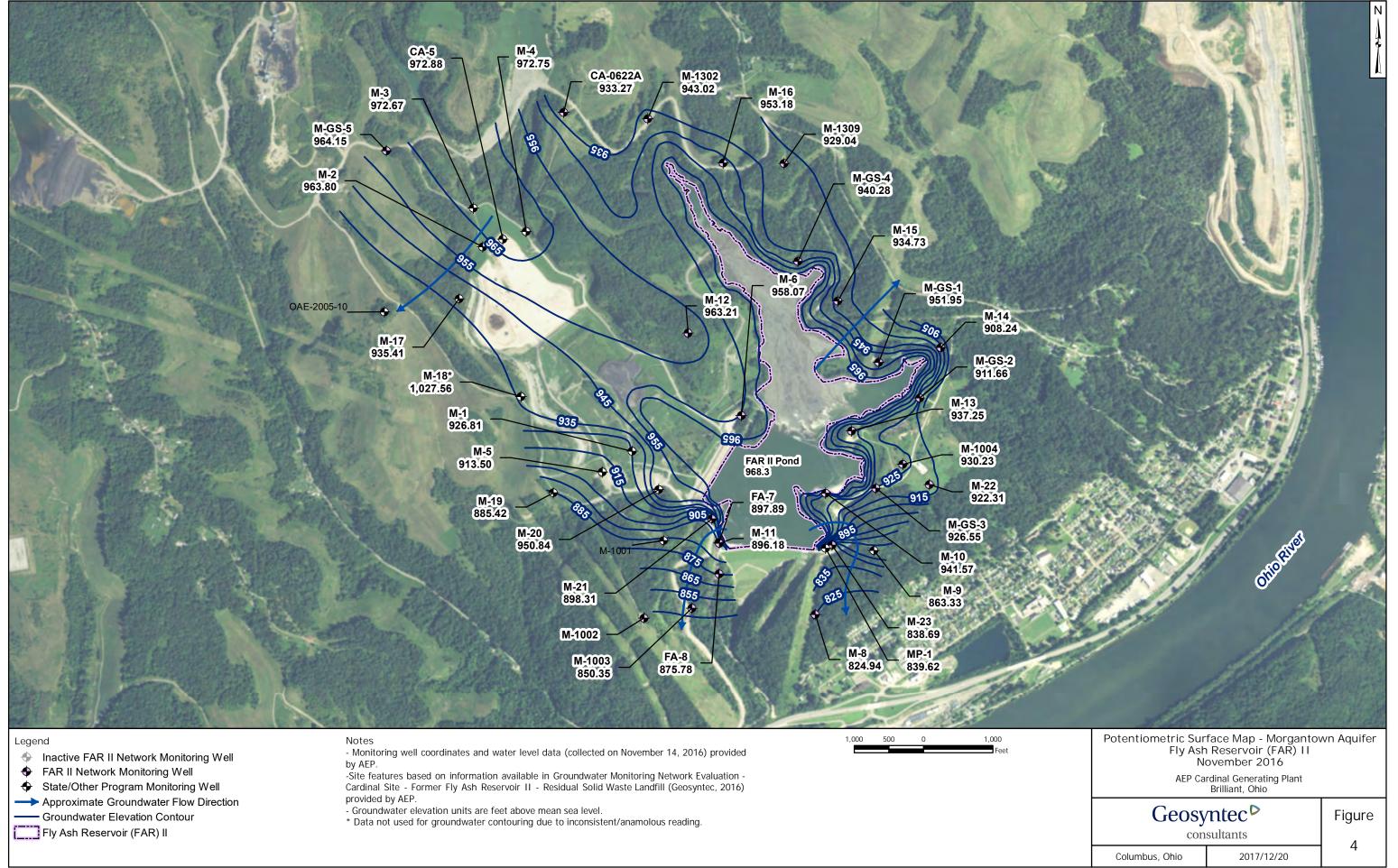
Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(2)(iii)

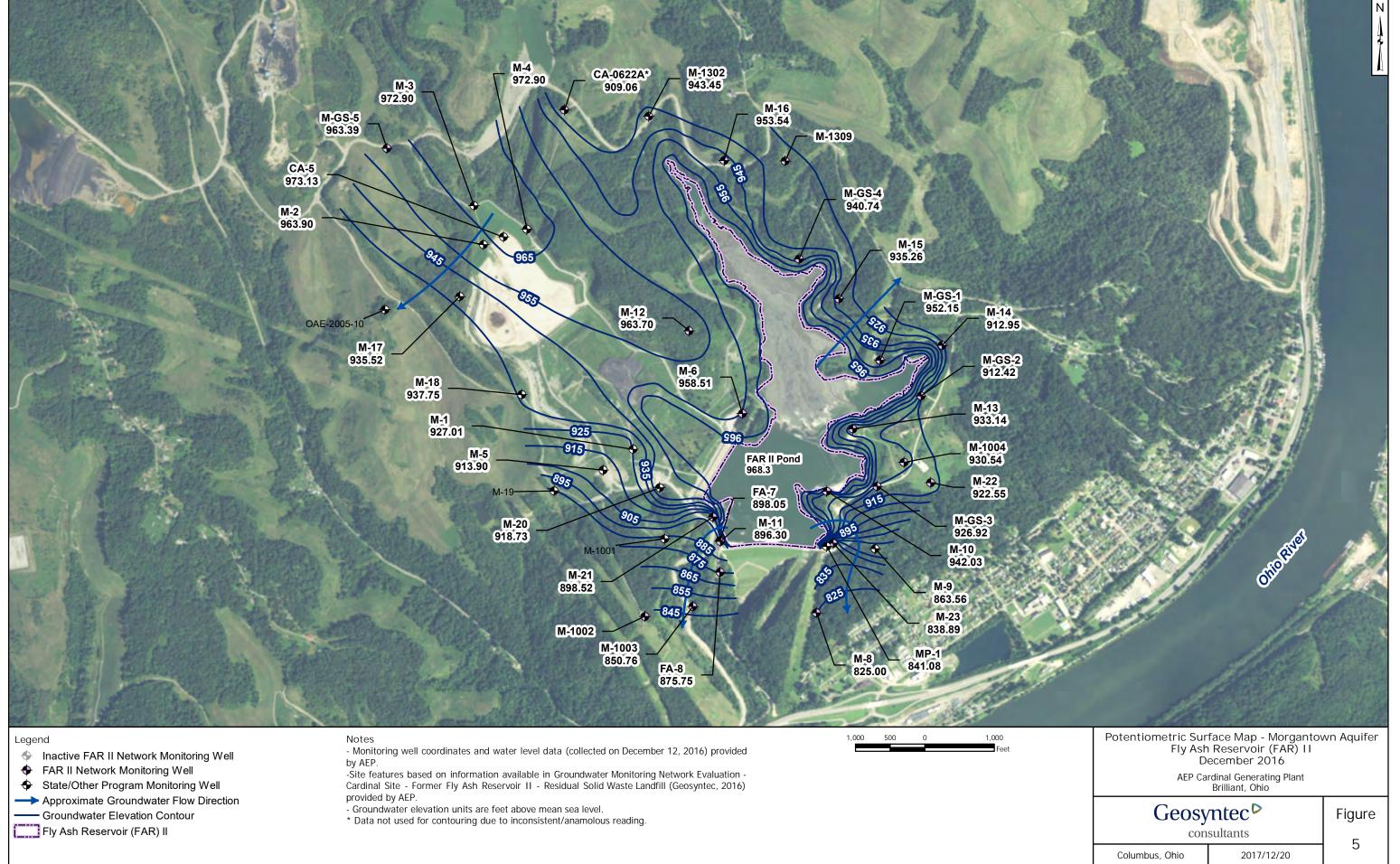
Maps that characterize the direction of groundwater flow accounting for seasonal variations

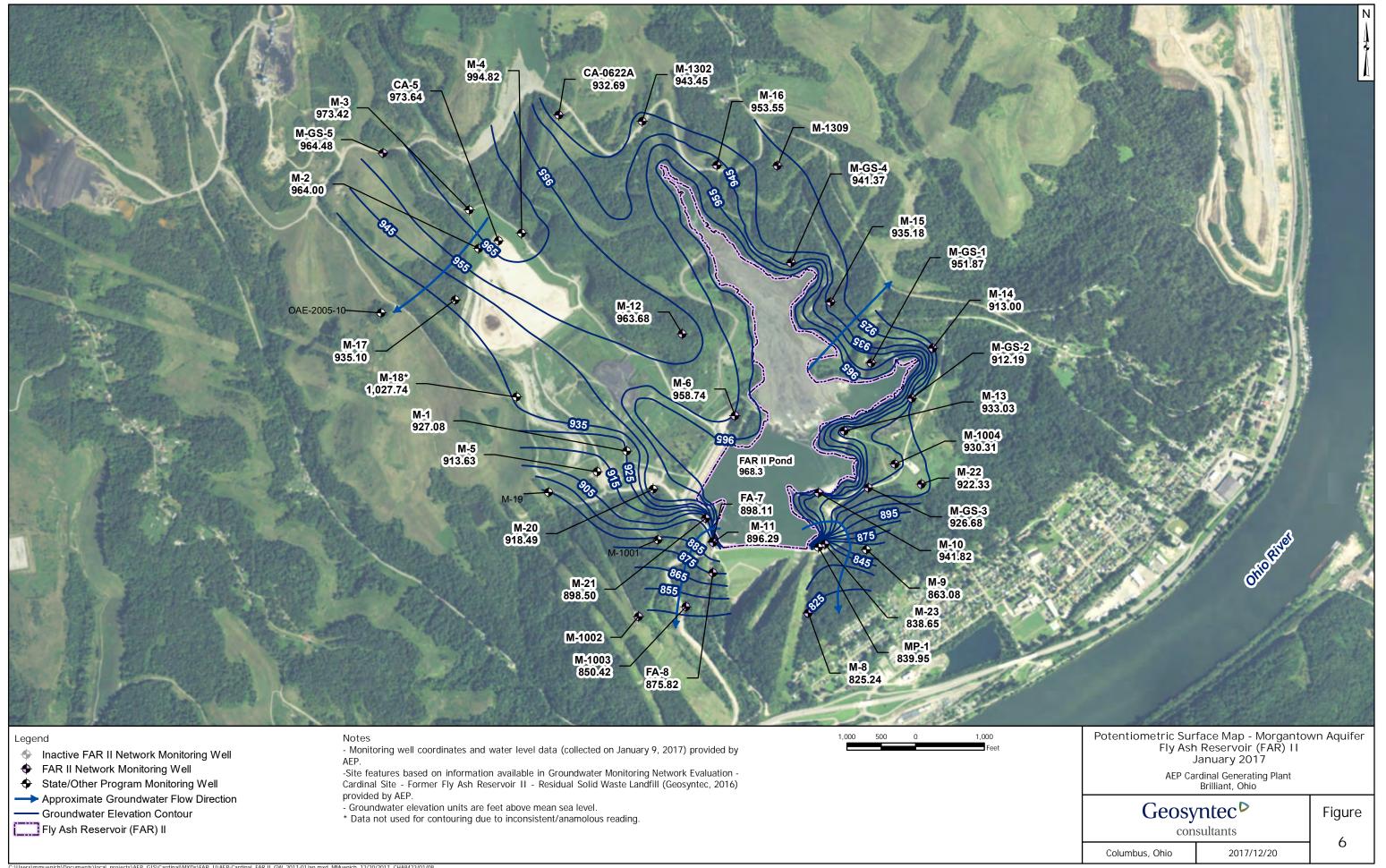


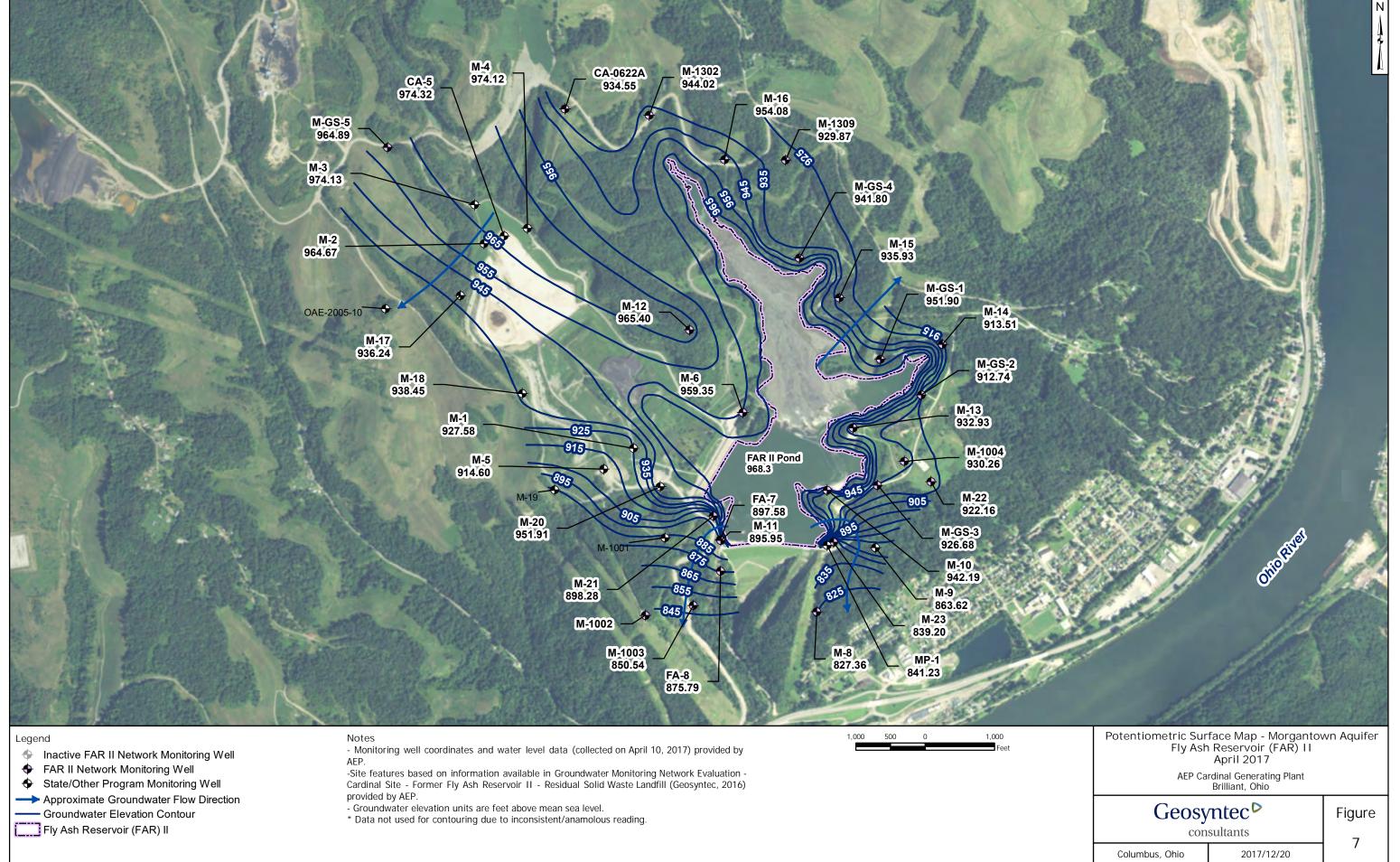


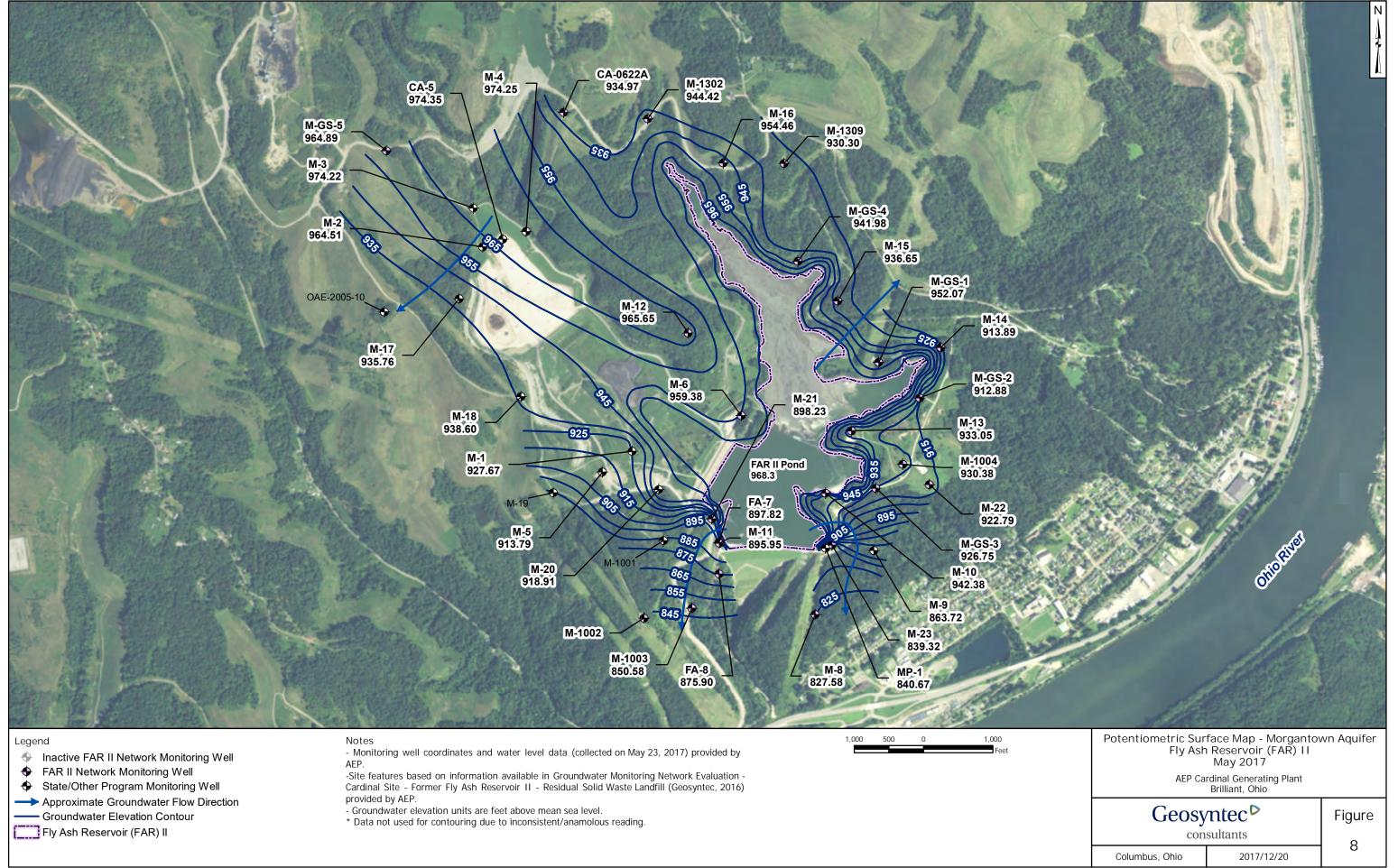


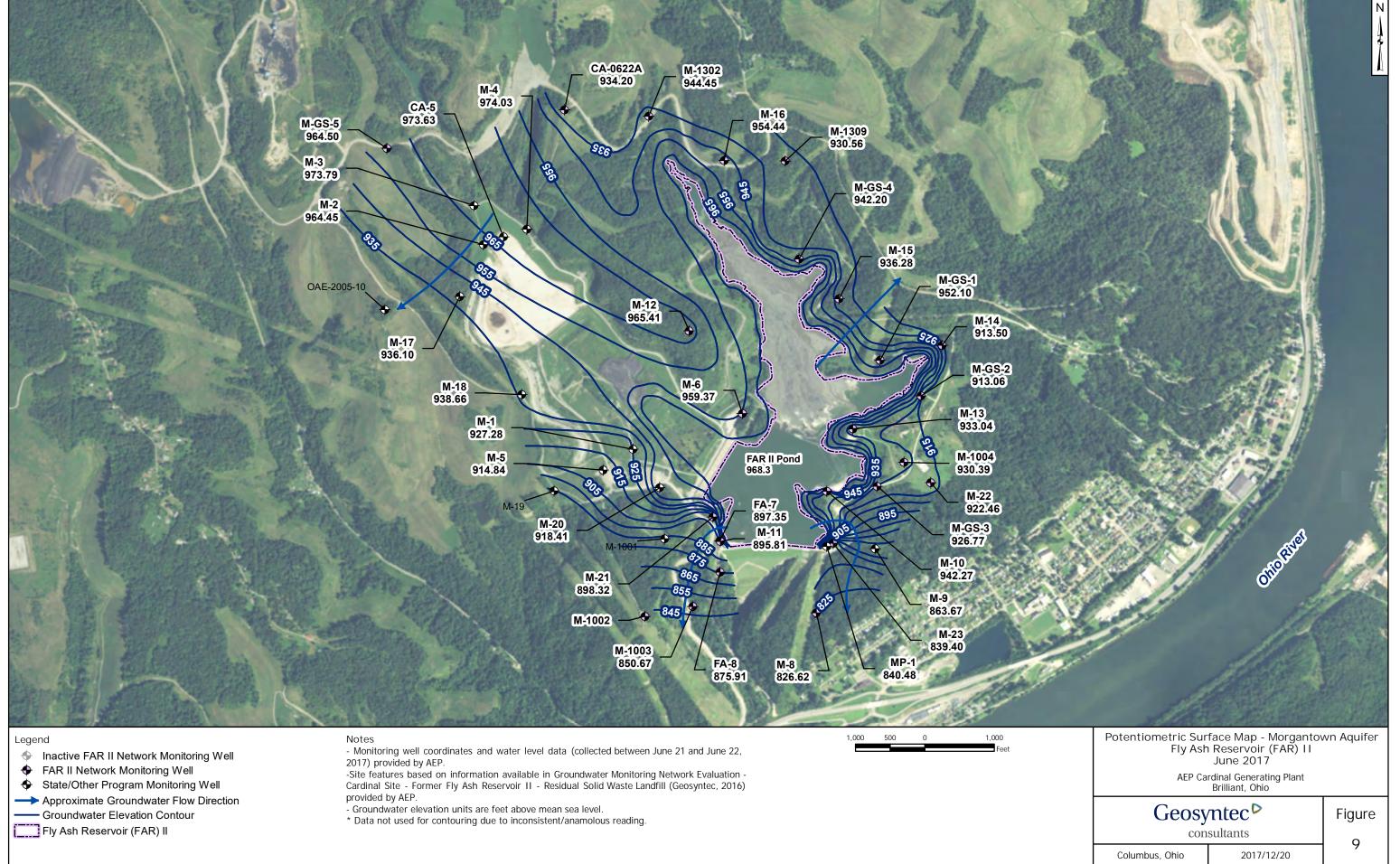


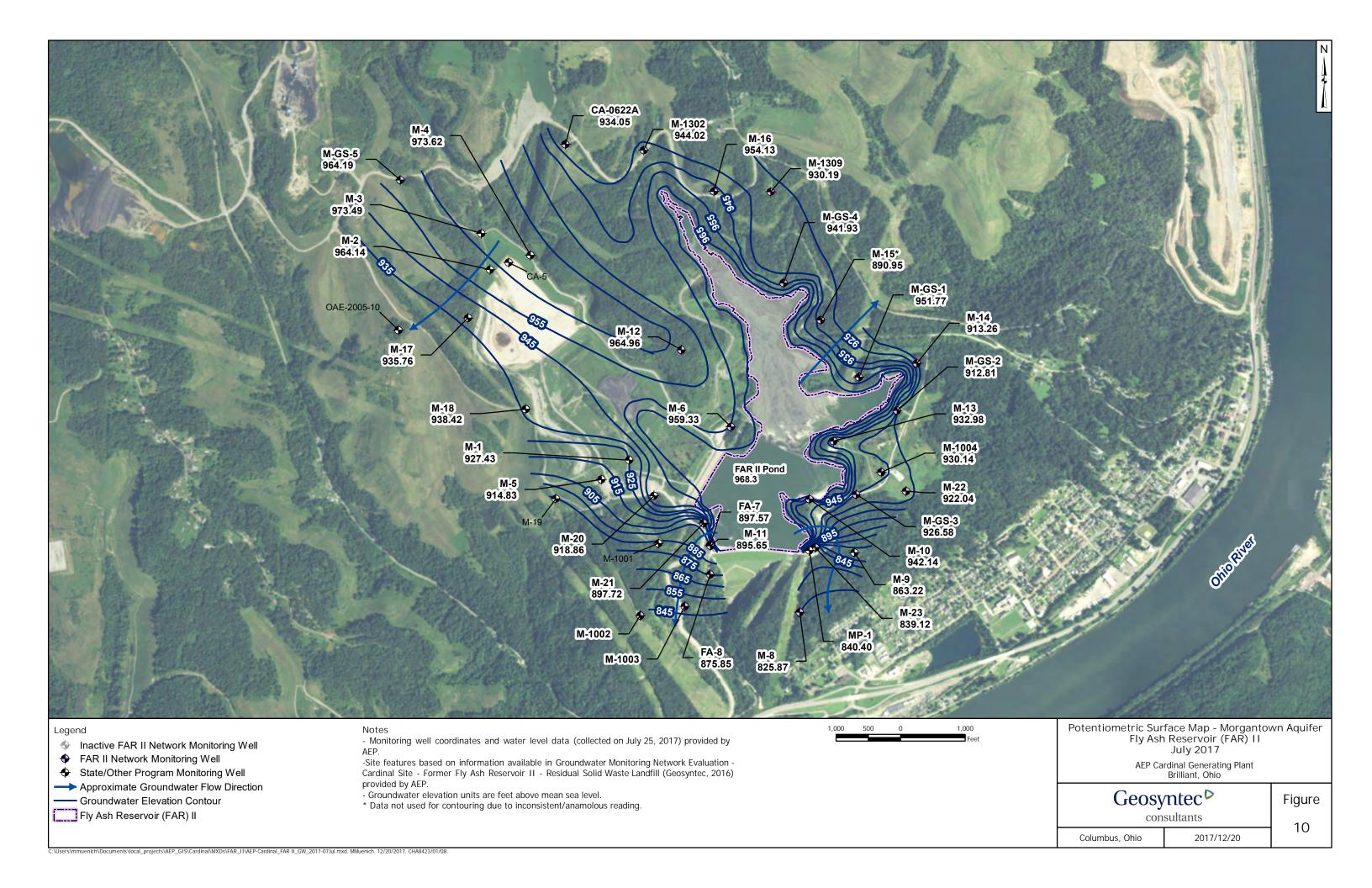


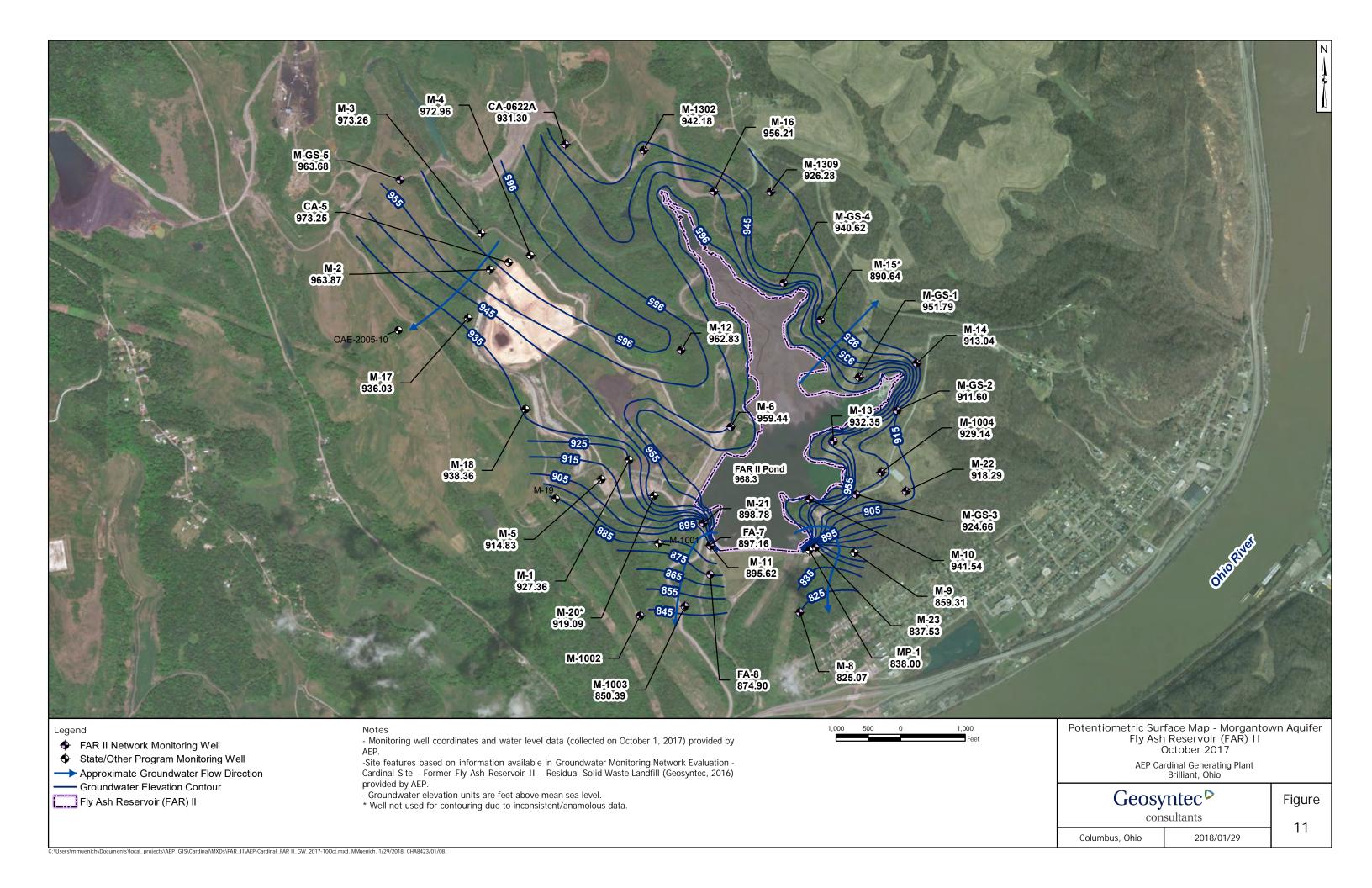












Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(3)

Constituent concentrations, summarized in table form, at each groundwater monitoring well monitored during each sampling event

Table 1: Groundwater Data Summary Cardinal Plant - Fly Ash Reservoir II

						CA-0622				
Parameter	Unit	10/25/2016	11/15/2016	12/14/2016	1/10/2017	4/18/2017	5/25/2017	6/20/2017	7/27/2017	9/27/2017
					Backg	round				Detection
Antimony	μg/L	0.37	1U	0.07	0.14	0.09J	0.03J	0.1J	0.2J	-
Arsenic	μg/L	4.32	6.4	16	19.1	36.9	31.8	24.9	25.4	-
Barium	μg/L	99.8	87.2	140	209	253	325	420	860	-
Beryllium	μg/L	0.142	0.2J	0.127	0.399	0.148	0.572	0.08J	0.08J	-
Boron	mg/L	0.385	0.366	0.293	0.306	0.314	0.447	0.305	0.276	0.331
Cadmium	μg/L	0.03	0.4U	0.02J	0.08	0.03J	0.06	0.02J	0.03J	-
Calcium	mg/L	32.3	22.3	22.8	35.8	37.9	64	48.4	67.7	85.4
Chloride	mg/L	119	135	743	1360	1330	1230	2380	3500	4190
Chromium	μg/L	3.52	5.06	3.96	9.48	5.21	11.6	3.02	3.1	-
Cobalt	μg/L	2.29	2.28	1.9	3.95	2.42	5.76	1.61	1.4	-
Combined Radium	pCi/L	2.059	0.601	1.581	1.947	1.421	2.37	3.78	4.93	-
Fluoride	mg/L	0.95	0.79	0.79	0.85	0.92	0.88	0.7J	0.5J	0.55
Lead	μg/L	3.04	1.86	2.33	5.22	2.71	10.5	1.74	1.59	-
Lithium	mg/L	0.031	0.035	0.036	0.054	0.039	0.059	0.054	0.068	-
Mercury	μg/L	0.006	0.003J	0.005U	0.005U	0.002J	0.005U	0.01	0.002J	-
Molybdenum	μg/L	24.2	45.8	60.1	54.5	48.4	22.2	46	32.5	-
Selenium	μg/L	0.6	1J	0.4	0.9	0.4	2	0.2J	0.2J	-
Total Dissolved Solids	mg/L	1180	28500	2240	3300	3100	2940	4590	5860	7140
Sulfate	mg/L	383	340	320	246	246	244	163	56.3	46.9
Thallium	μg/L	0.04J	1U	0.03J	0.209	0.05J	0.06J	0.2U	0.09J	
pН	SU	7.85	7.81	7.76	7.85	7.69	7.69	7.52	8.86	7.91

Notes:
mg/L: milligrams per liter
µg/L: micrograms per liter
pCi/L: picocuries per liter
SU: standard unit
U: Component was not present in concentrations above method detection limit
and is reported as the reporting limit
J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

						E4.0										M.					
	** *:					FA-8										M-6					
Parameter	Unit	10/27/2016	11/15/2016	12/13/2016	1/10/2017	5/2/2017	6/1/2017	6/27/2017	7/27/2017	9/26/2017	10/11/2016	4/20/2017	4/26/2017	7/26/2017	8/1/2017	8/21/2017	8/29/2017	9/6/2017	9/27/2017	10/4/2017	10/11/2017
					Backg	round				Detection					Backg	round					Detection
Antimony	μg/L	0.61	0.53	0.5	0.49	0.54	0.53	0.5	0.45	-	-	-	-	0.25	0.25	0.21	0.12	0.23	0.23	0.11	-
Arsenic	μg/L	7.85	5.11	4.04	3.91	6.07	5.99	6.47	6.41	-	-	-	-	1.85	3.2	3.31	2.83	3.37	5.36	4.36	-
Barium	μg/L	44.8	33.7	30	28.4	25.1	28.1	25.9	25.1	-	-	-	-	247	292	288	429	306	556	689	-
Beryllium	μg/L	0.058	0.02J	0.009J	0.009J	0.04U	0.008J	0.04U	0.04U	-	-	-	-	0.399	0.705	0.721	1.29	0.915	1.63	2.02	-
Boron	mg/L	5.46	5.05	4.49	4.84	4.64	4.44	5.05	4.34	4.86	-	-	-	0.212	0.201	0.246	0.198	0.287	0.216	0.234	0.248
Cadmium	μg/L	0.04	0.03	0.04	0.04	0.02J	0.02	0.04U	0.06	-	-	-	-	0.08	0.12	0.11	0.2	0.14	0.3	0.27	-
Calcium	mg/L	233	208	192	207	192	192	174	191	211	-	-	-	8.69	10.2	10.4	14.8	13.2	18.9	21.1	23.8
Chloride	mg/L	59.2	58.6	62.7	60.2	57.3	54.4	52.8	52.2	53.1	-	-	38	37.6	37.6	37.2	37.2	37.5	37.2	37.2	38.3
Chromium	μg/L	1.3	0.36	0.161	0.182	0.07J	0.143	0.131	0.324	-	-	-	-	3.16	5.13	5.09	2.99	4.22	4.89	3.28	-
Cobalt	μg/L	5.89	1.91	0.867	0.737	0.56	0.704	0.627	0.72	-	-	-	-	1.41	2.4	2.55	1.96	2.38	2.83	2.45	-
Combined Radium	pCi/L	1.867	1.197	0.943	0.3211	0.5468	0.16	1.123	1.254	-	-	-	-	3.412	4.68	4.33	9.81	4.43	8.11	7.15	-
Fluoride	mg/L	0.55	0.51	0.43	0.47	0.5	0.49	0.49	0.45	0.52	-	-	1.26	1.2	1.32	1.23	1.24	1.27	1.18	1.19	1.24
Lead	μg/L	2.1	0.523	0.279	0.374	0.061	0.156	0.062	0.08	-	-	-	-	7.25	11.4	11.3	16.4	13	26.8	22.7	-
Lithium	mg/L	0.229	0.228	0.206	0.218	0.207	0.198	0.184	0.199	-	-	-	-	0.015	0.012	0.018	0.011	0.018	0.014	0.016	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	-	-	-	0.005U	0.002J	0.005U	0.002J	0.005U	0.2U	0.005U	-
Molybdenum	μg/L	312	361	345	297	302	337	326	308	-	-	-	-	0.66	5.65	0.74	3.31	0.79	0.77	1.18	-
Selenium	μg/L	2.2	2.1	2.1	2.4	2.2	1.4	0.7	0.3	-	-	-	-	0.5	1.8	1.4	1.1	1.4	2.9	1.3	-
Total Dissolved Solids	mg/L	1500	1530	1540	1550	1540	1530	1510	1490	1560	-	-	588	594	580	564	594	612	562	134	288
Sulfate	mg/L	899	907	933	907	875	909	906	886	958	-	-	0.04	0.2	0.2	0.2	0.1J	0.1	0.1U	0.1	1.3
Thallium	μg/L	0.22	0.19	0.153	0.168	0.152	0.187	0.174	0.174		-	-		0.086	0.083	0.106	0.092	0.099	0.127	0.114	
pН	SU	6.49		7.22	7.22	6.79	7.15	7.21	6.97	7.82	8.37	8.4	8.24	7.66	7.18	7.73	8.51	8.25	8.31	8.71	8.4

Notes:

mg/L: milligrams per liter

µg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

							M-8										M.	-10				
Parameter	Unit	12/13/2016	1/16/2017	2/9/2017	3/9/2017	4/12/2017	4/18/2017	5/1/2017	5/31/2017	6/21/2017	8/1/2017	10/3/2017	10/27/2016	4/26/2017	7/26/2017	8/1/2017	8/21/2017	8/29/2017	9/6/2017	9/27/2017	10/4/2017	10/11/2017
		12/10/2010	1,10,201,	2///2017	6/5/2017		ground	0/1/201/	0.01/201/	0/21/2017	0/1/201/	Detection	10/2//2010	1/20/2017	772072017	0/1/201/	Background	0/2//2017	>10/ 2 017	2/2//2017	10/1/201/	Detection
Antimony	ug/L	0.23	0.08	0.02J	0.02J	0.05U	-	_	0.02J	0.05U	0.05U	-	-	-	0.11	0.02J	0.1	0.08	0.05J	0.03J	0.02J	-
Arsenic	μg/L	1.31	0.87	1.22	1.31	1.32	-	-	1.03	1.14	1.18	-	-	-	0.58	0.17	0.84	0.45	0.31	0.36	0.22	-
Barium	μg/L	148	142	119	133	129	-	-	136	125	128	-	-	-	106	97.6	163	109	96	77.5	73.5	-
Beryllium	μg/L	0.02J	0.008J	0.01J	0.01J	0.008J	-	-	0.009J	0.008J	0.01J	-	-	-	0.081	0.02J	0.315	0.078	0.074	0.025	0.01J	-
Boron	mg/L	0.027	0.01	0.038	0.024	0.029	-	-	0.033	0.035	0.01	0.017	-	-	0.51	0.566	0.553	0.501	0.609	0.56	0.661	0.577
Cadmium	μg/L	0.08	0.02J	0.008J	0.02U	0.02U	-	-	0.006J	0.36	0.05	-	-	-	0.89	0.04	0.45	0.46	0.17	0.21	0.07	-
Calcium	mg/L	94.3	88.6	105	98.2	93.6	-	-	92.4	97.5	99.2	93.7	-	-	10.7	13.8	13.7	13.6	14.5	13.3	14.1	13.5
Chloride	mg/L	5.92	5.76	5.79	5.75	-	-	5.86	5.89	5.87	5.8	5.68	-	-	12.3	12.7	12.2	13.1	12.9	12.5	12.9	13.5
Chromium	μg/L	0.38	0.211	0.116	0.06	0.077	-	-	0.096	0.076	0.161	-	-	-	0.777	0.175	1.18	0.547	0.322	0.255	0.04J	-
Cobalt	μg/L	0.438	0.378	0.68	0.454	0.385	-	-	0.309	0.272	0.327	-	-	-	0.173	0.038	0.374	0.146	0.117	0.058	0.032	-
Combined Radium	pCi/L	0.906	1.068	3.78	0.512	1.581	-	-	1.263	1.473	1.162	-	-	-	1.704	1.31	7.382	1.72	1.962	8.7	1.134	-
Fluoride	mg/L	0.09	0.11	0.11	0.11	-	-	0.11	0.1	0.09	0.12	0.09	-	-	0.62	0.76	0.69	0.66	0.74	0.68	0.67	0.66
Lead	μg/L	0.634	0.106	0.297	0.092	0.142	-	-	0.096	0.073	0.072	-	-	-	9.23	0.859	19.6	7.27	4.22	1.23	0.492	-
Lithium	mg/L	0.008	0.006	0.01	0.001U	0.008	-	-	0.011	0.007	0.008	-	-	-	0.02	0.025	0.024	0.014	0.023	0.019	0.02	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	-	-	0.005U	0.005U	0.004J	-	-	-	0.014	0.004J	0.005J	0.002J	0.005U	0.2U	0.01U	-
Molybdenum	μg/L	1.56	0.82	0.74	0.63	0.5	-	-	0.55	0.63	0.47	-	-	-	2.53	2.37	2.11	3.91	2.23	2.3	2.5	-
Selenium	μg/L	0.1	0.1U	0.1U	0.1U	0.1U	-	-	0.04J	0.1U	0.1U	-	-	-	0.05J	0.1U	0.3	0.1	0.03J	0.05J	0.1U	-
Total Dissolved Solids	mg/L	418	417	374	450	-	-	424	420	430	440	435	-	761	745	706	752	740	800	754	734	732
Sulfate	mg/L	94.5	90.5	95.4	93	-	-	94.4	97.4	98.5	97.7	94.2	-	-	127	135	127	135	139	137	128	133
Thallium	μg/L	0.153	0.02J	0.01J	0.05U	0.05U	-		0.01J	0.05U	0.05U		-	-	0.02J	0.05U	0.03J	0.01J	0.02J	0.05U	0.05U	
pН	SU	7.19	6.79	6.93	-	8.31	8.64	7.22	7.21	6.82	8.44	8.34	8.44	7.92	7.66	8.17	8.06	8.42	8.24	8.22	8.47	8.58

Notes:

mg/L: milligrams per liter

µg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

						M-										M-12				
Parameter	Unit	6/30/2016	8/10/2016	10/19/2016	10/26/2016	1/18/2017	4/26/2017	5/24/2017	6/21/2017	8/1/2017	10/4/2017	7/1/2016	8/5/2016	10/26/2016	11/16/2016	1/18/2017	4/20/2017	6/21/2017	8/1/2017	9/28/2017
						Background					Detection				Backg	round				Detection
Antimony	μg/L	0.95	1.1	0.97	-	0.96	0.83	0.63	0.84	0.71	-	0.04J	0.05J	0.05J	0.04J	0.02J	0.08J	0.03J	0.04J	-
Arsenic	μg/L	4.35	3.45	4.28	-	4.04	4.13	4.25	5.05	5.1	-	6.44	7.81	6.24	5.67	4.77	9.68	8.86	6.91	-
Barium	μg/L	26	26.6	27.9	-	29.4	29.4	26.6	25.7	26	-	36	58.8	35	61.2	45.5	51.6	33.9	41.8	-
Beryllium	μg/L	0.005J	0.02U	0.006J	-	0.02U	0.006J	0.02U	0.04U	0.02U	-	0.086	0.06	0.03J	0.038	0.02J	0.054	0.047	0.048	-
Boron	mg/L	4.88	4.91	4.43	-	4.64	4.93	4.87	4.92	5.08	4.69	0.391	0.273	0.295	0.283	0.264	0.266	0.377	0.324	0.276
Cadmium	μg/L	0.01J	0.03	0.01J	-	0.04	0.03	0.02U	0.04U	0.02U	-	0.15	0.09	0.04J	0.04	0.04	0.07	0.04J	0.05	-
Calcium	mg/L	230	207	215	-	201	211	209	203	212	191	341	273	196	112	91	303	307	207	102
Chloride	mg/L	57.2	55.4	58.5	-	57.7	56.9	55	54.6	52.3	52.6	284	288	476	402	658	205	257	391	448
Chromium	μg/L	0.3	0.1	0.1	-	0.168	0.088	0.03J	0.175	0.084	-	0.3	0.3	0.304	0.283	0.386	0.231	0.202	0.555	-
Cobalt	μg/L	0.974	0.749	0.641	-	0.982	0.917	0.546	0.735	0.744	-	26.9	17.5	4.54	4.04	1.3	12.8	13.2	7.39	-
Combined Radium	pCi/L	1.167	0.14	0.46	1.31	0.649	0.333	0.384	2.2142	1.006	-	0.579	1.018	1.408	1.183	2.042	1.302	1.996	6.708	-
Fluoride	mg/L	0.58	0.54	0.58	-	0.58	0.53	0.5	0.47	0.61	0.49	1.38	1.36	1.72	1.64	2.29	1.08	1.2	1.64	2.22
Lead	μg/L	0.171	0.27	0.62	-	6.89	0.757	0.149	0.155	0.127	-	1.44	0.515	0.446	0.494	0.597	1.07	0.366	0.503	-
Lithium	mg/L	0.251	0.208	0.203	-	0.214	0.217	0.223	0.22	0.222	-	0.14	0.107	0.095	0.08	0.06	0.123	0.122	0.098	-
Mercury	μg/L	0.005U	0.005U	0.005U	-	0.005U	0.2U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.01	0.005U	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	316	375	378	-	373	431	362	339	330	-	1.22	1.11	1.23	1.95	2.48	0.98	0.87	1.22	-
Selenium	μg/L	0.3	0.6	2.1	-	3.5	2.7	2	0.8	0.3	-	0.3	0.09J	0.2J	0.09J	0.07J	0.1J	0.2U	0.2U	-
Total Dissolved Solids	mg/L	1480	1510	1570	-	1620	1570	1560	1550	1530	1570	2560	2710	2440	1910	2280	2750	2690	2390	1850
Sulfate	mg/L	881	850	900	-	922	892	835	1000	936	886	1400	1380	898	488	419	1360	1370	1040	416
Thallium	μg/L	0.03J	0.351	0.141	-	0.282	0.375	0.211	0.116	0.074		0.11	0.06J	0.06J	0.02J	0.01J	0.04J	0.02J	0.03J	
pН	SU	7.9	7.62	7.59	-	7.3	7.65	7.67	7.66	7.89	8.44	6.93	6.91	7.09	7.24	7.22	7.6	7.63	7.53	7.9

Notes: mg/L: milligrams per liter

mg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

- Not sampled

-: Not sampled

						M	-13									М	[-14				
Parameter	Unit	6/30/2016	8/11/2016	10/19/2016	1/18/2017	4/13/2017	4/26/2017	6/1/2017	6/22/2017	7/31/2017	9/28/2017	7/5/2016	8/11/2016	10/18/2016	1/11/2017	4/13/2017	4/27/2017	6/1/2017	6/21/2017	7/31/2017	10/10/2017
						Background					Detection					Background	•				Detection
Antimony	μg/L	0.05J	0.02J	0.05J	0.03J	0.06	-	0.05	0.04J	0.04J	-	0.02J	0.01J	0.02J	0.02J	0.05U	-	0.05U	0.01J	0.05U	-
Arsenic	μg/L	1.07	1.36	1.39	1.11	1.23	-	1.3	1.19	1.03	-	0.36	0.29	0.32	0.24	0.17	-	0.16	0.14	0.15	-
Barium	μg/L	79.5	138	99.7	130	122	-	141	128	90.8	-	25	19.3	22.9	20.9	14.9	-	15.9	14.9	14.8	-
Beryllium	μg/L	0.121	0.027	0.139	0.255	0.184	-	0.132	0.114	0.116	-	0.062	0.026	0.044	0.035	0.006J	-	0.01J	0.008J	0.007J	-
Boron	mg/L	0.157	0.254	0.176	0.164	0.198	-	0.243	0.233	0.257	0.287	0.208	0.226	0.188	0.188	0.199	-	0.214	0.218	0.189	0.261
Cadmium	μg/L	0.17	0.02U	0.02	0.01J	0.01J	-	0.03	0.008J	0.009J	-	0.09	0.02U	0.006J	0.005J	0.02U	-	0.02U	0.02U	0.008J	-
Calcium	mg/L	6.77	13.1	8.65	9.01	8.86	-	10.5	11.5	7.71	7.8	1.17	0.736	1.05	0.739	0.526	-	0.534	0.595	0.531	0.485
Chloride	mg/L	2.76	2.03	2.74	2.62		2.65	2.42	2.23	2.53	2.43	1.36	1.29	1.33	1.38		1.39	1.31	1.38	1.49	1.4
Chromium	μg/L	0.9	0.3	0.9	0.285	0.846	-	0.657	0.544	0.606	-	0.4	0.2	0.8	0.332	0.092	-	0.117	0.084	0.127	-
Cobalt	μg/L	0.301	0.096	0.358	0.383	0.389	-	0.29	0.241	0.266	-	0.118	0.047	0.107	0.078	0.01	-	0.022	0.02J	0.01J	-
Combined Radium	pCi/L	2.687	0.976	1.243	4.1	2.36	-	1.854	2.583	2.284	-	1.002	1.088	1.229	1.543	0.3533	-	0.3893	1.469	1.353	-
Fluoride	mg/L	1.26	1.66	1.01	0.99		1.14	1.28	1.43	1.19	1.19	0.79	0.74	0.75	0.7		0.76	0.7	0.74	0.84	0.74
Lead	μg/L	0.859	0.221	1.06	1.33	1.47	-	1.11	0.933	0.853	-	0.763	0.285	0.68	0.512	0.037	-	0.082	0.053	0.031	-
Lithium	mg/L	0.021	0.013	0.008	0.01	0.007	-	0.009	0.015	0.012	-	0.009	0.008	0.003	0.005	0.007	-	0.006	0.0003J	0.008	-
Mercury	μg/L	0.005U	0.005U	0.002J	0.005U	0.005U	-	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	0.59	0.79	0.67	0.7	0.37	-	1.1	0.6	0.58	-	0.3	0.74	0.44	0.55	1.11	-	0.26	0.33	0.39	-
Selenium	μg/L	0.1	0.07J	0.2	0.06J	0.3	-	0.2	0.2	0.2	-	0.2	0.07J	0.1	0.07J	0.1U	-	0.1U	0.1U	0.1U	-
Total Dissolved Solids	mg/L	478	485	459	482	-	482	498	487	492	485	383	380	381	364	-	379	373	1010	395	381
Sulfate	mg/L	11.7	22.1	8.6	5.6		8.9	14	18	10.2	12.4	3	0.9	0.3	0.6	-	0.2	0.5	0.5	0.3	0.5
Thallium	μg/L	0.01J	0.05U	0.114	0.06	0.02J	-	0.02J	0.01J	0.02J	-	0.02J	0.05U	0.172	0.068	0.05U		0.05U	0.01J	0.05U	-
pН	SU	8.55	8.26	8.83	8.61	8.7	8.45	8.23	8.07	8.36	8.63	8.94	9.11	9.39	7.99	9.09	8.97	9.26	9.09	8.62	9.24

Notes:

mg/L: milligrams per liter

µg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

							4.7														
			•			M-	-	1									1-16				
Parameter	Unit	7/6/2016	8/11/2016	10/18/2016	1/17/2017	4/13/2017	4/27/2017	6/1/2017	6/26/2017	7/31/2017	9/27/2017	7/6/2016	8/10/2016	10/14/2016	1/11/2017	4/13/2017	4/27/2017	5/25/2017	6/26/2017	8/1/2017	10/2/2017
						Background					Detection					Background					Detection
Antimony	μg/L	0.02J	0.01J	0.02J	0.03J	0.01J	-	0.02J	0.02J	0.01J	-	0.05U	0.05U	0.05U	0.05U	0.05U	-	0.05U	0.05U	0.05U	-
Arsenic	μg/L	2.54	2.48	2.49	2.09	2.36	-	2.58	2.61	2.34	-	0.34	0.31	0.33	0.28	0.29	-	0.23	0.51	0.28	-
Barium	μg/L	46.3	49.3	51	47.3	46.7	-	55.1	49	44.6	-	44	44.2	42.5	42.5	40.9	-	42.7	40.3	41.8	-
Beryllium	μg/L	0.02U	0.02U	0.01J	0.02U	0.02U	-	0.045	0.01J	0.02U	-	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.02U	0.02U	0.02U	-
Boron	mg/L	0.244	0.266	0.225	0.231	0.227	-	0.243	0.115	0.225	0.272	0.174	0.177	0.171	0.171	0.164	-	0.196	0.235	0.185	0.191
Cadmium	μg/L	0.02U	0.09	0.005J	0.008J	0.02U	-	0.008J	0.08	0.009J	-	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.02U	0.02U	0.02U	-
Calcium	mg/L	1.94	1.9	2.1	1.79	1.6	-	1.67	0.369	1.73	1.85	2.26	2.22	2.09	2.19	2.06	-	2.23	2.24	2.37	2.11
Chloride	mg/L	29.6	27.5	28.1	31	-	29	28.7	28.1	28.1	28.5	9.2	8.98	9.37	8.92	-	9.21	9.04	9.06	8.93	9.26
Chromium	μg/L	0.5	0.1	0.2	0.178	0.01J	-	0.332	0.159	0.18	-	0.5	0.1	0.3	0.093	0.01J	-	0.05J	0.063	0.167	-
Cobalt	μg/L	0.027	0.02	0.059	0.033	0.009J	-	0.145	0.064	0.024	-	0.047	0.014	0.031	0.02	0.007J	-	0.01J	0.01J	0.01J	-
Combined Radium	pCi/L	0.551	1.204	2.224	1.806	0.598	-	0.791	1.242	1.645	-	0.209	0.381	0.6464	0.86	0.312	-	1.184	11.683	0.806	-
Fluoride	mg/L	1.32	1.25	1.29	1.18	-	1.21	1.15	1.14	1.34	1.29	0.41	0.37	0.39	0.38	-	0.37	0.35	0.31	0.4	0.33
Lead	μg/L	0.062	0.055	0.18	0.076	0.02U	-	0.414	0.19	0.078	-	0.065	0.02J	0.045	0.02J	0.006J	-	0.02J	0.02J	0.005J	-
Lithium	mg/L	0.009	0.009	0.004	0.008	0.007	-	0.002	0.004	0.009	-	0.01	0.012	0.012	0.015	0.012	-	0.015	0.008	0.013	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	0.7	0.74	0.74	1.06	0.92	-	0.72	1.18	0.76	-	0.19	0.39	0.15	0.59	0.28	-	0.27	11.4	0.24	-
Selenium	μg/L	0.1J	0.03J	0.04J	0.04J	0.1U	-	0.07J	0.05J	0.1U	-	0.1J	0.1U	0.03J	0.1U	0.1U	-	0.1U	0.1U	0.1U	-
Total Dissolved Solids	mg/L	588	578	612	565	-	567	578	574	588	572	776	764	758	764	-	776	775	778	795	737
Sulfate	mg/L	7.9	4.7	9.4	0.2J		3.2	2.1	2.4	2.6	3.1	252	251	253	242		250	240	252	273	247
Thallium	μg/L	0.081	0.05U	0.03J	0.02J	0.05U	-	0.02J	0.02J	0.05U	-	0.05U	0.05U	0.01J	0.01J	0.05U		0.05U	0.05U	0.05U	
pН	SU	8.79	9.01	8.98	7.64	9.03	8.84	8.74	8.69	9.39	8.34	8.67	9.08	9.01	8.29	8.98	9.27	8.93	8.6	8.66	8.4

Notes:

mg/L: milligrams per liter

µg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

							M-21											M-22					
Parameter	Unit	7/5/2016	8/9/2016	10/19/2016	1/18/2017	4/12/2017	4/20/2017	4/26/2017	5/30/2017	6/22/2017	8/1/2017	10/2/2017	6/30/2016	8/9/2016	10/18/2016	1/17/2017	4/13/2017	4/18/2017	4/27/2017	6/1/2017	6/26/2017	8/1/2017	9/27/2017
		Background Detection											Background										
Antimony	μg/L	0.27	0.09	0.17	0.07	0.3	-	-	0.07J	0.06J	0.04J	-	0.01J	0.05U	0.01J	0.02J	0.05U	-	-	0.05U	0.05U	0.05U	-
Arsenic	μg/L	5.49	2.66	4.24	2.92	5.7	-	-	1.89	2.59	4.62	-	0.49	0.47	0.53	0.62	0.55	-	-	0.56	0.52	0.51	-
Barium	μg/L	12.8	12.2	12.5	12.3	17.4	-	-	10.1	11.3	13.7	-	24.6	23.4	25.3	25.1	24.1	-	-	26.1	25.4	24.4	-
Beryllium	μg/L	0.915	0.379	0.739	0.247	0.512	-	-	0.244	0.191	0.091	-	0.041	0.032	0.038	0.041	0.033	-	-	0.04	0.037	0.037	-
Boron	mg/L	3.08	3.33	2.68	2.88	2.78	-	-	3.15	3.15	3.21	3.08	3.56	3.81	3.39	3.79	3.5	-	-	3.96	4.14	4.08	4.14
Cadmium	μg/L	0.03	0.01J	0.009J	0.006J	0.02J	-	-	0.04U	0.04U	0.01J	-	0.006J	0.01J	0.02U	0.004J	0.02U	-	-	0.02U	0.02U	0.02U	-
Calcium	mg/L	298	282	262	229	214	-	-	254	241	203	247	194	182	189	176	161	-	-	175	185	182	195
Chloride	mg/L	100	79.6	68.6	64.5	-	65.2	-	59.2	64.1	65.8	62.5	46.1	46.3	46.7	47.8	-	-	49.4	49.4	49.6	50.2	51.8
Chromium	μg/L	0.3	0.1	0.2	0.262	0.124	-	-	0.119	0.407	0.492	-	0.1	0.1	0.2	0.258	0.04J	-	-	0.04J	0.052	0.118	-
Cobalt	μg/L	2.95	2.07	2.41	2.02	1.89	-	-	2.66	2.99	1.89	-	0.922	1.17	1.13	1.16	1.17	-	-	1.15	1	1.06	-
Combined Radium	pCi/L	1.007	0.4449	0.393	0.838	0.811	-	-	1.19	1.306	1.849	-	1.947	1.197	3.244	2.084	1.255	-	-	1.494	5.156	0.769	-
Fluoride	mg/L	0.1J	0.1J	0.1J	0.1J	-	0.1J	-	0.07J	0.08J	0.1J	0.08	0.46	0.4	0.44	0.41	-	-	0.42	0.4	0.37	0.43	0.35
Lead	μg/L	1.52	0.446	0.983	0.544	1.67	-	-	0.392	0.349	0.328	-	0.007J	0.02J	0.039	0.029	0.027	-	-	0.01J	0.02J	0.02U	-
Lithium	mg/L	0.082	0.09	0.074	0.082	0.074	-	-	0.08	0.082	0.081	-	0.068	0.054	0.046	0.057	0.052	-	-	0.062	0.064	0.068	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	-	-	0.005U	0.005U	0.005U		0.005U	0.005U	0.005U	0.005U	0.005U	-	-	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	20.9	14.9	17.2	18.9	16.6	-	-	16.9	18.8	16.9		90.8	92.5	97.8	92.7	89	-	-	94.3	93.6	86.8	-
Selenium	μg/L	0.4	0.2	0.3	0.1	0.5	-	-	0.1J	0.07J	0.2U		0.1	0.1U	0.04J	0.05J	0.04J	-	-	0.05J	0.04J	0.1U	-
Total Dissolved Solids	mg/L	1940	1840	1810	1850	-	1850	891	1770	1830	1840	1830	883	913	916	929	-	-	938	946	961	963	985
Sulfate	mg/L	1070	995	990	986	-	990	-	1020	1030	1080	998	378	386	383	390	-	-	399	403	409	415	435
Thallium	μg/L	0.02J	0.02J	0.055	0.02J	0.05U	-	-	0.05J	0.1U	0.1U		0.04J	0.01J	0.064	0.082	0.05U	-		0.05U	0.01J	0.05U	
pH	SU	6.94	7.07	7.53	6.42	7.03	8.02	-	6.98	7.43	8.11	7.14	6.79	6.88	6.99	6.63	6.93	7.92	6.92	6.84	7.21	8.08	7.14

Notes:

mg/L: milligrams per liter

µg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

- Not sampled

-: Not sampled

							M-23						M-1003									
Parameter	Unit	6/30/2016	8/9/2016	10/19/2016	1/16/2017	4/12/2017	4/18/2017	5/1/2017	6/1/2017	6/26/2017	8/1/2017	10/3/2017	6/29/2016	10/25/2016	1/12/2017	5/2/2017	5/31/2017	6/22/2017	8/1/2017	8/4/2016	9/28/2017	
		Background Detection												Background								
Antimony	μg/L	0.03J	0.03J	0.02J	0.03J	0.03J	-	-	0.2U	0.2U	0.2U	-	0.02J	0.04J	0.03J	0.01J	0.01J	0.01J	0.02J	0.05J	-	
Arsenic	μg/L	1.31	1.22	1.08	1.18	0.97	-		1.11	1.21	1.15	-	0.35	1.05	0.73	0.32	0.36	0.21	0.68	1.92	-	
Barium	μg/L	9.59	9.26	8.85	9.33	9.04	-		9.75	8.67	8.76	-	76.4	90.5	81.7	73.6	82	71.7	78.2	102	-	
Beryllium	μg/L	0.02J	0.02J	0.02J	0.023	0.02J	-		0.02J	0.02J	0.02J	-	0.01J	0.039	0.027	0.009J	0.02J	0.007J	0.035	0.055	-	
Boron	mg/L	0.62	0.667	0.578	0.589	0.615	-		0.637	0.717	0.63	0.601	0.085	0.139	0.106	0.178	0.138	0.189	0.124	0.138	0.124	
Cadmium	μg/L	0.04U	0.04U	0.02J	0.009J	0.04U	-	-	0.06U	0.06U	0.06U	-	0.02U	0.006J	0.02U	0.02U	0.02U	0.02U	0.02U	0.01J	-	
Calcium	mg/L	121	106	112	92.1	89	-	-	106	100	110	104	67.7	65.1	59.2	60.8	63	68.8	62.8	56.8	61.6	
Chloride	mg/L	13	13.2	12.6	13.3	-	ı	13.7	12.3	13.8	12.5	12.8	4.51	4.61	4.58	4.66	4.76	4.72	4.92	4.56	4.9	
Chromium	μg/L	0.3	0.2	0.1	0.353	0.08J	-		0.291	0.208	0.575	-	0.4	0.488	0.494	0.113	0.16	0.107	0.562	0.7	-	
Cobalt	μg/L	0.455	0.438	0.376	0.377	0.29	-		0.57	0.598	0.486	-	0.195	0.304	0.3	0.249	0.267	0.289	0.346	0.828	-	
Combined Radium	pCi/L	3.12	2.108	2.033	6.71	2.879	-		2.77	3.92	3.08	-	2.104	4.18	5.06	2.57	2.8	3.13	3.97	4.06	-	
Fluoride	mg/L	0.59	0.57	0.55	0.57	-	-	0.54	0.5	0.52	0.6J	0.48	0.22	0.21	0.19	0.19	0.18	0.17	0.23	0.21	0.19	
Lead	μg/L	0.112	0.156	0.068	0.188	0.061	-	-	0.188	0.207	0.069	-	0.13	0.415	0.323	0.125	0.19	0.135	0.361	0.679	-	
Lithium	mg/L	0.055	0.049	0.048	0.054	0.048	-	-	0.05	0.05	0.059	-	0.026	0.012	0.013	0.012	0.008	0.015	0.013	0.011	-	
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.01U	-	-	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	
Molybdenum	μg/L	4.99	0.34	3.8	0.59	0.7	-	-	1.03	1.98	0.46	-	2.63	0.3	0.31	2.9	0.17	0.18	0.43	2.06	-	
Selenium	μg/L	0.2	0.2U	0.2U	0.06J	0.2U	-	-	0.3U	0.3U	0.3U	-	0.1J	0.1J	0.04J	0.03J	0.04J	0.1U	0.05J	0.06J	-	
Total Dissolved Solids	mg/L	3300	3280	3300	3240	-	-	3140	3220	3210	2980	3210	461	429	448	470	475	487	470	443	488	
Sulfate	mg/L	1730	1690	1660	1560	-	-	1610	1650	1690	1830	1620	88.7	91.9	89.8	105	106	113	91.9	79.9	105	
Thallium	μg/L	0.03J	0.04J	0.06J	0.078	0.02J			0.2U	0.2U	0.2U	-	0.05U	0.03J	0.01J	0.05U	0.02J	0.05U	0.02J	0.02J	-	
pН	SU	7	6.99	6.95	6.63	7.69	8.19	7.26	7.51	7.07	8.11	7.96	7.67	7.68	7.37	7.42	7.21	7.77	6.9	7.55	7.47	

Notes:
mg/L: milligrams per liter
μg/L: micrograms per liter
pCi/L: picocuries per liter
SU: standard unit
U: Component was not present in concentrations above method detection limit and is reported as the reporting limit
J: Estimated value. Component was detected in concentrations below the reporting limit
-: Not sampled

							M-1004						M-1302											
Parameter	Unit	6/30/2016	8/9/2016	10/18/2016	1/17/2017	4/12/2017	4/18/2017	4/27/2017	6/1/2017	6/22/2017	7/31/2017	9/27/2017	7/1/2016	8/10/2016	10/13/2016	11/15/2016	1/11/2017	4/13/2017	4/27/2017	5/25/2017	6/21/2017	8/1/2017	10/2/2017	
	_	Background Detection										Background												
Antimony	μg/L	0.03J	0.05	0.07	0.01J	0.06	-	-	0.03J	0.02J	0.03J	-	0.05U	0.05U	0.05U	0.05U	0.01J	0.05U	-	0.05U	0.05U	0.05U	-	
Arsenic	μg/L	2.48	4.01	7.55	2.41	5.26	-	-	3.53	2.94	3.19	-	0.09	0.07	0.08	0.06	0.12	0.05J	-	0.05J	0.05J	0.05J	-	
Barium	μg/L	73.1	94.3	102	84.1	90.4	-	-	76.9	67.3	82.3	-	83.6	84.8	94.5	81.9	88.5	95.5	-	69.4	83.9	85.8	-	
Beryllium	μg/L	0.09	0.145	0.247	0.093	0.133	-	-	0.11	0.085	0.127	-	0.02U	0.02U	0.02U	0.006J	0.02U	0.02U	-	0.02U	0.02U	0.02U	-	
Boron	mg/L	1.63	2.05	1.72	1.78	1.54	-	-	1.9	2.02	1.97	2.25	0.237	0.243	0.26	0.245	0.248	0.297	-	0.255	0.264	0.303	0.302	
Cadmium	μg/L	0.006J	0.009J	0.02J	0.02U	0.008J	-	-	0.02J	0.02U	0.007J	-	0.005J	0.007J	0.02U	0.02U	0.007J	0.02U	-	0.02U	0.02U	0.02U	-	
Calcium	mg/L	99.9	96.1	95.6	85.6	80.8	-	-	82.5	89.6	85.4	100	4.29	3.87	3.81	3.93	3.93	3.6	-	3.66	3.91	3.88	3.49	
Chloride	mg/L	26.1	27.8	29.6	28.5	-	-	29.3	30.3	32.5	30.1	32.1	26.3	25	25.8	25.8	24.5	-	25.7	25.5	26.2	25.8	26.5	
Chromium	μg/L	1.9	3.6	7.4	1.83	4.02	-	-	2.6	2.08	3.3	-	0.4	0.2	0.2	0.181	0.138	0.064	-	0.04J	0.056	0.163	-	
Cobalt	μg/L	0.588	1.13	2.57	0.369	1.48	-	-	0.902	0.756	0.988	-	0.019	0.015	0.02	0.012	0.038	0.01	-	0.007J	0.008J	0.009J	-	
Combined Radium	pCi/L	1.971	1.784	2.213	2.917	1.263	-	-	5.503	2.42	1.954	-	0.2456	0.404	0.878	0.795	1.08	0.186	-	0.76	1.1	1.806	-	
Fluoride	mg/L	1.46	1.54	1.53	1.65	-	-	1.86	1.48	1.37	1.69	1.38	1.31	1.14	1.43	1.11	1.26		1.16	1.03	1.23	1.38	1.37	
Lead	μg/L	0.527	1.11	2.69	0.544	1.34	-	-	1.03	0.743	0.943	-	0.035	0.039	0.025	0.01J	0.022	0.01J	-	0.02J	0.008J	0.004J		
Lithium	mg/L	0.035	0.023	0.02	0.022	0.025	-	-	0.023	0.026	0.024	-	0.015	0.013	0.014	0.014	0.018	0.013	-	0.013	0.013	0.016		
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	-	-	0.005U	0.005U	0.005U	-	0.005U	0.01U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U		
Molybdenum	μg/L	9.44	11.4	11.8	9.87	8.92	-	-	9.92	10.8	9.73	-	0.36	0.27	0.11	0.12	0.58	0.07J	-	0.3	0.1	0.19	-	
Selenium	μg/L	0.2	0.1	0.4	0.09J	0.2	-	-	0.1	0.1	0.2	-	0.08J	0.1U	0.1U	0.04J	0.1U	0.1U	-	0.1U	0.1U	0.1U	-	
Total Dissolved Solids	mg/L	1010	914	841	877	-	-	855	900	874	874	848	746	765	730	788	754	-	775	789	791	762	712	
Sulfate	mg/L	363	272	265	249	-	-	207	269	276	249	267	115	118	93.7	114	95	-	123	126	108	111	82.7	
Thallium	μg/L	0.01J	0.03J	0.053	0.02J	0.03J	-		0.02J	0.01J	0.02J		0.01J	0.05U	0.02J	0.02J	0.04J	0.05U		0.05U	0.05U	0.05U		
pН	SU	7.41	7.52	7.38	7.31	7.28	7.16	7.36	6.87	7.7	7.23	7.35	8.66	8.66	8.13	8.53	8.59	8.89	8.57	8.79	8.73	8.42	8.44	

Notes:
mg/L: milligrams per liter
µg/L: micrograms per liter
pCi/L: picocuries per liter
SU: standard unit
U: Component was not present in concentrations above method detection limit
and is reported as the reporting limit
J: Estimated value. Component was detected in concentrations below the reporting limit
-- Not sampled

-: Not sampled

						M-1309								MGS-1						
Parameter	Unit	7/6/2016	8/10/2016	10/26/2016	1/17/2017	5/1/2017	6/1/2017	6/26/2017	7/31/2017	9/28/2017	11/16/2016	12/13/2016	1/11/2017	2/9/2017	5/1/2017	6/1/2017	6/21/2017	7/31/2017	10/5/2017	
1 ai ainetei	Oint	7/0/2010	0/10/2010	10/20/2010	Backg		0/1/201/	0/20/2017	//31/2017	Detection	11/10/2010	12/13/2010	1/11/2017		round	0/1/201/	0/21/2017	//31/2017	Detection	
Antimony	μg/L									Detection	0.07 0.03J 0.02J 0.01J 0.01J 0.05U 0.05U 0.01J									
Arsenic	μg/L μg/L	3.53	2.68	2.56	2.51	2.43	2.33	2.85	0.01J 2.45		1.03	0.36	0.26	0.24	0.15	0.13	0.030	0.013	-	
Barium	μg/L μg/L	35.6	39.3	38.4	37.9	35.8	38.8	33.6	33.6		113	114	108	101	102	107	96.8	98.2	-	
Beryllium	μg/L μg/L	0.1	0.073	0.056	0.023	0.022	0.024	0.02J	0.01J		0.045	0.01J	0.01J	0.009J	0.01J	0.01J	0.009J	0.01J	-	
Boron	mg/L	0.282	0.264	0.276	0.252	0.256	0.284	0.345	0.268	0.278	0.312	0.26	0.28	0.342	0.304	0.313	0.286	0.25	0.268	
Cadmium	ug/L	0.02	0.03	0.02J	0.007J	0.01J	0.006J	0.02U	0.02U	0.276	0.02U	0.004J	0.02U	0.02U	0.02U	0.02U	0.02U	0.02U	-	
Calcium	mg/L	17.4	16.8	14.1	12.3	11.3	11.1	9.2	8.85	9.55	4.39	4.31	4.91	5.71	6.48	6.27	6.64	6.75	7.22	
Chloride	mg/L	46	40	39.6	38.6	39.9	38.6	38	39	39.2	19.8	35.9	34.5	34.6	36	35.7	36.5	36.5	36.7	
Chromium	μg/L	1.5	1	0.828	0.319	0.398	0.224	0.187	0.154	37.2	0.504	0.14	0.175	0.04J	0.218	0.078	0.079	0.127	-	
Cobalt	μg/L	2.34	1.16	0.904	0.506	0.51	0.406	0.336	0.271	_	0.226	0.066	0.025	0.214	0.02J	0.01J	0.01J	0.02J	_	
Combined Radium	pCi/L	0.527	0.825	2.84	0.562	0.642	0.695	4.64	3.539	_	2.668	1.745	1.495	0.932	0.526	0.77	1.156	0.959	_	
Fluoride	mg/L	1.34	1.11	1.13	1.2	1.21	1.04	1.03	1.26	1.1	0.62	0.45	0.52	0.54	0.56	0.51	0.5	0.64	0.5	
Lead	ug/L	1.17	0.924	0.718	0.231	0.261	0.232	0.181	0.122	-	0.665	0.085	0.043	0.025	0.02J	0.01J	0.009J	0.005J	-	
Lithium	mg/L	0.04	0.029	0.026	0.029	0.026	0.021	0.025	0.027	_	0.025	0.021	0.019	0.024	0.019	0.014	0.018	0.016	_	
Mercury	μg/L	0.005U	0.002J	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	
Molybdenum	μg/L	8.94	7.48	5.54	4	3.04	3.46	22.1	2.8	-	0.67	1.71	1.82	0.96	0.73	0.31	0.44	0.56	-	
Selenium	μg/L	0.3	0.2	0.2	0.1J	0.08J	0.05J	0.06J	0.1U	-	0.1	0.1U	0.1U	0.1U	0.03J	0.1U	0.1U	0.1U	-	
Total Dissolved Solids	mg/L	1070	866	815	804	789	778	790	760	769	496	642	628	642	624	640	640	644	632	
Sulfate	mg/L	323	222	193	172	151	149	161	154	149	41.3	84.3	86.5	80.6	83.8	85.6	87.3	87.6	82.4	
Thallium	μg/L	0.02J	0.01J	0.02J	0.05U	0.05U	0.02J	0.05U	0.05U	-	0.02J	0.04J	0.05U	0.01J	0.05U	0.05U	0.05U	0.05U	-	
pН	SU	7.77	7.79	8.21	7.9	7.99	7.67	8	9.14	8.2	6.86	7.57	7.58	7.18	7.75	8.27	7.95	7.27	8.58	

Notes:

mg/L: milligrams per liter

mg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

						MGS-2									MGS-3				
Parameter	Unit	7/5/2016	8/11/2016	12/13/2016	1/17/2017	5/1/2017	6/1/2017	6/21/2017	7/31/2017	10/5/2017	6/30/2016	8/9/2016	11/16/2016	1/11/2017	5/1/2017	6/1/2017	6/21/2017	7/27/2017	10/5/2017
					Backg	round				Detection				Backg	round				Detection
Antimony	μg/L	1.88	1.05	0.64	0.42	0.24	0.22	0.22	0.19	-	2.27	0.65	0.64	0.6	0.3	0.26	0.28	0.22	-
Arsenic	μg/L	16.4	16.3	18.2	16.4	12.1	14.4	13.5	12.3	-	79.1	83.5	101	76.5	11.9	12.7	19.9	20.3	-
Barium	μg/L	42.8	36.3	36.4	32.3	29.2	34.6	32.3	31.1	-	26.2	20.2	19.4	17	13.6	13.9	14.5	12	-
Beryllium	μg/L	0.02J	0.006J	0.009J	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.01J	0.01J	0.03J	0.02J	0.009J	0.01J	0.01J	0.01J	-
Boron	mg/L	0.207	0.222	0.202	0.226	0.208	0.245	0.249	0.205	0.203	0.381	0.614	0.756	0.759	0.423	0.507	0.637	0.734	0.87
Cadmium	μg/L	0.01J	0.006J	0.007J	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.02J	0.05	0.02J	0.02J	0.02J	0.01J	0.02J	0.01J	-
Calcium	mg/L	15.6	14.4	6.77	6.15	6.28	7.65	5.11	6.02	3.97	192	109	102	100	222	216	194	156	94.4
Chloride	mg/L	18.5	20.9	25.8	24.6	25.8	25.8	26.8	26	26.5	15.9	20.7	26.3	25.5	16.3	14.4	20.6	21.8	28.7
Chromium	μg/L	0.6	0.2	0.218	0.15	0.221	0.073	0.058	0.155	-	0.3	0.3	0.498	0.311	0.296	0.103	0.129	0.356	-
Cobalt	μg/L	0.596	0.517	0.603	0.731	0.627	0.449	0.473	0.521	-	5.47	7.16	6.8	4.54	2.15	1.55	2.08	2.2	-
Combined Radium	pCi/L	0.537	0.0543	0.568	1.141	0.2828	0.333	0.853	1.169	-	1.308	1.72	1.253	1.942	1.18	1.888	1.937	1.518	-
Fluoride	mg/L	0.43	0.41	0.36	0.42	0.43	0.39	0.4	0.48	0.4	0.34	0.34	0.34	0.29	0.23	0.2	0.21	0.2	0.24
Lead	μg/L	0.428	0.126	0.154	0.064	0.055	0.031	0.023	0.01J	-	0.461	0.289	0.457	0.479	0.15	0.088	0.263	0.173	-
Lithium	mg/L	0.013	0.016	0.009	0.015	0.016	0.016	0.01	0.017	-	0.06	0.05	0.067	0.062	0.041	0.04	0.05	0.055	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	7.11	5.11	1.65	2.6	1.39	0.93	0.95	1.19	-	9.26	4.64	3.14	3.29	1.9	6.93	2.95	1.77	-
Selenium	μg/L	0.2	0.1	0.07J	0.1U	0.1U	0.05J	0.04J	0.05J	-	0.4	0.06J	0.07J	0.2U	0.2U	0.2U	0.2U	0.2U	-
Total Dissolved Solids	mg/L	644	626	592	572	586	613	597	602	600	2130	2250	2480	2430	2110	2070	2280	2320	2320
Sulfate	mg/L	152	131	91.2	82.9	93.1	95	85.3	101	79.6	1270	1310	1470	1470	1240	1260	1460	1370	1360
Thallium	μg/L	0.01J	0.05U	0.01J	0.03J	0.05U	0.05U	0.05U	0.05U	-	0.241	0.353	0.37	0.39	0.212	0.234	0.292	0.368	-
pН	SU	7.46	7.77	7.59	7.59	7.77	7.21	7.95	7.29	8.45	6.85	6.72	6.14	6.57	6.86	6.98	7.38	8.77	7.84
		Notes: mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit U: Component was not present in concentrations above method detection limit and is reported as the reporting limit J: Estimated value. Component was detected in concentrations below the reporting limit -: Not sampled																	

						3400 A									3.500 F				
_			1			MGS-4	1	1		1		1	1	1	MGS-5	1	1		
Parameter	Unit	11/16/2016	12/13/2016	1/11/2017	2/9/2017	5/1/2017	6/1/2017	6/21/2017	7/31/2017	10/2/2017	11/17/2016	12/14/2016	1/10/2017	2/9/2017	4/17/2017	5/30/2017	6/21/2017	7/27/2017	10/3/2017
					Backg	round				Detection				Backg	round				Detection
Antimony	μg/L	0.37	0.08	0.13	0.05	0.06	0.05	0.05J	0.04J	-	2.9	0.36	0.19	0.12	0.1	0.07	0.06	0.05J	-
Arsenic	μg/L	3.57	5.09	9.43	11.1	15	15.6	15.8	14.9	-	5.47	22.7	20.8	20.7	25	24.1	23.1	22.6	-
Barium	μg/L	42	33.5	29.5	23	20.1	19.9	18	16.4	-	71.6	66.9	64	59.1	75.1	77.5	75.4	78.7	-
Beryllium	μg/L	0.066	0.02J	0.01J	0.005J	0.004J	0.02U	0.02U	0.02U	-	0.02J	0.073	0.029	0.01J	0.009J	0.006J	0.02U	0.02U	-
Boron	mg/L	0.178	0.151	0.161	0.267	0.209	0.175	0.191	0.142	0.183	0.168	0.251	0.297	0.36	0.287	0.3	0.302	0.3	0.287
Cadmium	μg/L	0.03	0.02J	0.02J	0.007J	0.02U	0.02U	0.02U	0.02U	-	0.01J	0.02	0.007J	0.009J	0.02U	0.02U	0.02U	0.01J	-
Calcium	mg/L	96.4	131	97.3	74	30.1	27.2	20.2	16.5	11.1	25.5	7.05	5.92	5.36	4.4	4.2	4.09	3.73	3.62
Chloride	mg/L	14.3	16.3	15.3	14.5	13.2	12.8	12.1	11.9	11.5	78.1	163	155	159	159	162	166	161	174
Chromium	μg/L	0.983	0.344	0.841	0.257	0.081	0.118	0.076	0.12	-	0.231	0.517	0.594	0.348	0.173	0.117	0.106	0.217	-
Cobalt	μg/L	3.93	3.21	2.61	1.4	0.484	0.507	0.28	0.243	-	1.46	0.656	0.165	0.065	0.052	0.027	0.024	0.032	-
Combined Radium	pCi/L	0.3336	1.105	1.752	0.635	0.571	0.184	0.3165	0.771	-	0.833	1.572	0.9	2.573	0.3421	1.232	1.868	2.005	-
Fluoride	mg/L	0.31	0.24	0.32	0.34	0.46	0.43	0.45	0.62	0.53	1.61	4.41	4.66	4.73	4.74	4.97	5.27	5.09	5.09
Lead	μg/L	1.62	0.304	0.409	0.133	0.062	0.047	0.027	0.01J	-	0.206	2.22	1.16	0.323	0.19	0.099	0.059	0.127	-
Lithium	mg/L	0.037	0.03	0.031	0.035	0.019	0.018	0.018	0.015	-	0.021	0.019	0.023	0.023	0.012	0.018	0.017	0.017	-
Mercury	μg/L	0.002J	0.005U	0.002J	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.002J	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	7.97	5.2	5.87	4.57	2.87	3.47	2.58	2.66	-	42	19	16	12.6	10.6	8.95	8.05	9.78	-
Selenium	μg/L	0.3	0.1	0.08J	0.05J	0.06J	0.04J	0.1U	0.1U	-	0.2	0.2	0.06J	0.08J	0.1U	0.03J	0.1U	0.03J	-
Total Dissolved Solids	mg/L	1540	1630	1410	1200	850	848	717	672	586	798	542	1140	1110	-	1080	1100	1060	1090
Sulfate	mg/L	848	914	741	556	283	298	189	152	98.3	211	94.3	74.3	45.8	29.9	24.8	20.4	14.6	2.7
Thallium	μg/L	0.04J	0.02J	0.125	0.02J	0.05U	0.05U	0.05U	0.05U	-	0.02J	0.02J	0.065	0.04J	0.05U	0.05U	0.05U	0.05U	-
pН	SU	6.64	7.21	7.36	7.1	7.84	7.87	8.03	8.97	8.34	6.98	8.29	8.53	7.96	8.72	8.79	8.47	8.11	8.27

Notes:

mg/L: milligrams per liter

mg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

Table 1: Groundwater Data Summary Cardinal Plant - Fly Ash Reservoir II

		CA-0	622A		FA-8		M		M	-8		M-10			M-11	
Parameter	Unit			1/24/2018		8/29/2018	5/16/2018				1/24/2018		8/23/2018	1/24/2018		8/27/2018
		Assess		Detection		sment	Asses			sment	Detection	Asses		Detection		ssment
Antimony	μg/L	0.100 J	0.5 U	-	0.460	0.530	0.110	0.5 U	0.0400 J	0.5 U	-	0.0200 J	0.5 U	-	0.570	0.5 U
Arsenic	μg/L	37.2	24.4	-	5.49	6.60	4.58	0.5 U	2.65	2.40	-	0.200	0.5 U	-	4.62	4.70
Barium	μg/L	1060	1240	-	25.1	23.1	413	189	120	126	-	69.7	88.4	-	26.0	26.1
Beryllium	μg/L	0.100	0.140	-	0.02 U	0.1 U	1.35	0.1 U	0.0300	0.1 U	-	0.0100 J	0.120	-	0.02 U	0.1 U
Boron	mg/L	0.368	0.331	5.16	4.97	5.47	0.247	0.229	0.0800	0.0282	0.599	0.663	0.591	5.10	5.17	5.24
Cadmium	μg/L	0.0300 J	0.1 U	-	0.0300	0.140	0.280	0.1 U	0.0400	0.1 U	-	0.0300	0.370	-	0.0300	0.160
Calcium	mg/L	80.8	67.8	-	214	196	17.1	5.51	102	89.6	-	12.6	12.6	-	224	205
Chloride	mg/L	3930	4300	-	54.7	6.80	37.1	37.5	6.15	52.3	-	13.4	13.8	-	53.3	50.4
Chromium	μg/L	1.98	3.70	-	0.206	1.30	3.35	1 U	1.14	1 U	-	0.208	1 U	-	0.149	1 U
Cobalt	μg/L	1.61	1.40	-	0.570	0.760	3.39	0.5 U	1.34	1.30	-	0.0360	0.5 U	-	0.699	0.760
Combined Radium	pCi/L	4.76	8.73	-	0.530	0.524	93.3	2.69	1.19	1.44	-	0.758	0.885	-	0.712	0.775
Fluoride	mg/L	0.600 J	0.05 U	-	0.590	0.0940	1.24	1.20	0.100 J	0.510	-	0.790	0.710	-	0.590	0.500
Lead	μg/L	2.63	2.30	-	0.167	0.5 U	22.7	0.520	1.35	1.20	-	0.664	5.10	-	0.315	0.5 U
Lithium	mg/L	0.0820	0.0738	-	0.204	0.218	0.00700	10 U	0.001 U	10 U	=	0.0150	0.0198	-	0.213	0.211
Mercury	μg/L	0.005 U	0.0123	-	0.005 U	0.5 U	0.00900	0.0166	0.00200 J	0.00179	-	0.005 U	0.00300	-	0.005 U	0.000530
Molybdenum	μg/L	18.9	8.00	-	285	336	0.510	0.5 U	0.550	0.900	-	2.25	2.30	-	324	337
рН	SU	7.63	7.98	7.34	7.29	7.29	8.15	8.35	7.29	7.30	8.01	8.13	7.42	7.75	7.47	7.11
Selenium	μg/L	0.400 J	0.5 U	-	2.20	0.810	2.40	0.5 U	0.100	0.5 U	-	0.0400 J	0.5 U	-	2.80	0.5 U
Total Dissolved Solids	mg/L	5960	6980	-	1530	1520	598	548	428	437	-	749	726	-	1600	1550
Sulfate	mg/L	57.8	62.5	945	937	99.3	1.30	0.370	99.1	959	-	128	146	-	942	849
Thallium	μg/L	0.0600 J	0.5 U	-	0.148	0.5 U	0.146	0.5 U	0.0400 J	0.5 U	-	0.0200 J	0.5 U	-	0.343	0.5 U

mg/L: milligrams per liter μg/L: micrograms per liter SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Fly Ash Reservoir II

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		M-	·12	M-	·13	M-	14	M-	-15	M-	-16		M-21			M-22	
Parameter	Unit	5/16/2018	8/27/2018	5/16/2018	8/23/2018	5/16/2018	8/23/2018	5/16/2018	8/22/2018	5/16/2018	8/22/2018	1/24/2018	5/22/2018	8/28/2018	1/24/2018	5/17/2018	8/23/2018
		Asses	sment	Asses	sment	Assess	sment	Asses	sment	Asses	sment	Detection	Asses	ssment	Detection	Asses	sment
Antimony	μg/L	0.0500 J	0.5 U	0.0200 J	0.5 U	0.05 U	0.5 U	0.0200 J	0.5 U	0.05 U	0.5 U	-	0.0800 J	0.5 U	-	0.0100 J	0.5 U
Arsenic	μg/L	7.18	4.30	0.860	0.690	0.120	0.5 U	2.50	2.00	0.300	0.5 U	-	1.90	2.80	1	0.460	0.5 U
Barium	μg/L	58.6	27.2	100	122	14.1	14.3	50.2	46.3	39.4	37.8	-	9.87	13.0	-	26.3	27.8
Beryllium	μg/L	0.0740	0.1 U	0.0620	0.1 U	0.02 U	0.1 U	0.0100 J	0.1 U	0.02 U	0.1 U	-	0.419	0.1 U	-	0.0380	0.1 U
Boron	mg/L	0.388	0.364	0.285	0.242	0.350	0.225	0.341	0.262	0.215	0.180	3.24	3.41	3.37	4.26	4.35	4.38
Cadmium	μg/L	0.0700	0.1 U	0.0100 J	0.1 U	$0.00800 \mathrm{J}$	0.1 U	0.00900 J	0.1 U	0.02 U	0.1 U	-	0.0200 J	0.1 U	-	0.0100 J	0.1 U
Calcium	mg/L	320	285	9.17	11.2	0.587	0.534	1.85	1.61	2.49	2.15	-	266	180	-	187	177
Chloride	mg/L	237	284	2.79	3.70	1.56	1.90	27.0	28.2	9.72	10.7	-	59.4	61.1	-	52.6	50.7
Chromium	μg/L	0.496	1 U	0.359	1 U	0.175	1 U	0.237	1 U	0.148	1 U	-	0.212	1 U	-	0.211	1 U
Cobalt	μg/L	13.6	9.00	0.131	0.5 U	0.00900 J	0.5 U	0.0630	0.5 U	0.0100 J	0.5 U	-	2.66	1.60	-	0.985	0.5 U
Combined Radium	pCi/L	1.12	0.450	2.21	0.997	0.414	0.491	0.887	0.806	0.755	1.51	-	1.17	0.738	-	2.12	2.17
Fluoride	mg/L	1.12	0.990	1.24	1.30	0.800	0.810	1.34	1.40	0.410	0.350	0.100 J	0.100 J	0.05 U	-	0.450	0.460
Lead	μg/L	0.770	0.5 U	0.465	0.5 U	0.0350	0.5 U	0.245	0.5 U	0.0290	0.5 U	-	0.791	0.5 U	-	0.0300	0.5 U
Lithium	mg/L	0.136	0.116	0.00500	0.0103	0.001 U	10 U	0.00500	10 U	0.00800	0.0108	-	0.0960	0.0699	-	0.0650	0.0655
Mercury	μg/L	0.005 U	0.00201	0.005 U	0.000880	0.005 U	0.5 U	0.005 U	0.00130	0.005 U	0.5 U	-	0.005 U	0.000940	1	0.005 U	0.000920
Molybdenum	μg/L	0.590	0.5 U	0.390	0.5 U	0.260	0.5 U	0.650	0.520	0.210	0.5 U	-	15.5	15.5	1	83.2	82.9
pН	SU	6.73	6.83	8.64	8.42	9.01	9.34	8.88	8.92	8.82	8.91	7.14	7.09	7.29	7.02	6.92	7.40
Selenium	μg/L	0.100 J	0.5 U	0.0700 J	0.5 U	0.1 U	0.5 U	0.0400 J	0.5 U	0.0300 J	0.5 U	-	0.300	0.5 U	-	0.1 U	0.5 U
Total Dissolved Solids	mg/L	2800	2800	465	450	376	365	573	548	770	784	-	1780	1840	-	961	914
Sulfate	mg/L	1470	1510	10.1	13.8	0.400	0.800	3.60	4.40	255	287	-	1020	1060	421	415	437
Thallium	μg/L	0.0400 J	0.5 U	0.05 U	0.5 U	0.05 U	0.5 U	0.0100 J	0.5 U	0.0200 J	0.5 U	-	0.0400 J	0.5 U	-	0.05 U	0.5 U

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Fly Ash Reservoir II

			1.6.00		3.7.4					3.5.1	202	3.5.1	200		3.666.1	
			M-23		M-I	.003		M-1004		M-1	302	M-1	.309		MGS-1	
Parameter	Unit	1/24/2018	5/17/2018	8/24/2018	5/16/2018	8/28/2018	1/24/2018	5/17/2018	8/27/2018	5/16/2018	8/22/2018	5/16/2018	8/29/2018	1/23/2018	5/16/2018	8/28/2018
		Detection	Asses	sment	Asses	sment	Detection	Asses	sment	Asses	sment	Asses	sment	Detection	Assess	sment
Antimony	μg/L	-	0.0200 J	0.5 U	0.0100 J	0.5 U	-	0.0100 J	0.5 U	0.05 U	0.5 U	0.0100 J	0.5 U	-	0.05 U	0.5 U
Arsenic	μg/L	-	0.510	0.750	0.440	0.5 U	-	1.81	1.80	0.0400 J	0.5 U	2.54	2.40	-	0.0500	0.5 U
Barium	μg/L	-	8.99	8.40	77.0	76.7	-	48.6	48.1	72.9	92.1	36.9	34.4	-	89.7	90.7
Beryllium	μg/L	-	0.0100 J	0.1 U	0.0200	0.1 U	-	0.0600	0.1 U	0.02 U	0.1 U	0.0200 J	0.1 U	-	0.00900 J	0.1 U
Boron	mg/L	0.684	0.748	0.731	0.150	0.159	1.89	2.37	2.56	0.284	0.283	0.313	0.296	-	0.326	0.314
Cadmium	μg/L	-	0.02 U	0.1 U	0.0300	0.1 U	-	$0.0100 \mathrm{J}$	0.1 U	$0.00700 \mathrm{J}$	0.1 U	0.0100 J	0.1 U	-	0.02 U	0.1 U
Calcium	mg/L	-	118	104	61.8	47.6	-	99.6	89.6	4.17	3.58	6.90	5.49	-	10.3	10.0
Chloride	mg/L	-	13.4	15.5	5.73	6.60	-	33.6	35.0	26.5	27.7	41.5	41.9	35.8	36.8	37.3
Chromium	μg/L	-	0.0860	1 U	0.268	1 U	-	0.775	1 U	0.135	1 U	0.277	1 U	-	0.104	1 U
Cobalt	μg/L	-	0.432	0.5 U	0.168	0.5 U	-	0.197	0.5 U	0.00700 J	0.5 U	0.285	0.5 U	-	0.0100 J	0.5 U
Combined Radium	pCi/L	-	2.49	3.51	4.13	2.77	-	1.62	0.929	0.684	0.253	0.576	0.547	-	0.267	1.11
Fluoride	mg/L	-	0.590	0.300	0.220	0.190	-	1.40	1.30	1.16	1.60	1.26	1.20	-	0.630	0.590
Lead	μg/L	-	0.0320	0.5 U	0.200	0.5 U	-	0.202	0.5 U	0.0210	0.5 U	0.200	0.5 U	-	0.0100 J	0.5 U
Lithium	mg/L	-	0.0470	0.0549	0.00900	10 U	-	0.0150	0.0165	0.0110	0.0140	0.0150	0.0182	-	0.0130	0.0184
Mercury	μg/L	-	0.005 U	0.000600	0.005 U	0.00178	-	0.005 U	0.000730	0.005 U	0.51 U	0.005 U	0.00356	-	0.005 U	0.5 U
Molybdenum	μg/L	-	0.450	0.5 U	0.120	0.5 U	-	9.89	10.4	0.0700 J	0.5 U	1.76	1.70	-	0.320	0.5 U
рН	SU	7.21	7.09	7.35	7.82	7.53	7.42	7.16	7.72	8.57	8.70	8.24	7.64	7.49	7.42	7.30
Selenium	μg/L	-	0.0300 J	0.5 U	0.1 U	0.5 U	-	0.0600 J	0.5 U	0.1 U	0.5 U	0.0600 J	0.5 U	-	0.1 U	0.5 U
Total Dissolved Solids	mg/L	-	3190	3450	459	433	-	871	876	774	671	755	713	-	614	604
Sulfate	mg/L	-	1580	1690	92.7	96.7	-	290	315	141	97.8	123	121	-	78.9	83.9
Thallium	μg/L	-	0.0200 J	0.5 U	0.05 U	0.5 U	-	0.0100 J	0.5 U	0.05 U	0.5 U	0.0100 J	0.5 U	-	0.05 U	0.5 U

mg/L: milligrams per liter µg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Fly Ash Reservoir II

		1.40	10.0		7		1.60	10. 1	1.60	10 -
			S-2		MGS-3			S-4		S-5
Parameter	Unit	5/16/2018	8/28/2018	1/24/2018	5/17/2018	8/28/2018	5/16/2018	8/23/2018	5/15/2018	8/21/2018
		Asses	sment	Detection	Asses	sment	Asses	sment	Asses	sment
Antimony	μg/L	0.100	0.5 U	-	0.220	0.5 U	0.0400 J	0.5 U	0.0400 J	0.5 U
Arsenic	μg/L	9.29	8.00	-	8.68	10.8	9.52	7.10	18.7	16.0
Barium	μg/L	28.5	27.1	-	11.8	10.4	12.1	11.7	91.9	104
Beryllium	μg/L	0.02 U	0.1 U	-	0.02 U	0.1 U	0.02 U	0.1 U	0.02 U	0.1 U
Boron	mg/L	0.313	0.238	0.338	0.444	0.924	0.319	0.192	0.433	0.331
Cadmium	μg/L	0.02 U	0.1 U	-	0.0100 J	0.1 U	0.02 U	0.1 U	0.02 U	0.1 U
Calcium	mg/L	8.83	9.21	-	327	150	9.39	6.45	3.15	2.92
Chloride	mg/L	25.7	26.2	-	11.3	24.2	11.5	12.2	167	171
Chromium	μg/L	0.156	1 U	-	0.152	1 U	0.157	1 U	0.272	1 U
Cobalt	μg/L	0.426	0.5 U	-	0.359	1.60	0.142	0.5 U	0.0260	0.5 U
Combined Radium	pCi/L	0.709	0.456	-	1.94	1.15	0.228	0.941	1.62	1.43
Fluoride	mg/L	0.470	0.420	-	0.210	0.110	0.610	0.610	5.50	5.10
Lead	μg/L	0.0250	0.5 U	-	0.0780	0.5 U	0.0310	0.5 U	0.0430	0.650
Lithium	mg/L	0.0110	0.0152	-	0.0280	0.0514	0.00600	10 U	0.0100	0.0151
Mercury	μg/L	0.005 U	0.5 U	-	0.005 U	0.5 U	0.005 U	0.000800	0.005 U	0.000760
Molybdenum	μg/L	1.26	1.40	-	1.66	1.30	2.72	2.10	3.26	2.90
рН	SU	7.58	7.53	6.81	6.74	6.47	8.16	8.50	8.36	8.52
Selenium	μg/L	0.1 U	0.5 U	-	0.0400 J	0.5 U	0.1 U	0.5 U	0.1 U	0.5 U
Total Dissolved Solids	mg/L	630	583	-	1870	2220	600	519	1100	1090
Sulfate	mg/L	117	115	-	1100	1380	121	73.1	3.60	3.80
Thallium	μg/L	0.05 U	0.5 U	-	0.0890	0.5 U	0.0100 J	0.5 U	0.0100 J	0.5 U

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

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J: Estimated value. Parameter was detected in concentrations below the reporting limit

Dawamatan	Unit	CA-0	622A	FA	1-8	M	-6	M	-8	M	-10	M-	·11
Parameter	Unit	3/26/2019	10/1/2019	4/4/2019	10/9/2019	3/28/2019	10/3/2019	4/1/2019	10/3/2019	4/2/2019	10/3/2019	4/5/2019	10/9/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.700	0.620
Arsenic	μg/L	29.2	25.8	7.70	9.20	4.10	4.70	0.940	1.10	0.500 U	0.500 U	3.00	5.40
Barium	μg/L	934	952	24.5	22.2	435	442	127	120	78.0	80.6	23.9	20.8
Beryllium	μg/L	0.100 U	0.100 U	0.100 U	0.100 U	1.50	1.80	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U
Boron	μg/L	284	337	4,980	4,740	231	239	26.6	25.8	580	536	4,670	5,000
Cadmium	μg/L	0.100 U	0.100 U	0.240	0.150	0.230	0.260	0.100 U	0.100 U	0.100 U	0.190	0.400	0.180
Calcium	μg/L	75,400	74,200	198,000	218,000	16,000	15,600	108,000	102,000	14,100	12,400	195,000	219,000
Chloride	mg/L	4,900	3,470	43.4	46.6	32.9	39.8	6.00	6.10	12.6	12.6	44.2	45.7
Chromium	μg/L	1.50	1.20	1.00 U	1.10	9.20	11.8	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U
Cobalt	μg/L	0.620	0.630	0.870	0.920	4.90	5.60	0.660	0.500 U	0.500 U	0.500 U	1.20	1.20
Combined Radium	pCi/L	11.6	11.9	0.188	1.17	6.51	5.15	0.476	0.776	1.68	0.815	0.453	1.28
Fluoride	mg/L	0.470	0.0500 U	0.600	0.570	1.20	1.10	0.0880	0.0500 U	0.690	0.670	0.580	0.550
Lead	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	22.0	25.9	0.590	0.730	0.850	1.20	0.980	0.640
Lithium	μg/L	89.8	86.7	198	194	18.3	18.6	10.0 U	10.0 U	19.6	20.0	193	188
Mercury	μg/L	0.00108	0.00185	0.000500 U	0.000570	0.0104	0.00694	0.00116	0.00214	0.000570	0.000500 U	0.000500 U	0.000610
Molybdenum	μg/L	3.40	1.90	321	303	1.00 U	1.20	0.500 U	0.500 U	2.40	2.30	316	338
Selenium	μg/L	0.500 U	1.00	2.00	0.500 U	1.00 U	1.70	0.500 U	0.500 U	0.500 U	0.500 U	4.50	0.640
Sulfate	mg/L	72.0	40.4	885	762	2.00	7.70	95.8	99.9	133	134	960	781
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Total Dissolved Solids	mg/L	6,680	7,250	1,430	1,360	478	695	421	409	711	710	1,400	1,360
рН	SU	7.49	7.85	7.12	7.02	7.71	7.95	7.37	7.33	8.40	8.44	8.01	7.15

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

D	TI . 4	M-	-12	M-	-13		M-14			M-15			M-16	
Parameter	Unit	4/3/2019	10/9/2019	4/3/2019	10/8/2019	3/27/2019	7/2/2019	10/7/2019	3/25/2019	5/1/2019	9/30/2019	3/27/2019	7/2/2019	10/1/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Arsenic	μg/L	5.60	2.10	1.40	1.00	0.500 U	-	0.500 U	2.00	-	2.00	0.500 U	-	0.500 U
Barium	μg/L	89.1	28.3	218	159	14.6	-	14.3	43.2	-	45.4	37.1	-	37.4
Beryllium	μg/L	0.100	0.100 U	0.930	0.100 U	0.100 U	-	0.100 U	0.100 U	-	0.100 U	0.100 U	-	0.100 U
Boron	μg/L	324	290	261	285	224	-	231	228	-	258	192	-	184
Cadmium	μg/L	0.130	0.100 U	0.100 U	0.100 U	0.100 U	-	0.100 U	0.100 U	=	0.100 U	0.100 U	=	0.100 U
Calcium	μg/L	371,000	188,000	14,600	15,400	513	=	603	1,550	=	1,490	2,240	=	2,260
Chloride	mg/L	184	270	2.10	2.10	-	1.80	1.50	-	25.7	26.0	-	10.0	10.2
Chromium	μg/L	1.00 U	1.00 U	3.40	1.00 U	1.00 U	-	1.00 U	1.00 U	-	1.00 U	1.00 U	-	1.00 U
Cobalt	μg/L	31.7	6.20	1.20	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Combined Radium	pCi/L	1.14	1.19	3.87	1.56	0.680	=	1.22	0.681	=	0.00	0.553	=	0.805
Fluoride	mg/L	0.990	1.30	1.70	1.80	-	0.730	0.780	-	1.40	1.30	=	0.350	0.350
Lead	μg/L	1.20	0.500 U	3.30	0.530	0.500 U	=	0.500 U	0.500 U	=	0.500 U	0.500 U	=	0.500 U
Lithium	μg/L	106	108	15.8	10.8	10.0 U	=	10.0 U	10.0 U	=	10.0 U	10.4	=	11.0
Mercury	μg/L	0.00583	0.00169	0.00267	0.000510	0.000500 U	=	0.000500 U	0.000500 U	=	0.000500 U	0.000500 U	=	0.000500 U
Molybdenum	μg/L	0.500 U	0.500 U	0.730	0.680	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.610
Sulfate	mg/L	1,590	1,020	28.8	30.5	-	0.800	1.10	-	3.00	1.50	-	332	276
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	=	0.500 U
Total Dissolved Solids	mg/L	2,910	2,290	510	492	347	-	347	540	-	552	780	=	757
pН	SU	667	7.22	8.15	7.48	9.19	_	8.83	9.13	-	9.03	8.34	-	8.81

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

D	Unit	M	-21	M	[-22	M	[-23	M-1	003	M-1	004		M-1302	
Parameter	Unit	4/3/2019	10/8/2019	4/3/2019	10/9/2019	4/1/2019	10/3/2019	4/8/2019	10/9/2019	4/2/2019	10/7/2019	3/25/2019	5/1/2019	10/1/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U
Arsenic	μg/L	5.40	3.60	0.500 U	0.500 U	3.40	0.930	0.530	0.500 U	1.50	1.50	0.500 U	I	0.500 U
Barium	μg/L	14.1	13.8	25.1	21.8	27.3	8.50	84.2	79.4	47.2	44.8	107	I	106
Beryllium	μg/L	1.40	0.780	0.100 U	0.100 U	0.250	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	ı	0.100 U
Boron	μg/L	3,210	3,100	3,990	3,760	695	696	128	130	2,310	2,680	244	ı	295
Cadmium	μg/L	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	I	0.100 U
Calcium	μg/L	237,000	176,000	188,000	178,000	125,000	105,000	69,100	68,300	97,900	115,000	3,490	ı	3,340
Chloride	mg/L	51.9	63.8	44.0	43.9	12.0	13.4	5.80	6.20	31.8	35.6	=	26.6	28.4
Chromium	μg/L	1.00 U	1.00 U	1.00 U	1.00 U	3.50	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	I	1.00 U
Cobalt	μg/L	1.80	0.910	0.500 U	1.50	2.60	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	I	0.500 U
Combined Radium	pCi/L	0.573	0.980	0.776	1.18	2.30	2.21	2.10	3.24	0.890	1.25	0.771	I	0.421
Fluoride	mg/L	0.100	0.130	0.520	0.380	0.330	0.390	0.230	0.200	1.20	1.20	=	1.20	1.70
Lead	μg/L	3.70	1.00	0.500 U	0.500 U	3.30	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	I	0.500 U
Lithium	μg/L	80.0	66.5	52.6	52.5	57.9	48.6	10.0 U	10.0 U	21.0	18.0	13.1	I	11.7
Mercury	μg/L	0.00366	0.00156	0.000680	0.000500 U	0.0127	0.000500 U	0.000500 U	0.000640	0.000500 U	0.000510	0.000510 U	-	0.000510 U
Molybdenum	μg/L	21.3	16.6	56.5	79.1	0.500 U	0.500 U	0.500 U	0.500 U	9.40	11.6	0.500 U	-	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.560	0.530	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.590
Sulfate	mg/L	1,170	968	382	400	1,570	1,750	98.3	112	272	341	=	111	60.9
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U
Total Dissolved Solids	mg/L	1,810	1,760	896	905	3,320	3,210	466	437	859	869	699	-	721
рН	SU	7.21	7.21	7.11	7.09	7.21	7.14	7.56	7.39	7.48	7.27	8.79	-	8.51

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

Danamatan	Unit		M-1309		M-2	000	MG	SS-1	MG	GS-2	MG	S-3	MC	GS-4	MO	GS-5
Parameter	Unit	4/4/2019	10/10/2019	11/12/2019	4/5/2019	10/9/2019	3/27/2019	10/7/2019	4/2/2019	10/9/2019	4/3/2019	10/4/2019	4/1/2019	10/8/2019	3/26/2019	10/2/2019
Antimony	μg/L	0.500 U	0.500 U	-	0.500 U											
Arsenic	μg/L	2.20	2.00	-	0.530	0.880	0.500 U	0.500 U	11.7	12.9	36.7	10.3	5.40	5.10	14.1	12.5
Barium	μg/L	33.6	34.6	-	86.7	25.1	92.0	89.6	38.6	45.7	12.2	9.40	12.3	13.0	103	105
Beryllium	μg/L	0.100 U	0.100 U	-	0.100 U											
Boron	μg/L	285	283	ı	254	4,970	288	321	214	169	536	879	193	204	335	271
Cadmium	μg/L	0.100 U	0.100 U	-	0.100 U	0.100	0.100 U									
Calcium	μg/L	5,690	4,390	-	218,000	216,000	13,200	13,300	20,900	46,300	284,000	147,000	8,170	8,040	2,950	2,730
Chloride	mg/L	37.8	38.9	-	46.6	50.0	32.7	38.3	20.6	21.1	16.8	27.6	12.1	12.1	170	206
Chromium	μg/L	1.00 U	1.00 U	-	1.00 U											
Cobalt	μg/L	0.500 U	0.500 U	-	0.500 U	1.10	0.500 U	0.500 U	0.770	1.20	0.660	0.840	0.500 U	0.500 U	0.500 U	0.500 U
Combined Radium	pCi/L	0.936	1.71	-	1.72	1.24	0.316	0.901	0.307	0.177	1.37	0.850	0.0710	0.221	0.181	0.527
Fluoride	mg/L	1.10	1.20	-	0.370	0.380	0.650	0.640	0.370	0.370	0.170	0.140	0.510	0.510	5.40	6.60
Lead	μg/L	0.500 U	0.500 U	-	0.500 U											
Lithium	μg/L	23.4	17.2	-	201	190	17.7	16.6	13.5	13.2	38.0	47.6	10.0 U	10.0 U	16.7	14.0
Mercury	μg/L	0.00170	-	0.000850	0.000500 U											
Molybdenum	μg/L	1.50	1.10	-	0.500 U	208	0.500 U	0.500 U	4.30	10.3	2.30	2.10	4.20	5.50	2.30	1.90
Selenium	μg/L	0.500 U	0.500 U	-	0.500 U											
Sulfate	mg/L	94.6	88.1	Ī	820	830	78.9	91.2	164	162	1,330	1,290	98.1	86.7	3.50	1.60
Thallium	μg/L	0.500 U	0.500 U	Ī	0.500 U											
Total Dissolved Solids	mg/L	693	687	-	721	1,440	616	597	618	651	2,030	2,000	572	522	1,030	1,070
pН	SU	7.94	7.57	-	6.83	6.80	7.75	7.47	7.62	7.51	679	6.59	8.46	8.13	8.70	8.50

Notes:

mg/L: milligrams per liter μ g/L: micrograms per liter SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

Spring 2020 App III & IV Parameters_Rev 1 Cardinal Plant - Fly Ash Reservoir II

Parameter	Unit	CA-0622A	FA-8	M-6	M-8	M-10	M-11	M-12	M-13	M-14	M-15	M-16	M-21	M-21	M-22	M-23	M-1003	M-1004	M-1302	M-1309	MGS-1	MGS-2	MGS-3	MGS-4	MGS-5
														Resamp											
		4/09/2020	4/20/2020	4/14/2020	4/15/2020	4/15/2020	4/17/2020	4/15/2020	4/16/2020	4/21/2020	4/08/2020	4/09/2020	4/21/2020	6/22/2020	4/15/2020	4/17/2020	4/21/2020	4/16/2020	4/08/2020	4/20/2020	4/16/2020	4/15/2020	4/10/2020	4/09/2020	4/13/2020
Antimony	μg/L	2.5 U	0.50 U	0.50 U	0.50 U	0.50 U	0.63	0.50 U	2.8	0.5 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U				
Arsenic	μg/L	25.8	8.6	3.7	0.65	0.50 U	3.3	3.6	0.57	0.50 U	1.8	0.50 U	440	1.7	0.50 U	0.8	0.50 U	1.5	0.50 U	1.8	0.50 U	6	35.9	4.1	8.5
Barium	μg/L	1080	24.3	381	116	78.8	22.2	71.1	127	13.7	45.2	36.7	1650	12.6	24.1	7.6	72.5	40	114	29.7	81.1	28.1	9.6	12.9	113
Beryllium	μg/L	0.50 U	0.10 U	1.6	0.10 U	168	0.26	0.10 U	0.50 U																
Boron	ug/L	279	4770	209	25.7	537	4760	316	240	219	250	175	3780	3400	3790	666	140	2380	270	277	273	219	774	181	310
Cadmium	μg/L	0.50 U	0.10 U	0.2	0.10 U	0.10 U	0.10 U	0.15	0.10 U	0.10 U	0.10 U	0.10 U	12.3	0.1 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U	0.10 U
Calcium	ug/L	76700	208000	14500	101000	11900	206000	368000	11500	500 U	1660	2340	352000	230000	170000	103000	74500	108000	3180	3960	11800	7320	163000	4890	2510
Chloride	mg/L	4450	48.5	36.7	6.9	13.7	53.4	163	2.6	1.8	28.7	11	61	50.6	51.5	15.7	7.2	37.6	30	39.5	35.4	26.6	35.8	21.2	162
Chromium	μg/L	5.0 U	1.3	10	1.0 U	95.7	1.0 U																		
Cobalt	μg/L	2.5 U	0.92	4.6	0.50 U	0.50 U	0.97	32.4	0.50 U	0.50 U	0.50 U	0.50 U	143	2.4	1	0.50 U	0.82	0.50 U	2.5 U						
Combined Radium	pCi/L	10.5	0.755	7.41	1.4	0.792	0.5	0.41	1.44	0.205	1.35	0.592	8.46	1.58	1.25	2	2.47	1.06	0.963	0.0571	0.245	0.515	0.886	0.321	1.99
Fluoride	mg/L	0.050 U	0.59	1.2	0.16	0.81	0.68	1.3	1.9	0.85	1.3	0.37	0.14	0.13	0.49	0.63	0.23	1.3	1.8	1.1	0.67	0.45	0.19	0.54	5.7
Lead	μg/L	0.50 U	0.50 U	21.2	0.50 U	1.1	0.50 U	1.1	0.50 U	0.50 U	0.50 U	0.50 U	689	1.3	0.50 U										
Lithium	ug/L	99.5	194	22.3	10.0 U	20.4	191	149	11.8	10.0 U	10.0 U	12.2	122	87.8	65.1	54	11.3	25.7	14.8	19.4	17.4	16.5	54.9	10.0 U	16
Mercury	μg/L	0.00073	0.00062	0.00819	0.0018	0.00066	0.00103	0.00187	0.00139	0.00077	0.0005 U	0.0005 U	0.679	0.00381	0.0005 U	0.0005 U	0.00091	0.0005 U	0.00051 U	0.00178	0.0005 U				
Molybdenum	μg/L	2.5 U	298	1.1	0.50 U	2.2	289	0.50 U	707	29.7	77.2	0.50 U	0.50 U	9.9	0.50 U	1	0.50 U	2.3	2.4	4.2	2.5 U				
pH	SU	7.52	7.09	7.4	7.24	8.35	7.23	7.18	8.01	8.99	8.87	8.65	6.84	7.2	6.94	7.12	7.3	7.17	8.57	8.19	7.47	7.48	6.65	8.17	8.51
Selenium	μg/L	2.5 U	1.4	1	0.50 U	0.50 U	3.2	0.50 U	79.8	29.7	0.50 U														
Total Dissolved	mg/L	6890	1300	606	422	710	1290	2600	511	340	548	791	900	1650	942	3250	440	850	719	679	599	613	2020	586	1130
Solids																									
Sulfate	mg/L	47	740	5.2	103	133	778	1630	29.7	0.85	1.2	300	1030	914	403	1740	125	312	54.2	83.7	75.7	105	1400	134	42.8
Thallium	μg/L	0.50 U	2.5 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U	0.50 U										

Notes:

mg/L: milligrams per liter

High: micrograms per liter PCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

2019 ANNUAL GROUNDWATER MONITORING REPORT

FEDERAL CCR RULE

CARDINAL PLANT – FLY ASH RESERVOIR II BRILLIANT, OHIO

Submitted to



Cardinal Operating Compnay

306 County Road 7E Brilliant, Ohio 43913

Submitted by



engineers | scientists | innovators

941 Chatham Lane, Suite 103 Columbus, Ohio 43221

January 13, 2020

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Attachment A: Monitoring Well M-2000 Construction Diagram

LIST OF ACRONYMS AND ABBREVIATIONS

ACM Assessment of Corrective Measures

CCR Coal Combustion Residuals

CFR Code of Federal Regulations

ESP Electrostatic Precipitator

FAD Fly Ash Dam

FAR Fly Ash Reservoir

FGD Flue Gas Desulfurization

GWPS Groundwater Protection Standards

MCL Maximum Contaminant Level

NPDES National Pollutant Discharge Elimination System

RSL Risk-Based Screening Level

RSW Residual Solid Waste

SCR Selective Catalytic Reduction

SSI Statistically Significant Increase

SSL Statistically Significant Level

USEPA United States Environmental Protection Agency

1. INTRODUCTION

The Federal Coal Combustion Residuals (CCR) Rule (40 Code of Federal Regulations [CFR] Part 257.90(e)) (USEPA, 2015) requires owners and or operators of existing CCR landfills and surface impoundments to prepare a Groundwater Monitoring and Corrective Action Report (Report) no later than January 31 annually. Geosyntec Consultants (Geosyntec) has prepared this Report for the Fly Ash Reservoir (FAR) II, an existing CCR unit at the Cardinal Plant in Brilliant, Ohio (Site). This Report summarizes the groundwater monitoring activities conducted pursuant to the CCR Rule through December 31, 2019.

2. SITE SUMMARY

2.1 Site Description

The Site is located one mile south of Brilliant, Ohio in Jefferson County (**Figure 1**) and is operated by Buckeye Power, Inc. (Buckeye Power). Located along the Ohio River, the generating station consists of three coal-powered units with an 1,800-megawatt capacity and annual coal use of 5.2 million tons (Geosyntec, 2017a). Units 1 and 2 began operation in 1967 and Unit 3 began operation in 1977. As of 2012, all three units were equipped with an electrostatic precipitator (ESP), a selective catalytic reduction (SCR) system, and a flue gas desulfurization (FGD) system.

FAR II is an existing wet fly ash disposal reservoir that is located approximately one mile north of the plant site and immediately east of the FAR I Residual Solid Waste (RSW) Landfill. The reservoir is contained within Blockhouse Hollow (also referred to as Blockhouse Run in references and drawings) by Fly Ash Dam (FAD) 2 and the decommissioned FAD I. FAR II receives sluiced fly ash from the generating units' ESPs and collected stormwater and leachate from the FAR I RSW Landfill. FAR II/FAD 2 has a permitted discharge through the National Pollutant Discharge Elimination System (NPDES) Outfall 019 (Geosyntec, 2017a).

2.2 Regional Physiographic Setting

The Site is underlain by horizontal sequences of lower Permian and upper Pennsylvanian sedimentary rock. The Conemaugh Group, 500 feet (ft) thick in Jefferson County, consists of shale, sandstone, limestone, claystone, and coal. This group includes the Morgantown Sandstone underlain by the Elk Lick Limestone, the Skelly Limestone and Shale, the Ames Limestone, and the Cow Run Sandstone (Geosyntec, 2017a). Above the current grade of the RSW Landfill lies the Monongahela Group consisting of shale, sandstone, limestone, coal, claystone, and siltstone. Overlying the Monongahela Group, at approximately 1,250 feet in elevation, is the Permian-age Dunkard Group.

The uppermost aquifer at the Site lies within the Morgantown Sandstone, which is overlain by a shale aquitard. Groundwater in the uppermost aquifer generally flows south-southeast towards the Ohio River with hydraulic conductivity ranging from 1×10^{-1} to 1×10^{-4} centimeters per second

(cm/s). The hydraulic conductivity of the confining shale layer ranges from 1×10^{-7} to 1×10^{-9} cm/s (AEP, 2006).

3. GROUNDWATER MONITORING SYSTEM

The FAR II's groundwater monitoring network was designed to comply with 40 CFR 257.91. The groundwater monitoring network utilizes monitoring wells initially installed as part of a separate site-wide hydrogeologic investigation and is used to monitor groundwater quality in the uppermost aquifer at the Site. Monitoring well construction and soil boring logs were provided in the *Groundwater Monitoring Network Design Report* (Geosyntec, 2017a).

The FAR II groundwater monitoring network consists of twenty-three monitoring wells, as shown in **Figure 2.** Five upgradient monitoring wells (CA-0622A, M-12, M-1302, M-6, and MGS-5) are used to measure background conditions and eighteen downgradient monitoring wells (FA-8, M-10, M-1003, M-1004, M-11, M-13, M-1309, M-14, M-15, M-16, M-21, M-22, M-23, M-8, MGS-1, MGS-2, MGS-3, and MGS-4) are used as compliance wells.

4. CCR RULE GROUNDWATER KEY ACTIVITIES COMPLETED

4.1 2018 Statistical Evaluation Activities

A Groundwater Protection Standard (GWPS) was established for each Appendix IV parameter in accordance with the United States Environmental Protection Agency (USEPA's) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities – Unified Guidance* (Unified Guidance; USEPA, 2009) and the Site's Statistical Analysis Plan (Geosyntec, 2017b). The established GWPSs were determined to be the greater value of the background concentration and the maximum contaminant level (MCL) or risk-based screening level (RSL) for each Appendix IV parameter. GWPSs determined in 2018 are provided in the *2018 Annual Groundwater Monitoring Report* (Geosyntec, 2019a). A statistical evaluation of the 2018 assessment monitoring data compared against the GWPS was completed in January 2019 and is described in the *Statistical Analysis Summery – Fly Ash Reservoir* (Geosyntec, 2019b). The statistical analysis report identified statistically significant levels (SSLs) of lithium and molybdenum above their respective GWPS at monitoring wells FA-8 and M-11. An alternate source was not identified for the SSLs and the CCR unit transitioned to corrective measures, as discussed in **Section 4.3**.

4.2 2019 Sampling and Data Evaluation Activities

4.2.1 Assessment Monitoring Program

Assessment monitoring sampling events were conducted in March and October 2019 in accordance with 40 CFR 257.95(b) and 40 CFR 257.95(d)(1). Samples were analyzed for all Appendix III and Appendix IV parameters, with results provided in **Table 1**. A revision of the

GWPS and statistical evaluation of the 2019 assessment monitoring data is ongoing and will be completed outside of the timeframe of this report.

4.2.2 Groundwater Elevation and Flow Velocities

Prior to sampling, a synoptic round of groundwater level measurements was collected from the compliance and background monitoring wells. Potentiometric surface maps based on groundwater elevations measured during the March and October 2019 assessment monitoring events are presented on **Figure 3** and **Figure 4**, respectively. The potentiometric maps show that groundwater near FAR II flows southeast towards the Ohio River. The groundwater residence times within the wells at the FAR II ranged from 0.6 days at M-GS-2 to 21.6 days at M-12. A summary of hydraulic gradients and groundwater residence times at the FAR II is provided in **Table 2**.

4.2.3 Data Usability

Upon receipt of laboratory analytical reports, the data were evaluated for usability. Analytical data were checked for the following:

- Samples were analyzed within the method specified hold times;
- Samples were received within holding temperature;
- Chain of custody forms were complete;
- Precision was within control limits using relative percent differences of blind duplicate samples;
- Matrix spike and matrix spike duplicate recoveries and laboratory control samples were within the control limits; and
- Potential for positive bias was evaluated using method blanks.

Samples collected in March 2019 from monitoring wells M-15 and M-1302 were not analyzed by USEPA method 9056 for chloride, fluoride, and sulfate within an acceptable hold time as a result of laboratory error. Monitoring wells M-15 and M-1302 were re-sampled in May 2019 and analyzed for the USEPA 9056 anions only. All other data received during 2019 were considered complete and usable.

4.3 Corrective Measures Program

Following detection of lithium and molybdenum SSLs at FA-8 and M-11, a Notification of Exceedance of Groundwater Protection Standards was published to the public internet site on February 7, 2019 in accordance with 40 CFR 257.105(h) (Buckeye Power, 2019). As required for characterization of the nature and extent of the release, monitoring well M-2000 was installed in accordance with 40 CFR 257.95(g)(1) on March 8, 2019 and sampled during the March and October 2019 assessment monitoring events. Monitoring well installation and sampling efforts are described in the *Groundwater Characterization Report, Cardinal Site – Fly Ash Reservoir II*

(Geosyntec, 2019c). The boring and construction log for monitoring well M-2000 is provided as **Attachment A**.

An Assessment of Corrective Measures (ACM) Report was completed in July 2019 in accordance with 40 CFR 257.96 and published to the public internet site (Geosyntec, 2019d). The ACM report lists four potential corrective measures that may be appropriate for addressing the elevated lithium and molybdenum concentrations in Site groundwater. A public meeting was held on September 4, 2019 in Steubenville, Ohio where the selection and implementation of potential corrective measures outlined in the ACM Report were reviewed and discussed.

4.4 Problems Encountered and Resolutions

No problems were encountered during 2019 which were related to assessment monitoring activities at the FAR II. Monitoring well M-2000 was installed in 2019 to facilitate characterization of the nature and extent of the release within the corrective measures program. No monitoring wells were gauged dry or abandoned within the well network during 2019.

Samples for chloride, fluoride, and sulfate at M-14 and M-16 were not collected during the March 2019 event due to sampling error. Samples were instead collected from M-14 and M-16 in July 2019 and submitted for analysis of anions. The mercury sample collected at M-1309 on October 10, 2019 was unable to be analyzed due to laboratory error. An additional sample was collected from M-1309 in November 2019 and submitted for mercury analysis. The March 2019 samples for chloride, fluoride, and sulfate at M-15 and M-1302 were analyzed out of hold time. These data will not be included in any statistical evaluation and additional samples were collected in May 2019. All other analytical data received were deemed to be of acceptable quality.

5. STATUS OF MONITORING PROGRAM

The Site was in the assessment monitoring program from May 2018 through January 2019 and transitioned to the corrective measures program in February 2019. Assessment monitoring events were conducted in March and October 2019. FAR II will remain in the corrective measures program in 2020.

6. PLANNED KEY ACTIVITIES FOR 2020

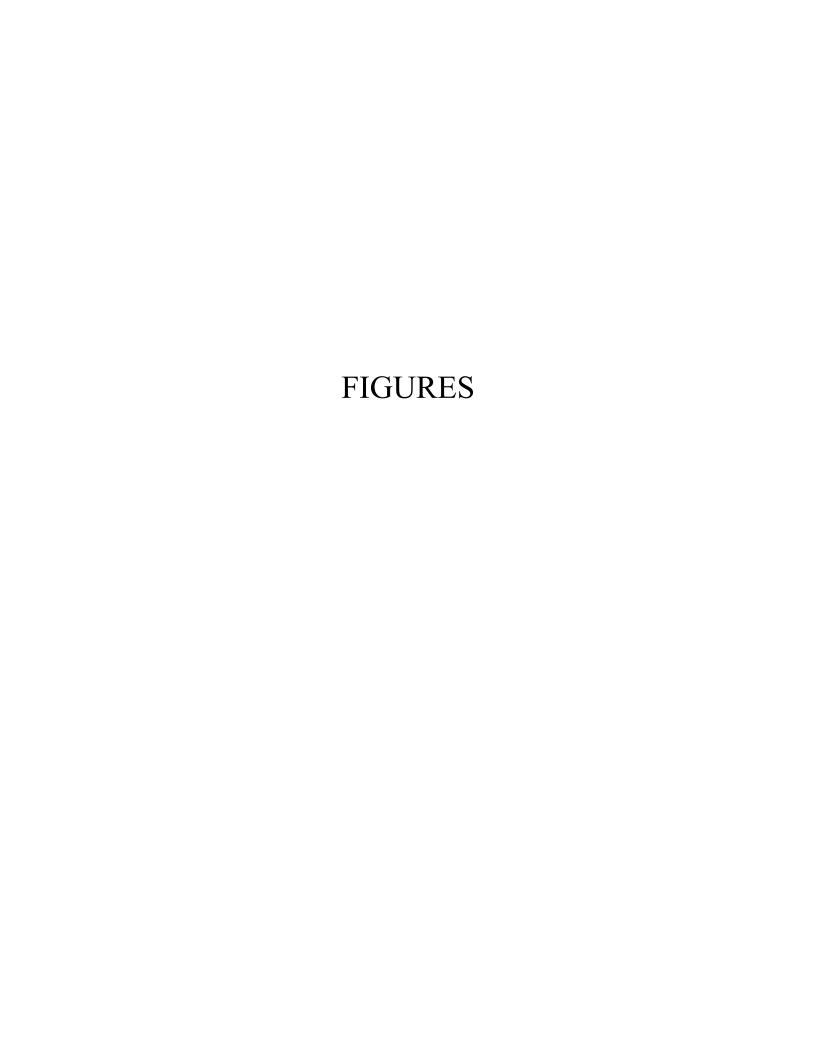
The following activities are planned for 2020 at the FAR II:

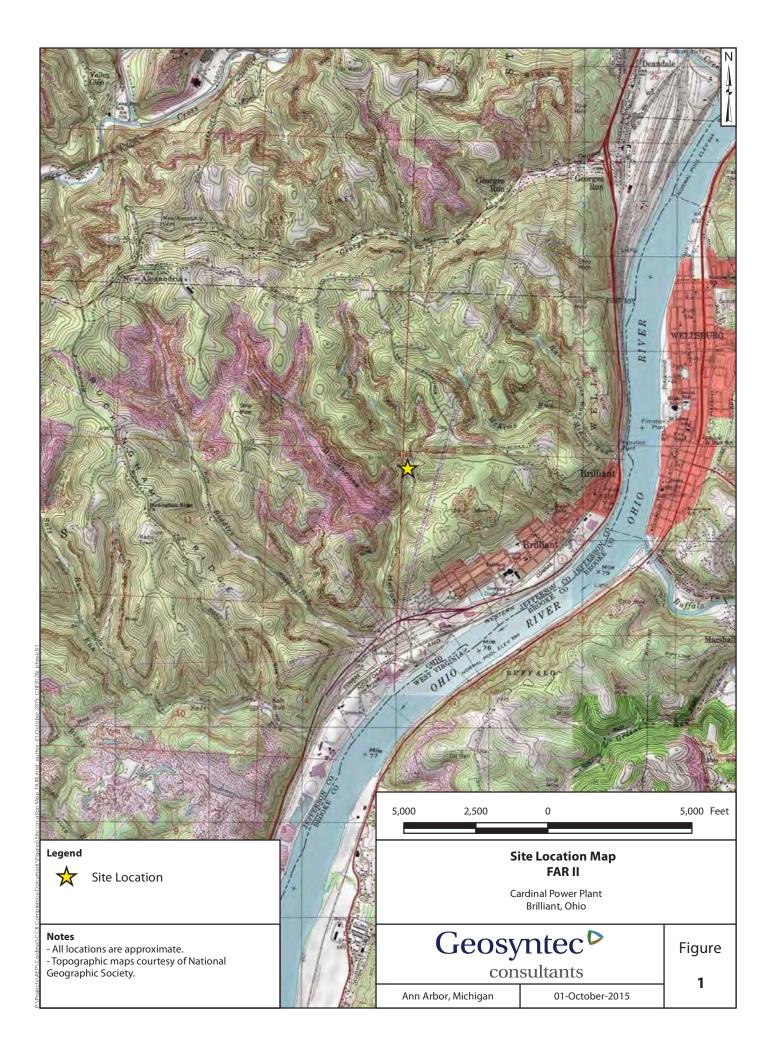
- The 2019 Annual Groundwater Monitoring Report will be entered into the facility's operating record and posted to the public internet site;
- The assessment monitoring statistics revision for data collected in 2019 will be completed and the potential for SSLs of Appendix IV parameters and Statistically Significant Increases (SSIs) of Appendix III parameters over background will be evaluated;

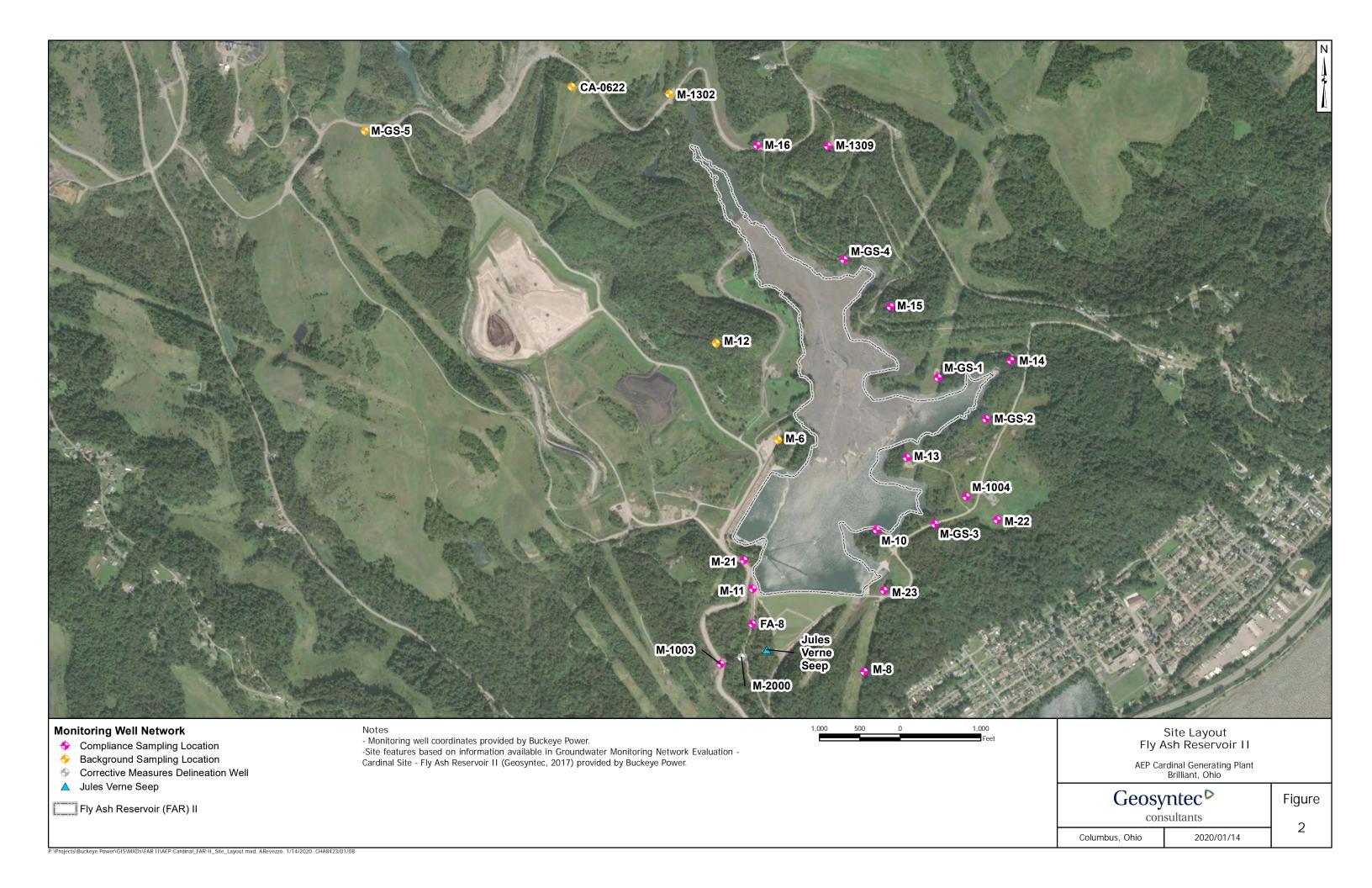
- Two semi-annual groundwater assessment monitoring program events will be conducted;
- A semi-annual report describing the progress in selecting and designing the remedy will be prepared and posted to the public internet site;
- A remedy, outlined in the ACM, will be selected in accordance with 40 CFR 257.97. A final report describing the selected remedy, and initiation of remedial activities will be prepared and posted to the public internet site; and
- The 2020 Annual Groundwater Monitoring Report will be prepared for submittal in January 2021.

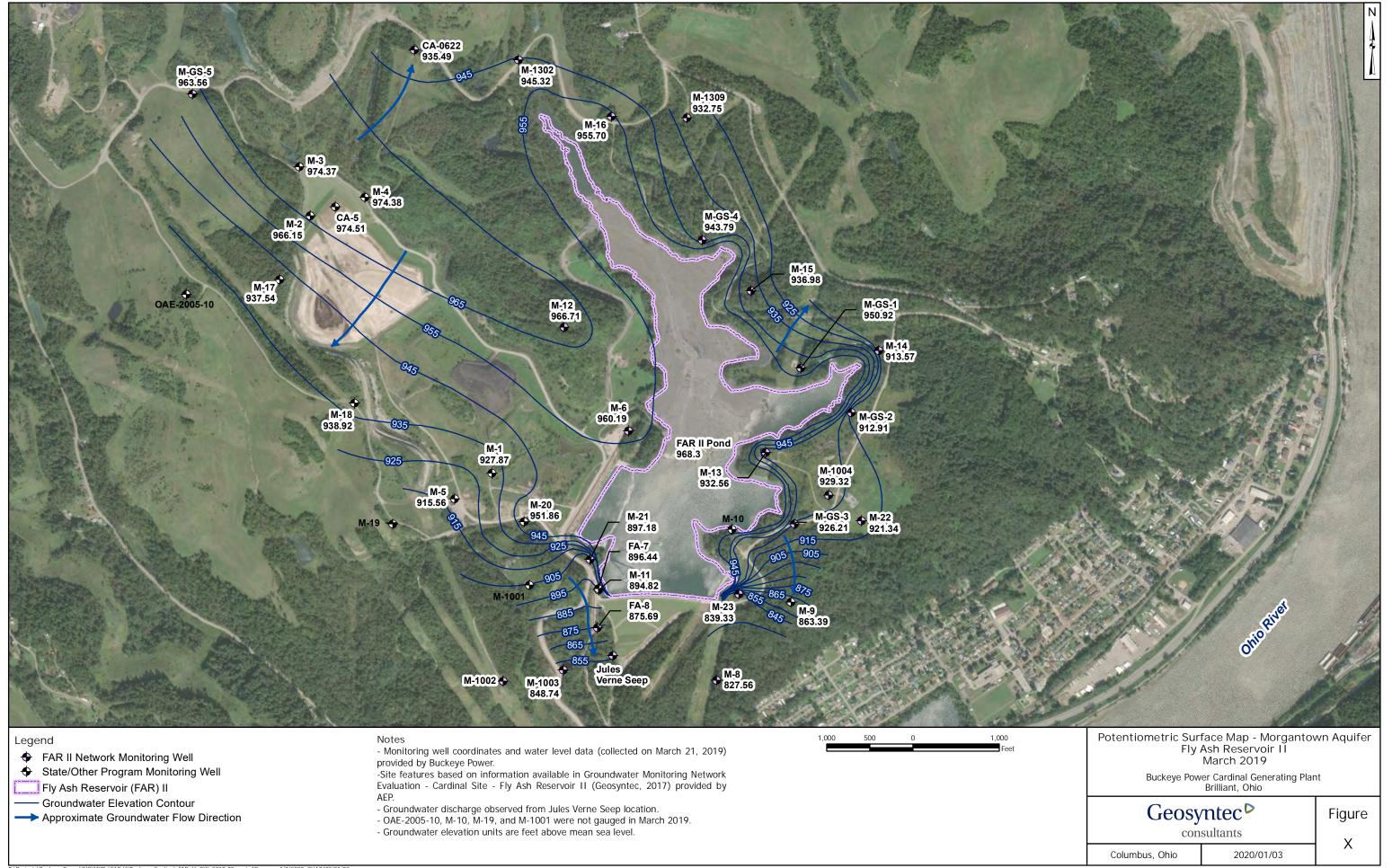
7. REFERENCES

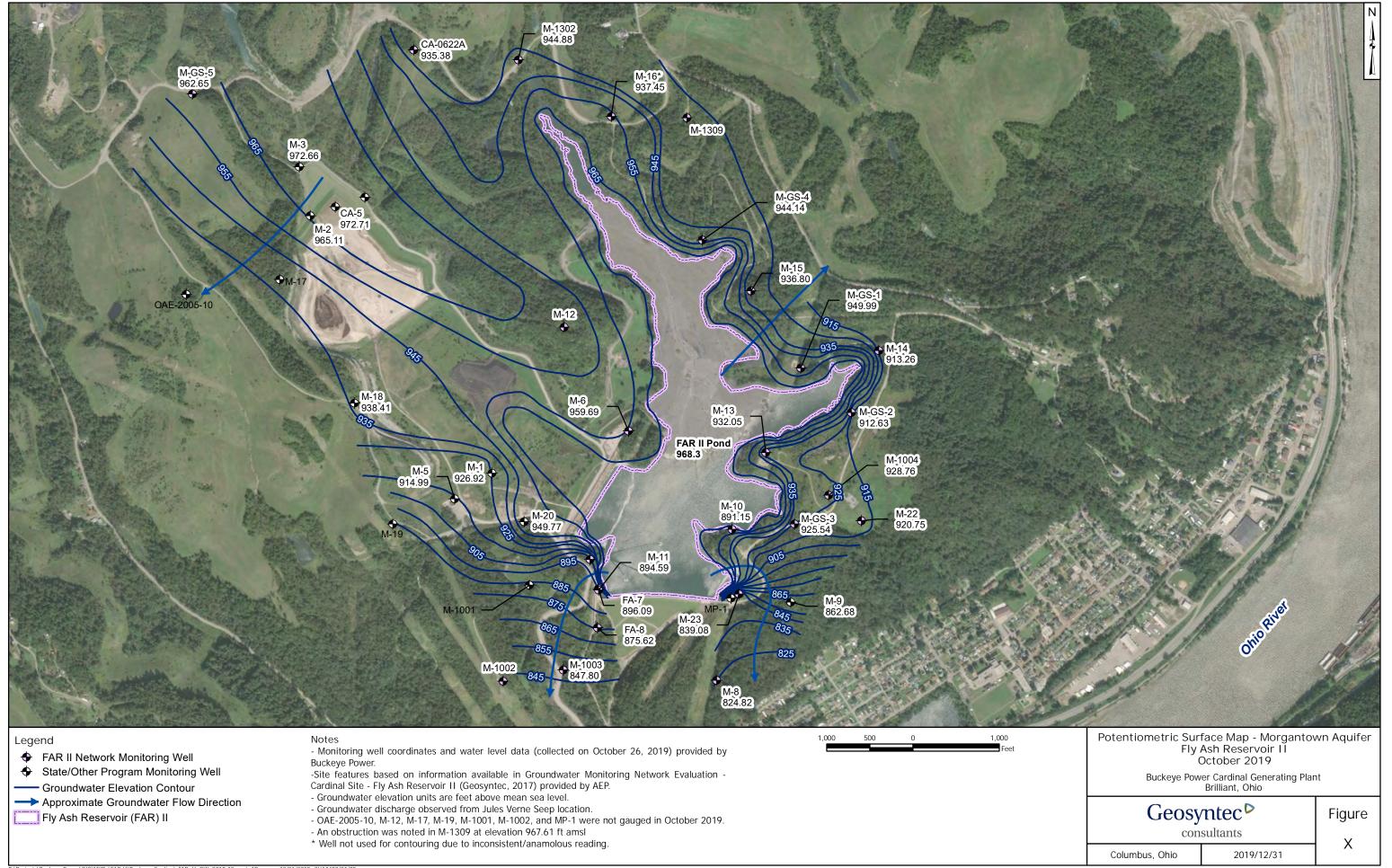
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- Geosyntec Consultants, Inc. 2019b. Statistical Analysis Summary Fly Ash Reservoir II. January.
- Geosyntec Consultants, Inc, 2019c. Groundwater Characterization Report, Cardinal Site Fly Ash Reservoir II. July.
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- United States Environmental Protection Agency (USEPA). 2015. Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities (Final Rule). Fed. Reg. 80 FR 21301, pp. 21301-21501, 40 CFR Parts 257 and 261, April.

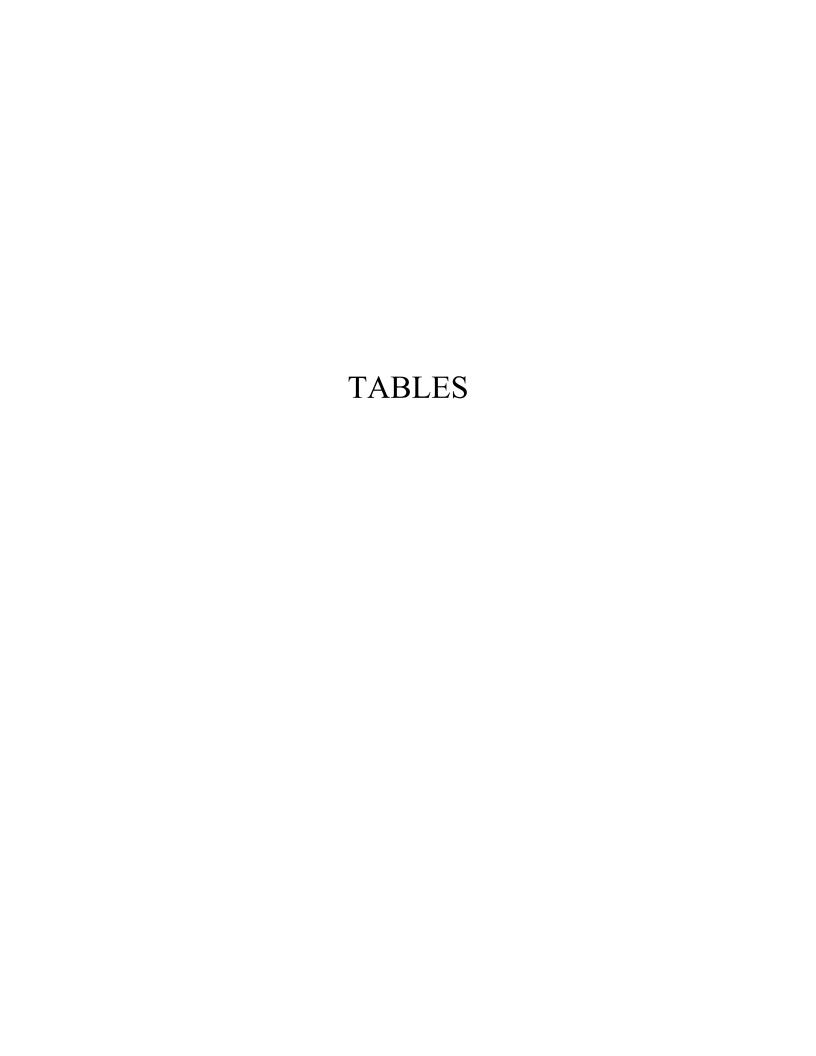












Dawamatan	Unit	CA-0622A		FA-8		M-6		M-8		M-10		M-11	
Parameter	Unit	3/26/2019	10/1/2019	4/4/2019	10/9/2019	3/28/2019	10/3/2019	4/1/2019	10/3/2019	4/2/2019	10/3/2019	4/5/2019	10/9/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.700	0.620
Arsenic	μg/L	29.2	25.8	7.70	9.20	4.10	4.70	0.940	1.10	0.500 U	0.500 U	3.00	5.40
Barium	μg/L	934	952	24.5	22.2	435	442	127	120	78.0	80.6	23.9	20.8
Beryllium	μg/L	0.100 U	0.100 U	0.100 U	0.100 U	1.50	1.80	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U
Boron	μg/L	284	337	4,980	4,740	231	239	26.6	25.8	580	536	4,670	5,000
Cadmium	μg/L	0.100 U	0.100 U	0.240	0.150	0.230	0.260	0.100 U	0.100 U	0.100 U	0.190	0.400	0.180
Calcium	μg/L	75,400	74,200	198,000	218,000	16,000	15,600	108,000	102,000	14,100	12,400	195,000	219,000
Chloride	mg/L	4,900	3,470	43.4	46.6	32.9	39.8	6.00	6.10	12.6	12.6	44.2	45.7
Chromium	μg/L	1.50	1.20	1.00 U	1.10	9.20	11.8	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U
Cobalt	μg/L	0.620	0.630	0.870	0.920	4.90	5.60	0.660	0.500 U	0.500 U	0.500 U	1.20	1.20
Combined Radium	pCi/L	11.6	11.9	0.188	1.17	6.51	5.15	0.476	0.776	1.68	0.815	0.453	1.28
Fluoride	mg/L	0.470	0.0500 U	0.600	0.570	1.20	1.10	0.0880	0.0500 U	0.690	0.670	0.580	0.550
Lead	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	22.0	25.9	0.590	0.730	0.850	1.20	0.980	0.640
Lithium	μg/L	89.8	86.7	198	194	18.3	18.6	10.0 U	10.0 U	19.6	20.0	193	188
Mercury	μg/L	0.00108	0.00185	0.000500 U	0.000570	0.0104	0.00694	0.00116	0.00214	0.000570	0.000500 U	0.000500 U	0.000610
Molybdenum	μg/L	3.40	1.90	321	303	1.00 U	1.20	0.500 U	0.500 U	2.40	2.30	316	338
Selenium	μg/L	0.500 U	1.00	2.00	0.500 U	1.00 U	1.70	0.500 U	0.500 U	0.500 U	0.500 U	4.50	0.640
Sulfate	mg/L	72.0	40.4	885	762	2.00	7.70	95.8	99.9	133	134	960	781
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Total Dissolved Solids	mg/L	6,680	7,250	1,430	1,360	478	695	421	409	711	710	1,400	1,360
рН	SU	7.49	7.85	7.12	7.02	7.71	7.95	7.37	7.33	8.40	8.44	8.01	7.15

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

D	TI .24	M-12		M-13		M-14			M-15			M-16		
Parameter	Unit	4/3/2019	10/9/2019	4/3/2019	10/8/2019	3/27/2019	7/2/2019	10/7/2019	3/25/2019	5/1/2019	9/30/2019	3/27/2019	7/2/2019	10/1/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Arsenic	μg/L	5.60	2.10	1.40	1.00	0.500 U	-	0.500 U	2.00	-	2.00	0.500 U	-	0.500 U
Barium	μg/L	89.1	28.3	218	159	14.6	-	14.3	43.2	-	45.4	37.1	-	37.4
Beryllium	μg/L	0.100	0.100 U	0.930	0.100 U	0.100 U	-	0.100 U	0.100 U	-	0.100 U	0.100 U	-	0.100 U
Boron	μg/L	324	290	261	285	224	-	231	228	-	258	192	-	184
Cadmium	μg/L	0.130	0.100 U	0.100 U	0.100 U	0.100 U	-	0.100 U	0.100 U	=	0.100 U	0.100 U	=	0.100 U
Calcium	μg/L	371,000	188,000	14,600	15,400	513	=	603	1,550	=	1,490	2,240	=	2,260
Chloride	mg/L	184	270	2.10	2.10	-	1.80	1.50	-	25.7	26.0	-	10.0	10.2
Chromium	μg/L	1.00 U	1.00 U	3.40	1.00 U	1.00 U	-	1.00 U	1.00 U	-	1.00 U	1.00 U	-	1.00 U
Cobalt	μg/L	31.7	6.20	1.20	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Combined Radium	pCi/L	1.14	1.19	3.87	1.56	0.680	=	1.22	0.681	=	0.00	0.553	=	0.805
Fluoride	mg/L	0.990	1.30	1.70	1.80	-	0.730	0.780	-	1.40	1.30	=	0.350	0.350
Lead	μg/L	1.20	0.500 U	3.30	0.530	0.500 U	=	0.500 U	0.500 U	=	0.500 U	0.500 U	=	0.500 U
Lithium	μg/L	106	108	15.8	10.8	10.0 U	=	10.0 U	10.0 U	=	10.0 U	10.4	=	11.0
Mercury	μg/L	0.00583	0.00169	0.00267	0.000510	0.000500 U	=	0.000500 U	0.000500 U	=	0.000500 U	0.000500 U	=	0.000500 U
Molybdenum	μg/L	0.500 U	0.500 U	0.730	0.680	0.500 U	=	0.500 U	0.500 U	=	0.500 U	0.500 U	=	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	=	0.500 U	0.500 U	=	0.500 U	0.500 U	=	0.610
Sulfate	mg/L	1,590	1,020	28.8	30.5	-	0.800	1.10	-	3.00	1.50	-	332	276
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U	0.500 U	-	0.500 U
Total Dissolved Solids	mg/L	2,910	2,290	510	492	347	-	347	540	-	552	780	=	757
pН	SU	667	7.22	8.15	7.48	9.19	_	8.83	9.13	-	9.03	8.34	-	8.81

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

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J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

D	Unit	M-21		M-22		M-23		M-1003		M-1004		M-1302		
Parameter	Unit	4/3/2019	10/8/2019	4/3/2019	10/9/2019	4/1/2019	10/3/2019	4/8/2019	10/9/2019	4/2/2019	10/7/2019	3/25/2019	5/1/2019	10/1/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U
Arsenic	μg/L	5.40	3.60	0.500 U	0.500 U	3.40	0.930	0.530	0.500 U	1.50	1.50	0.500 U	I	0.500 U
Barium	μg/L	14.1	13.8	25.1	21.8	27.3	8.50	84.2	79.4	47.2	44.8	107	I	106
Beryllium	μg/L	1.40	0.780	0.100 U	0.100 U	0.250	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	ı	0.100 U
Boron	μg/L	3,210	3,100	3,990	3,760	695	696	128	130	2,310	2,680	244	ı	295
Cadmium	μg/L	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	I	0.100 U
Calcium	μg/L	237,000	176,000	188,000	178,000	125,000	105,000	69,100	68,300	97,900	115,000	3,490	ı	3,340
Chloride	mg/L	51.9	63.8	44.0	43.9	12.0	13.4	5.80	6.20	31.8	35.6	=	26.6	28.4
Chromium	μg/L	1.00 U	1.00 U	1.00 U	1.00 U	3.50	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	1.00 U	I	1.00 U
Cobalt	μg/L	1.80	0.910	0.500 U	1.50	2.60	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	I	0.500 U
Combined Radium	pCi/L	0.573	0.980	0.776	1.18	2.30	2.21	2.10	3.24	0.890	1.25	0.771	I	0.421
Fluoride	mg/L	0.100	0.130	0.520	0.380	0.330	0.390	0.230	0.200	1.20	1.20	=	1.20	1.70
Lead	μg/L	3.70	1.00	0.500 U	0.500 U	3.30	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	I	0.500 U
Lithium	μg/L	80.0	66.5	52.6	52.5	57.9	48.6	10.0 U	10.0 U	21.0	18.0	13.1	I	11.7
Mercury	μg/L	0.00366	0.00156	0.000680	0.000500 U	0.0127	0.000500 U	0.000500 U	0.000640	0.000500 U	0.000510	0.000510 U	-	0.000510 U
Molybdenum	μg/L	21.3	16.6	56.5	79.1	0.500 U	0.500 U	0.500 U	0.500 U	9.40	11.6	0.500 U	-	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.560	0.530	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.590
Sulfate	mg/L	1,170	968	382	400	1,570	1,750	98.3	112	272	341	=	111	60.9
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	-	0.500 U
Total Dissolved Solids	mg/L	1,810	1,760	896	905	3,320	3,210	466	437	859	869	699	-	721
pН	SU	7.21	7.21	7.11	7.09	7.21	7.14	7.56	7.39	7.48	7.27	8.79	-	8.51

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

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J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

Danamatan	Parameter Unit		M-1309			M-2000		MGS-1		MGS-2		MGS-3		MGS-4		MGS-5	
Parameter	Unit	4/4/2019	10/10/2019	11/12/2019	4/5/2019	10/9/2019	3/27/2019	10/7/2019	4/2/2019	10/9/2019	4/3/2019	10/4/2019	4/1/2019	10/8/2019	3/26/2019	10/2/2019	
Antimony	μg/L	0.500 U	0.500 U	-	0.500 U												
Arsenic	μg/L	2.20	2.00	-	0.530	0.880	0.500 U	0.500 U	11.7	12.9	36.7	10.3	5.40	5.10	14.1	12.5	
Barium	μg/L	33.6	34.6	-	86.7	25.1	92.0	89.6	38.6	45.7	12.2	9.40	12.3	13.0	103	105	
Beryllium	μg/L	0.100 U	0.100 U	-	0.100 U												
Boron	μg/L	285	283	-	254	4,970	288	321	214	169	536	879	193	204	335	271	
Cadmium	μg/L	0.100 U	0.100 U	-	0.100 U	0.100	0.100 U										
Calcium	μg/L	5,690	4,390	-	218,000	216,000	13,200	13,300	20,900	46,300	284,000	147,000	8,170	8,040	2,950	2,730	
Chloride	mg/L	37.8	38.9	-	46.6	50.0	32.7	38.3	20.6	21.1	16.8	27.6	12.1	12.1	170	206	
Chromium	μg/L	1.00 U	1.00 U	-	1.00 U												
Cobalt	μg/L	0.500 U	0.500 U	-	0.500 U	1.10	0.500 U	0.500 U	0.770	1.20	0.660	0.840	0.500 U	0.500 U	0.500 U	0.500 U	
Combined Radium	pCi/L	0.936	1.71	-	1.72	1.24	0.316	0.901	0.307	0.177	1.37	0.850	0.0710	0.221	0.181	0.527	
Fluoride	mg/L	1.10	1.20	ı	0.370	0.380	0.650	0.640	0.370	0.370	0.170	0.140	0.510	0.510	5.40	6.60	
Lead	μg/L	0.500 U	0.500 U	ı	0.500 U												
Lithium	μg/L	23.4	17.2	ı	201	190	17.7	16.6	13.5	13.2	38.0	47.6	10.0 U	10.0 U	16.7	14.0	
Mercury	μg/L	0.00170	-	0.000850	0.000500 U												
Molybdenum	μg/L	1.50	1.10	ı	0.500 U	208	0.500 U	0.500 U	4.30	10.3	2.30	2.10	4.20	5.50	2.30	1.90	
Selenium	μg/L	0.500 U	0.500 U	-	0.500 U												
Sulfate	mg/L	94.6	88.1	-	820	830	78.9	91.2	164	162	1,330	1,290	98.1	86.7	3.50	1.60	
Thallium	μg/L	0.500 U	0.500 U	ı	0.500 U												
Total Dissolved Solids	mg/L	693	687	-	721	1,440	616	597	618	651	2,030	2,000	572	522	1,030	1,070	
pН	SU	7.94	7.57	-	6.83	6.80	7.75	7.47	7.62	7.51	679	6.59	8.46	8.13	8.70	8.50	

Notes:

mg/L: milligrams per liter μ g/L: micrograms per liter SU: standard unit

pCi/L: picocuries per liter

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J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

Table 2: Residence Time Calculation Summary Cardinal Plant - Fly Ash Reservoir II

			201	9-03	201	9-10
CCR Management Unit	Monitoring Well	Well Diameter (inches)	Groundwater Velocity (ft/year)	Groundwater Residence Time (days)	Groundwater Velocity (ft/year)	Groundwater Residence Time (days)
	CA-0622/A ^[1]	2.0	9.4	6.5	13.0	4.7
	FA-8 ^[2]	2.0	20.5	3.0	17.7	3.4
	M-10 ^[2]	0.75	NC	NC	35.0	0.7
	M-1003 ^[2]	2.0	20.0	2.7	13.8	3.8
	M-1004 ^[2]	2.0	9.1	6.7	5.0	12.1
	M-11 ^[2]	1.0	15.7	1.9	17.5	1.7
	M-12 ^[1]	2.0	2.8	21.6	NC	NC
	M-13 ^[2]	2.0	11.6	5.2	5.7	10.6
	M-1302 ^[1]	2.0	7.9	7.7	25.4	2.4
	M-1309 ^[2]	2.0	5.2	11.8	NC	NC
F1 4 1	M-14 ^[2]	2.0	65.8	0.9	62.9	1.0
Fly Ash Reservoir II	M-15 ^[2]	2.0	17.2	3.5	16.9	3.6
Reservoir ir	M-16 ^[2]	2.0	12.2	5.0	21.9	2.8
	M-21 ^[2]	2.0	7.5	8.2	8.9	6.9
	M-22 ^[2]	2.0	3.7	16.3	3.8	15.9
	M-23 ^[2]	2.0	4.3	14.0	3.9	15.5
	M-6 [1]	1.0	13.3	4.6	11.9	5.1
	M-8 ^[2]	2.0	7.0	8.7	13.1	4.7
	M-GS-1 [2]	2.0	13.9	4.4	19.1	3.2
	M-GS-2 [2]	2.0	100.9	0.6	89.1	0.7
	M-GS-3 [2]	2.0	20.7	2.9	20.7	2.9
	M-GS-4 [2]	2.0	39.6	1.5	20.0	3.0
	M-GS-5 [1]	2.0	4.6	13.4	9.5	6.4

Notes:

[1] - Background Well

[2] - Downgradient Well

NC - Groundwater residence time could not be calculated

ATTACHMENT A

Monitoring Well M-2000 Construction Diagram

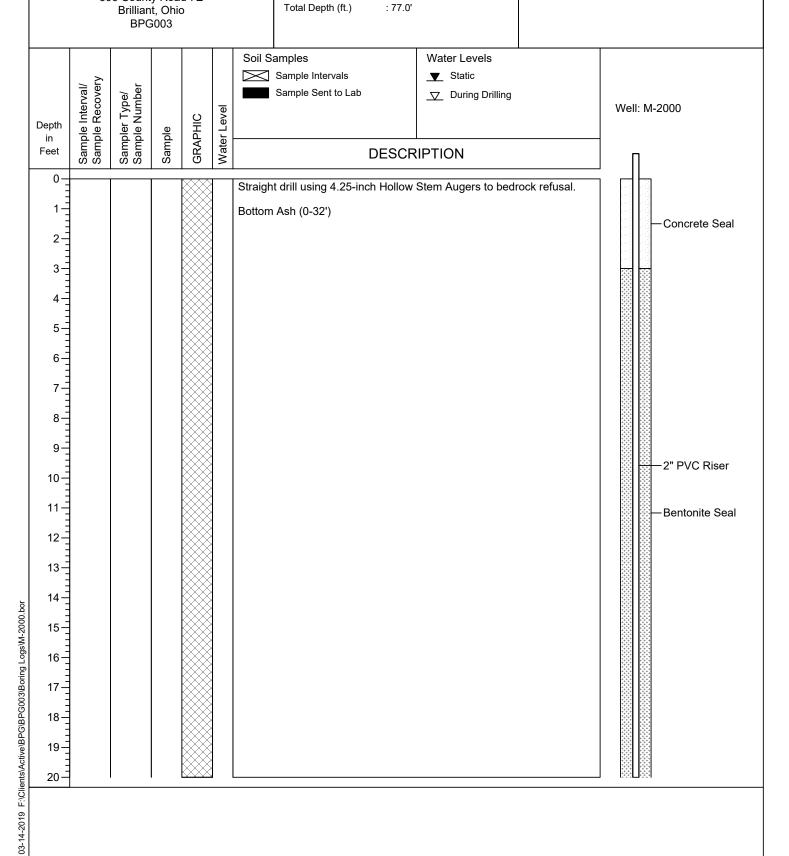


Cardinal Operating Company 306 County Road 7E Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel
Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 1 of 4)



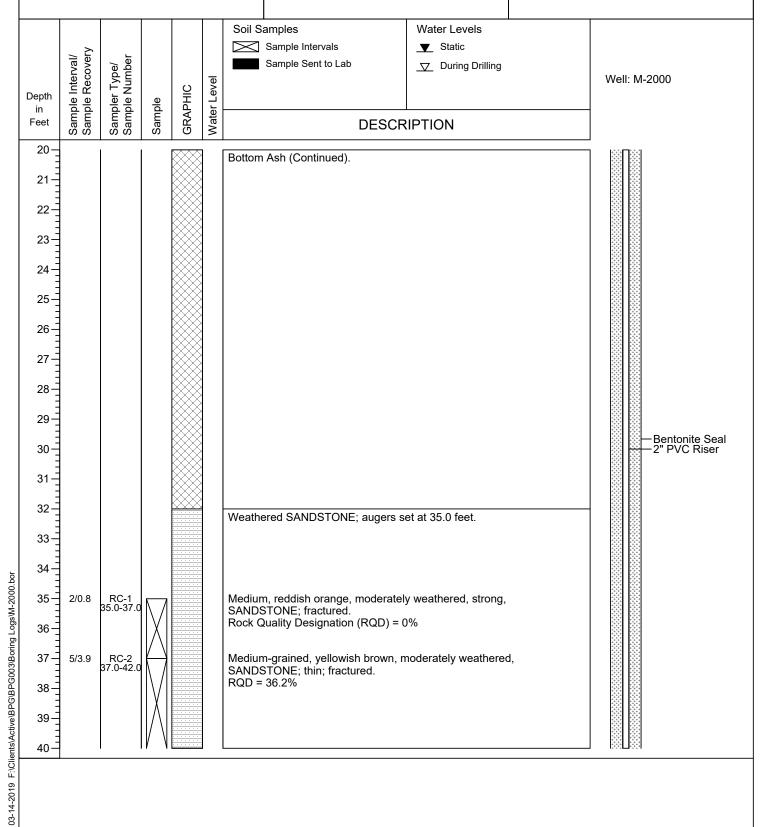


Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Testing

Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel
Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 2 of 4)





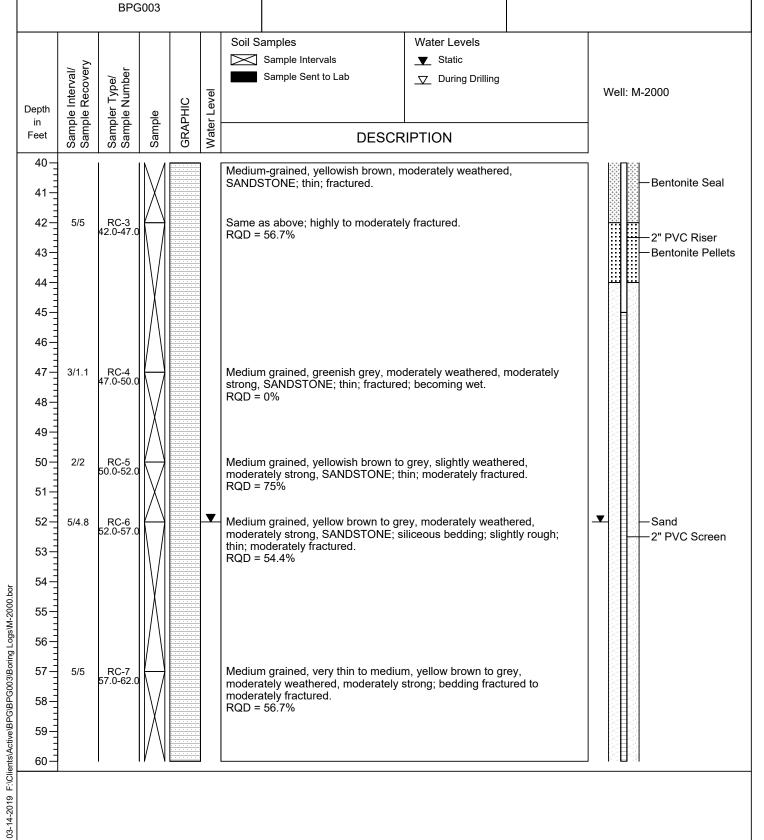
Cardinal Operating Company 306 County Road 7E Brilliant, Ohio Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner
Drilling Contractor : Terra Tes

Drilling Contractor : Terra Testing
Drilling Method : 4.25" HSA
Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 3 of 4)





Cardinal Operating Company 306 County Road 7E Brilliant, Ohio BPG003 Date Started : 3-6-2019
Date Completed : 3-8-2019
Logged by : Mielecki
Reviewed by : J. Ardner

Reviewed by : J. Ardner

Drilling Contractor : Terra Testing

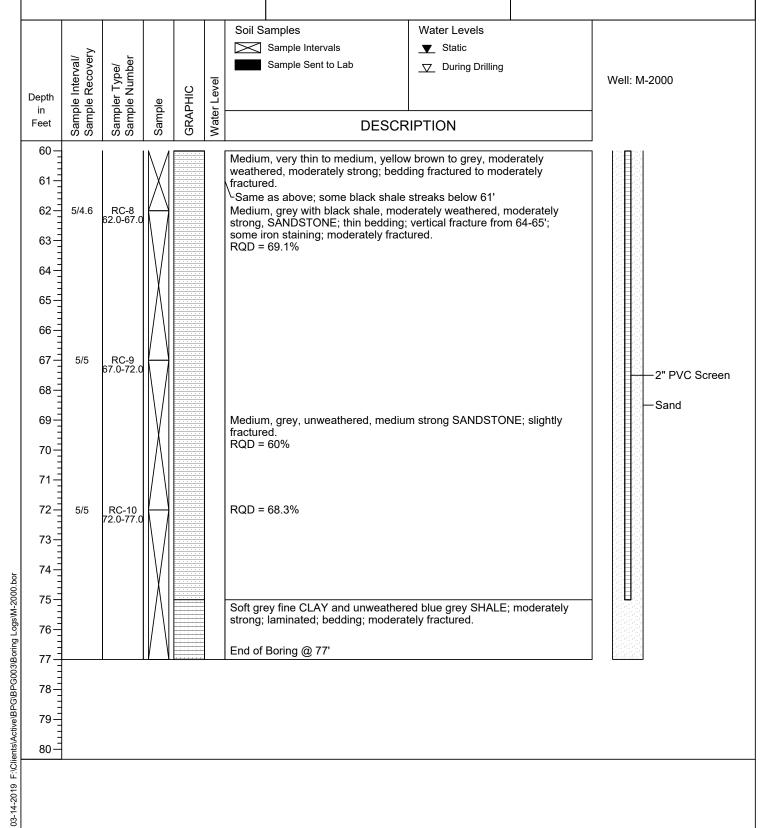
Drilling Method : 4.25" HSA

Sampling Method : 4" Core Barrel

Total Depth (ft.) : 77.0'

LOG OF BORING M-2000

(Page 4 of 4)



Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(4)

A description of site hydrogeology including stratigraphic cross-sections



- Dam Raising Design Summary Cardinal Fly Ash Retention Pond II Waste Water PTI Application, April 2012, Submitted to OEPA Division of Water Surface, AEP Service Corp.
- Dam Raising Design Report Cardinal Fly Ash Reservoir No. 2, January 2013, Submitted to ODNR Division of Soil and Water Resources, AEP Service Corp. and S&ME, Inc., and Revised Permit Application Comment Response, January 16, 2013.

2.4 <u>Hydrogeologic Setting</u>

2.4.1 Climate and Water Budget

The major drainage feature of FAR II is Blockhouse Run, which drains into the Ohio River. Approximately one mile upstream, Blockhouse Run splits into the East Branch and West Branch. The West Branch drains the western watershed and was dammed to form the former FAR 1, while the East Branch drains the eastern watershed. The FAR II inundates the East Branch, and runoff from the western watershed drains into the FAR II. The total area of the western watershed is 677 acres, while the eastern watershed is 675 acres.

The 2015 average monthly temperature and precipitation values for the Brilliant, Ohio area are presented in the table below (NOAA, 2016). The climatological data was collected from the nearest weather station (USC00338025) located in Steubenville, OH.

NOAA Climatological Summary (2015)			
Month	Average Temperature (°F)	Average Precipitation (inches)	
January	23.0	2.16	
February	16.0	1.34	
March	30.9	4.02	
April	51.1	3.60	
May	64.6	2.95	
June	70.0	10.69	
July	71.4	4.66	
August	70.5	2.81	
September	69.3	6.70	
October	53.2	2.56	
November	47.8	1.17	
December	46.6	3.24	



2.4.2 Regional and Local Geologic Setting

The geology at FAR II and the vicinity consists of nearly horizontal sequences of lower Permian and upper Pennsylvanian sedimentary rock. The Permian-age Dunkard Group occurs only on the tops of some ridges above an elevation of approximately 1250 feet (ft), northwest and west of the FAR 1 RSW Landfill and FAR II sites.

The Monongahela Group is up to 230 feet thick in Jefferson County, consisting of shale, sandstone, limestone, coal, claystone and siltstone. These rocks form much of the slopes above the current levels of the FAR II and FAR 1 RSW Landfill sites. Below the Monongahela Group is the Conemaugh Group, which is generally over 500 feet thick in Jefferson County. The Conemaugh Group consists of shale, sandstone, limestone, coal, claystone and siltstone, including the Morgantown Sandstone, which is a developed aquifer in the area. Beneath the Morgantown Sandstone is a sequence of the Conemaugh Group including the Elk Lick Limestone, the Skelly Limestone and shale, the Ames Limestone, several thick shale sequences, the Cow Run Sandstone and the Buffalo Sandstone.

2.4.3 Surface Water and Surface Water-Groundwater Interactions

Both surface stormwater and leachate from the FAR 1 RSW Landfill is transferred to FAR II as FAR II serves as the facilities sedimentation pond and leachate collection pond. The intermittent stream of the western branch of Blockhouse Hollow at the northwest end of the FAR 1 RSW Landfill was historically re-routed during surface mining operations and flows into FAR II. Streams within the watersheds of the western and eastern branches of Blockhouse Run are recharged by precipitation. The entirety of the western and eastern watersheds, including approximately 1,033 acres of woodland, drains into the FAR II Reservoir. Blockhouse run discharges to the Ohio River approximately 1.0 mile further downstream to the east. According to USACE maps, the next nearest tributary which discharges to the Ohio River is Riddles Run, which is located approximately 0.75 miles to the southwest (USACE, 2003).

Recharge of the Morgantown Sandstone aquifer occurs through vertical infiltration of precipitation at upgradient outcrops. The Morgantown Sandstone is also directly recharged by the FAR II Reservoir as it is incised through the Morgantown Sandstone unit.

2.4.4 Water Users

According to water well records obtained from the Ohio Department of Natural Resources (ODNR), the nearest water supply well is located approximately 2,000 feet east of FAR II. Additionally, ODNR records indicate a series of water supply wells in the Tidd-Dale Subdivision of Brilliant, Ohio, approximately 3,000 to 4,000 feet southeast of FAR II. These water supply wells are developed in the deeper Buffalo Sandstone, which underlies the uppermost aquifer. The ground surface elevation for these wells, generally around 750 feet, is lower than the elevation of the bottom



of the Morgantown Sandstone, generally ranging from approximately 780 feet to 800 feet in the vicinity of FAR II. One of these water supply wells has a reported pumping rate of 3.0 gallons per minute (gpm).

Approximately one mile west of FAR II, a series of water supply wells develop several limestone horizons, the Arnoldsburg and Benwood Limestone units. These well logs report pumping rates ranging from approximately 1.0 gpm to 8.0 gpm with significant drawdown (Geosyntec, 2006).

According to the 2014 Drinking Water Consumer Confidence Report prepared by the Jefferson County Water and Sewer District, there are no surface water intakes supplying water to the town of Brilliant, Ohio. Brilliant's water source comes from two groundwater wells located at a water treatment plant approximately one mile east of FAR II. ODNR records indicate these wells are screened within the alluvial deposits of the Ohio River and exhibit pumping rates of up to 700 gpm.



3. MONITORING NETWORK EVALUATION

3.1 **Hydrostratigraphic Units**

3.1.1 Horizontal and Vertical Position relative to CCR Unit

The principal regional aquifer is comprised of the alluvial sediments along the Ohio River, located east of FAR II. The hydrogeology around FAR II is characterized by an uppermost aquifer comprised of sandstone, shale and limestone units, specifically the Morgantown Sandstone, which lies below a shale aquitard that caps the Morgantown Sandstone. FAR II is positioned within a former river valley and is incised into the Morgantown Sandstone. Geologic cross-sections illustrating the horizontal and vertical position of FAR II relative to the uppermost aquifer are provided in Appendix B.

3.1.2 Overall Flow Conditions

Based on monitoring well data in the vicinity of the FAR II, the uppermost aquifer is the Morgantown Sandstone unit. A shale aquitard above the Morgantown Sandstone has very low hydraulic conductivity values, in the range of 1 x 10-7 to 1 x 10-9 cm/sec. Hydraulic conductivity values of the Morgantown Sandstone are in the range of 1 x 10-1 to 1 x 10-6 cm/sec and tends to be driven by interconnected fracture flow. The Morgantown Sandstone has a gradient to the east, southeast, and southwest, generally flowing away from FAR II (AEP, 2014). Contours depicting the groundwater elevations in the Morgantown Sandstone are shown in Figure 3-1.

3.2 Uppermost Aquifer

3.2.1 CCR Rule Definition

According to the 2015 CCR rule, the term "uppermost aquifer" has the same provisions as in §257.40: "the geologic formation nearest the natural ground surface that is an aquifer, as well as lower aquifers that are hydraulically interconnected with this aquifer within the facility's property boundary. This definition includes a shallow, deep, perched, confined, or unconfined aquifer, provided that it yields usable water" (40 CFR 257.60).

For the purposes of this report, it is assumed that the uppermost useable aquifer has the following characteristics: (1) groundwater production rate over a 24-hour period of at least 0.1 gallons per minute (gpm); and (2) groundwater quality with total dissolved solids (TDS) less than 10,000 milligrams per liter (mg/L).



3.2.2 Identified Onsite Hydrostratigraphic Unit

The hydrostratigraphy in the vicinity of FAR II is characterized by an uppermost aquifer system comprised of Morgantown Sandstone unit, which lies below the shale aquitard that caps the Morgantown Sandstone. FAR II is partially incised through the Morgantown Sandstone.

Based on ODNR water well logs, the nearest wells with a recorded pumping rate (not including wells screened in the alluvial sediments near the Ohio River) occur approximately one mile west of FAR II. These wells are screened within limestone and shale units, and at a similar elevation to the upper aquifer system at FAR II. These wells have recorded pumping rates ranging from 1.0 to 8.0 gpm. Another series of wells occurs approximately 3 miles southwest of FAR II, and are screened within sandstone and siltstone units at a similar elevation to the Morgantown Sandstone near FAR II.

Based on the information gathered from ODNR, previous analytical data, and geological conditions at FAR II, the uppermost continuous and usable aquifer is considered to be the Morgantown Sandstone.

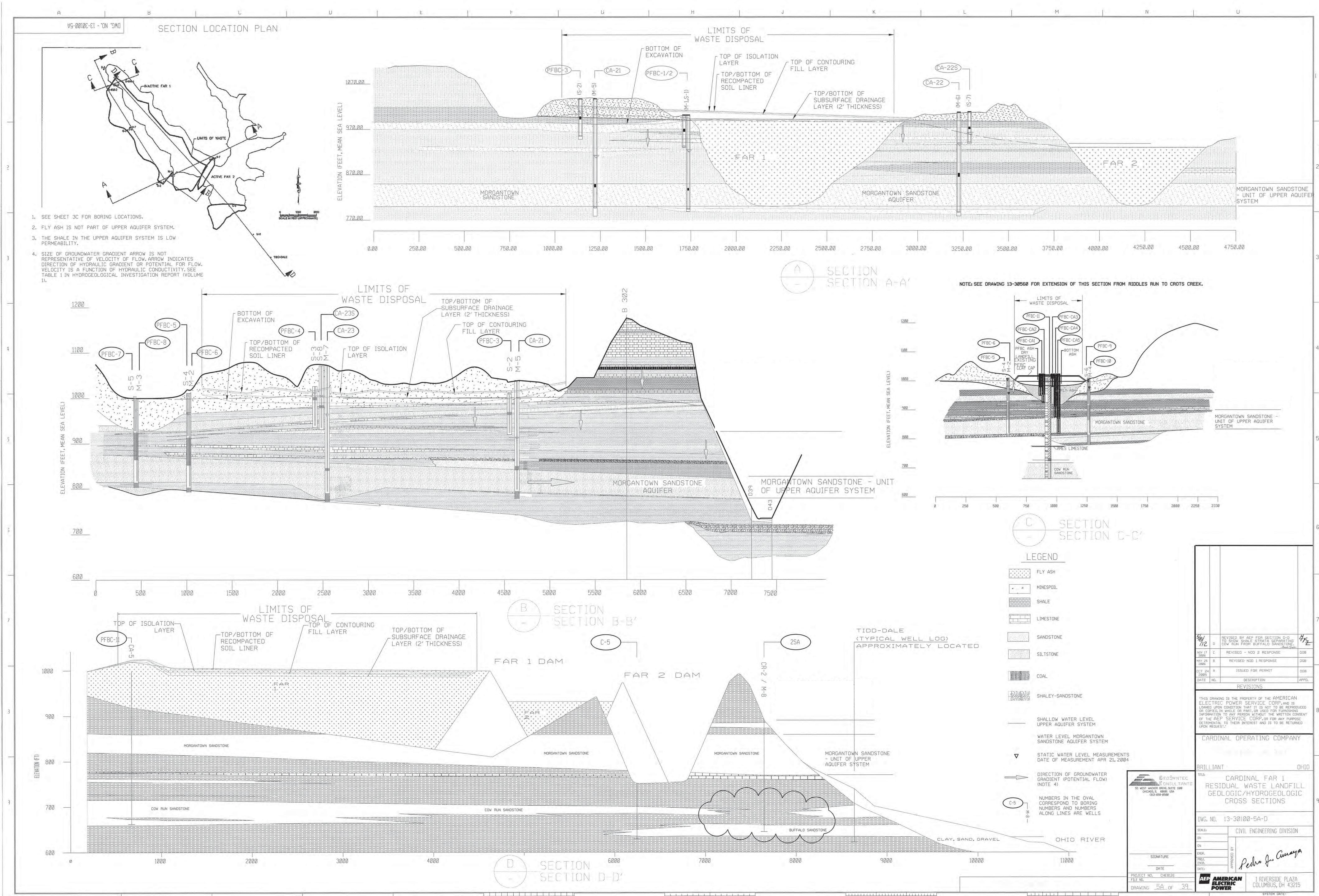
3.3 Review of Existing Monitoring Network

3.3.1 Overview

The groundwater monitoring network is shown on Figure 3-2 and consists of five (5) monitoring wells (CA-0622, M-6, M-12, M-1302 and M-GS-5) located upgradient and 18 monitoring wells (FA-8, M-8, M-10, M-11, M-13, M-14, M-15, M-16, M-21, M-22, M-23, M-1003, M-1004, M-1309, M-GS-1, M-GS-2, M-GS-3 and M-GS-4) and Seep-1, also referred to as the Jules Verne Seep, located downgradient of FAR II. The groundwater monitoring wells and Seep-1 provide detection monitoring for the uppermost aquifer (Morgantown Sandstone). The number, spacing, and depth of groundwater monitoring wells included in the groundwater monitoring network are based on site-specific geochemical, geologic and hydrogeologic information and span the full thickness of the uppermost aquifer system. Well construction details are summarized in Table 3-1. Boring and well construction logs for the groundwater monitoring well network wells are provided in Appendix C and Appendix D, respectively.

3.3.2 Compliance Assessment

Review of the existing groundwater monitoring well network in relation to the geologic and hydrogeologic conditions in the area of FAR II indicates that the monitoring well network consists of a sufficient number of wells installed at the appropriate depths to collect groundwater samples from the uppermost aquifer that accurately represent the groundwater quality upgradient and downgradient of FAR II. The groundwater monitoring well network is also capable of providing upgradient background groundwater quality and downgradient detection monitoring for a potential contaminant release to the uppermost aquifer (Morgantown Sandstone) nearest the waste boundary.



Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(5)

Any corrective measures assessment conducted as required at 40 CFR 257.96

ASSESSMENT OF CORRECTIVE MEASURES CARDINAL SITE – FLY ASH RESERVIOR II - REV.1 BRILLIANT, OHIO

Prepared for

Cardinal Operating Company 306 County Road 7E Columbus, Ohio 43213



Prepared by



engineers | scientists | innovators

941 Chatham Lane, Suite 103 Columbus, OH 43221 Project Number CHA6468

November 2020

CARDINAL FAR II ASSESSMENT OF CORRECTIVE MEASURES RECORD REVISION

Date	Date Changes Made		
7/09/2019	Initial Report Completed, Uploaded to Facility Operating Record		
11/30/20	Supplemental information added as "Attachment 1" to support Site Characterization for Site Specific Alternative Closure Demonstration		

Geosyntec consultants

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Figure 5: Potentiometric Surface Map – Morgantown Aquifer



LIST OF ACRONYMS AND ABBREVIATIONS

ACM Assessment of Corrective Measures

AEP American Electric Power Service Corporation

AMSL Above Mean Sea Level
BAC Bottom Ash Complex
CCR Coal Combustion Residual
CFR Code of Federal Regulations
ESP Electrostatic Precipitator

FAD Fly Ash Dam FAR Fly Ash Reservoir

FGD Flue Gas Desulfurization

GWPS Groundwater Protection Standards
MNA Monitored Natural Attenuation
MSE Mechanically Stabilized Earth

MW Megawatts

NPDES National Pollutant Discharge Elimination System

PMF Probable Maximum Flood

PTI Permit to Install

RCC Roller Compacted Concrete

RSW Residual Solid Waste

SCR Selective Catalytic Reduction SSL Statistically Significant Levels

TDS Total Dissolved Solids

USEPA United States Environmental Protection Agency

1. INTRODUCTION

On behalf of our client, Cardinal Operating Company, Geosyntec has produced this Assessment of Corrective Measures (ACM) report for the Fly Ash Reservoir II (FAR II) at the Cardinal Generating Plant (the Site or Facility). The Site is located one mile south of Brilliant, Ohio in Jefferson County, along the Ohio River. Under the United States Environmental Protection Agency (USEPA) Coal Combustion Residual (CCR) Rule (40 Code of Federal Regulations (CFR) 257 Subpart D), groundwater monitoring is required to assess impacts of CCR activities to groundwater compared to background conditions. In 2018, statistically significant levels (SSL) of lithium and molybdenum above their respective groundwater protection standards (GWPS) were observed at the Site, requiring an ACM under 40 CFR 257.96. This document was developed to identify potential corrective measures that may be appropriate for addressing elevated lithium and molybdenum concentrations in site groundwater and was prepared in accordance with 40 CFR 257.96.

1.1 Background

The Facility is located approximately one mile south of Brilliant, Ohio in Jefferson County along the Ohio River (**Figure 1**). The generating station consists of three units with a nominal capacity of 1,830 megawatts (MW). Units 1 and 2 began operation in 1967 and Unit 3 began operation in 1977. All three units are coal powered, with an average annual coal use of 5.2 million tons for the entire plant. As of 2012, all three units were equipped with an electrostatic precipitator (ESP), a selective catalytic reduction (SCR) system, and a flue gas desulfurization (FGD) system. Fly ash generated at the plant was formerly sluiced to the Fly Ash Reservoir I (FAR I), which was impounded by Fly Ash Dam 1 (FAD 1) from 1977 through 1988 when it was filled to capacity. The closure process for FAR I began in 1990 per Permit to Install (PTI) Application No. 17-709 (Buckeye Power, 2019).

The three CCR storage units currently utilized by the Facility, the Bottom Ash Complex (BAC), the FAR I Residual Solid Waste Landfill (FAR I RSW Landfill), and the FAR II reservoir are shown in **Figure 1**. Fly ash is currently sluiced to FAR II, which is impounded by FAD 1 and FAD 2. The construction of FAD 2 and subsequent dam raisings are discussed further in **Section 2.4**. FAR II receives sluiced fly ash from the generating unit's ESPs and collected stormwater and leachate from the FAR I RSW Landfill. FAR II/FAD 2 has a permitted discharge (Outfall 019) through the national pollutant discharge elimination system (NPDES) (Geosyntec, 2017). Monitoring wells within the CCR rule monitoring network and select other locations of interest are shown in **Figure 2**.

1.2 ACM Objective

The purpose of this ACM Report is to identify and evaluate potential technologies that may be appropriate for reducing lithium and molybdenum present in site groundwater to acceptable regulatory cleanup levels in accordance with 40 CFR 257.96. The target cleanup levels are the GWPS defined under 40 CFR 257.95(h). The site-specific GWPS for lithium and molybdenum are 140 μ g/L and 100 μ g/L, respectively. This ACM relies on the Groundwater Characterization Report for the FAR II Unit prepared by Geosyntec in 2019 to focus the evaluation of remedial technologies that will achieve the most efficient and cost-effective method of obtaining concentrations of lithium and molybdenum below the GWPS.

1.3 Report Organization

The remainder of this ACM Report is organized as follows:

Section 2: Summary of Site Conditions – This section provides a brief description of the site setting, history, and summarizes the investigations performed to support the ACM for the Site, as well as a description of anticipated future conditions at the Site.

Section 3: Evaluation of Corrective Measure Alternatives – This section provides evaluation criteria, primary corrective measure technologies, as well as a comprehensive evaluation of the most appropriate groupings of technologies identified to remediate the lithium and molybdenum groundwater impacts at the Site.

Section 4: *Next Steps* – This section presents a summary of follow-on actions pertaining to remedy selection and schedule for implementation and completion.

Section 5: *References* – This section provides a listing of the references cited in this ACM Report

2. SUMMARY OF CURRENT CONDITIONS

2.1 Site Setting and History

2.1.1 Site Geology

The Site is underlain by horizontal sequences of lower Permian and upper Pennsylvanian sedimentary rock. In the vicinity of the Site, the Dunkard Group is the upper most stratigraphic unit of the Washington Formation, and is characterized by non-marine cyclic sequences of sandstone, siltstone, shale, limestone, and coal. Associated rock outcrops appear along the northwest and west ridges of the FAR I/FAD 1 RSW Landfill.

Underlying the Dunkard Group is the Monongahela Group, which is approximately 230 feet thick in the vicinity of the Site. The Monongahela Group consists of sandstone and shale, siltstone, limestone, sandstone, and coal (American Electric Power Service Corporation [AEP], 2006).

Beneath the Monongahela Group, is the Conemaugh Group, which consists of shale, sandstone, limestone, claystone, and coal and is approximately 500 feet thick in Jefferson County (AEP, 2006). This group includes the Morgantown Sandstone underlain by the Elk Lick Limestone, the Skelly Limestone and Shale, the Ames Limestone, the Cow Run Sandstone, and the Buffalo Sandstone. The Morgantown Sandstone is a fractured and jointed conglomeratic sandstone that is approximately 75 to 100 feet thick in the vicinity of the western abutment of FAD 2 (Sanborn Head & Associates, Inc. [Sanborn Head], 2018). In the vicinity of FAD 2, the base of the Morgantown Sandstone slopes south from M-21 to the Jules Verne Seep, and east from M-1003 to the Jules Verne Seep (Sanborn Head, 2018). The Elk Lick Limestone, the Skelly Limestone and Shale and the Ames Limestone vary in a combined thickness of approximately 80 feet. At the bottom of the Conemaugh Group, the Cow Run Sandstone is approximately 20 to 30 feet thick (AEP, 2006).

Prior to the development of the FAR II, overburden in the FAR II valley consisted of 10 to 30 feet of residual soils, mine spoil, landside debris and alluvial deposits (AEP, 1984; AEP, 2006). Along the valley walls, the overburden consisted of clayey colluvium (Amaya et al., 2009). Prior to the construction of FAD 2, a landslide upstream of the western abutment of FAD 2 occurred, exposing the face of the Morgantown Sandstone at approximately 880 feet above mean sea level (AMSL).

FAR II incises the Monongahela Group and partially incises the Conemaugh Group, including the Morgantown Sandstone. Cross sections for the geology at FAD 2 are shown in **Figure 3** and **Figure 4**.

2.1.2 Site Hydrogeology

Groundwater at FAR II is present in three aquifers of interest to the ACM: the surficial aquifer, the Morgantown Sandstone, and the Cow Run Sandstone.

The surficial aquifer is contained in the Monongahela group, primarily the Connellsville Sandstone, the Summer Field Limestone, the Bellaire Sandstone, former room and pillar mines, and mine spoils. The groundwater flow in the shallow aquifer tends to follow local topography and generally has high hydraulic conductivity, ranging from 1×10^{-1} to 1×10^{-4} centimeters per second (cm/sec; AEP, 2006). The surficial aquifer and the Morgantown Sandstone are separated by a shale aquitard with a hydraulic conductivity ranging from 1×10^{-7} to 1×10^{-9} cm/s (AEP, 2006).

Regionally, the Morgantown Aquifer flows south-southeast towards the Ohio River in the vicinity of the Site. Groundwater flow in the vicinity of FAR II flows around the eastern and western abutments of the FAD 2 structure (**Figure 5**). Along the western abutment, the Morgantown Sandstone outcrops and groundwater is discharged through the Jules Verne Seep (**Figure 4**)

The Cow Run Sandstone is separated from the Morgantown Sandstone by approximately 50 to 100 feet of low permeability shale and limestone beds. The Cow Run Sandstone Aquifer generally flows south-southeast towards the Ohio River in the vicinity of the Site. Regionally, the Cow Run Aquifer is a saline aquifer, with total dissolved solids (TDS) concentrations at CR-1 and CR-2 frequently reported above 2,000 milligrams per liter (mg/L).

2.1.3 Construction of FAD 2

Construction of FAR II began in 1985 under PTI 06-1250 (Buckeye, 2019). The FAR II foundation consists of a claystone and shale, and the abutment consists of the Monongahela Group and a portion of the Conemaugh Group including the Morgantown Sandstone. Prior to the construction of FAR II, permeability testing was conducted on the abutment and foundation rock structures which indicated that the Morgantown Sandstone would be relatively impervious except where the rock face was exposed to the surface of the FAR II unit. The clayey colluvium overburden was left in place along the abutment to provide a naturally impervious barrier (Amaya et.al, 2009). However, prior the construction of FAD 2, a small landslide occurred in the clayey colluvium overburden covering the Morgantown Sandstone just upgradient of the western abutment of FAD 2 at approximately 880 feet AMSL. A cut to rock was made and a grout curtain was installed (AEP, 2016). The abutment was installed such that the clay core contacted the competent rock at 90-degree angles on the upstream side of the abutment to prevent

seepage beneath the dam and reduce cracking of the core (AEP 2016). The dam had a final crest height of 925 feet AMSL (AEP, 1997).

The FAD 2 structure has been raised twice since the initial construction. In 1997, the dam elevation was raised to 970 feet AMSL (AEP, 1997). The raising included an earthen embankment with a Roller Compacted Concrete (RCC) zone. The RCC zone was supported on the downstream side of FAD 2 with mine spoils. In 2013, the dam was raised again to a crest height of 983 feet AMSL with a back-to-back mechanically stabilized earth (MSE) wall. The MSE wall consists of a vinyl sheet pile wall that extends from the existing clay core to the Probable Maximum Flood (PMF) level (AEP, 2016). The current maximum operating stage of the FAR II unit is 974 feet AMSL (AEP, 2016).

2.1.4 Summary of ACM Investigations

Additional investigation work was completed in spring 2019 in accordance with 40 CFR 257.95(g)(1). Monitoring well M-2000 was installed in March 2019 to delineate the lithium and molybdenum release and to serve as the additional monitoring well at the facility boundary (FAR II Unit). Additional sampling of the wells in the monitoring network, M-2000, and seeps along the FAD II abutment were sampled in March, April, and May 2019. Concentrations of lithium and molybdenum above the GWPSs were observed at monitoring wells FA-8, M-11, M-2000, and the Jules Verne Seep. These results suggest impacts to the Morgantown Aquifer extend from M-11 to the Jules Verne Seep. These investigation activities and their results were documented in a *Groundwater Characterization Report* (Geosyntec, 2019a).

2.2 <u>Characterization of Release</u>

The FAR II unit discharges into the Morgantown Aquifer and impacts from the FAR II unit are limited to monitoring wells FA-8, M-11, M-2000, and the Jules Verne Seep. The Morgantown Aquifer consists of a fractured and jointed conglomeratic sandstone with fractures and joints through which water from the FAR II unit flows around the FAD 2 structure on the western side and ultimately to the Jules Verne Seep. As shown in **Figure 3**, the hydraulic head in the Morgantown Aquifer along the north-south transect of the dam is from north to south (M-11 to M-2000). Along the east-west transect, the hydraulic gradient is from west to east and ultimately discharges through the Jules Verne Seep (M-1003 to Jules Verne Seep; **Figure 4**). Therefore, impacts from FAR II enters the Morgantown Aquifer in the vicinity of M-11 and discharges through the outcrop of the Morgantown Sandstone at Jules Verne Seep. Groundwater discharging from the Jules Verne Seep is collected at the base of FAD 2 and discharged to the Ohio River through NPDES Permitted Outfall 19.

2.3 **Anticipated Future Conditions**

As required under 40 CFR 257.101(a)(1), by October 31, 2020 the facility will cease placing CCR and non-CCR waste streams into the FAR II unit and close the unit in accordance with 40 CFR 257.102. This change in waste disposal practices will be achieved through operational changes to dry ash handling.

Following closure, the facility will comply with the post-closure care and maintenance requirements for a period of 30 years or more, as required by 40 CFR 257.104. These post-closure requirements include maintaining the final cover system, maintaining the leachate collection system, maintaining the groundwater monitoring system, and monitoring groundwater in accordance with 40 CFR 257.90 through 257.98.

3. EVALUATION OF CORRECTIVE MEASURES ALTERNATIVES

3.1 Evaluation Criteria

The evaluation criteria used to determine the appropriateness of the proposed remedies are outlined in 40 CFR 257.96 and include (1) performance, (2) reliability, (3) ease of implementation, (4) potential impacts, (5) time to begin/complete remedy, and (6) institutional requirements. Each of the evaluation criteria are defined and briefly described in the following paragraphs.

3.1.1 Performance

Corrective measure remedies must be protective of human health and the environment. Human health can be protected by preventing exposures through engineering and institutional controls or by reducing concentrations of all chemicals in all media to levels that meet the required corrective measure standards¹.

¹ The risk to human health and the environment from exposure to CCR-related constituents in groundwater at the Site was assessed (Geosyntec, 2019b). The risk assessment included an exposure assessment, and a screening-level risk evaluation. The purpose of the exposure assessment was to identify potentially complete exposure pathways by which human or ecological receptors may contact lithium or molybdenum in groundwater, while the purpose of the screening-level risk evaluation was to quantitatively evaluate receptor-exposure scenarios for pathways identified as complete or assumed-to-be compete.

The assessment evaluated current conditions at the Site and assumed that any changes in site conditions, such as FAR II no longer receiving fly ash, likely result in an overestimate of potential exposures and risks. Based on the results of the exposure assessment and screening-level risk evaluation, lithium and molybdenum in FAR II groundwater are unlikely to pose an unacceptable risk to human or ecological receptors in the vicinity of the site under current or near-term future conditions. Anticipated future site conditions are expected to further reduce these risks in the future; however, in the interim, additional actions are not necessary to protect human health and the environment.

Preference is generally given to techniques that include source control or reduce the potential for future environmental releases, continuing migration or exposures to human health and the environment by reducing the toxicity, mobility, or volume of source material released.

3.1.2 Reliability

This evaluation criterion is used to consider future conditions, which is important for locations where remedial goals and objectives will take several decades or more to be achieved. Corrective measures that incorporate some degree of source removal or control are more effective and reliable in the long-term than technologies that rely on perpetual operation. Alternatives are compared in terms of the risk remaining at the site after the cleanup objectives have been met; the long-term impacts of any adverse consequence of any alternative; operation and maintenance requirements; and the continuity of institutional controls through administrative changes and ownership transactions.

3.1.3 Ease of Implementation

This criterion addresses both technical and administrative feasibility of executing a remedial alternative and the availability of various services and materials required during its implementation. The ease of implementation considers:

- Availability of materials and skilled workers to construct, operate, and maintain the system;
- Ease of undertaking or implementing additional remedial actions, off-site storage, or disposal services;
- Consistency of approach with measures that are already operating at the Site;
- Time for full-scale implementation; and
- Time required for beneficial results to be achieved.

Administrative ease of implementation, which involves evaluation of the time and practicability of obtaining needed permits, rights-of-way, or any other administrative approvals, is addressed in the Institutional Requirements evaluation criteria.

3.1.4 Potential Impacts

This evaluation criterion considers the potential impacts of the corrective measure implementation. Per 40 CFR 257.96, these impacts include "safety impacts, cross-media impacts and control of exposure to residual contamination." Impacts may be negative such as increased risk of accidents due to trucking, or carbon emissions due to pumping requirements. Some impacts may be unknown due to data gaps, such as potential

alteration of the geochemistry resulting in mobilization of other constituents or a reduction of groundwater base flow to adjacent waterbodies.

3.1.5 Time Requirements

This evaluation criterion considers the time to begin and complete the remedy to minimize risk in the interim. This evaluation includes the timing of construction, start-up and completion. In this way the assessment may consider the immediate to short-term reduction in exposure risk to receptors. Remedial actions that offer more rapid reduction of COCs in media of concern are favored over remedies that may not reach full effect for years or decades.

3.1.6 Institutional Requirements

This evaluation criterion addresses how the specific corrective measure activities will be conducted in compliance with all applicable local, state and federal regulations (e.g., waste handling, closure requirements, land disposal restrictions, discharge permits).

3.2 Development of Remedial Technologies

An initial screening was conducted across a range of existing remedial technologies including containment, in-situ treatment, mass removal, ex-situ and integrated approaches. This screening resulted in the identification of five primary corrective measure technologies that could feasibly be implemented within the limitations of the physical setting and geochemistry of the FAR II Unit. The five technologies are (1) Monitored Natural Attenuation (MNA), (2) Vertical Barrier, (3) Cap & Operational Modification, (4) Groundwater Extraction, and (5) Ex-Situ Treatment.

3.2.1 MNA

MNA is an in-situ remedial technology that relies on natural processes occurring in aquifers to attenuate dissolved contaminants and thereby reduce their concentrations in groundwater. MNA is effective at sites where the source is controlled, the contaminant plume is stable, and contaminant concentrations are low. Natural attenuation of lithium mainly relies on the dilution process. Dilution is a physical attenuation mechanism that reduces concentrations by distributing constituent concentrations over large volumes of groundwater. Molybdenum is geochemically more reactive and may be attenuated further through precipitation or sorption processes. Precipitation and sorption are chemical mechanisms that reduce concentrations by immobilizing constituents in groundwater.

As concluded in the risk evaluation (Section 1.2), lithium and molybdenum are unlikely to pose unacceptable risks to nearby human or ecological receptors. Additionally, the concentrations of these inorganic constituents in groundwater is low, with concentrations



remaining less than one order of magnitude above the GWPS. Due to the low risk to human and ecological receptors and low constituent concentrations, MNA is a viable remedial option.

Advantages:

One of the main advantages of MNA technology is the ability to utilize naturally occurring processes to attenuate concentrations in groundwater. In addition, MNA requires little infrastructure and causes minimal disruption to remediation areas.

Disadvantages:

The MNA remedial option requires that groundwater impacts be stable, otherwise source treatment and control may be required. Another disadvantage for application of MNA for molybdenum is that attenuation of metals does not result in their destruction and the attenuation processes could be reversed under changed subsurface conditions.

3.2.2 Vertical Barrier

Vertical barriers are remedial technologies that utilize low-permeability vertical barriers, such as slurry walls or grout curtains, installed around or downgradient of the waste mass to limit the future migration of groundwater impacts. Soil-bentonite slurry walls are commonly used and are installed by either conventional trenching, continuous trenching, or bio-polymer slurry trenching. Grout curtains are typically installed using injection of cement-based grout into underlying bedrock. Slurry walls and/or grout curtains are installed generally with surface caps for more complete containment. Gradient control systems can be used in conjunction with the vertical barrier technology to prevent groundwater mounding behind the barrier. Because this approach does not rely on the geochemical properties of lithium and molybdenum, it is likely to be equally successful for both constituents of interest.

Advantages:

Employment of vertical barriers is a proven technology that is a reliable source control measure for the entire suite of CCR constituents of interest, especially when used in combination with other technologies, such as capping and gradient control systems. Specifically, slurry walls are an effective technology that prevents groundwater migration in the subsurface and grout curtains mitigate groundwater flow through fractured bedrock. Barriers can also be implemented at both active and closed CCR sites.

Disadvantages:

The vertical barrier technology is limited by installation depth and the requirement to find a suitable low permeability layer. In addition, geologic considerations at the site may make it difficult to construct the barrier. For example, variability in fractured bedrock creates difficulty in ensuring the full continuity of the grout curtain. Moreover, dewatering or groundwater extraction may be necessary to relieve backpressure from groundwater flow prior to grouting. Additionally, groundwater extraction may be required after grouting to relieve backpressure as groundwater flow is restricted behind the barrier.

3.2.3 Cap & Operational Modification

The capping technology includes a low permeability cover installed over the waste surface to prevent vertical infiltration of stormwater into the CCR unit and reduce impacted groundwater generation. The implementation of a cap system would require operational modification to dry ash handling and subsequent unit closure.

Advantages:

Caps are an effective means for source control by preventing vertical infiltration and generation of impacted groundwater.

Disadvantages:

Although caps are effective at minimizing stormwater infiltration, the effectiveness increases when used with other technologies.

3.2.4 Groundwater Extraction

Groundwater extraction technology consists of a network of vertical or horizontal extraction wells to capture and remove contaminated groundwater. Wells can be located both downgradient and within the waste to effectively limit horizontal migration of the groundwater plume and reduce total contaminant mass. Because lithium and molybdenum are not attenuated, they can be readily extracted with groundwater. The extracted groundwater will require ex-situ treatment and permitted discharge.

Advantages:

Groundwater extraction is a proven technology effective at source capture and removal. Groundwater extraction can be used successfully in bedrock aquifers.

Disadvantages:

Groundwater extraction systems will likely require a large quantity of extraction wells to provide adequate hydraulic containment. Complex site geology and anisotropic conditions could challenge the effectiveness of the extraction system. This technology also requires ex-situ water treatment system with additional operation and maintenance considerations.

3.2.5 Ex-Situ Treatment

Ex-situ treatment consists of various technologies that treat extracted groundwater prior to permitted discharge. Such technologies include; precipitation/co-precipitation, adsorption, and membrane filtration.

Precipitation/co-precipitation uses chemicals to transform dissolved contaminants into an insoluble solid. The precipitation/co-precipitated solid is then removed from the liquid phase by clarification or filtration. Adsorption is accomplished by passing contaminated groundwater through a column where the contaminants are adsorbed into the column media. The column must be regenerated or replaced when the media becomes full. Lastly, membrane filtration separated contaminants from water by passing it through a semi-permeable barrier or membrane.

Advantages:

Ex-situ treatment can be combined with other technologies to facilitate their application.

Disadvantages:

This approach has limited applicability for lithium lithium precipitation is limited and dependent on pH and other variables. Likewise, lithium adsorption is rarely favorable. Both lithium and molybdenum require additional removal steps and produce large volumes of residuals. Additionally, the potential for high concentrations of competing contaminants and the fouling of the adsorption media due to the presence of other suspended or dissolved matter could inhibit its efficiency

3.3 Description and Assessment of Corrective Measure Options

The five identified technologies discussed in **Section 3.2** were then assembled into four corrective measure options:

- (1) MNA includes MNA only
- (2) Closure and Monitor FAR II unit closure and capping with long-term monitoring

- (3) *Bedrock Grouting* –bedrock grouting of west FAD 2 abutment, with groundwater extraction to control hydraulic gradients and ex-situ treatment if needed
- (4) *Hydraulic Gradient Control* Groundwater extraction upgradient of west FAD 2 abutment with ex-situ treatment

Each of the four corrective measure options is described and evaluated based on the evaluation criteria presented in Section 3.1. The findings of this section are summarized in **Table 1**.

3.3.1 Option #1 – Monitored Natural Attenuation

MNA relies on natural attenuation processes to achieve site-specific GWPS within a reasonable time frame. It requires demonstration of attenuation mechanisms and aquifer attenuation capacity over the long term. While there are few potential impacts and a high ease of implementation, the time to complete the remedy cannot be determined at this time as it relies on performance and this technology performs best when paired with source control.

3.3.2 Option #2 – Closure of FAR II unit with Long-Term Monitoring

Anticipated operational changes to dry ash handling allows for unit closure and capping. As part of closure, the unit will be dewatered and the proposed cap will prevent infiltration of precipitation in to the groundwater system. Closure will be completed in accordance with 40 CFR 257.100 through 257.104. This plan will incorporate long-term monitoring and will address any potential long-term impacts, including any groundwater issues associated with future Site conditions.

3.3.3 Option #3 – Bedrock Grouting or Cutoff Wall

This option will include bedrock grouting of the FAD 2 western abutment to cut off flow of impacted groundwater from the vicinity of M-11 to Jules Verne seep. Groundwater extraction will be required to minimize hydraulic head such that the grouting can be implemented. Data gaps associated with complex fractured bedrock geology limit evaluation of the performance of this option. Long-term groundwater extraction may be required to control groundwater flow following implementation of bedrock grouting if it changes groundwater direction or hydrostatic pressure behind the dam. If long-term extraction is required, the time to complete remedy could be infeasible. Extracted groundwater may require treatment prior to discharge.

3.3.4 Option #4 – Hydraulic Gradient Control

Extraction wells are used to capture impacted groundwater and hydraulically contain impacts. Captured groundwater may subsequently require ex-situ treatment and discharge to a permitted outfall. Reliability and performance may be limited due to the complex, fractured bedrock geology. Additionally, the time to complete this remedy could be infeasible.

4. NEXT STEPS

According to the 40 CFR 257.96, the owner or operator must discuss the results of this ACM in a public meeting with interested and affected parties at least 30 days prior to the selection of the remedy. Remedy selection will occur as soon as feasible based on the need to fill data gaps prior to remedy selection. The remedy selection will include a schedule for implementation and completion. The unit will cease receiving waste no later than October 31, 2020, which will initiate the closure and post-closure care process.

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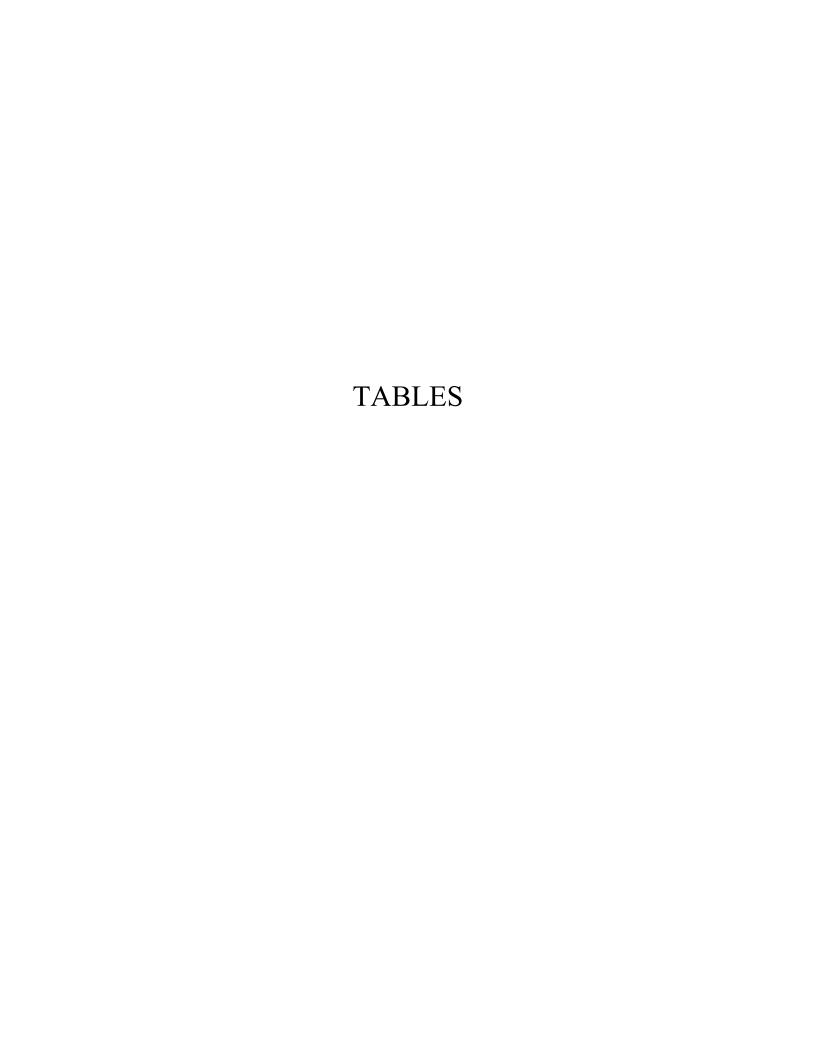


TABLE 1 - DEVELOPMENT AND EVALUATION OF POSSIBLE CORRECTIVE MEASURE OPTIONS SUMMARY

Assessment of Potential Corrective Measures for Groundwater CCR Unit - Fly Ash Reservoir II
Cardinal Plant, Brilliant, Ohio

Possible Corrective Measures Technologies / Evaluation Criteria Considered		easures Cardinal Plant, Brilliant, Onio Corrective Measures Options			
		CMO #1: MNA	CMO #2: Closure and Monitoring	CMO #3: Bedrock Grouting	CMO #4: Hydraulic Gradient Control
Primary Corrective Measure Technologies	Monitored Natural Attenuation	•	•		
	Vertical Barrier			•	
	Cap & Oper. Modification		•		
	Groundwater Extraction			•	•
	Ex-Situ Treatment			0	•
Summary	Description of CMO	species relies on physical and chemical processes to achieve site-specific groundwater protection standards (GWPS) within a reasonable time frame. This approach requires demonstration of attenuation mechanisms and aquifer capacity over the performance period. The primary	requires operational changes to dry ash handing allowing for unit closure, dewatering and capping. Capping acts as source control to prevent influx of precipitation and production of leachate. As with CMO #1, MNA relies on natural attenuation processes to achieve site-specific GWPS within a reasonable time frame and to monitor impacts	The bedrock grouting corrective measure involves grouting fractured bedrock along the western abutment of Fly Ash Dam II in order to limit the migration of impacted groundwater. Groundwater extraction will be required to reduce hydraulic head prior to grouting. Additional measures, including groundwater extraction and treatment may be necessary to address groundwater migration through fractured bedrock.	in the vicinity of the western abutment upgradient of the existing groundwater seeps. Groundwater extraction wells are used to capture and hydraulically contain impacted groundwater. Extracted groundwater would subsequently require on site ex-situ treatment and discharge to
Assessment Criteria (40 CFR §257.96)	Performance	within a reasonable time frame. Currently, dilution is the primary attenuation process that reduces exposure pathways. Additional data is needed to further evaluate the attenuation capacity of the site and determine the future performance of the MNA corrective measure because the FAR II is incised into the Morgantown sandstone and upgradient groundwater will continue to migrate through the fly ash within the unit. Additionally, precipitation may infiltrate the fly ash and mobilize lithium and molybdenum. However, performance of MNA is enhanced when it is used in combination with source control technologies.	effectively achieve GWPS. Currently, dilution is the primary attenuation process that eliminates exposure risks. After dewatering and closure, it is expected that the hydrostatic head within the impoundment should approximately equal historic groundwater elevations in the Morgantown sandstone at the west abutment. Additional data will be collected after the closure of the unit to address any post-closure concerns.		limit infiltration, control hydraulic gradient, and reduce hydraulic head behind the dam. Additional data from pump tests, flow modeling, and capture zone analysis will need to be collected to adequately evaluate performance.
	Reliability	data necessary to evaluate the reliability of the MNA option. It is important to plan a tiered	thus achieving source control. Given that no current exposure risks were identified, MNA is a sufficient method to monitor downgradient concentrations.	bedrock grouting ranges from moderately uncertain to moderately reliable as a source control measure to prevent migration of	geology, complete capture of groundwater using

TABLE 1 - DEVELOPMENT AND EVALUATION OF POSSIBLE CORRECTIVE MEASURE OPTIONS SUMMARY

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Cardinal Plant, Brilliant, Ohio

Possible Corrective Measures Technologies / Evaluation Criteria Considered		Corrective Measures Options			
		CMO #1: MNA	CMO #2: Closure and Monitoring	CMO #3: Bedrock Grouting	CMO #4: Hydraulic Gradient Control
rimary Cor Measur Technolo	Monitored Natural Attenuation	•	•		
	Vertical Barrier			•	
	Cap & Oper.Modification		•		
	Groundwater Extraction			•	•
	Ex-Situ Treatment			o	•
Assessment Criteria (40 CFR 257.96)	ID OT 1	straightforward with respect to the installation of infrastructure. The current groundwater monitoring well network should continue to provide adequate monitoring capability for mass flux calculations needed as part of MNA.	respect to infrastructure as capping of the unit is a significant effort. A sufficient groundwate monitoring well network currently exists Additional groundwater sampling which will be	The ease of implementation is moderate with respect to construction. Additional data is required to aid in design of both the groundwater extraction system and the bedrock grouting approach. If utilized, the groundwater extraction and water treatment systems will have additional operation and maintenance requirements.	groundwater extraction and water treatment systems will have additional operation and
	Potential Impacts	the aquifer; therefore, surface and subsurface impacts that are adverse to treatment are unlikely. Although exceedances have been	construction and include land disturbance trucking and equipment activity, and carbon emissions. Any long-term impacts will be	g Intermediate impacts include changes to groundwater flow/rerouting and increase in hydrostatic pressure behind the dam. While unlikely, dam weakening is a potential impact. Additional data will need to be collected to determine the potential impacts from changed groundwater conditions.	of spent media from the ex-situ treatment process.
		MNA option is very short. However, it will take some additional time to collect the data necessary to establish groundwater flow characteristics and attenuation capacity. The time to complete the remedy cannot be determined at this time, as it relies on MNA	Time to implement capping and monitoring will be moderate. It will take time to complete dewatering operations, cap design, cap construction. Upon completion, monitoring can begin immediately since the groundwater monitoring network is already established. The time to complete the remedy cannot be determined at this time, as it relies on MNA performance.	moderate. It will take time to complete the design, groundwater extraction system installation, ex-situ water treatment system installation (if needed), and bedrock grouting operations. The groundwater extraction and	The time to implement the corrective measure is moderate. It will take time to complete the design, groundwater extraction system installation, and ex-situ treatment installation. The groundwater extraction and treatment systems must be maintained long term; therefore, the time to complete is indefinite.
		Groundwater is currently captured and discharged under the existing NPDES permit. There are no anticipated changes to present operations and water will continue to be discharged under the existing NPDES permit. Given that receptors are currently not at risk of exposure, no additional changes are required to minimize risk.	and discharged under the NPDES permit after mixing with other discharge streams.	1 .	A permit would be required for discharge of extracted groundwater.

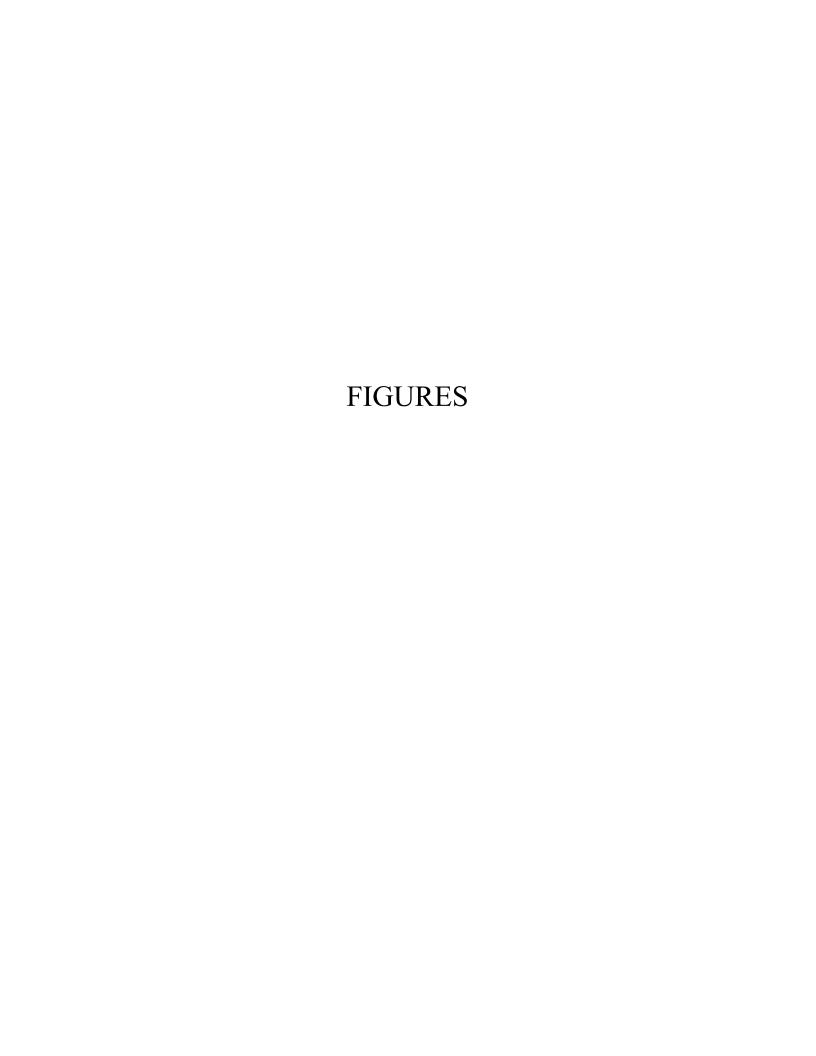
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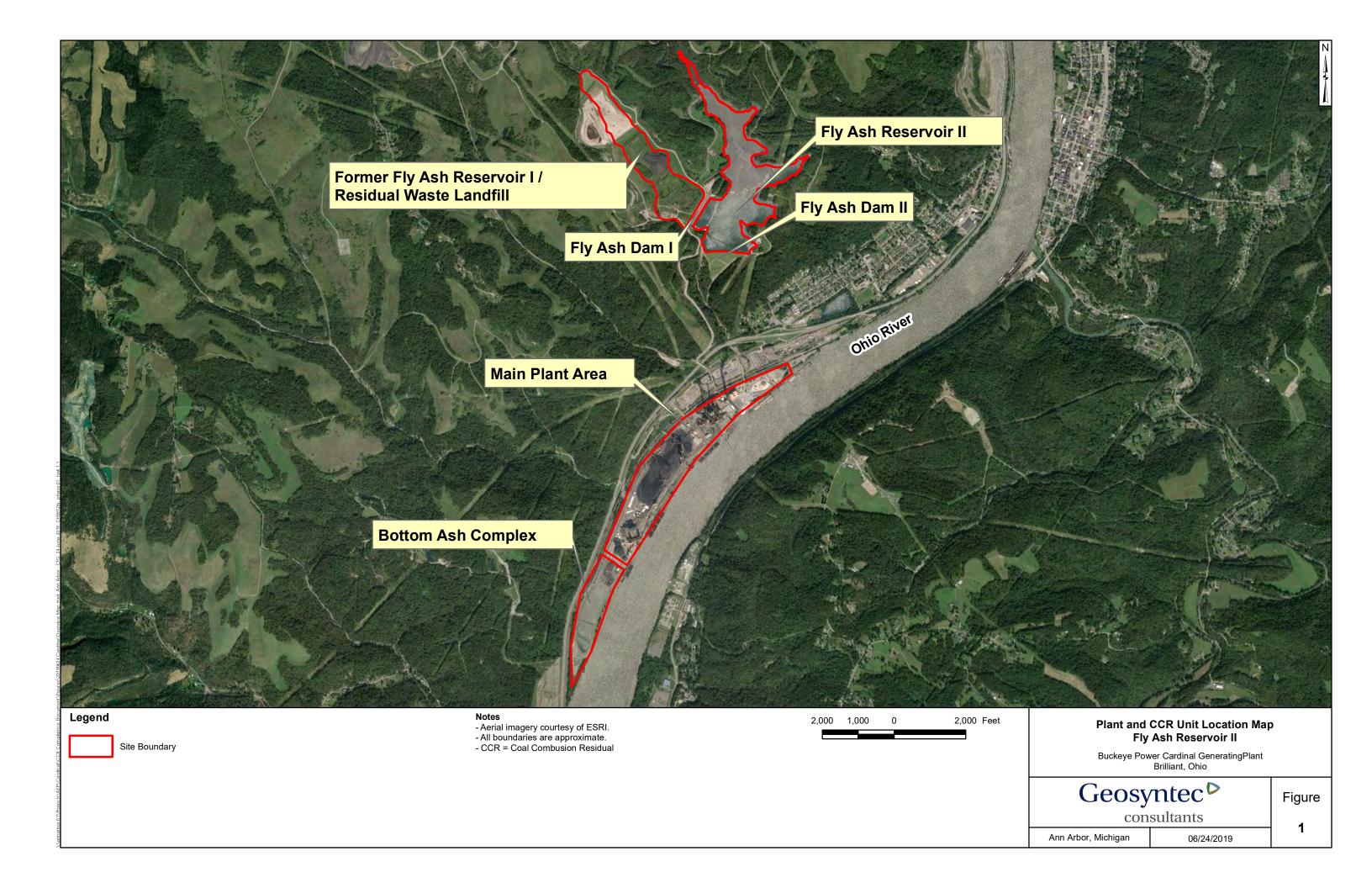
CMO - corrective measures option

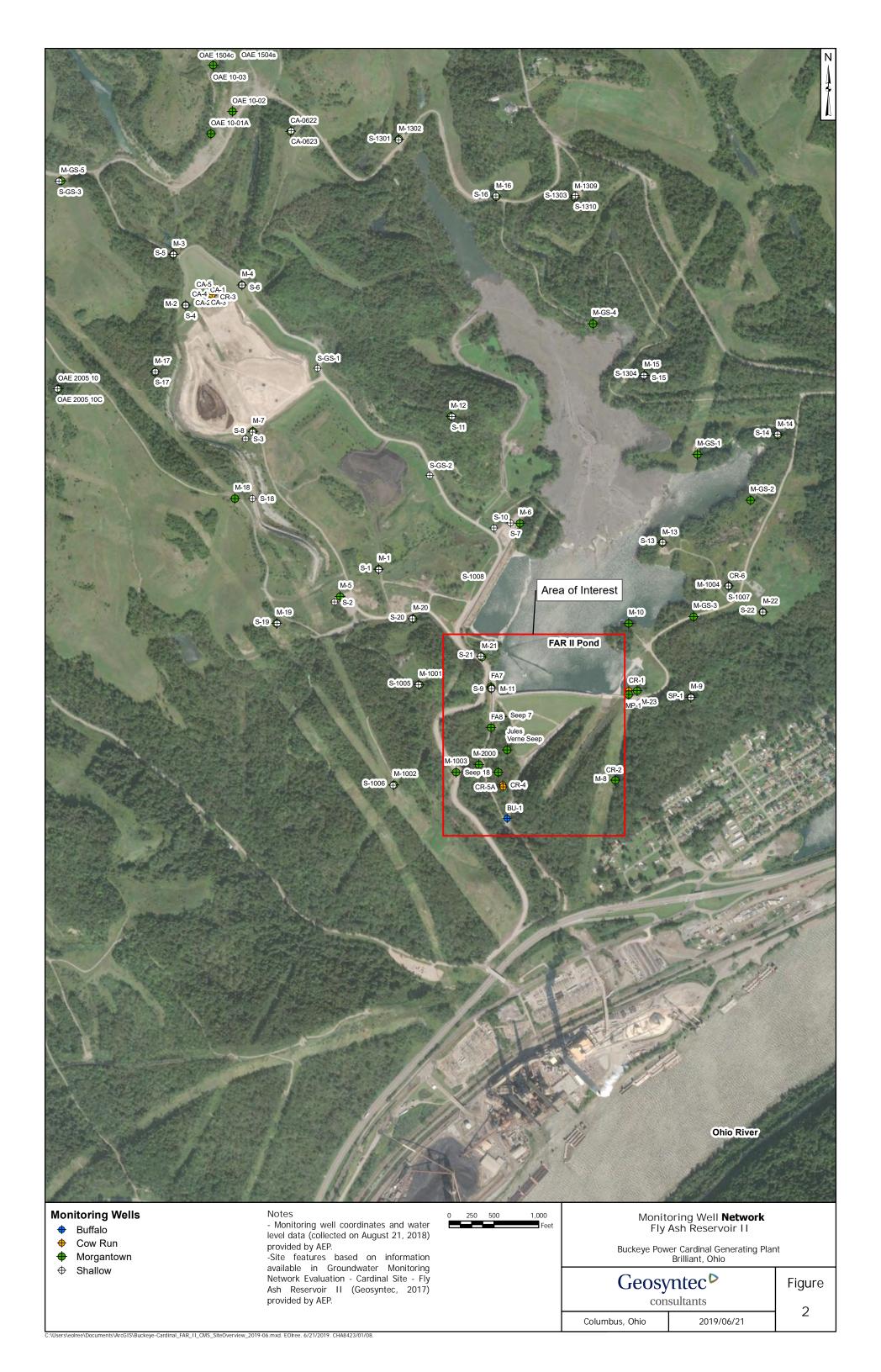
MCL - federal drinking water maximum contaminant level

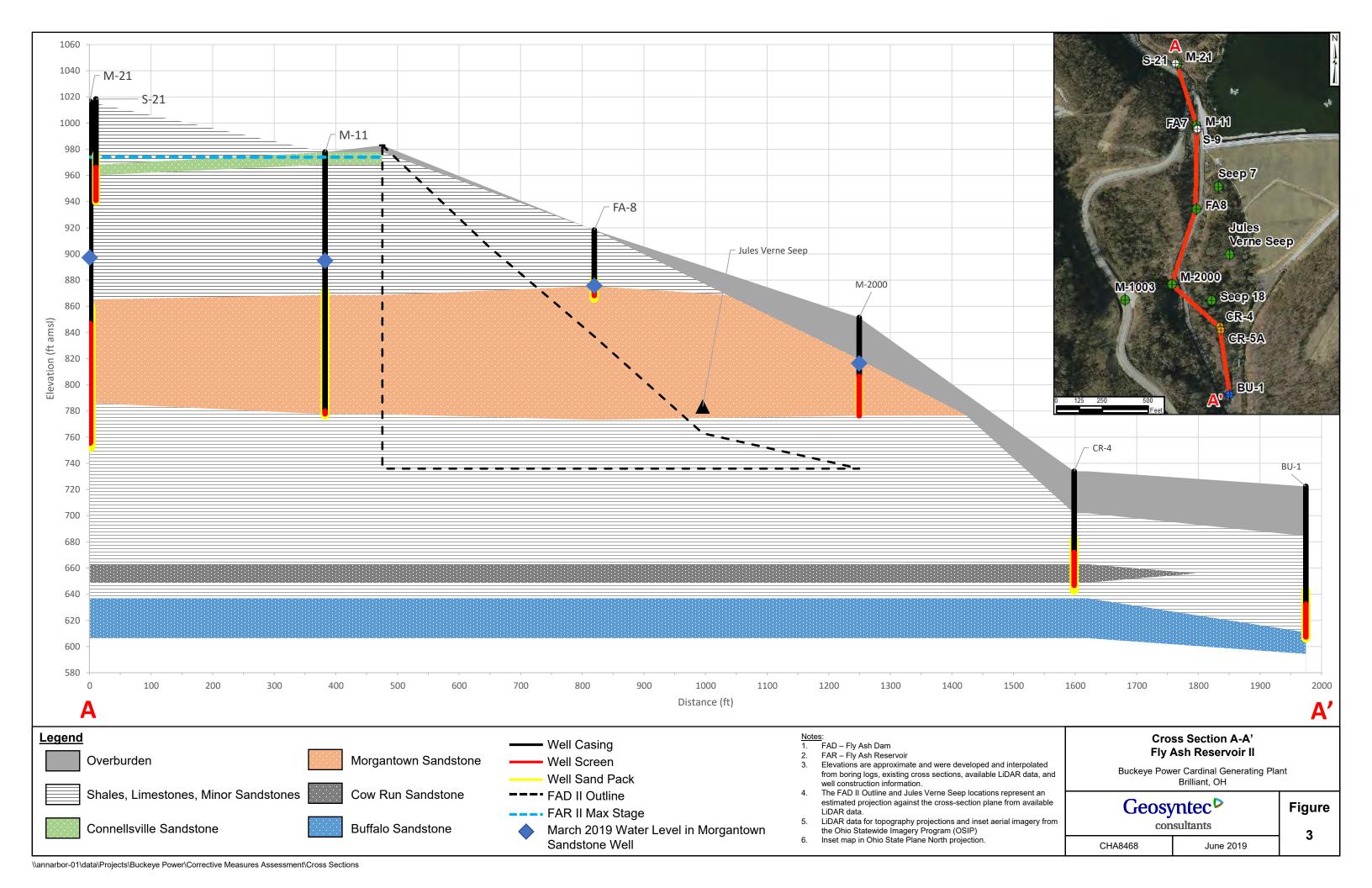
• = technology is part of CMO

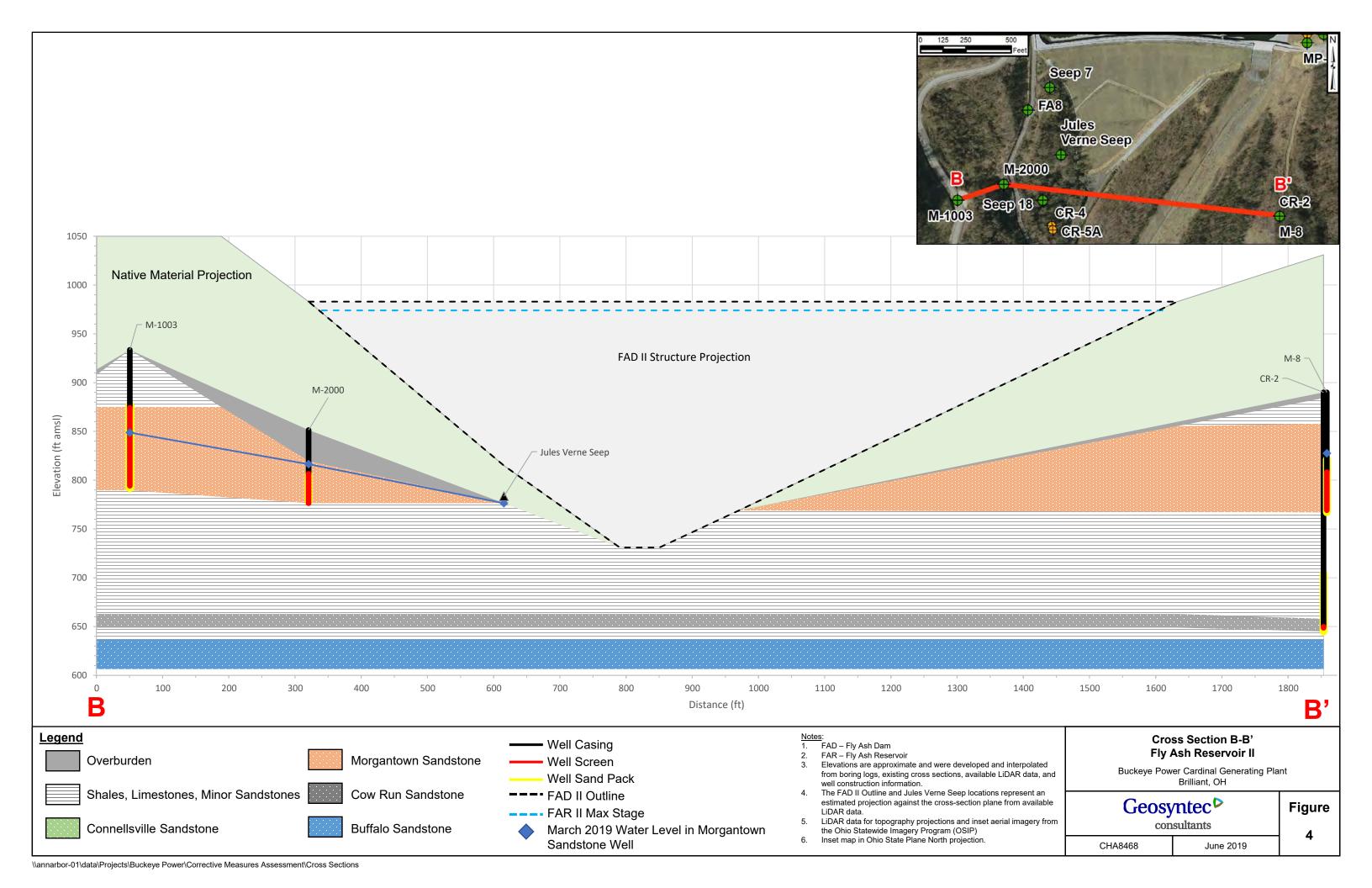
o = technology may be required for success of CMO

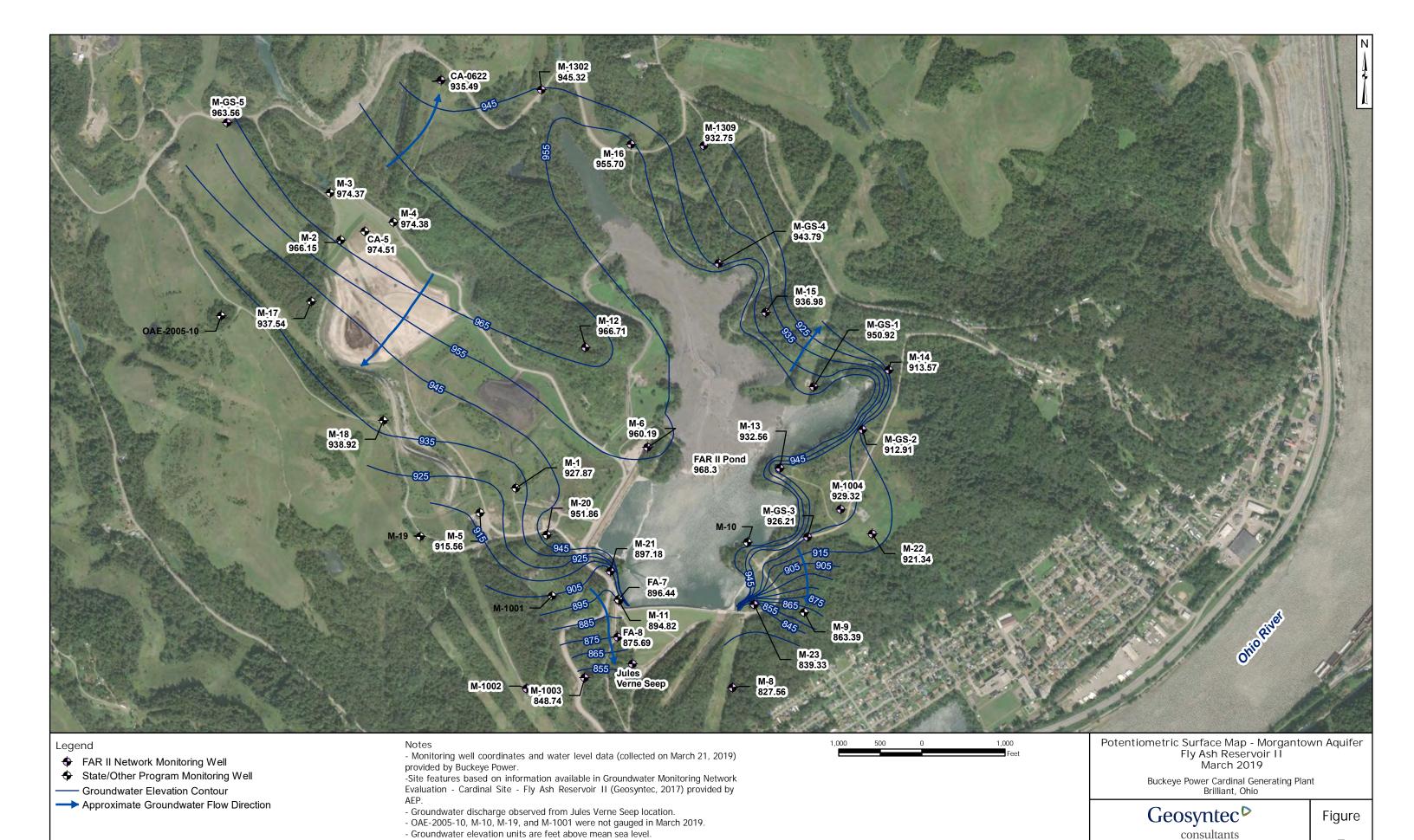












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2019/06/24

Columbus, Ohio

Attachment 1

Supplemental Information to the FAR II Assessment of Corrective Measures for US EPA CCR Extension Request

CARDINAL SITE – FLY ASH RESERVIOR II BRILLIANT, OHIO

Prepared for

Cardinal Operating Company 306 County Road 7E Columbus, Ohio 43213



Prepared by



engineers | scientists | innovators

941 Chatham Lane, Suite 103 Columbus, OH 43221 Project Number CHA8468

November 2020



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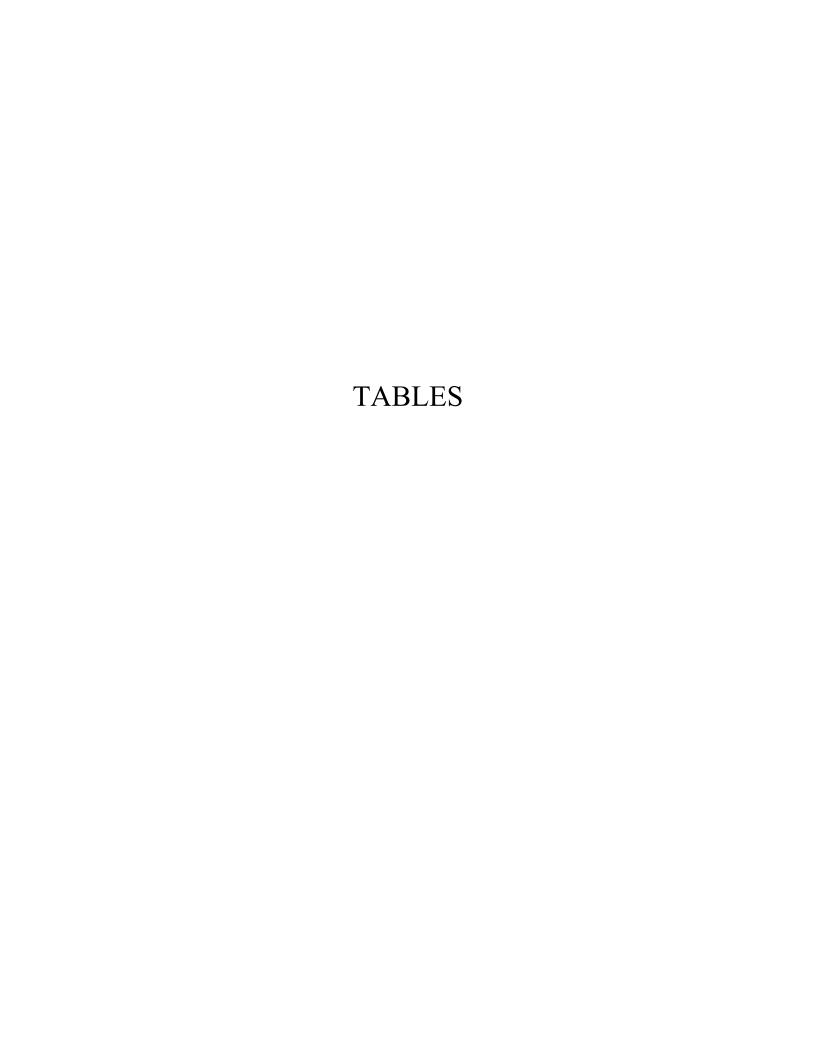


Table 1: Groundwater Elevation Data - March 2019 Cardinal Plant - Fly Ash Reservoir II

Monitoring Well	Top of Casing Elevation	Water Level	Groundwater Elevation	
	(ft AMSL)	(ft AMSL)	(ft AMSL)	
BU-1	736.72	29.77	706.95	
CR-4	724.92	17.92	707.00	
CR-5A	735.83	29.00	706.83	
FA-8	921.03	45.34	875.69	
M-1003	935.88	87.14	848.74	
M-11	980.21	85.39	894.82	
M-2000	854.30	37.90	816.40	
M-21	1,018.61	121.43	897.18	
S-21	1,018.40	49.93	968.47	
S-9	980.56	39.04	941.52	

Notes:

ft AMS: feet above mean sea level.

1. Groundwater elevations were collected on March 21, 2019.

Table 2: Groundwater Analytical Data - Spring 2019 Cardinal Plant - Fly Ash Reservoir II

Parameter		Lithium		Molybdenum		Sodium	
		Dissolved	Total	Dissolved	Total	Dissolved	Total
Site-Specific (GWPS	NA	140	NA	100	NA	NA
CR-4	3/26/2019	10.9	-	<1.0	-	161,000	-
CR-5A	3/26/2019	13.1	-	<1.0	-	113,000	-
FA-8	4/4/2019	-	198	-	321	-	143,000
гА-о	5/22/2019	-	193	-	313	-	-
M-8	4/1/2019	-	<10	-	< 0.5	-	18,300
M 11	3/12/2019	-	196	-	323	-	-
M-11	4/5/2019	-	193	-	316	-	138,000
M-21	4/3/2019	-	80	-	21.3	-	296,000
M-1003	4/8/2019	-	<10	-	< 0.5	-	76,700
M-2000	3/12/2019	-	200	-	184	-	-
	5/22/2019	-	184	-	201	-	-
S-19	3/28/2019	-	-	-		-	124,000
Jules Verne Seep	4/4/2019	-	264	-	413	-	199,000
Seen 18	3/26/2019	-	88.7	-	69	-	-
	5/22/2019	-	104	-	75.5	-	-
Face of Dam	3/11/2019	-	236	-	160	-	-

Notes:

-: Not sampled

μg/L: micrograms per liter

GWPS: Groundwater Protection Standard

NA: GWPSs not applicable for parameter.

- 1. Groundwater concentrations are shown in units of micrograms per liter.
- 2. The Site-specific GWPSs were established in accordance with United Stated Environmental Protection Agency 40 CFR 257.95.
- 3. Bolded values exceed the GWPS.

Table 3: Jules Verne Flow Rates Cardinal Plant - Fly Ash Reservoir II

Date	Flow Rate (GPM)
10/20/2017	75
11/17/2017	75
12/15/2017	75
6/29/2018	75
7/27/2018	75
8/24/2018	75
9/10/2018	75
9/25/2018	75
10/25/2018	60
11/21/2018	75
12/19/2018	75
1/18/2019	75
2/15/2019	75
3/15/2019	75
4/12/2019	30
5/10/2019	75
6/7/2019	75
Average	71

Notes:

GPM: gallons per minute

1. Flow rates are collected as part of the monthly dam inspection.

Table 4: Mass of Lithium and Molybdenum Released Cardinal Plant - Fly Ash Reservoir II

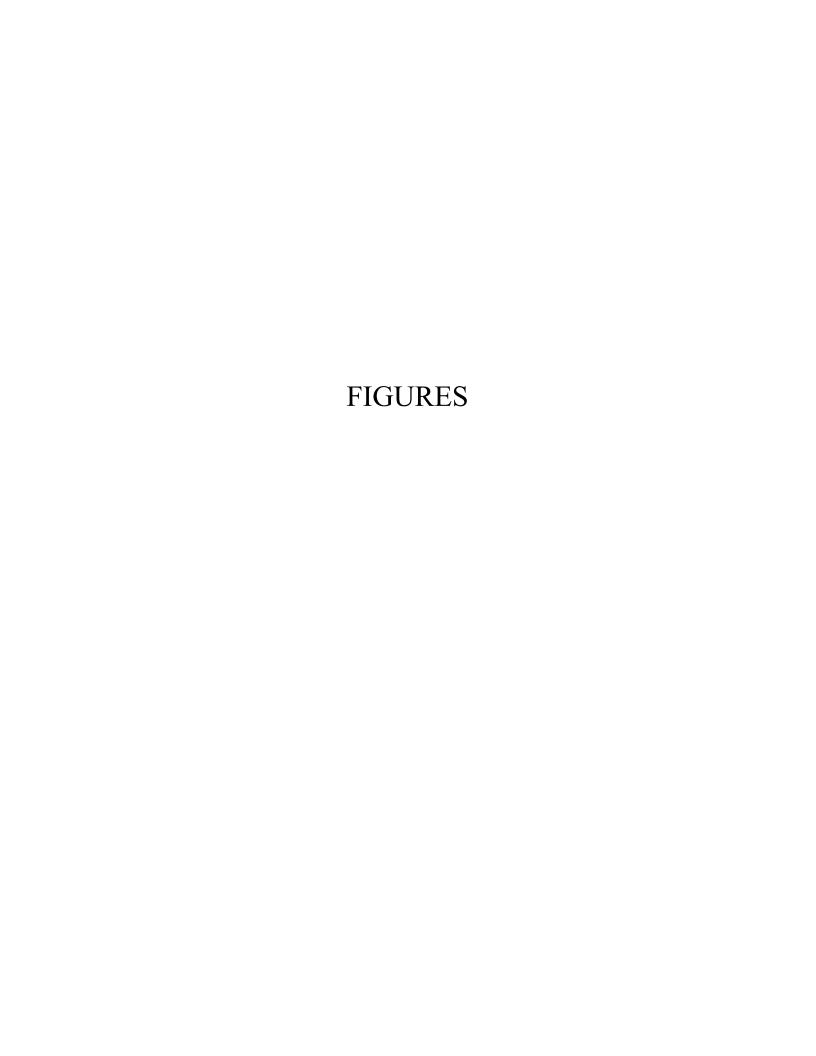
Parameter	Average Concentration	Mass Released
	(µg/L)	(lbs)
Lithium	265	2,124
Molybdenum	463	3,702

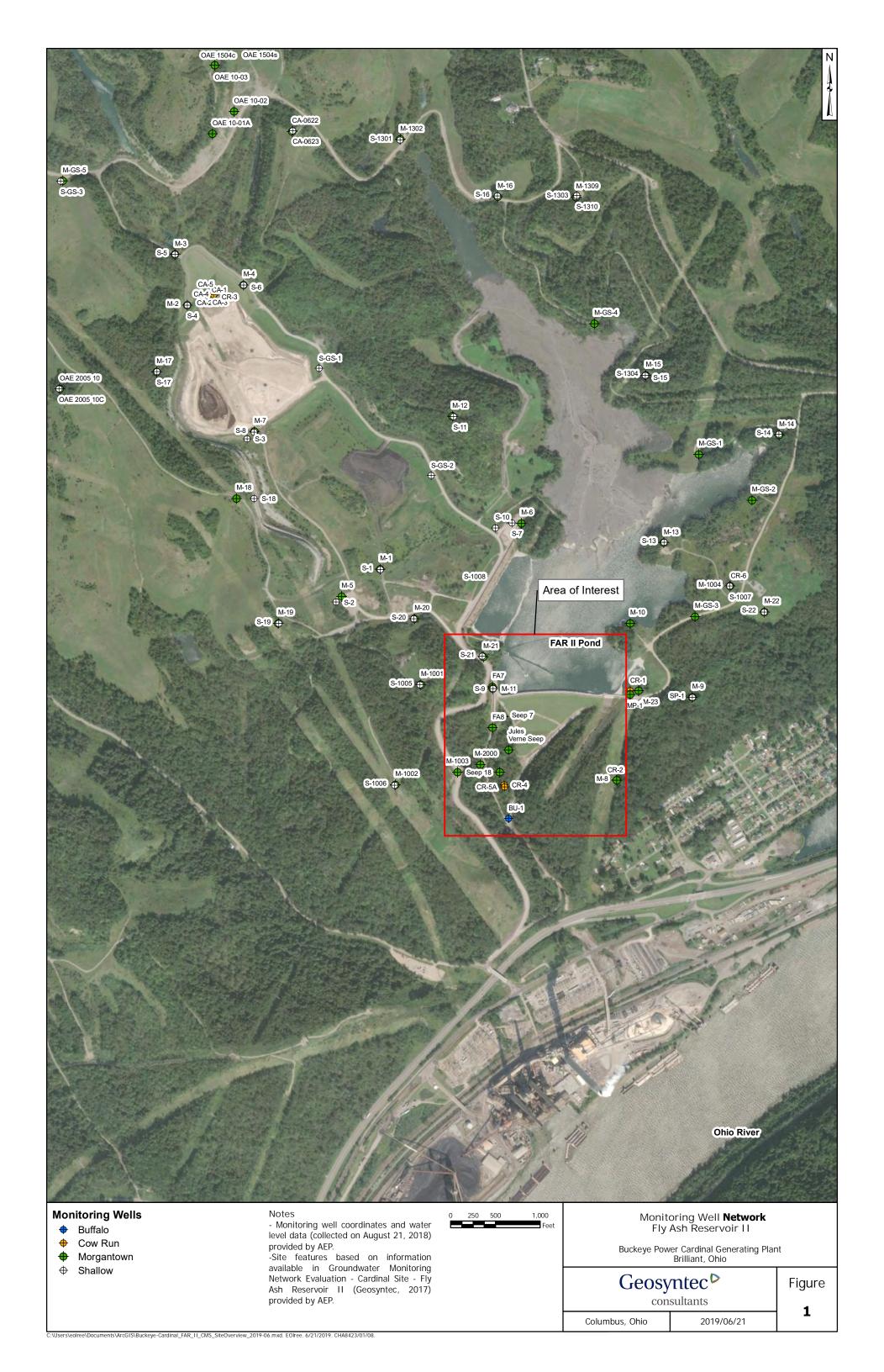
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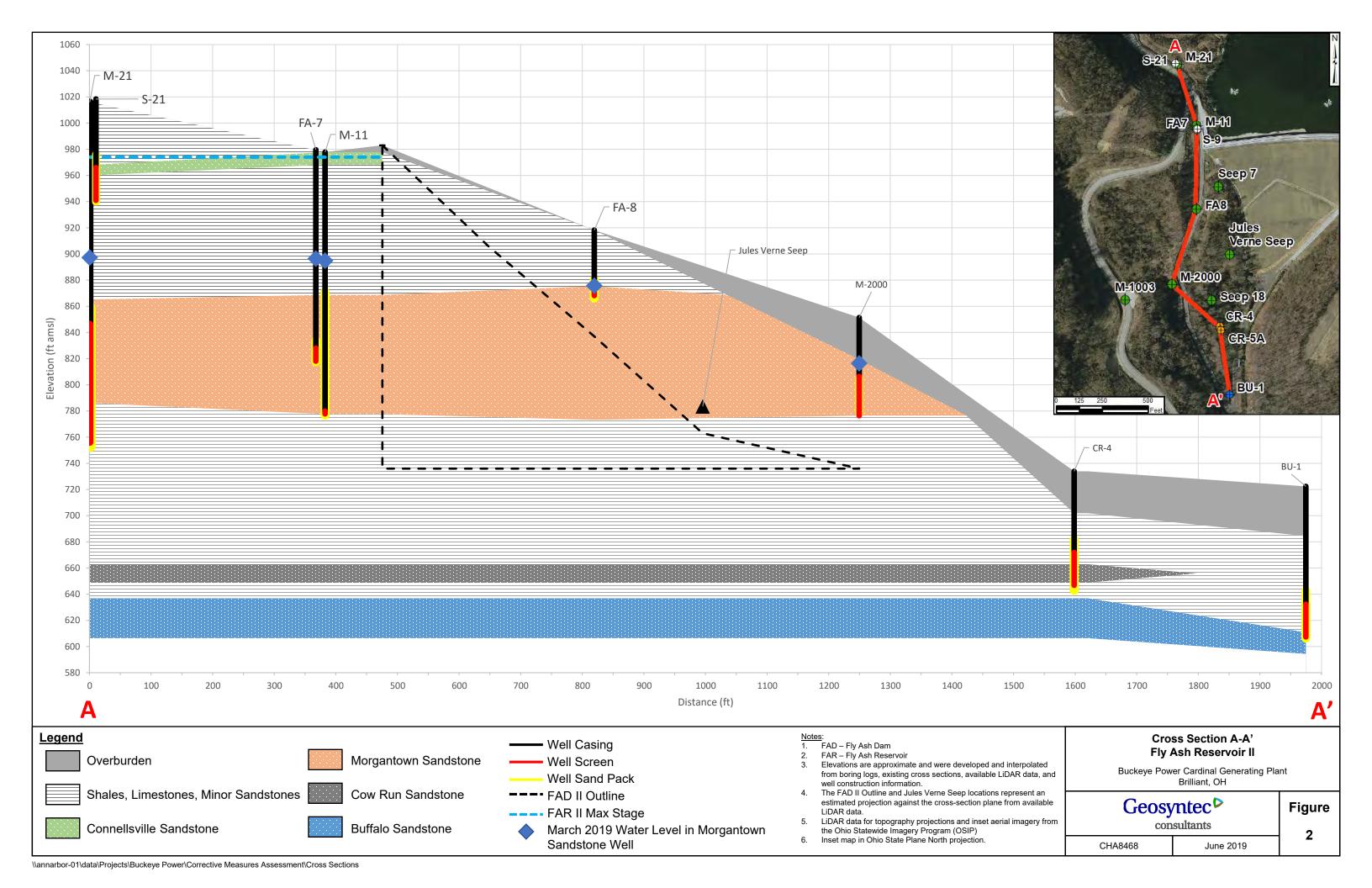
μg/L: micrograms per liter

lbs: pounds

- 1. Mass of parameter released calculated as the historical average concentration multiplied by the historical average seepage rate over a 24.5 year period (January 1995 through July 2019).
- 2. Table 3 contains historical analytical data for Jules Verne Seep. A combination of total and dissolved metals data was used to calculate the average concentration at Jules Verne Seep. For sample dates where total and dissolved metals were analyzed, the maximum value was used to calculate the average.







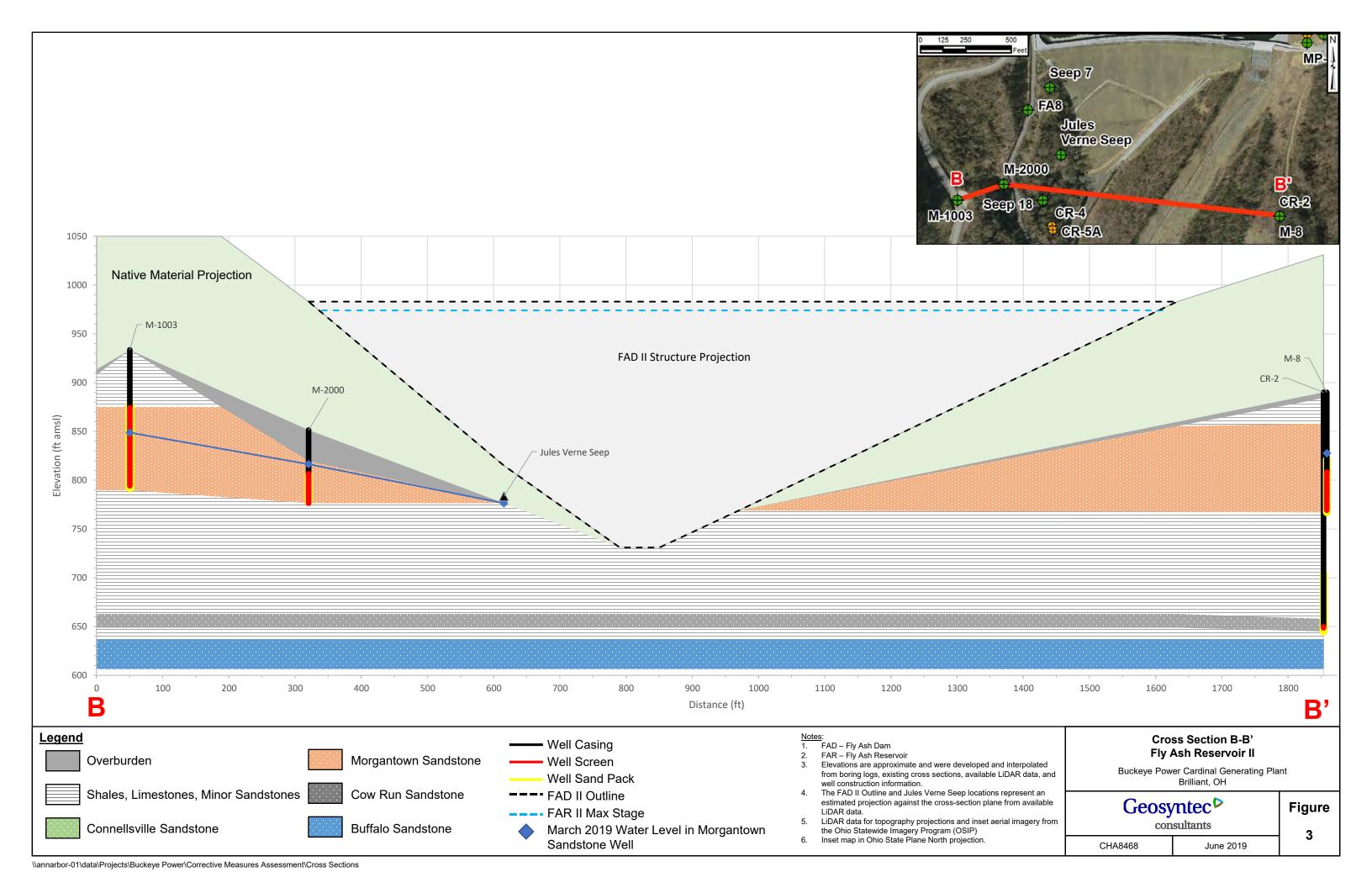
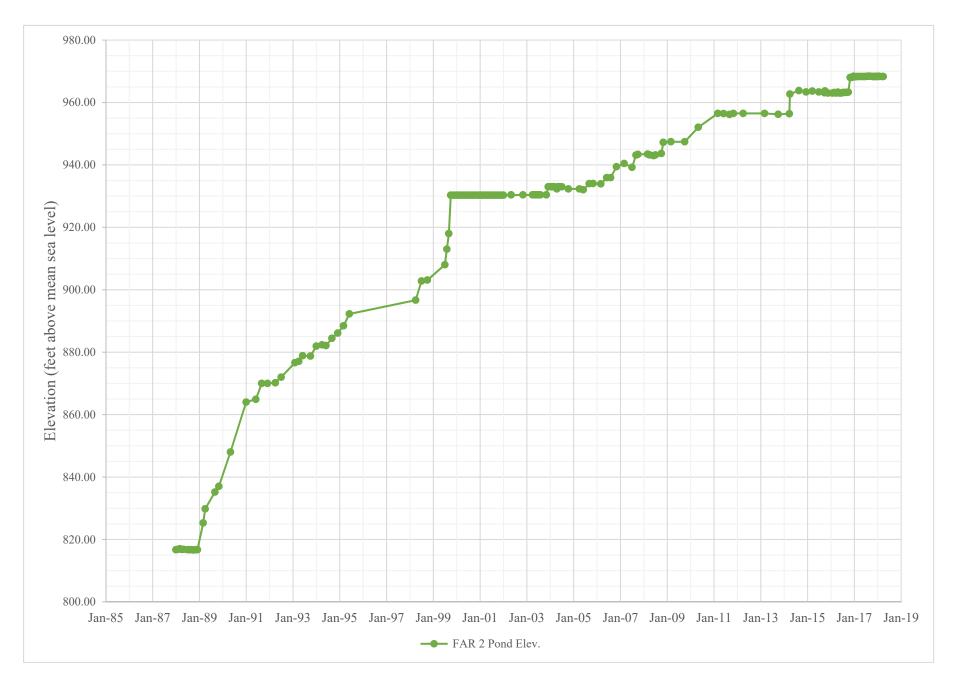
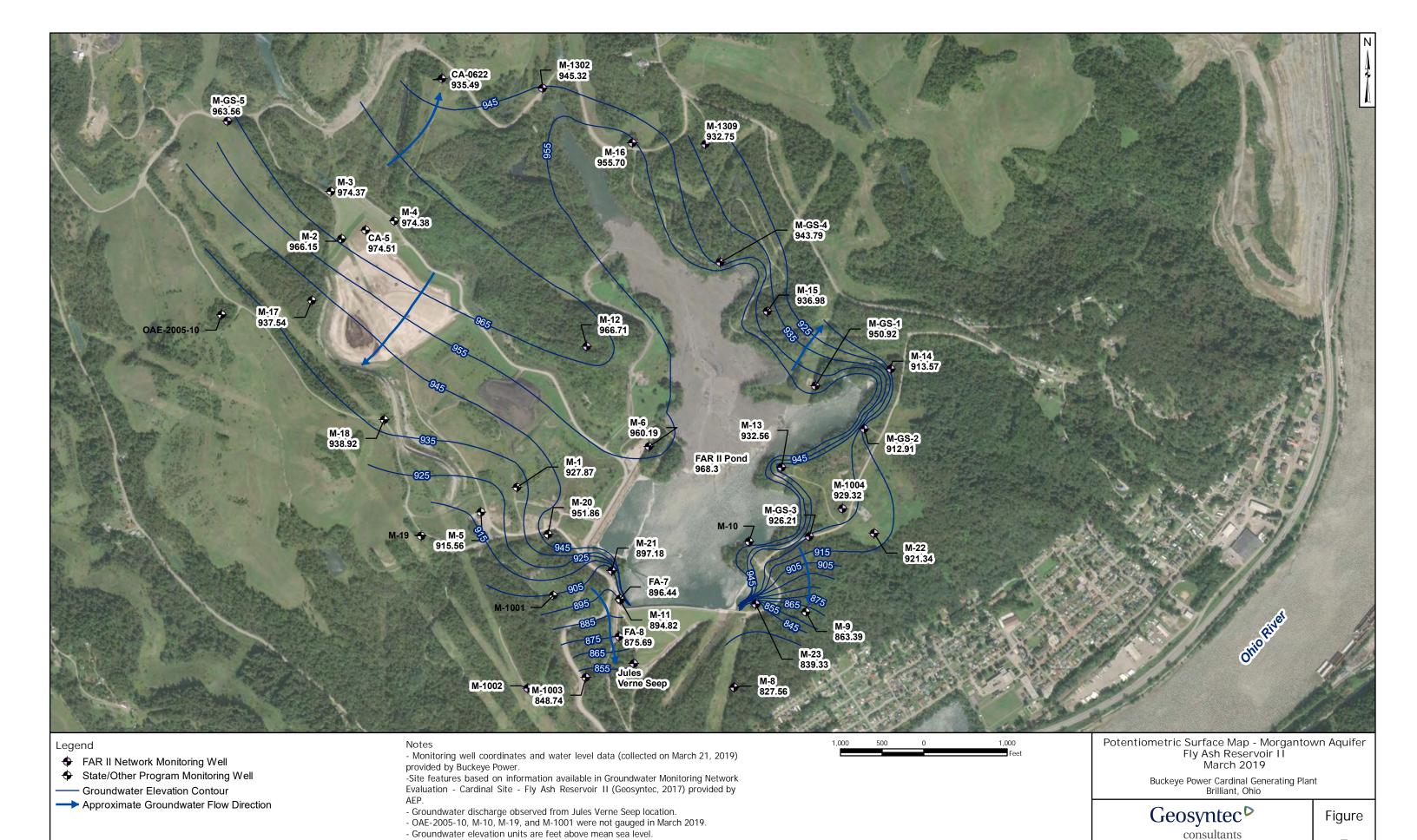


Figure 4
Historical FAR II Groundwater Elevation





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2019/06/24

Columbus, Ohio

Figure 6
Sodium vs Lithium in Morgantown Aquifer and Seeps

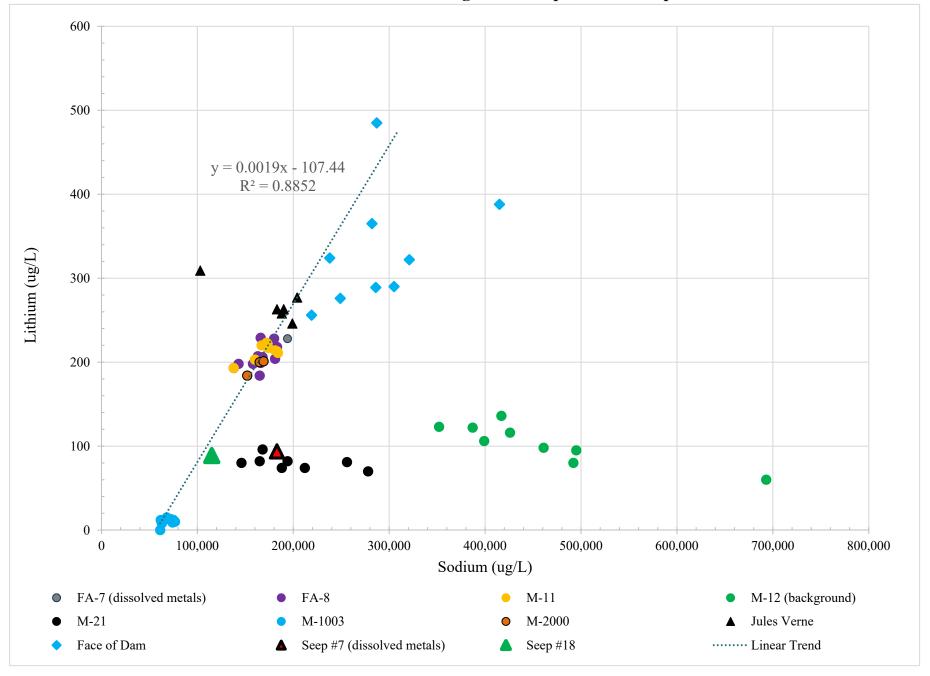
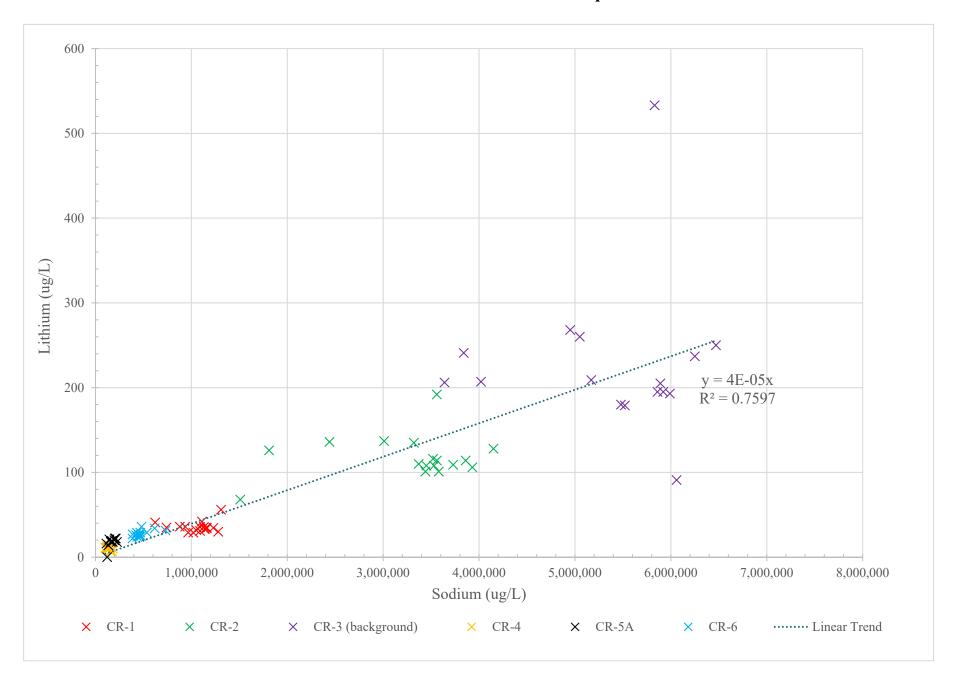


Figure 7
Sodium vs Lithium in Cow Run Aquifer



Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(6)

Any progress reports on corrective action remedy selection and design and the report of final remedy selection required at 40 CFR 257.97(a)

March 6, 2020

Cardinal Operating Company 306 County Road 7 E Brilliant, Ohio 43913

Semi-Annual Progress Report on Groundwater Corrective Measures at FAR II

This Progress Report is prepared to provide an update on the status of selecting and designing the remedy/ corrective measure in accordance with CCR Rule § 257.97(a).

A public meeting was held on September 4th, 2019 to discuss the Assessment of Corrective Measures (ACM) Report for groundwater protection exceedances at Monitoring Wells M-11 and FA-8. The ACM outlined four corrective measure options which Cardinal has been reviewing since the report was prepared.

As described in § 257.97(b), Cardinal must select a remedy that:

- Protects human health and environment;
- Attains the groundwater protection standards;
- Controls the source of the release to reduce or eliminate, to the maximum extent feasible, further releases of constituents in Appendix IV to the environment;
- Removes from the environment as much of the contaminant material that was released from the CCR Unit as is feasible; and
- Comply with standards for waste management.

Additionally, Cardinal shall consider the following evaluation factors when selecting a remedy, as prescribed in § 257.97(c):

- The long term and short-term effectiveness and protectiveness of the remedy, along with the degree of certainty that the remedy will prove successful;
- The effectiveness of the remedy in controlling the source to reduce further releases;
- The ease or difficulty of implementing a potential remedy; and
- The degree to which community concerns are addressed.

At this time Cardinal is still evaluating groundwater flow conditions at the Facility to determine which remedy would be most effective at meeting the requirements stated above. Cardinal believes a decision on the corrective measure should be made prior to the end of the year.

Once the corrective measure is selected, a report detailing the implementation, schedule and effectiveness of meeting § 257.97(b), will be published on Cardinal's CCR Compliance Website.

September 4, 2020

Cardinal Operating Company 306 County Road 7 E Brilliant, Ohio 43913

Semi-Annual Progress Report on Groundwater Corrective Measures at FAR II

This Progress Report is prepared to provide an update on the status of selecting and designing the remedy/ corrective measure in accordance with CCR Rule § 257.97(a).

A public meeting was held on September 4th, 2019 to discuss the Assessment of Corrective Measures (ACM) Report for groundwater protection exceedances at Monitoring Wells M-11 and FA-8. The ACM outlined four corrective measure options which Cardinal has been reviewing since the report was prepared.

As described in § 257.97(b), Cardinal must select a remedy that:

- Protects human health and environment;
- Attains the groundwater protection standards;
- Controls the source of the release to reduce or eliminate, to the maximum extent feasible, further releases of constituents in Appendix IV to the environment;
- Removes from the environment as much of the contaminant material that was released from the CCR Unit as is feasible; and
- Comply with standards for waste management.

At this time, Cardinal is in the final phases of selecting a remedy and is currently preparing the Remedy Selection Report in accordance with § 257.97(a). As stated in the first semi-annual progress report, Cardinal is still on schedule to make a decision on the Remedy and upload the Remedy Selection Report to the Facility's publicly available internet website, prior to the end of the year.

REMEDY SELECTION REPORT CARDINAL SITE – FLY ASH RESERVIOR II BRILLIANT, OHIO

Prepared for

Cardinal Operating Company

306 County Road 7E Brilliant, Ohio 43913



Prepared by



engineers | scientists | innovators

941 Chatham Lane, Suite 103 Columbus, OH 43221 Project Number CHA6468

October 2020



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LIST OF ACRONYMS AND ABBREVIATIONS

Acronym	Definition
μ g/L	Micrograms per Liter
cm/s	Centimeters per Second
ACM	Assessment of Corrective Measures
AEP	American Electric Power
AMSL	Above Mean Sea Level
BAC	Bottom Ash Complex
CCR	Coal Combustion Residual
CFR	Code of Federal Regulations
ESP	Electrostatic Precipitator
FAD 1	Fly Ash Dam 1
FAD 2	Fly Ash Dam 2
FAR I	Fly Ash Reservoir I
FAR II	Fly Ash Reservoir II
FGD	Flue Gas Desulfurization
GWPS	Groundwater Protection Standards
LLDPE	Low-Density Polyethylene
MCL	Maximum Contaminant Level
MNA	Monitored Natural Attenuation
MW	Megawatts
NPDES	National Pollutant Discharge Elimination System
OAC	Ohio Administrative Code
OEPA	Ohio Environmental Protection Agency
PTI	Permit to Install
QA/QC	Quality Assurance and Quality Control
RCRA	Resource Conservation and Recovery Act
RSR	Remedial Selection Report
RSW	Residual Solid Waste Landfill
SAP	Statistical Analysis Plan
SCR	Selective Catalytic Reduction (SCR) System
SSL	Statistically Significant Levels
UCL	Upper Confidence Limit
USEPA	United States Environmental Protection Agency

SECTION 1

INTRODUCTION

On behalf of our client, Cardinal Operating Company (Cardinal), Geosyntec Consultants, Inc. (Geosyntec) has produced this Remedy Selection Report (RSR) for the Fly Ash Reservoir II (FAR II), a regulated impoundment at the Cardinal Generating Plant (the Site or Facility). The Site is located one mile south of Brilliant, Ohio in Jefferson County, along the Ohio River (Figure 1). Under the United States Environmental Protection Agency (USEPA) Coal Combustion Residual (CCR) Rule (40 Code of Federal Regulations (CFR) 257 Subpart D), groundwater monitoring is required to assess impacts of CCR activities to groundwater compared to background conditions.

In 2019, an Assessment of Corrective Measures Report (ACM) and a Closure Plan for FAR II were prepared for the Site to address statistically significant levels (SSLs) of lithium and molybdenum above their respective groundwater protection standards (GWPS) that were observed at the Site in 2018 (Geosyntec, 2019a). This RSR has been prepared as required by and in accordance with 40 CFR 257.97 and was developed to select remedial measures for addressing elevated lithium and molybdenum concentrations in site groundwater.

1.1 **Purpose and Scope**

The purpose of this RSR is to present the selected remedial strategies and technologies for the reduction of lithium and molybdenum present in Site groundwater to acceptable regulatory cleanup levels in accordance with 40 CFR 257.97. The target cleanup levels are the GWPS defined under 40 CFR 257.95(h). The current site-specific GWPS for lithium and molybdenum are 149 micrograms per liter (μ g/L) and 100 μ g/L, respectively. While the ACM identified a GWPS of 140 μ g/L for lithium, this value was updated to 149 μ g/L following completion of the first semiannual assessment monitoring event of 2020 (Geosyntec, 2020a).

This RSR report relies on the 2019 Assessment of Corrective Measures, the 2019 Groundwater Characterization Report prepared by Geosyntec Consultants and the 2019 Final Closure Plan prepared by TRC Engineers, Inc. (TRC) to focus the selection of remedial technologies that will achieve the most efficient and reliable method of reducing concentrations of lithium and molybdenum to below the GWPS.

1.2 Remedial System Requirements

Per 40 CFR 257.97, the selected remedial system is required to, at minimum:

- Be protective of human health and the environment;
- Attain the groundwater protection standards pursuant to 40 CFR 257.95(h);
- Control the source of the releases so as to reduce or eliminate, to the maximum extent feasible, further releases of constituents in Appendix IV to 40 CFR 257;
- Remove from the environment as much of the contaminated material that was
 released from the CCR unit as is feasible, taking into account factors such as
 avoiding inappropriate disturbance of sensitive ecosystems; and
- Comply with standards for management of wastes as specified in 40 CFR 257.98(d).

The effectiveness of the selected remedy in meeting these requirements is discussed in **Section 4**.

SECTION 2

SITE BACKGROUND

2.1 Site Operational History

The Site is located approximately one mile south of Brilliant, Ohio in Jefferson County along the Ohio River (**Figure 1**). The generating station consists of three units with a nominal capacity of 1,830 megawatts (MW). Units 1 and 2 began operation in 1967 and Unit 3 began operation in 1977. All three units are coal powered, with an average annual coal use of 5.2 million tons for the entire plant. As of 2012, all three units were equipped with an electrostatic precipitator (ESP), a selective catalytic reduction (SCR) system, and a flue gas desulfurization (FGD) system.

The regulated CCR storage unit addressed in the RSR and currently used by the Facility is the FAR II reservoir. The locations of the FAR II unit is shown in **Figure 1**. Fly ash is currently sluiced to FAR II, which is impounded by Fly Ash Dam 2 (FAD 2). FAR II/FAD 2 has a permitted discharge (Outfall 019) through the national pollutant discharge elimination system (NPDES).

Construction of FAR II began in 1985 under PTI 06-1250 (Cardinal, 2019b). The FAR II foundation consists of a bedrock base (claystone and shale), and geology adjacent to the eastern and western abutments consists of bedrock units, the Monongahela Group and a portion of the Conemaugh Group including the Morgantown Sandstone.

Prior to the construction of FAD 2, a colluvium landslide upstream of the western abutment of FAR II occurred, exposing the face of the Morgantown Sandstone. The abutment was installed such that the clay core contacted the competent bedrock at 90-degree angles on the upstream side of the abutment to prevent seepage beneath the dam and reduce cracking of the core (American Electric Power [AEP], 2016). The dam was constructed with an open cut to rock and a grout curtain was installed (AEP, 2016). The dam had a final crest height of 925 feet above mean sea level (AMSL; AEP, 1997).

The FAD 2 structure has been raised twice since the initial construction. The dam was raised to an elevation of 970 AMSL in 1997 and the final crest height of 983 ft AMSL in 2013 (AEP, 1997; AEP, 2016).

Groundwater monitoring for FAR II is conducted in accordance with 40 CFR 257. Monitoring wells within the CCR rule monitoring network and select other locations of interest are shown in **Figure 2**.

2.2 **Geologic Site Conditions**

The Site is underlain by horizontal sequences of lower Permian and upper Pennsylvanian age sedimentary bedrock. The geologic units of interest in the vicinity of FAR II/FAD 2 are the Pennsylvanian aged Monongahela Group and the Conemaugh Group. The Monongahela group is approximately 203 ft thick in the vicinity of the Site and consists of sandstone and shale, siltstone, limestone, sandstone, and coal (AEP, 2006).

The Conemaugh group is approximately 500 feet thick in the vicinity of the Site and consists of shale, sandstone, limestone, claystone, and coal. This group includes the Morgantown Sandstone underlain by the Elk Lick Limestone, the Skelly Limestone and Shale, the Ames Limestone, the Cow Run Sandstone, and the Buffalo Sandstone. The Morgantown Sandstone is a fractured and jointed conglomeratic sandstone that is approximately 75 to 100 feet thick in the vicinity of the western abutment of FAD 2 (Sanborn Head & Associates, Inc. [Sanborn Head], 2018). In the vicinity of FAD 2, the base of the Morgantown Sandstone slopes south from M-21 to the Jules Verne Seep, and east from M-1003 to the Jules Verne Seep (Sanborn Head, 2018). The Elk Lick Limestone, the Skelly Limestone and Shale and the Ames Limestone vary in a combined thickness of approximately 80 feet. At the bottom of the Conemaugh Group, the Cow Run Sandstone is approximately 20 to 30 feet thick (AEP, 2006).

Prior to the development of the FAR II, overburden in the FAR II valley consisted of 10 to 30 feet of residual soils, mine spoil, landside debris and alluvial deposits (AEP, 1984; AEP, 2006). Along the valley walls, the overburden consisted of clayey colluvium (Amaya et al., 2009). Prior to the construction of FAD 2, a landslide upstream of the western abutment of FAD 2 occurred, exposing the face of the Morgantown Sandstone at approximately 880 feet AMSL. FAR II incises the Monongahela Group and partially incises the Conemaugh Group, including the Morgantown Sandstone. Cross sections for the geology at FAD 2 are shown in **Figure 3** and **Figure 4**.

2.3 Hydrogeologic Site Conditions

Groundwater in the vicinity of FAR II is present in three aquifers: the surficial aquifer, the Morgantown Sandstone, and the Cow Run Sandstone. The surficial aquifer is comprised of the Conemaugh group, primarily the Connellsville Sandstone, the Summerfield Limestone, the Bellaire Sandstone, former room and pillar mines, and mine spoils. The groundwater flow in the surficial aquifer tends to follow local topography. Underlying the surficial aquifer is a shale aquitard.

The Morgantown Sandstone aquifer is found below the shale aquitard and consists of a fractured and jointed conglomeratic sandstone with fractures. Regionally, groundwater in the Morgantown Aquifer flows south-southeast towards the Ohio River southeast of the Site. In the vicinity of FAD 2, groundwater in the Morgantown Aquifer travels through FAR II and around FAD 2 with discharges on the eastern and western abutments. Along the western abutment, the Morgantown Sandstone outcrops, and groundwater is discharged through the Jules Verne Seep (**Figure 4**).

Underlying the Morgantown Sandstone is approximately 50 to 100 feet of low permeability shale and limestone beds followed by the Cow Run Sandstone Aquifer. The Cow Run Sandstone Aquifer generally flows south-southeast towards the Ohio River in the vicinity of the Site. Additional details of the hydrogeologic conditions at the Site are discussed in the 2019 ACM report.

2.4 **Groundwater Quality**

A groundwater sampling program is in place at the Site to monitor background groundwater conditions and groundwater conditions downgradient of the FAR II unit in accordance with 40 CFR 257. In 2018, SSLs of lithium and molybdenum above their respective GWPS were observed at the Site (Geosyntec, 2019a).

Efforts completed in 2019 to delineate groundwater impacts found that although the FAR II unit discharges into the Morgantown Aquifer, the impacts from the FAR II are limited to monitoring wells FA-8, M-11, M-2000, and the Jules Verne Seep (Geosyntec, 2019c). Additionally, concentrations of lithium and molybdenum in the Cow Run Aquifer were generally much lower than concentrations in the impacted Morgantown Aquifer monitoring wells indicating that there is little to no vertical migration from the Morgantown Aquifer to the Cow Run Aquifer.

Groundwater flow and geochemical analysis of water from the Jule Verne Seep indicates that the seep water originates from the FAR II unit (Sanborn Head, 2018; Geosyntec, 2019c). The entry point for the water from FAR II is likely the location of the colluvium landside that occurred in the native overburden at 880 ft AMSL feet during the installation of FAD 2.

The hydraulic gradient in the Morgantown Aquifer along the north-south transect of the dam is from north to south (M-11 to M-2000) as shown in **Figure 3.** Along the east-west transect, the hydraulic gradient is from west to east and ultimately discharges through the Jules Verne Seep (M-1003 to Jules Verne Seep; **Figure 4**). Therefore, impacts from FAR II likely enter the Morgantown Aquifer in the vicinity of M-11 and discharge through the

outcrop of the Morgantown Sandstone at the Jules Verne Seep. Groundwater discharging from the Jules Verne Seep is collected at the base of FAD 2 and discharged to the Ohio River through NPDES Permitted Outfall No. 19.

SECTION 3

SELECTED REMEDY

3.1 Overview

The selected remedy for the Site to mitigate and remediate SSLs of lithium and molybdenum in the affected portion of the Morgantown Aquifer includes the closure of the FAR II unit via dewatering and capping and long-term monitoring in accordance with the closure plan.

The FAR II unit will be closed by closure in place in accordance with 40 CFR 257.102(d) commencing in 2021. Closure in place will be achieved by:

- Removal of free water from the CCR material (unwatering),
- dewatering the CCR material,
- regrading the CCR material, leaving the existing CCR material within the unit in place, and
- installing a geomembrane cover system in accordance with 40 CFR 257.102(d) with drainage channels to divert water away from the capped CCR unit.

The existing dam and spillway are proposed to remain.

A written final closure plan was developed by TRC (TRC, 2019) in accordance with 40 CFR 257.102(b) and approved by Ohio Environmental Protection Agency (OEPA) on February 2, 2020. The closure process is expected to take approximately five years, after which groundwater impacts will be addressed through long-term groundwater monitoring. The individual steps that will be taken to achieve the remedial system requirements presented in **Section 1.2** are discussed in detail in the following subsections.

3.2 Remedy Selection Process

Four remedial alternatives were assessed in the 2019 ACM report, including monitored natural attenuation (MNA); closure of the FAR II unit with long-term monitoring; installation of bedrock grouting or a cutoff wall; and, hydraulic gradient control

(Geosyntec, 2019c). The corrective measure alternatives were evaluated based on the criteria provided in 40 CFR 257.96(c).

Prior to the selection of the remedy, the results of the ACM were presented at a public meeting with interested and affected parties on September 4, 2019, which was at least 30 days prior to the selection of the remedy as required by 40 CFR 257.96.

The conclusions of the ACM and public comments resulted in the selection of closure of the FAR II unit with long-term monitoring as the selected remedial approach as detailed in **Section 3.3**.

3.3 <u>Selected Remedial System</u>

3.3.1 Removal of Free Water

The FAR II unit currently receives sluiced fly ash waste from the generating unit's ESP and stormwater runoff from the FAR I RSW Landfill. Operational changes from wet to dry ash handling will result in the termination of disposal of sluiced fly ash in the FAR II. Additionally, as part of the FAR II unit closure plan, stormwater will be diverted from FAR I and FAR II to sedimentation ponds via earthen berms and ultimately discharged through NPDES Permitted Outfall No. 19.

The changes in operation of the FAR II unit will allow the start of the free water removal process from the FAR II unit (unwatering). Free water will be removed by lowering the stop logs of the existing service spillway and with pumps when needed.

3.3.2 CCR Dewatering

The CCR material in the FAR II unit will be dewatered to provide a stable surface for the final cap. Dewatering is anticipated to reduce pore water elevations within FAR II to below the elevation of the colluvium landslide (880 ft AMSL) which is the main entry point for water to enter the Morgantown from FAR II and discharge at Jules Verne seep. The final dewatering process will be followed as described in the Closure Plan (TRC, 2019).

3.3.3 CCR Stabilization

Once the FAR II unit has been dewatered, the CCR material will be stabilized to prevent sloughing or movement of the final cover system. CCR stabilization will be completed as described in the Closure Plan (TRC, 2019).

3.3.4 CCR Regrading

The CCR in the FAR II unit will be regraded to achieve the planned final grade of the cover system. As presented in the 2019 Permit-to-Install Modification Application, the site will be regraded to provide a final slope for the cover system of 1% to 2% from east to west in the main length of FAR II with general side grading of 3% to 5% with a maximum slope of 3:1 (TRC, 2019). The surface of FAR II will also include grading for stormwater collection and redirection of runoff towards the NPDES Permitted Outfall No. 19.

3.3.5 Cover Installation

The cover system will be constructed to control, minimize, or eliminate, to the maximum extent feasible, infiltration of precipitation into the FAR II unit as prescribed by 40 CFR 257.102(d)(i). The system will cover approximately 160 acres of CCR. The system will be installed directly over the dewatered and regraded CCR material and will consist of:

- a 40-mil linear low-density polyethylene (LLDPE) geomembrane placed directly on the CCR material;
- a geocomposite drainage layer within the swale or a cushion geotextile;
- an infiltration layer that contains 18 inches of earthen material, and
- six inches of earthen material capable of supporting native vegetation (TRC, 2019).

The Closure Plan states: "The geomembrane or general fill material will be selected such that the permeability of the cover system is less than or equal to the permeability of the natural subsoils and is not greater than $1x10^{-5}$ centimeters per second (cm/s)" (TRC, 2019).

3.3.6 Final Site Restoration

The final cover system will be vegetated to prevent erosion. Maintenance of the cover system will include mowing. The final cover will be inspected and maintained, including the drainage channels, the cover, the final cover surface, and the surface drainage system.

3.3.7 Long Term Monitoring

The Facility will comply with the post-closure care and maintenance requirements for a period of 30 years, as required by 40 CFR 257.104. These post-closure requirements include maintaining the final cover system, maintaining the leachate collection system, maintaining the groundwater monitoring system, and monitoring groundwater in

accordance with 40 CFR 257.90 through 257.98. A post-closure plan has been developed in accordance with 40 CFR 257.104(d) (TRC, 2019).

Groundwater will continue to be monitored at the site after closure. Groundwater upgradient, down gradient and cross gradient to FAR II will continue to be monitored during closure and post-closure in accordance with 40 CFR 257.90 through 257.98 and with the site-specific CCR Groundwater Monitoring Design Network and Statistical Analysis Plan (TRC, 2019; Geosyntec, 2020b).

SECTION 4

EFFECTIVENESS OF SELECTED REMEDY

In accordance with 40 CFR 257.97(b), this section provides an evaluation of the effectiveness of the selected remedy at protecting human health and the environment, the attaining groundwater protection standards, controlling the source, removing released material, and managing wastes during the implementation of the remedy. Additionally, this section addresses the consideration of the evaluation factors listed in 40 CFR 257.97(c).

4.1 Protection of Human Health and the Environment

Under 40 CFR 257.97(b)(1), the selected remedy must be protective of human health and the environment. The risk to human health and the environment from exposure to CCR-related constituents in groundwater at the Site was assessed (Geosyntec, 2019b). The risk assessment included an exposure assessment and a screening-level risk evaluation. The purpose of the exposure assessment was to identify potentially complete exposure pathways by which human or ecological receptors may contact lithium or molybdenum in groundwater, while the purpose of the screening level risk evaluation was to quantitatively evaluate receptor-exposure scenarios for pathways identified as complete or assumed-to-be complete.

Based on the results of the exposure assessment and screening-level risk evaluation, lithium and molybdenum in FAR II groundwater are unlikely to pose an unacceptable risk to human or ecological receptors in the vicinity of the site under current or near-term future conditions. Until the remedy can be implemented, additional actions are not necessary to protect human health and the environment. Anticipated future remedy implementation and resulting site conditions are expected to further reduce these risks.

4.2 Ability to Attain the Groundwater Protection Standards

Under 40 CFR 257.97(b)(2), the selected remedy must be able to attain the GWPSs developed for the Site pursuant to 40 CFR 257.95(h). GWPSs must be established for each detected Appendix IV constituent. The GWPS shall be the greater of the background concentration and the maximum contaminant level (MCL) established by the USEPA for that constituent. The selected remedy will achieve GWPS by reducing impacts from FAR II to groundwater in the vicinity of the unit. Evaluation of whether the remedy has achieved the GWPSs will follow the statistical approach outlined in Section 4.2.1.

4.2.1 Corrective Action Effectiveness Evaluation

Following implementation of remedial activities, a corrective action groundwater monitoring program will be established in accordance with 40 CFR 257.98(a)(1). The effectiveness of the corrective action will be evaluated by comparing groundwater monitoring results to the site GWPSs developed in 2020. A Statistical Analysis Plan (SAP) has been prepared for the Site in accordance with the CCR Rule (Geosyntec, 2020b) and USEPA's Statistical Analysis of Groundwater monitoring Data at Resource Conservation and Recovery Act (RCRA) Facilities, Unified Guidance (USEPA, 2009). The SAP incorporates a logic process regarding the appropriate statistical analysis of groundwater data collected in compliance with the CCR Rules. Additionally, the SAP describes the statistical procedures to be used to establish background conditions and implement corrective action monitoring.

The conclusion that the remedy has successfully decreased concentrations below the GWPS is made when average concentrations of monitoring well-constituent pairs where an SSL has previously been identified are less than the GWPS (i.e., when the *upper* confidence limit [UCL] is *less* than the GWPS). Further, a remedy is considered complete when, among other things, confidence intervals constructed for Appendix IV constituents for monitoring wells identified with SSLs have not exceeded the GWPS for three consecutive years [40 CFR 257.98(c)(2)]. The statistical analysis plan includes a detailed path for calculating the UCL for the monitoring well-constituent pairs based on the nature of the data (i.e. seasonality, distribution of data, significant non-detects, etc.).

If a corrective action monitoring program is in place, it must meet the requirements of an assessment monitoring program [40 CFR 257.98(a)(1)(i)].

4.3 Source Control

In accordance with 40 CFR 257.97(b)(3), the remedy must control the source such that further releases are reduced to the "maximum extent feasible". The selected remedy should result in minimal further releases, as capping and dewatering the unit to below the elevation of the colluvium landslide is expected to eliminate the main pathway of water entry from FAR II to the environment.

4.4 Removal of Released Material

Under 40 CFR 257.97(b)(4), the selected remedy must remove from the environment as much of the contaminated material that was released from the CCR unit as is feasible. As

discussed in **Section 2.4**, seep water from the Jules Verne Seep is currently collected and discharged to a NPDES permitted outfall.

Assessment of the hydrogeology along the western abutment of FAD 2 indicates that collection of groundwater at the Jules Verne Seep is an effective way of capturing lithium and molybdenum impacted water released from FAR II. This practice will continue until the flow of seep water ceases after installation of the cap and dewatering of the CCR material or concentrations of lithium and molybdenum in seep water decrease below GWPS. Groundwater upgradient, within, and downgradient of the impacted portion of the aquifer will continue to be monitored to assess the post-closure groundwater concentrations as discussed in **Section 3.3.7**.

4.5 Compliance with Standards for Management of Wastes

The CCR material will be managed in compliance with applicable RCRA requirements as required under 40 CFR 257.98(d).

4.6 Evaluation Factors

In selecting the remedy, the evaluation factors listed in 40 CFR 257.97(c) were considered. A brief summary of each evaluation is provided below.

4.6.1 Long-Term and Short-Term Effectiveness and Protectiveness

In accordance with 40 CFR 257.97(c)(1), the long-term and short-term effectiveness and protectiveness of the potential remedy was evaluated, along with the degree of certainty that the remedy will prove successful based on consideration of multiple factors.

4.6.1.1 Short-Term Effectiveness and Protectiveness

As discussed in **Section 2.4**, water impacted with SSLs of lithium and molybdenum are released from the FAR II unit into the Morgantown Aquifer and ultimately discharge to the Jules Verne Seep. Impacts from the FAR II are limited to monitoring wells FA-8, M-11, M-2000, and the Jules Verne Seep (Geosyntec, 2019c). Impacted water discharged at the Jules Verne Seep is currently collected at the base of FAD 2 and discharged to the Ohio River through NPDES Permitted Outfall No. 19.

Assessment of the hydrogeology along the western abutment of FAD 2 indicates that collection of groundwater at the Jules Verne Seep is an effective way of capturing lithium and molybdenum impacted water released from FAR II. The risk assessment found that lithium and molybdenum in FAR II groundwater are unlikely to pose an unacceptable

risk to human or ecological receptors in the vicinity of the site under current or near-term future conditions (Geosyntec, 2019b).

4.6.1.2 Long-Term Effectiveness

Dewatering and capping of FAR II will provide long-term source control of lithium and molybdenum at the Site. Ponded water in the FAR II unit will be removed to a sufficient elevation to provide structural stability and capped as part of the closure plan. Dewatering will be sufficient to reduce the hydraulic head in the CCR material in FAR II to below the elevation of the landslide in the native colluvium that is the assumed entry point for the water from FAR II into the Morgantown Aquifer as discussed in **Section 2.4**. Capping will reduce to the maximum extent possible infiltration of precipitation into the groundwater system, which will reduce the future potential for groundwater flow from FAR II to the Morgantown Aquifer.

Once the remedy is in place, a groundwater monitoring program will be implemented similar to the existing and on-going monitoring program under the Federal CCR Rule. As discussed in **Section 4.1.3**, an SAP has been developed for the Site which includes a logic process regarding the appropriate statistical analysis of groundwater for corrective action monitoring. The monitoring program will meet the requirements of 40 CFR 257.98(a)(1)(i) and progress towards remedy completion will be documented in an annual report that will include [40 CFR 257.95(d)(3)]:

- Analytical results for Appendix III and detected Appendix IV constituents,
- Background concentrations for all Appendix III and Appendix IV constituents, and
- GWPSs established for detected Appendix IV constituents.

4.6.2 Effectiveness of the Remedy

In accordance with 40 CFR 257.97(c)(3), the effectiveness of the remedy in reducing further releases should include consideration of the extent to which containment practices will reduce further releases and the extent to which treatment technologies may be used. The selected remedy uses industry-standard containment technologies which are anticipated to reduce the potential for further releases. The use of treatment technologies is not included in the design of the proposed remedy.

4.6.3 Ease of Implementation

While closure of the unit is a significant effort, the remedy can be implemented with respect to infrastructure. A written Closure Plan for FAR II has been developed in accordance with 40 CFR 257.102(b) and was approved by OEPA on February 2, 2020. The Closure Plan includes a plan for Quality Assurance and Quality Control (QA/QC) during construction which will facilitate long-term operational reliability of the implemented remedy. Closure and capping of FAR II is anticipated to take five years.

4.6.4 Community Concerns

Prior to the selection of the remedy, the results of the ACM were presented at a public meeting with interested and affected parties on September 4, 2019. Attendees of the meeting expressed no direct concerns with any of the proposed remedies.

4.7 Remedy Completion

The remedy will be considered complete when compliance with the GWPS have been achieved at all points within the plume of contamination that lie beyond the groundwater monitoring well system and confidence intervals constructed for Appendix IV constituents for wells identified with SSLs have not exceeded the GWPS for three consecutive years.

Upon completion of the remedy, the facility must prepare a notification that the remedy has been completed. The notification must be certified by a qualified professional engineer or approved by the State Director or USEPA and placed in the operating record [40 CFR 257.98(e)].

SECTION 5

REMEDY IMPLEMENTATION

The proposed remedy implementation schedule was developed in accordance with 40 CFR 257.97(d) and the anticipated schedule for the closure for the FAR II unit.

5.1 Schedule of Remedial Activities

The engineering and design for the closure of FAR II was approved by OEPA in 2020 (OEPA, 2020). The plant will stop receiving process water and divert storm water flows in 2021 and begin FAR II closure. CCR closure activities are expected to take five years to complete. Experience has shown that completion of remedial activities in five years at a pond of this size is within a reasonable period of time. Post-closure care, including groundwater monitoring, is expected to continue for 30 years after closure, in accordance with 40 CFR 257.104 (c).

5.2 Schedule Implementation Factors

The proposed remedy implementation schedule considers the factors established in CFR 257.97(d), as discussed in the **Section 5.2.1** through **Section 5.2.5**.

5.2.1 Extent and Nature of Contamination

The extent of lithium and molybdenum groundwater impacts has been defined to the area near the Jules Verne Seep. Impacted water discharged at the Jules Verne Seep is currently collected at the base of FAD 2 and discharged to the Ohio River through NPDES Permitted Outfall No. 19. Collection of seep water will continue until the flow of seep water ceases or concentrations of lithium and molybdenum in seep water decrease below GWPS.

The extent and nature of contamination does not strongly influence the remedy implementation schedule. The extent of contamination in groundwater is limited to onsite impacts and is unlikely to pose an unacceptable risk to human health or ecological receptors under current or near-term future conditions (Geosyntec, 2019b).

5.2.2 Reasonable Probability of Remedial Technologies in Achieving Compliance

The selected remedy is highly likely to achieve compliance with the GWPS established for the site. As the water level within FAR II is reduced below the elevation of the colluvium landslide, the main entry point for impacts to enter the groundwater will be eliminated. Following reduction in inputs of impacts to groundwater, concentrations are

expected to decline below the GWPS and groundwater flow through the Jules Verne Seep is expected to significantly decline or cease over time.

Consequently, the reasonable probability of the selected remedy achieving compliance does not strongly influence the remedy selection implementation schedule.

5.2.3 Availability of Treatment or Disposal

Impacted groundwater is currently collected at the base of FAD 2 and discharged to the Ohio River through NPDES Permitted Outfall No. 19. Collection of seep water will continue until the flow of seep water ceases or concentrations of lithium and molybdenum in seep water decrease below GWPS.

Consequently, the availability of treatment for impacted does not strongly influence the remedy selection implementation schedule.

5.2.4 Potential Risks to Human Health and the Environment

The risk assessment conducted by Geosyntec (Geosyntec, 2019b) concluded that lithium and molybdenum in FAR II groundwater are unlikely to pose an unacceptable risk to human or ecological receptors in the vicinity of the site under current or near-term future conditions. Until the remedy can be implemented, additional actions are not necessary to protect human health and the environment.

Consequently, potential risks to human health and the environment do not strongly influence the remedy implementation schedule.

5.2.5 Resource Value of the Aquifer

Impacts of lithium and molybdenum at the Site have been delineated, with no off-site migration of impacts observed. Because there are no off-site impacts and there are no current or future uses of groundwater from the impacted aquifer on-site, the resource value of the aquifer is not affected in a way that would strongly influence the remedy implementation schedule. Additionally, the risk assessment found that lithium and molybdenum in FAR II groundwater are unlikely to pose an unacceptable risk to human or ecological receptors in the vicinity of the site under current or near-term future conditions (Geosyntec, 2019b). There are abundant alternate water supplies near the Site, with highly productive wells installed in the sand and gravel aquifer adjacent to the Ohio River, which is located less than one mile from the Site, provides abundant alternative water supplies. These resources provide additional support for the conclusion that the schedule for remedy implementation is not affected by the resource value of the aquifer.

SECTION 6

CERTIFICATION BY A PROFESSIONAL ENGINEER

By means of this certification, I certify that I have reviewed the Remedy Selection Report for the Fly Ash Reservoir II unit at the Cardinal Operating Company's Cardinal Plant and it meets the requirements of Section 40 CFR 257.97.

John Seymour, P.E.		11111111111111111111111111111111111111
Printed Name of Registere	ed Professional Engineer	JOHN SEYMOUR E-85326
Signature Serv	Mice	SSIONAL ENGINEER
E-85326	OHO	10/19/2020

Registration State

Date

Registration No.

SECTION 7

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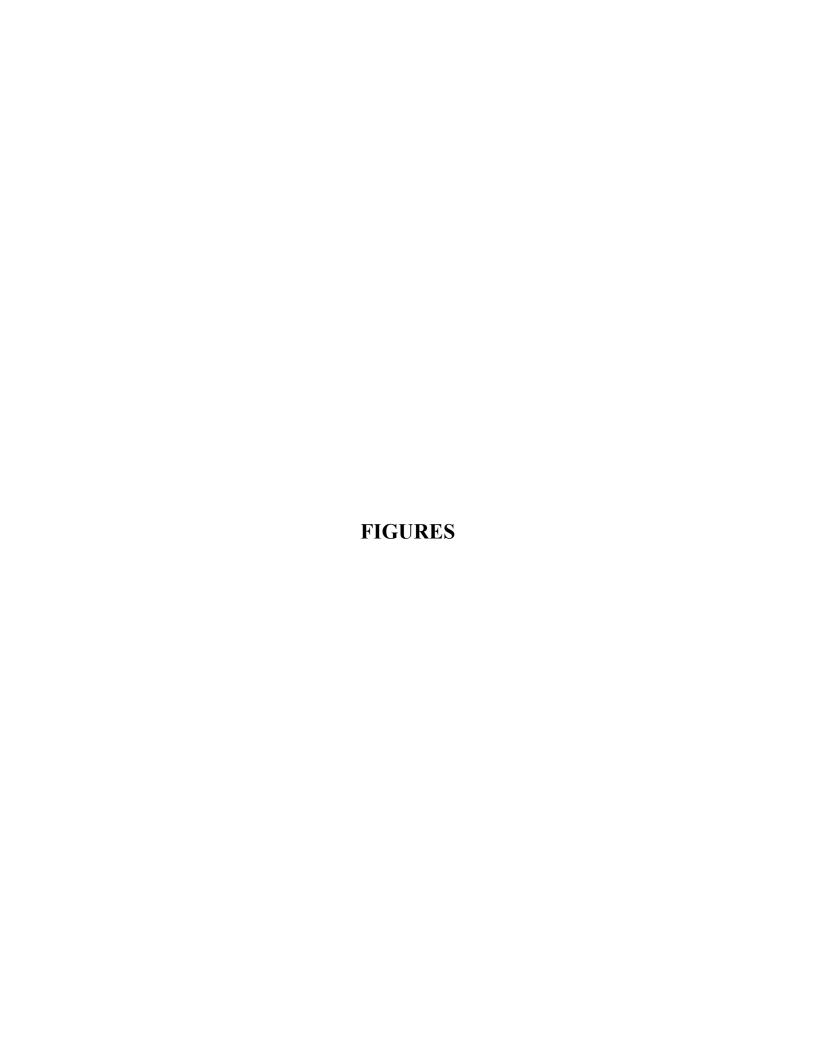
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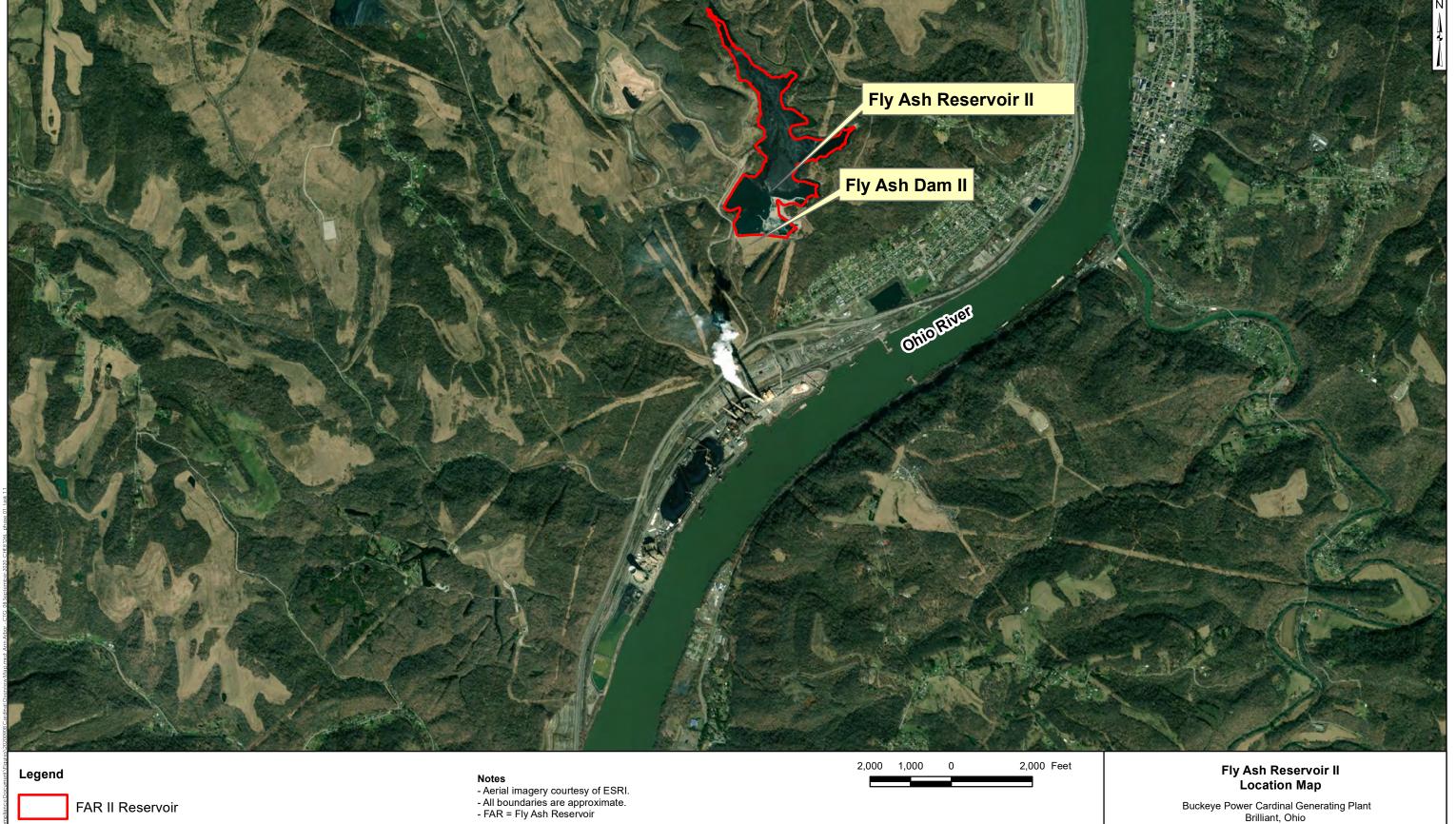
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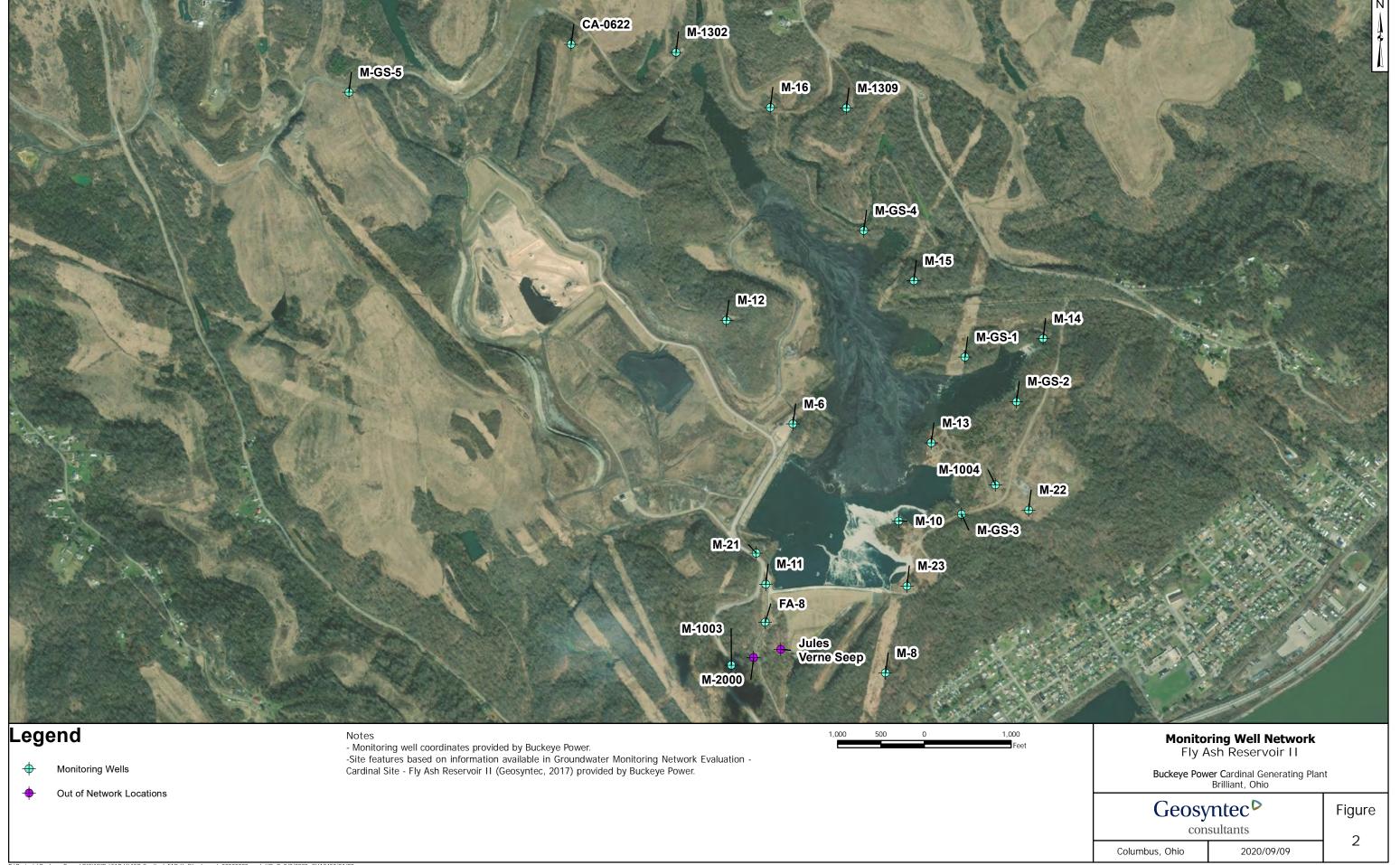
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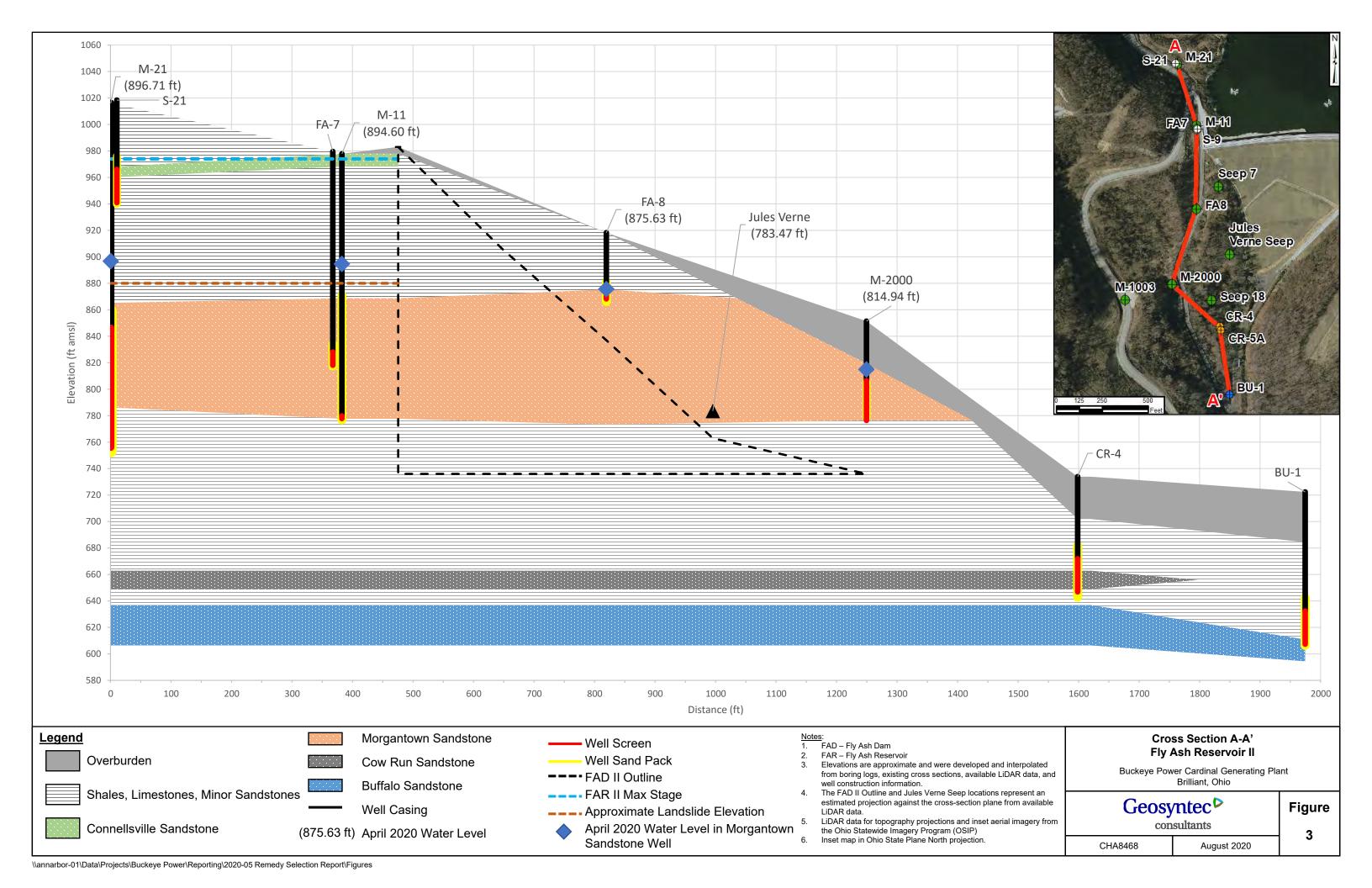


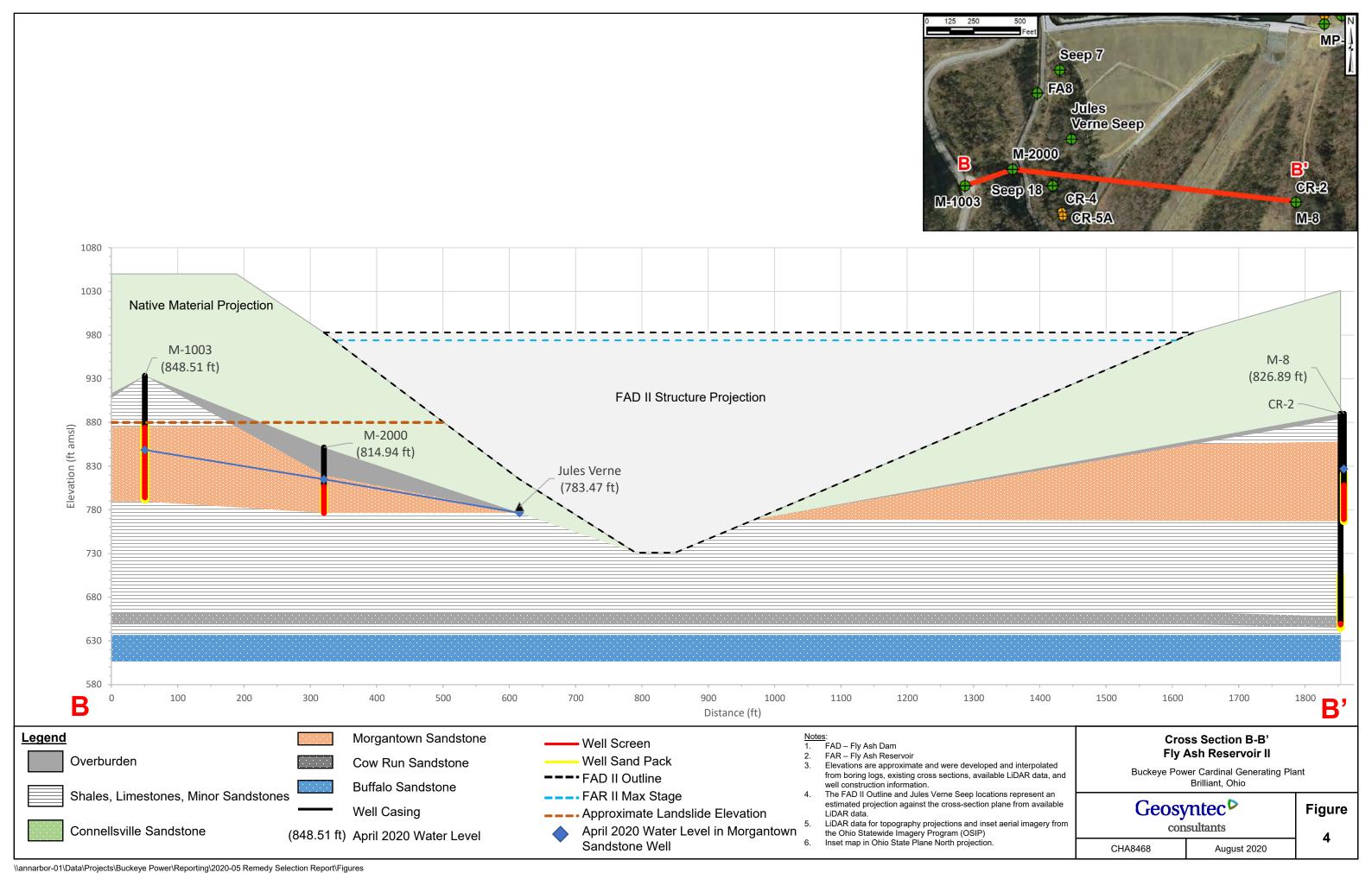
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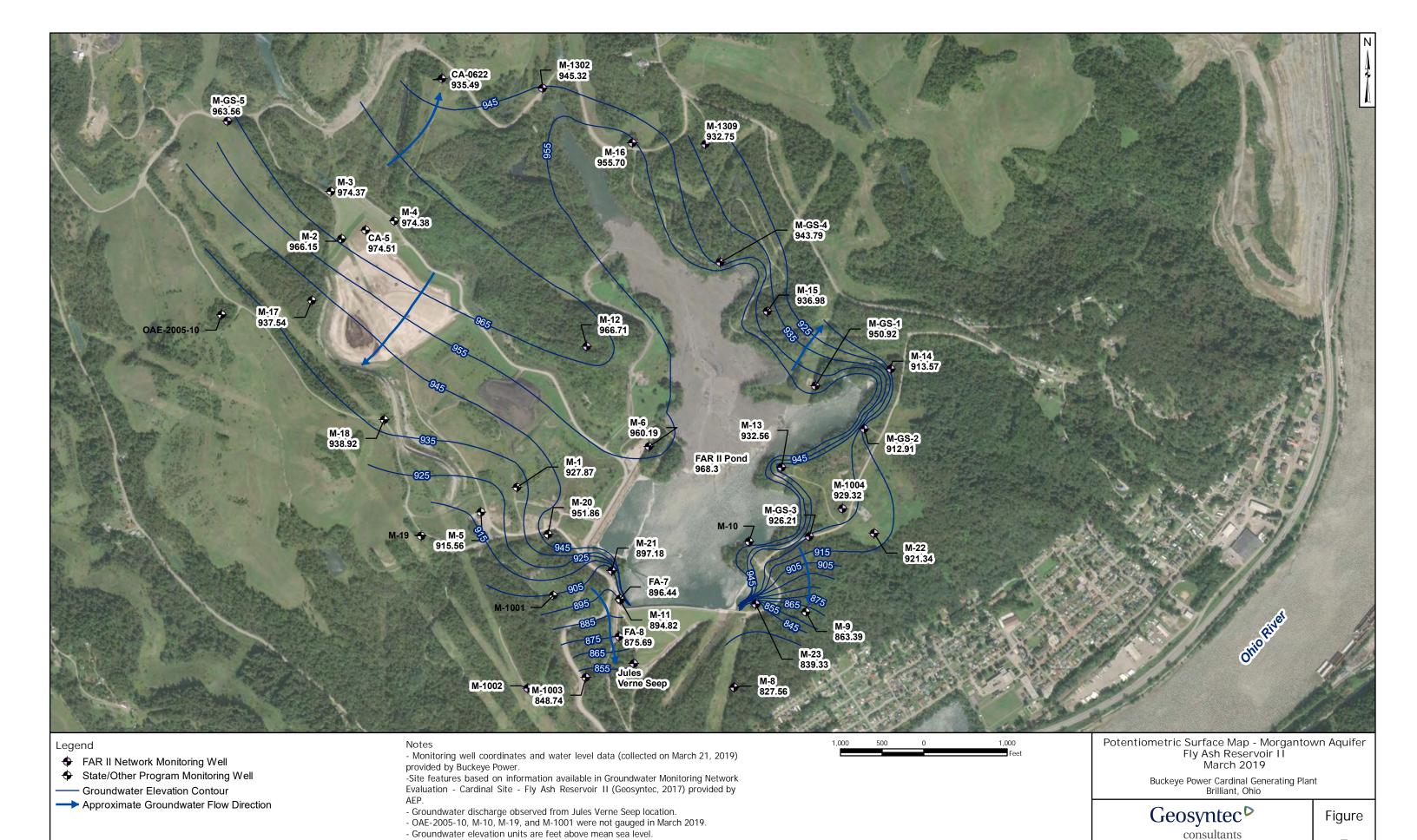
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Figure









5

2019/06/24

Columbus, Ohio

Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(7)

The most recent structural stability assessment required at 40 CFR 257.73(d)

STRUCTURAL STABILITY ASSESSMENT CFR 257.73(d)

Fly Ash Reservoir II

Cardinal Plant Brilliant, Ohio

October, 2016

Prepared for: Cardinal Operating Company - Cardinal Plant

Brilliant, Ohio

Prepared by: Geotechnical Engineering Services

American Electric Power Service Corporation

1 Riverside Plaza

Columbus, OH 43215



STRUCTURAL STABILITY ASSESSMENT CFR 257.73(d) FLY ASH RESERVOIR II CARDINAL PLANT

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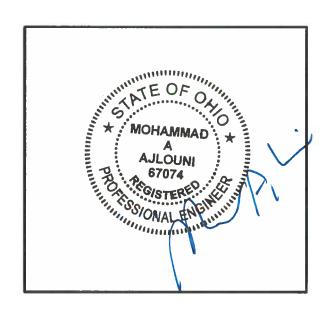
PREPARED BY	DATE 9/2	28/2016
Mohammad A. Afl	ougi, Ph.D.,P.E.	
REVIEWED BY	DATE	, ,

APPROVED BY

DATE

10/5/2016

Manager - AEP Geotechnical Engineering



I certify to the best of my knowledge, information and belief that the information contained in this structural stability assessment meets the requirements of 40 CFR 257.73(d)

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1.0 OBJECTIVE 257.73(d)

This report was prepared by AEP- Geotechnical Engineering Services (GES) section to fulfill requirements of CFR 257.73(d) and document whether the design, construction, operations, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices. This is the initial assessment as per the Rule.

2.0 NAME AND DESCRIPTION OF CCR SURFACE IMPOUNDMENT

The Cardinal Power Plant in Wells Township, Jefferson County, near the town of Brilliant in eastern Ohio. It is owned by Buckeye Power and AEP Generation Resources (GENCO) and is operated by Cardinal Operating Company. The facility operates two surface impoundments for storing CCR; the Bottom Ash Complex and Cardinal Fly Ash Reservoir II (FAR II) Dam. The focus of this report is the FAR II Dam.

The FAR II Dam is a valley filled dam with a unique structure whose current configuration is the result of the original earth fill dam and two separate raisings. The original earth fill dam (Stage 1) consisted of a 180 feet high arched earth embankment incorporating a zoned cross section. At 925 feet NGVD, the dam featured a 70-foot wide by 1,055-feet long crest. The maximum operating pool that could be achieved with the original configuration was El. 913. In 1997, the original dam was raised, referred to as Stage 2. Following this raising, the dam was 237 feet high with a 30-foot wide crest. In 2013, the dam was raised 13 feet using back-to-back MSE walls, bringing the dam into its current, Stage 3 configuration. The principal features of the typical section are the MSE wall themselves and a vinyl sheet pile wall extending from the existing clay core to the top of the PMF flood level for seepage cutoff purposes.

3.0 STABLE FOUNDATION AND ABUTMENTS 257.73(d)(1)(i)

[Was the facility designed for and constructed on stable foundations and abutments? Describe any foundation improvements required as part of construction.]

Since the overburden is saturated and appeared to be heterogeneous, with some material having a softer consistency than that of the sample tested, it was determined to be unsuitable as a foundation material, and was removed in the area beneath the dam and along the valley slopes up to approximately elevation 800 feet NGVD.

Based on the design drawings, a foundation key was constructed along the centerline of the dam. The key was excavated 6-8 feet into the rock beneath the dam and along the valley slopes up to approximately elevation 800 feet NGVD.

At the abutments location, a cut to rock was made at the proposed abutment. The orientation of the trimmed faces has been designed so that the upstream core of the dam intersects the abutments at right angles. This symmetrical configuration resulted in balanced seating of the clay core against the rock which reduces interface seepage and minimizes the potential for cracking of the core.

A grout curtain was provided in the abutments of the dam. The dam was arched in the upstream direction and camber was provided to compensate for settlement. Slope protection consisted of RCC

Facing for stage 2 in the upstream and grass and riprap on the downstream for stage 1 and 2 slopes with riprap in the groin of the dam. Stage 3 does not require slope protection.

Based on recent subsurface investigations, the density and description of the foundation materials are adequate for this CCR unit.

4.0 SLOPE PROTECTION 257.73(d)(1)(ii)

[Describe the slope protection measures on the upstream and downstream slopes.]

Slope protection consisted of RCC Facing for stage 2 in the upstream and grass on the downstream for stage 1 and 2 slopes with riprap in the groin of the dam. Stage 3 does not require slope protection. Any erosion that may occur is repaired within a timely period.

5.0 EMBANKMENT CONSTRUCTION 257.73 (d)(1)(iii)

[Describe the specifications for compaction and/or recent boring to give a relative comparison of density.]

The design drawings show that the embankment materials were to be compacted to 90% Modified proctor density. Recent borings through the embankment indicate that the material is stiff and representative of compacted earthen materials.

6.0 VEGETATION CONTROL 257.73 (d)(1)(iv)

[Describe the maintenance plan for vegetative cover.]

The vegetative areas are mowed to facilitate inspections and maintain the growth of the vegetative layer; and prevent the growth of woody vegetation.

7.0 SPILLWAY SYSTEM 257.73(d)(1)(v)

[Describe the spillway system and its capacity to pass the Inflow Design Flood as per its Hazard Classification.]

The spillway system consists of a primary weir box and pipe for normal operations and an open channel spillway to pass flood events. The CCR unit has a high Hazard rating and design flood is the PMF flood. The facility can safely pass this flood (PMF) without overtopping the dam crest.

7.1 SERVICE SPILLWAY

The existing service spillway is a vertical concrete shaft structure with side opening for effluent discharge connecting into a sloping concrete shaft structure with one side opening, four feet wide, connecting into a 54 inch diameter pre-stressed concrete cylinder pipe (PCCP), designed for 200 feet of internal hydraulic pressure and 200 feet of overburden pressure. During most operating conditions, discharge through the service spillway is controlled by the-weir flow over the side openings in the shaft. The bottom of the sloping concrete shaft and the entire 54-inch concrete pipe were constructed within bedrock as part of the 1997 raising. Stop logs are utilized to maintain settling action and control the operating pool level.

Results of the reservoir routings establish a maximum operating level of 974.0 feet, with the 50-year design flood reaching a level of 975.5 feet, 1.5 feet above the maximum operating pool.

7.2 EMERGENCY SPILLWAY

As of 2013 construction, the existing emergency was raised to El. 975.5 through the use of a mass concrete gravity section in conjunction with reinforced concrete training walls, in a manner similar to the existing configuration. The new walls direct the flow into the existing spillway outlet channel.

In accordance with State of Ohio dam safety requirements for Class 1 dams, the new emergency spillway was designed to pass the design probable maximum flood (PMF) without overtopping the dam. The new spillway features a 108 foot long by 15 foot wide concrete control section positioned at El. 975.5, or 1.5 feet above the maximum operating pool. The training walls are located above elevation 975.5 and will consequently not be exposed to a continuous pool reducing corrosion concerns.

Based on the flood routing, the calculated peak discharge from the dam is 5,409 cfs at a maximum pool elevation of 981.9 feet NGVD. The PMF routing was also checked with the service spillway blocked, which resulted in a maximum pool elevation of 982.8 and 0.2 feet of freeboard.

8.0 BURIED HYDRAULIC STRUCTURES 257.73 (d)(1)(vi)

[Describe the condition of the sections of any hydraulic structure that in buried beneath and/or in the embankment.]

The principal outlet pipe from FAR II Pond passes though the dam near the southwestern side of the impoundment. The portion of the outlet pipe that passes though the embankment is a 54 inch diameter pre-stressed concrete cylinder pipe (PCCP), designed for 200 feet of internal hydraulic pressure and 200 feet of overburden pressure. The entire 54-inch concrete pipe was constructed within bedrock as part of the 1997 raising. There are no performance issues with the outlet pipe that would indicate plugging or failure of the pipe. Given that this portion of pipe is reinforced concrete, structural integrity is not considered to be an issue. In general reinforced concrete pipe has a long service life under a range of conditions and is an appropriate design for this application.

Based on recent video inspection of the pipe, the concrete pipe is in excellent conditions with no signs of deformation or deterioration.

9.0 SUDDEN DRAWDOWN 257.73 (d)(1)(vii)

[If the downstream slope is susceptible to inundation, discuss the stability due to a sudden drawdown.]

The downstream slope of the Fly Ash Reservoir II is not expected to be inundated from any adjacent water bodies.

Fly Ash Reservoir II 40 CFR 257.101 (f)(1)(iv)(B)(8)

The most recent safety factor assessment required at 40 CFR 257.73(e)

Fly Ash Reservoir II Dam Initial Safety Factor Assessment

Cardinal Power Plant Brilliant, Ohio S&ME Project No. 7217-15-006A



Prepared for:
American Electric Power
1 Riverside Plaza, 22nd Floor
Columbus, Ohio 43215

Prepared by: S&ME, Inc. 6190 Enterprise Court Dublin, OH 43016

September 18, 2015



Brilliant, Ohio S&ME Project No. 7217-15-006A

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Appendices

Appendix I – Safety Factor Assessment Figures

September 18, 2015





Brilliant, Ohio S&ME Project No. 7217-15-006A

1.0 Introduction

1.1 Background

In April of 2015, the US EPA formally published national regulations for disposal of coal combustion residuals (CCR) from electric facilities. As part of the rule, the owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that aspects of the CCR impoundments are in accordance with the rules. Based on our understanding of the Request for Fee Estimate received from AEP on April 29, 2015, AEP specifically requested P.E. certification to fulfill the requirements of 40 CFR § 257.73(e), *Periodic Safety Factor Assessments*. S&ME performed the design and construction administration for the dam raising completed in 2014. Due to our familiarity with the site, S&ME was selected to perform the Safety Factor Assessment for this facility. S&ME understands that certification and/or documentation for other structural integrity criteria will be performed by AEP or other consultants.

1.2 Location and Historic Overview

The Cardinal Power Plant is located along the Ohio River, approximately 8 miles south of Steubenville in Jefferson County. Then Fly Ash Reservoir II is an on-stream reservoir within the east branch of Blockhouse Run, located approximately, three-quarters of a mile north of the plant. Completed in 1986, the original earth fill dam, referred to as Stage 1, consisted of a 180 feet high arched dam constructed as a conventional zoned earth embankment. At 925 feet NGVD, the dam featured a 70-foot wide by 1,055feet long crest. The maximum operating pool that could be achieved with the original configuration was El. 913. Construction of the first dam raising, referred to as Stage 2, was completed in 1997 which brought the dam to a maximum height of 225 feet with a 30 foot wide crest at Elevation 970 feet and a maximum operating pool Elevation of 960 feet. The dam raising was achieved through the use of an upstream soil cement block (cement stabilized bottom ash) in conjunction with a downstream earth fill along with extensions of the upstream bottom ash filter, clay core, chimney drain and downstream mine spoil shell. At the completion of the 1997 raising, the upper portion of the entire dam crest consisted of a minimum of 9 feet of RCC to both protect the dam from erosion and serve as a roadway. In 2013, the dam was raised an additional 13 feet with the construction of a double-sided mechanically stabilized earth (MSE) wall system on top of the RCC, raising the maximum operating pool to Elevation 974 feet. To control seepage, a cement-bentonite slurry wall was constructed which penetrated into the existing clay core. A non-structural vinyl sheet pile wall was then inserted full depth through the slurry wall and extended to the top of the raised dam in between the MSE reinforced zones. The raised dam also includes a modified auxiliary spillway composed of mass concrete, and a precast service spillway extension.





Figure 1-1 – Location Map

1.3 Previous Investigations and Design Work

In 2010, the undersigned senior engineer, when in the employment of BBC&M Engineering, Inc., completed a supplemental geotechnical assessment of the FAR-II Dam. The assessment consisted of performing slope stability analysis for various steady-state, seismic, rapid drawdown, and surcharge loading cases load cases which were not previously addressed.

S&ME began design work for the FAR-II dam raising in 2011. In support of the design, S&ME conducted a subsurface investigation consisting of soil borings, test pits, and core samples of the soil cement block. S&ME then worked closely with AEP and state dam safety officials to permit this unique structure,





Brilliant, Ohio S&ME Project No. 7217-15-006A

including evaluating a variety of seepage and stability failure modes as well as the potential for corrosion of the reinforced concrete wall panels. S&ME then served in a construction administration role for the duration of construction. Upon completion of the project in April of 2014, S&ME issued an Engineering Certification Letter to the Ohio Department of Natural Resources, Division of Soil and Water. S&ME also completed a First Filling Plan and updated the Operation, Maintenance, and Inspection Manual and Emergency Action Plan.

2.0 Scope of Work

In accordance with AEP's request, the following work items were performed by S&ME:

- 1. S&ME completed a cursory review of the previously conducted design work for the recent dam raising, as well as a previous design reports and construction documents made available by AEP.
- 2. S&ME visited the site along with personnel from AEP to observe the facility. It should be noted that the ODNR Division of Soil and Water, Dam Safety Section conducted the 1-year inspection of the dam in June of 2015 and concluded that construction was performed in accordance with the terms of the permit, plans, specifications, and approved changes.
- 3. Action values relating to instrumentation measurements were determined based on slope stability analyses using the critical cross-section and examination of historical piezometer readings provided by AEP.
- 4. Upon completing Tasks 1 through 4, S&ME's determined that there was sufficient information to certify the structural integrity of the surface impoundment in accordance with the requirements of 40 CFR § 257.73(e). A separate letter has been prepared to this effect.

3.0 Information Review and Site Visit

To support the safety factor assessment, S&ME conducted a cursory review of previous documents relating to the FAR-II Dam and conducted a site visit at the facility. While not a comprehensive list, AEP provided S&ME with the following documents during the course of our involvement with this facility:

- Design Report: Proposed Dam for Fly Ash Retention Pond II, December 1984
- Construction Plans, Fly Ash Dam 2
- Final Design Report: Proposed Earth Fill-Roller Compacted Concrete Raising of Dam for Fly Ash Retention Pond II, March, 1997
- Construction Plans, Dam Raising of Fly Ash Retention Dam II, March 1997
- RCC QA/QC Plan for 1997 Raising, July 1998
- 1997 Failure Repair Report
- 1999 Post Construction Performance Report
- 2004 Seepage Report

On August 18, 2015, the undersigned S&ME personnel met with Dr. Mohammad Ajlouni (AEP Civil Engineering) and Mr. Randy Sims (Landfill Operations) at the Cardinal Plant and conducted a site visit at the FAR-II Dam. The participants observed the site and discussed recent monitoring results, as well as



Fly Ash Reservoir II Dam Initial Safety Factor Assessment Cardinal Power Plant

Brilliant, Ohio S&ME Project No. 7217-15-006A

tentative plans to raise the pool level by adding additional stop logs. S&ME observed slight rutting along the wheel path on top of the dam, as well as minor settlement of the granular infill adjacent to the panels. Instrumentation readings from 12 tiltmeter sensors placed on the MSW wall panels indicate that both the upstream and downstream MSE wall panels are leaning outward slightly, however the rate of movement has now generally leveled off. This outward tilt appears to be an expression of the rotational movement needed to fully engage the geogrid reinforcement. S&ME understands that AEP is closely monitoring the ongoing instrumentation readings. While the site visit was not a formal inspection, visual observations of the FAR-II Dam did not reveal any dam safety concerns, and the downstream slopes appear to be in a similar condition as observed during construction of the recent dam raising.

4.0 Safety Factor Assessment

As part of the safety factor assessment, S&ME completed Parts 1 and 2 of Section 257.73(e) of the Final Rules for the Disposal of Coal Combustion Residuals from Electric Utilities published on April 17, 2015 in the Federal Register. In accordance with the Rule, the analysis was performed for the critical cross-sections(s) that are anticipated to be most susceptible of all cross-sections to structural failure based on appropriate engineering considerations. The Rule specified the following loading conditions for analysis:

- i. Static Factor of Safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- ii. Calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.50
- iii. The calculated seismic factor of safety must equal or exceed 1.00
- iv. For dikes constructed of soils susceptible to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

4.1 Limit Equilibrium Analyses

Our 2013 Dam Raising Design Report discusses in detail the subsurface investigation, laboratory testing, parameter justification, seepage analyses and limit equilibrium slope stability analyses that were performed to develop safety factors for the FAR-II Dam Raising design. S&ME focused on evaluating the cross-section through the high point of the dam with additional slope stability runs performed for the section through the existing emergency spillway. Two dimensional slope stability analyses were performed under End of Construction, Long Term Maximum Pool (Static), Maximum Surcharge Pool, Rapid Drawdown, and seismic loading conditions in conformance with the US Army Corps of Engineers Manual 1110-2-1902 entitled Slope Stability. The phreatic surface was modeled based on current piezometer data collected from at the site and the results of the finite element seepage analysis. However, the phreatic surface was entered manually to minimize the potential for computation uncertainty as compared to directly using the finite element analysis output pressures.

Shear strength parameters representing the existing dam zones were developed by AEP and their consultants for the design of the Stage 1 and Stage 2 dams. These values were used as the starting point for the Stage 3 global stability analyses but were modified in some cases to reflect the results of the current investigation or to investigate particular failure modes. Additionally, the Stage 3 raising included several new material zones: the MSE wall reinforced zone (No. 57 stone), ODOT Item 304 surface course,



Fly Ash Reservoir II Dam Initial Safety Factor Assessment Cardinal Power Plant

Brilliant, Ohio S&ME Project No. 7217-15-006A

the cement-bentonite slurry wall, and the vinyl sheet pile wall. The shear strength parameters for these new materials were estimated based on past experience. It should also be noted that the strength of these materials does not appreciably impact the global stability analyses. Additional discussion of the shear strength values for the most critical zones is provided in the 2013 Final Design Report.

4.2 Liquefaction Potential of Embankment Soils

S&ME reviewed the material and compaction specifications of the embankment fill for the original dam construction and subsequent raisings. The dam was constructed entirely of engineered materials and was designed in accordance with the methods used to design conventional water reservoirs. The embankment fill consists of fine grained overburden soil and mine spoil fill from near the project site. With the exception of the blanket drain, chimney drain, and rip rap zone, all earthen material was compacted to 100% of the standard proctor compaction test. Based on this understanding, the embankment soils are considered non-liquefiable. Furthermore, liquefaction of the foundation soils is not a concern as the overburden beneath the dam was removed prior to fill placement, with the dam supported directly on bedrock

4.3 Summary of Results

Based on our previous investigations and current assessment of the Bottom Ash Pond facility, S&ME certifies that this assessment meets the requirements of 40 CFR § 257.73(e), *Periodic Safety Factor Assessments*. A summary of the computed safety factors for the critical cross-section is provided in Table 5-1. Also included in the table are the minimum values defined in 40 CFR § 257.73(e)(1) subparts (i) through (iv). Graphical output corresponding to the analysis cases are presented in Appendix II.

Table 4-1 – Safety Factor Summary

Analysis Case	Minimum Safety Factor	Computed Safety Factor
Long-term, maximum storage pool	1.50	1.75
Maximum surcharge pool	1.40	1.68
Pseudo-static seismic loading	1.00	1.11
Embankment Liquefaction	1.20	Non-liquefiable



Brilliant, Ohio S&ME Project No. 7217-15-006A

5.0 Certification

Based on our previous investigation, design, and construction administration work associated with the Fly Ash Reservoir II Dam, S&ME certifies that this assessment meets the requirements of 40 CFR § 257.73(e), *Periodic Safety Factor Assessments*. A summary of the computed safety factors for the critical cross-section is provided in the table below. Also included in the table are the minimum values defined in 40 CFR § 257.73(e)(1) subparts (i) through (iv).

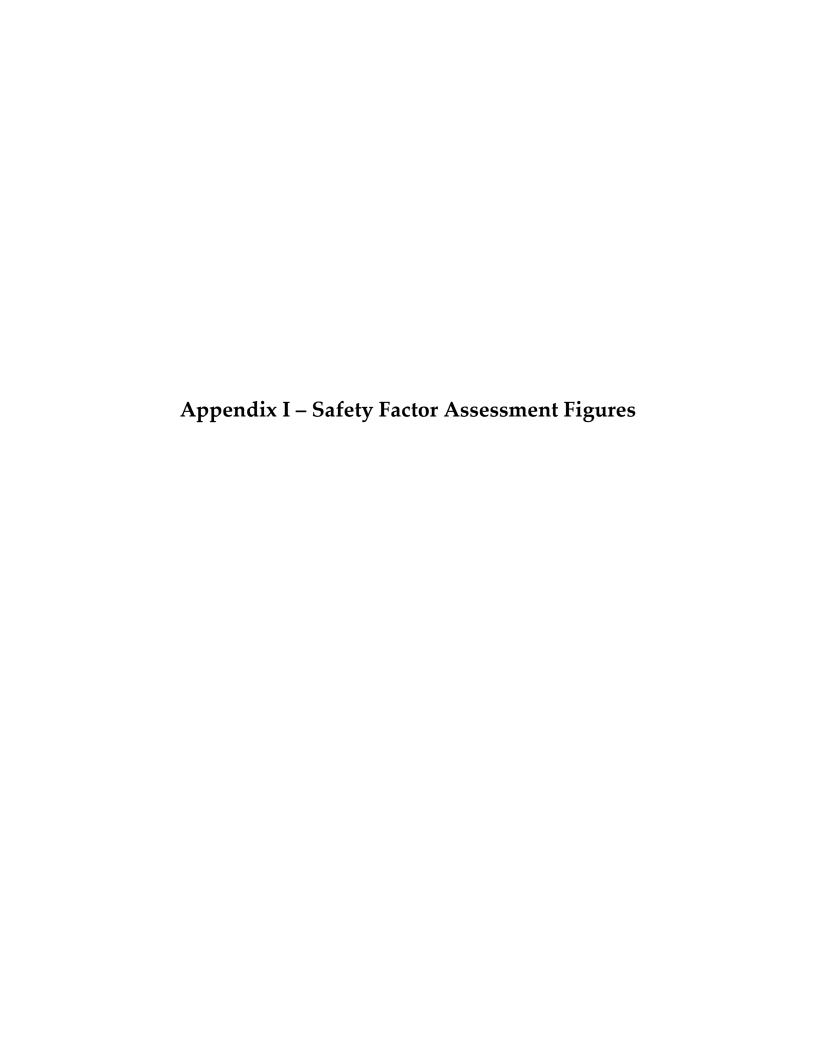
Michael T. Romanello, P.E.

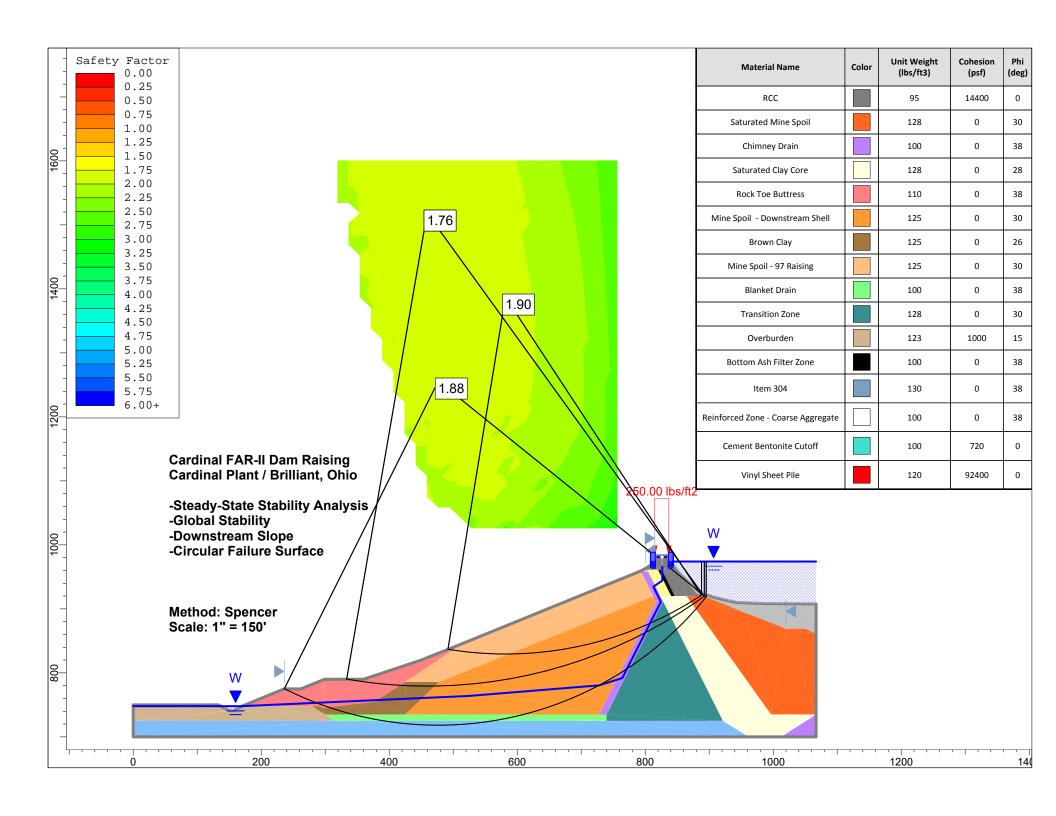
Project Engineer

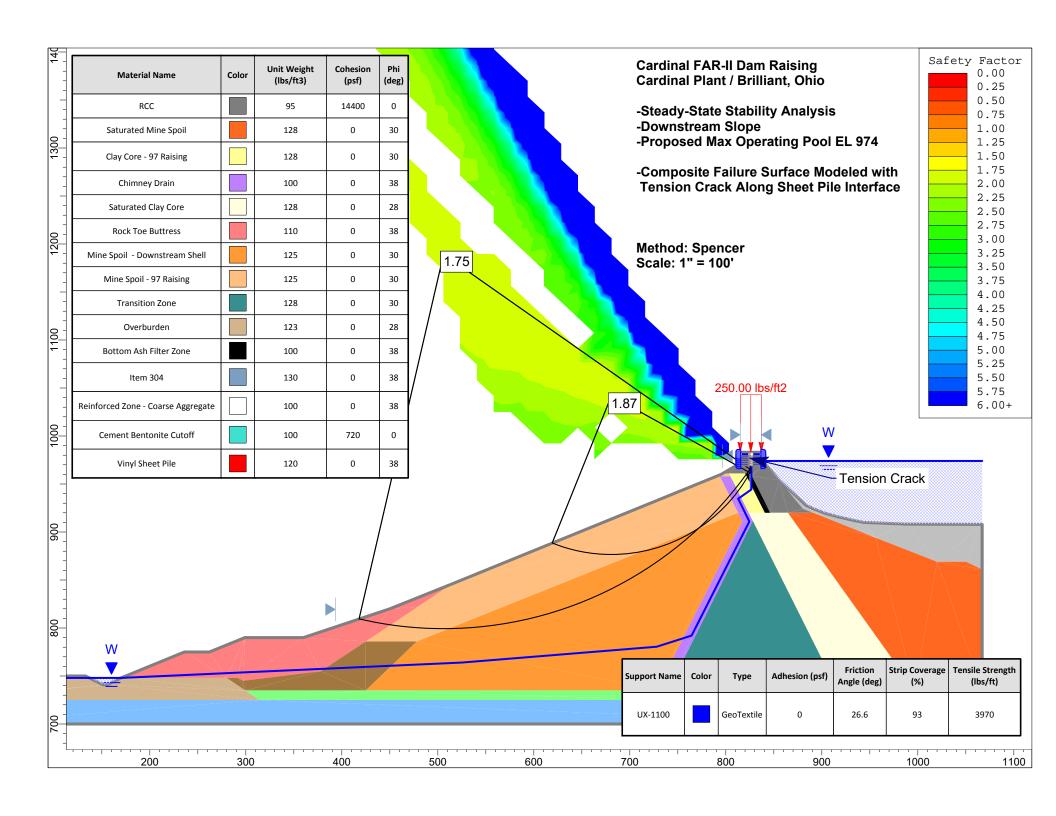
Registration No. 74384

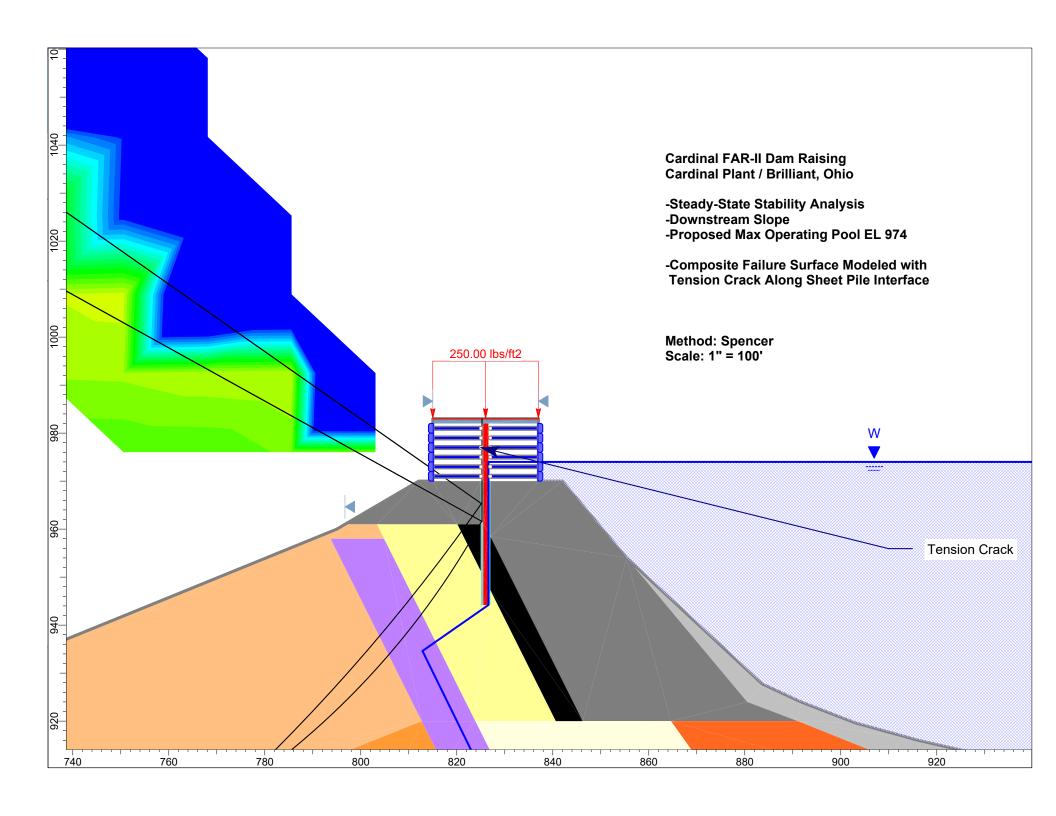
Michael G. Rowland, P.E. Senior Engineer Registration No. 65559

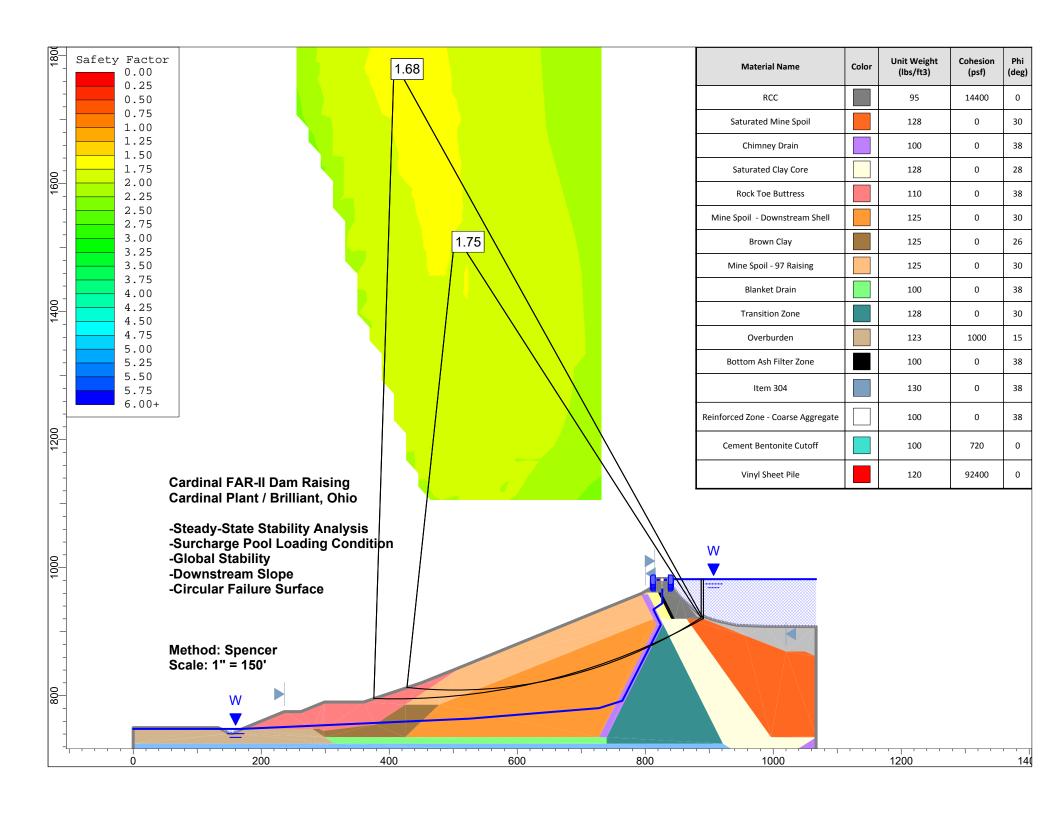


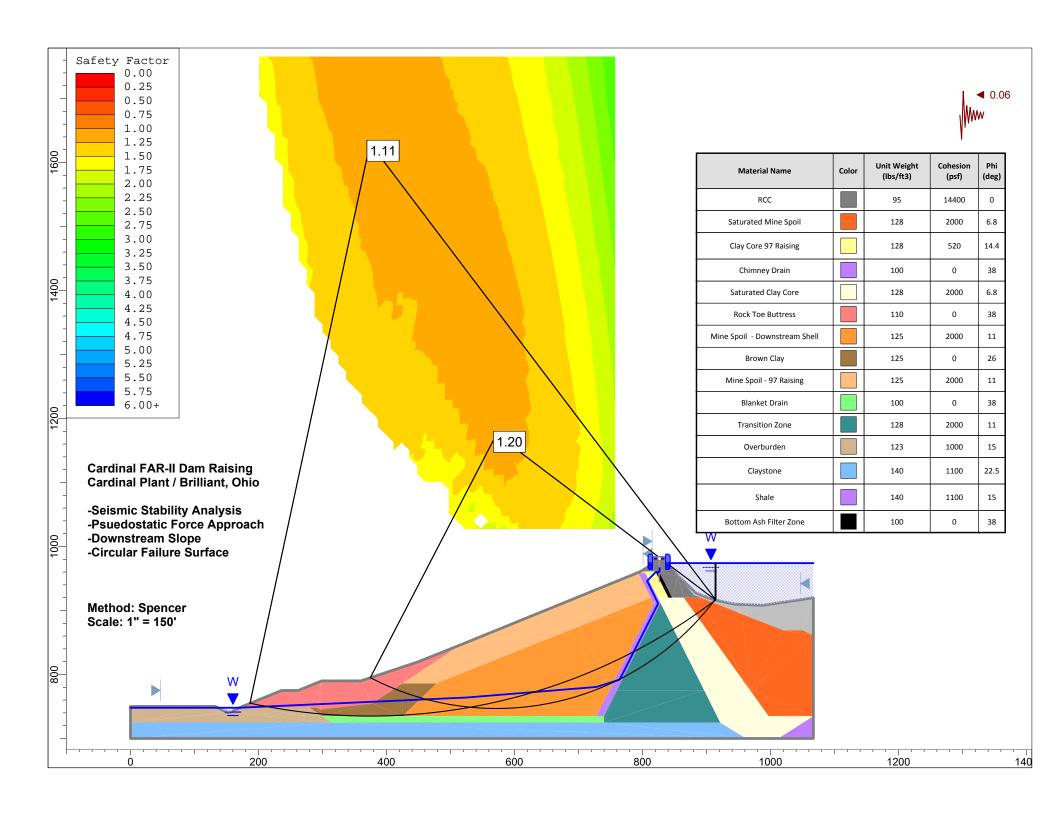




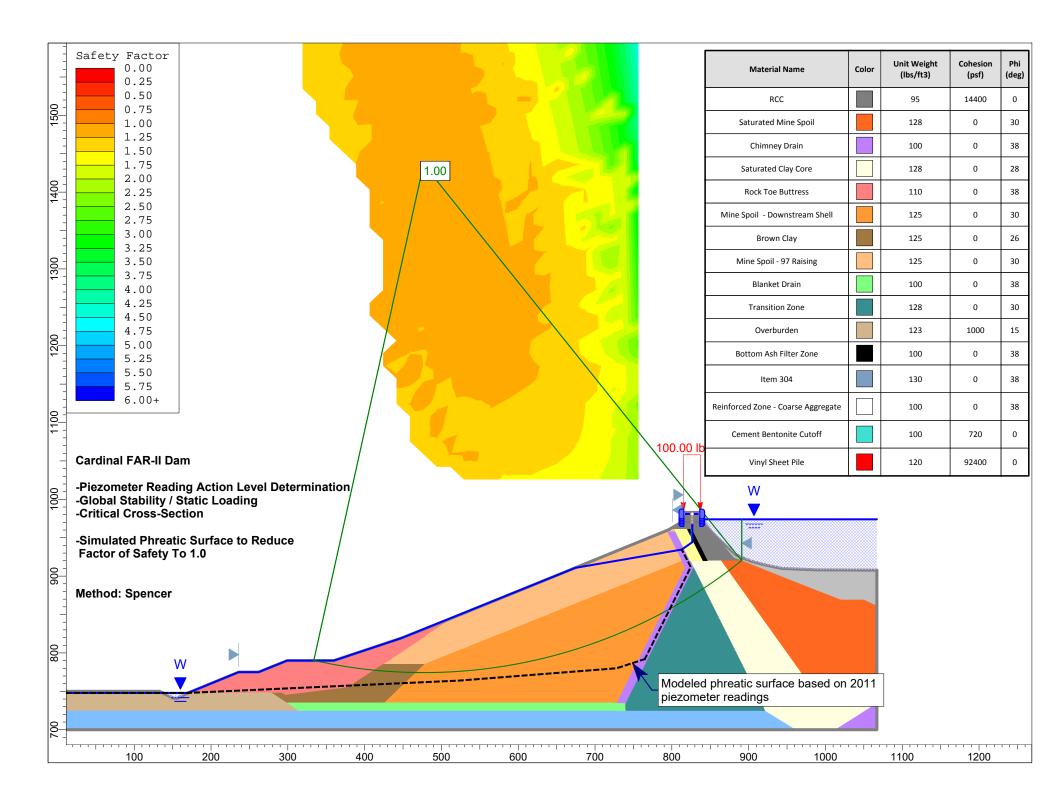








Appendix II – Action Value Recommendation Figures



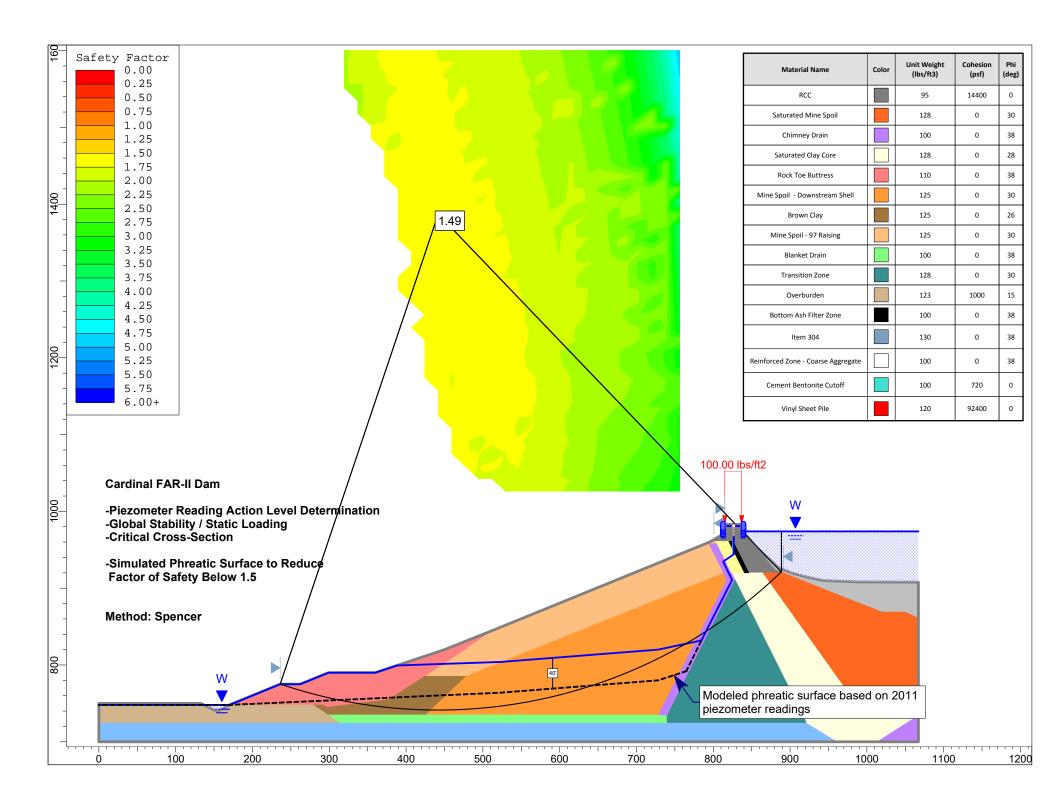
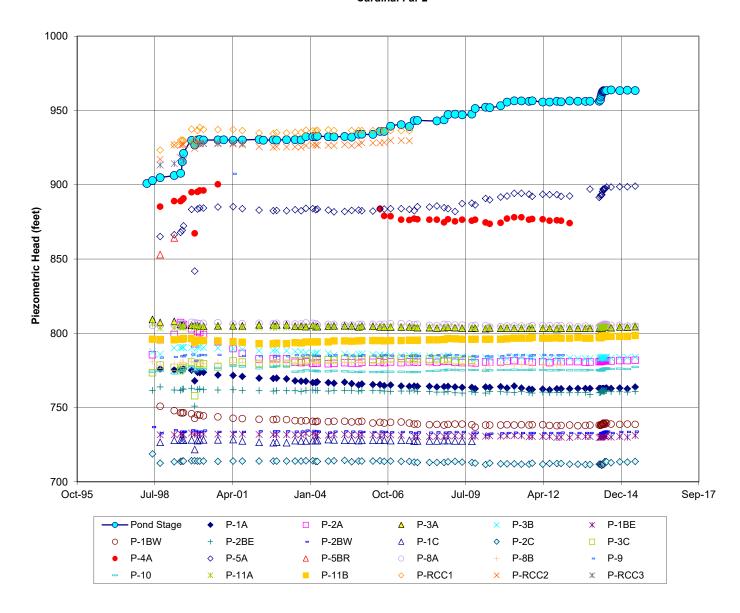
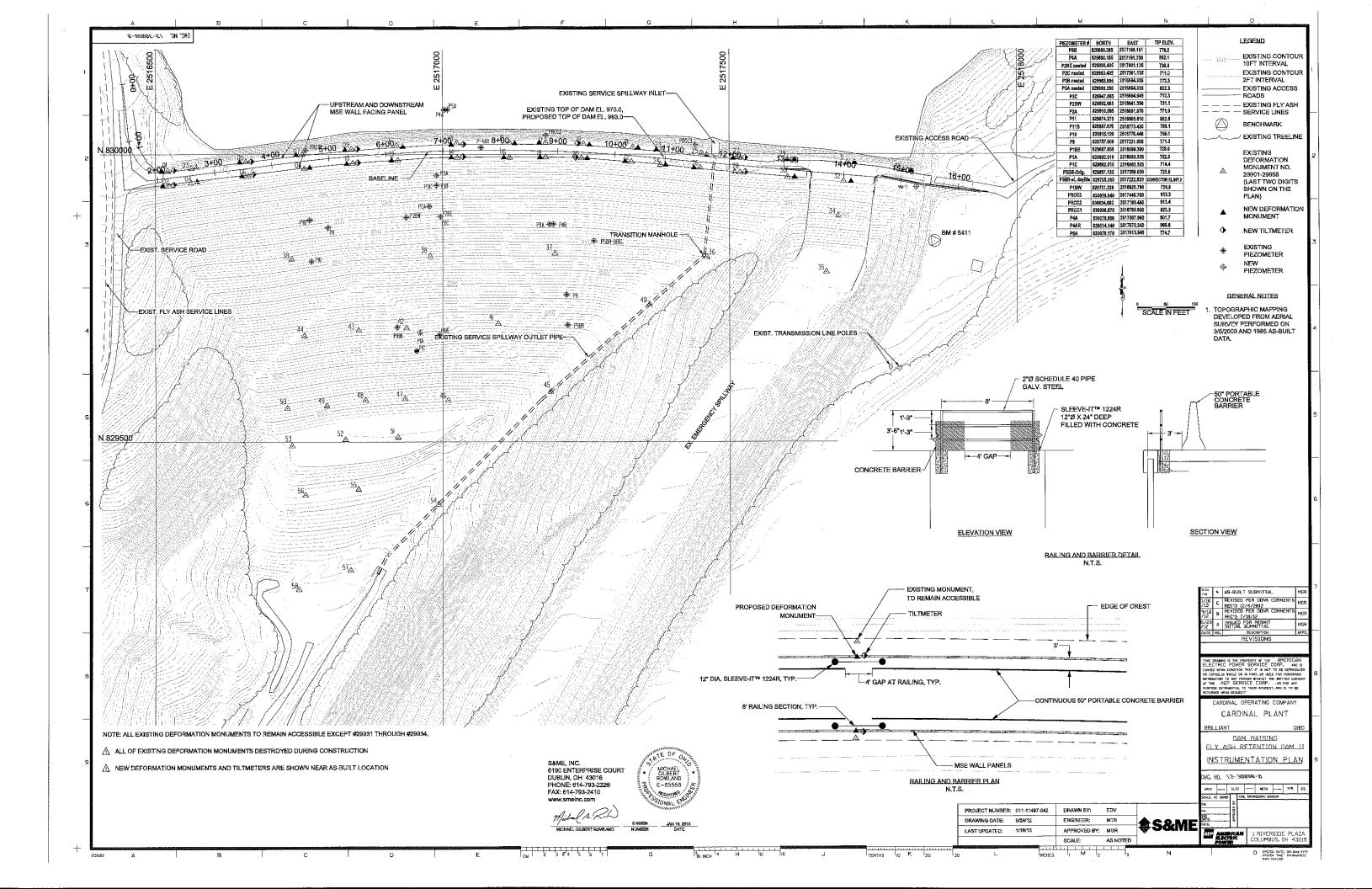


Figure 5a Cardinal Far 2





Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(2)(i)

Maps of Groundwater monitoring well locations in relation to CCR Unit



- Compliance Sampling Location
- Background Sampling Location

Bottom Ash Pond

- Monitoring well coordinates provided by Buckeye Power.
 Site features based on information available in Groundwater Monitoring Network Evaluation Cardinal Site Bottom Ash Pond (Geosyntec, 2016) provided by Buckeye Power.

Site Layout Bottom Ash Complex

Buckeye Power Cardinal Generating Plant Brilliant, Ohio

2018/01/25

Geosyntec D consultants

Columbus, Ohio

Figure 2

C:\Users\mmuenich\Documents\local_projects\AEP_GIS\Cardinal\MXDs\BAP\AEP-Cardinal_BAP_Site_Layout.mxd. MMuenich. 1/25/2018. CHA8423/01/08.

Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(2)(ii)

Well construction diagrams and drilling logs for all groundwater monitoring wells

EXPLANATION OF SYMBOLS AND TERMS USED ON BORING LOGS FOR SAMPLING AND DESCRIPTION OF SOIL

SAMPLING DATA



- Blocked-in "SAMPLES" column indicates sample was attempted and recovered within this depth interval.



- Sample was attempted within this interval but not recovered.

2/5/9

- The number of blows required for each 6-inch increment of penetration of a "Standard" 2-inch O.D. split-barrel sampler, driven a distance of 18 inches by a 140-pound hammer freely falling 30 inches. Addition of one of the following symbols indicates the use of a split-barrel other than the 2" O.D. sampler:

2S

- 2½"O.D. split-barrel sampler

3S

- 3" O.D. split-barrel sampler

1

P - Shelby tube sampler, 3" O.D., hydraulically pushed.

- Refusal of sampler in very-hard or dense soil, or on a resistant surface.

R 50-2"

- Number of blows (50) to drive a split-barrel sampler a certain number of inches (2), other than the normal 6-inch increment.

S/D

- Split-barrel sampler (S) advanced by weight of drill rods (D),

S/H

- Split-barrel sampler (S) advanced by combined weight of rods and drive hammer (H).

SOIL DESCRIPTIONS

All soils have been classified basically in accordance with the Unified Soil Classification System, but this system has been augmented by the use of special adjectives to designate the approximate percentages of minor components as follows:

<u>Adjective</u>	Percent by Weight
trace	1 to 10
little	11 to 20
some	21 to 35
"and"	36 to 50

The following terms are used to describe density and consistency of soils:

Term (Granular Soils)	Blows per foot
Very-loose	Less than 5
Loose	5 to 10
Medium-dense	11 to 30
Dense	31 to 50
Very-dense	Over 50
Term (Cohesive Soils)	Qu (tsf)
Very-soft	Less than 0.25
Soft	0.25 to 0.5
Medium-stiff	0.5 to 1.0
Stiff	1.0 to 2.0
Very-stiff	2.0 to 4.0
Hard	Over 4.0

Page 1 of 2

2010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007B

LOG OF BORING NO. MW-BAP-1 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,305, E. 2,513,927 12/4/15 - 12/10/15 ELEVATION: 669.8 DATE: DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: 52.0' SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT TEST NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION E RESULTS **AGGREGATE - 34 INCHES** 100 75 667.0 FILL: Hard brown silty clay, some fine to coarse 64 100 2 H=3.0sand, some fine to coarse gravel, cobbles, moist. 665.3 18 3 27 FILL: Medium-dense gray fine to coarse grave, 5 little to some fine to coarse sand, trace silt to some silty clay, cobbles, dry. 59 67 4 662.8 FILL: Stiff to very-stiff brown silty clay, some to "and" fine to coarse sand, some fine to coarse 11 87 H=2.5gravel, contains fine to coarse sand seams and sandstone fragments, damp. 19 100 H = 3.010-16 100 H=2.5-3.5658.3 FILL: Medium-dense fine to coarse gravel, some 19 67 to "and" fine to coarse sand, some clayey silt, 8 damp becoming moist. 11 100 - 3" pocket of sand at 14.5'. 15 13 53 H=1.25653.8 Stiff gray clayey silt, "and" fine to coarse sand, 8 11 67 H=1.25little to some fine gravel, moist. 652.3 Loose brown fine to coarse sand, "and" silty clay, 8 12 53 H = 1.0some fine to coarse gravel, moist. 650.6 Loose to medium-dense brown fine to coarse 16 93 13 H=4.520gravel, some to "and" fine to coarse sand, some silty clay, damp to moist. 14 14 80 15 15 67 16 6 80 25 643.8 Hard brown mottled with gray and dark-gray silty H=3.0-4.011 73 17 clay, little fine to coarse sand, trace fine to coarse gravel (shale fragments), slightly organic, damp. 641.0 Stiff dark-brown clayey silt, little to some fine to 100 H=1.25-2.25 medium sand, slightly organic, damp. SYMBOLS USED TO Drill Rod Energy Ratio: 0.75 WATER LEVEL: 31.0 - Gradation-- Uncon Comp H - Penetrometer (tsf) WATER NOTE: Inside Well **Last Calibration Date:** 8/2/2013 Inside HSA T - Triax C - Consol Separate W - Unit Dry Wt (pcf) Triax Comp 12/15/15 12/7/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE: ATV 550-2

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LOG OF BORING NO. MW-BAP-1 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,305, E. 2,513,927 12/4/15 - 12/10/15 ELEVATION: 669.8 DATE: 4-1/4" I.D. Hollow-stem Auger 52.0' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE EFFORT DEPTH FEET **TEST** NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION EL RESULTS 30-638.8 SH Very-soft to medium-stiff brown, gray and 0 19 100 H=0.0-0.25SH, dark-gray organic clayey silt, little fine sand, SH contains silt seams and lenses, contains seams of fine to coarse sand, wet. SH20 0 100 H=0.0-0.7535 2010 NEW DEFAULT BORING LOG-W/ 21 100 H=0.0-0.75SH. SH 0 100 SH 630.7 H=1.0-1.522B SH, Very-loose gray fine to coarse sand, interbedded 22C with silty clay seams, wet. 40-628.8 Loose brown fine to coarse sand, trace fine 6 60 gravel, trace silt, wet. 626.8 Dense brown fine to coarse gravel, some to "and" fine to coarse sand, trace silt. 45 24 50 47 25 40 67 50-618.8 Medium-dense brown fine to coarse sand, trace 26 19 47 fine gravel, trace clay. 617.3 - Encountered water at 31.0'. - Encountered cobbles at 4.4 and 18.2'. 55 - Borehole converted to monitoring well upon completions. See separate well completion diagram. - Boring locations and elevation surveyed by AEP. - Datum: Ohio State Plane South. - NAD 27/NAVD 29 (Plant Grid). 60-G - Gradation
Q - Uncon Comp
T - Triav Drill Rod Energy Ratio: 0.75 WATER LEVEL: 31.0 27.5 H - Penetrometer (tsf) WATER NOTE: Inside Well **Last Calibration Date:** 8/2/2013 **Inside HSA** Separate W - Unit Dry Wt (pcf) Triax Comp DATE: 12/7/15 12/15/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME ATV 550-2

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2010 NEW DEFAULT BORING LOG-W/

LOG OF BORING NO. MW-BAP-2 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 819,792, E. 2,513,707 12/2/15 - 12/4/15 ELEVATION: 669.9 DATE: DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: 45.0' SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT ^{9}Z DESCRIPTION \mathbf{E} RESULTS **AGGREGATE - 23 INCHES** 668.0 45 87 11. FILL: Dense to very-dense dark-gray fine to coarse sand, trace to little fine gravel, trace to 79 47 2 little silt, moist. 33. 666.3 FILL: Stiff to hard brown and dark-brown silty clay, some to "and" fine to coarse sand, little to 24 60 3 H=2.05 some fine to coarse gravel, few pockets of gravel, dry becoming damp. 67 4 30 H=4.528 80 19 60 - 10-13 87 23 80 H=2.0-4.58 10 14 53 H=2.5655.4 FILL: Medium-stiff to very-stiff brown mottled 15 10 14 67 H = 3.5with gray silty clay, some fine to coarse sand, little fine to coarse gravel, moist. 9 87 H=1.0-2.2510 67 H=0.75-1.5 650.3 9 13 87 H=2.0FILL: Very-loose to loose dark-gray fine to 20coarse sand, trace to little fine gravel, little silt, moist becoming wet. 67 9 14 SH - Contains sand seams at 20.0' to 20.3'. 0 15 SH, 100 3 100 16 644.7 -25 3 17 100 H=0.5FILL: Very-loose dark-gray silt, trace fine to 643.9 coarse sand, slightly organic, wet. Stiff gray mottled with brown silty clay, some 18 0 53 H=2.0fine sand, trace medium to coarse sand, slightly 641.9 organic, silt seams, damp. Medium-stiff dark-gray organic clayey silt, little fine sand, damp. 0 100 H=1.0SYMBOLS USED TO INDICATE TEST RESULT Ţ Drill Rod Energy Ratio: 0.75 WATER LEVEL: 29.2 - Gradation-- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: Inside Well 8/2/2013 Separate W - Unit Dry Wt (pcf) - Triax Comp 12/15/15 DATE: Curves D - Relative Dens (%) **Drill Rig Number:** S&ME ATV 550-2

Page 2 of 2

LOG OF BORING NO. MW-BAP-2 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 819,792, E. 2,513,707 12/2/15 - 12/4/15 ELEVATION: 669.9 DATE: 4-1/4" I.D. Hollow-stem Auger 45.0' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION EL RESULTS 30-639.4 Stiff gray mottled with brown silty clay, little fine sand, trace medium to coarse sand, slightly 5 20 100 H=1.5organic, damp. 636.2 Loose fine to coarse sand, trace fine gravel, little 21 8 100 H=1.5to some silt, slightly organic, moist. 35 634.4 2010 NEW DEFAULT BORING LOG-W/ N60 Loose brown fine to coarse sand, trace fine gravel, trace to little silt. 10 53 5 67 40-6 100 5 100 624.9 45 - Encountered water at 14.5'. to 16.0'. - Borehole converted to monitoring well upon completion - See separate well completion digram. - Boring location and elevation surveyed by AEP. - Datum: Ohio State Plane South 50-- NAD 27/NAVD 29 (Plant Grid). 55 60-₹ Drill Rod Energy Ratio: 0.75 WATER LEVEL: <u> 29.2</u> - Gradation-- Uncon Comp H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: **Inside Well** 8/2/2013 Separate W - Unit Dry Wt (pcf) Triax Comp DATE: 12/15/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME

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:010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007B

LOG OF BORING NO. MW-BAP-3 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 819,111, E. 2,513,519 11/11/15 - 11/12/15 ELEVATION: 669.9 DATE: 4-1/4" I.D. Hollow-stem Auger 55.0' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $^{\circ}_{\rm N}$ DESCRIPTION EL RESULTS **AGGREGATE - 12 INCHES** 668.9 FILL: Medium-dense to dense gray and brown 28 87 fine to coarse gravel, some to "and" fine to coarse H=3.512 sand, little to some silt or silty clay (variers), contains pockets of fine to coarse sand, dry. 39 80 2 H=4.013 43 3 67 5 44 4 100 25 67 661.4 FILL: Hard gray and brown silty clay, some fine 29 100 H=4.5+10 to coarse and, little fine to coarse gravel, damp. 659.9 10-FILL: Very-dense fine to coarse black and gray 71 7 67 sand, some fine to coarse gravel, damp. 658.4 FILL: Very-stiff brown silty clay, some to "and" 19 100 8 fine to coarse sand, some fine to coarse gravel, H = 3.5damp. 35 87 H=3.5-4.0655.4 FILL: Loose to medium-dense brown fine to 15 10 14 80 coarse gravel, some to "and" fine to coarse sand, some silty clay, damp to moist. 14 80 H=4.5- Contains zones of hard silty clay at 16.0'. 13 93 8 13 67 20-649.4 Medium-stiff to stiff brown clayey silt, "and" fine 9 14 53 to coarse sand, some fine to coarse gravel, wet. H=1.0-2.015A 6 100 H=0.5647.3 Loose gray fine to medium sand, trace coarse 15B sand, trace fine gravel, little silt, wet. 16 5 100 644.4 Very-loose gray silt, little fine to medium sand, wet. 100 17 0 641.9 Soft to stiff dark-brown mottled with dark-gray slithly organic to organic clayey silt, little to some 100 H=1.0-1.5fine to medium sand, contains silt seams, fine SYMBOLS USED TO INDICATE TEST RES Drill Rod Energy Ratio: 0.75 WATER LEVEL: 28.2 - Gradation-- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: Inside Well 8/2/2013 Separate W - Unit Dry Wt (pcf) - Triax Comp 12/11/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE: C - Consol ATV 550-2

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2010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007B

LOG OF BORING NO. MW-BAP-3 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 819,111, E. 2,513,519 11/11/15 - 11/12/15 **ELEVATION:** 669.9 DATE: 4-1/4" I.D. Hollow-stem Auger 55.0' DRILLING METHOD: COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION E RESULTS sand seams and roots, wet. Soft to stiff dark-brown mottled with dark-gray slithly organic to organic clayey silt, little to some 5 100 fine to medium sand, contains silt seams, fine sand seams and roots, wet. 100 -35-634.4 Soft to medium-stiff dark-brown mottled with gray slightly organic to organic clayey silt, some 21 4 100 to "and" fine to medium sand, wet. 100 40-629.4 Soft to medium-stiff gray mottled with brown silty clay, trace to some fine to coarse sand, 23 100 slightly organic, contains fine sand seams, wet. 14 100 624.9 45 Medium-dense to very-dense brown fine to coarse gravel, some to "and" fine to coarse sand, trace to little silt, wet. 80 25 35 - Contains zones of fine to coarse sand at 49.0'. 26 75 53 50-27 20 33 614.9 55 - Encountered seepage at 16.0'. - Encountered water at 20.5'. - Borehole converted to monitoring well upon completion - See separate well completion diagram. - Datum: Ohio State Plane South. NAD 60 SYMBOLS USED TO INDICATE Drill Rod Energy Ratio: 0.75 WATER LEVEL: 28.2 - Gradation-- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: **Inside Well** 8/2/2013 Separate W - Unit Dry Wt (pcf) T - Triax Comp C - Consol. _ DATE: 12/11/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME

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2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. MW-BAP-3 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



11/11/15 - 11/12/15 LOCATION: N. 819,111, E. 2,513,519 669.9 DATE: ELEVATION: DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: 55.0' SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE SAMPLE EFFORT NATURAL CONSISTENCY INDEX DEPTH, FEET **TEST** ELEV NATURAL MOISTURE CONTENT $\overset{\circ}{\mathsf{Z}}$ **DESCRIPTION RESULTS** 60-27/NAVD 29 (Plant Grid). 65 70-75 80-85 └90-SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 G - Gradation
Q - Uncon Comp
T - Triax Comp 28.2 See H - Penetrometer (tsf) WATER NOTE: Inside Well Last Calibration Date: 8/2/2013 W - Unit Dry Wt (pcf) D - Relative Dens (%) -Separate T - Triax Comp C - Consol. DATE: _ 12/11/15 Curves **Drill Rig Number:** S&ME

JOB: 7217-15-007B

ATV 550-2 PLATE 9 Page 1 of 2

2010 NEW DEFAULT BORING LOG-W/

LOG OF BORING NO. MW-BAP-4 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,880, E. 2,513,617 11/20/15 - 11/23/15 ELEVATION: 661.1 DATE: 40.0' DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION E RESULTS **AGGREGATE - 12 INCHES** 660.1 FILL: Medium-dense to dense gray and brown 39 87 fine to coarse gravel, some to "and" fine to coarse H=4.25-4.5 sand, little to some silt, dry. 18 53 2 20 67 3 5 655.8 655.3 FILL: Very-soft brown and gray silty clay, "and" fine to coarse sand, little fine to coarse gravel. 31 4 87 FILL: Dense brown fine to coarse sand, little fine 20 5 653.6 50-3"R to coarse gravel, "and" clayey silt, cobbles, moist. Stiff to very-stiff dark-brown mottled with dark-gray silty clay, little fine to coarse sand, trace fine gravel, slightly organic, damp. 9 3 87 H=2.0-3.0- 10-P H=1.25-2.515 644.9 Very-stiff brown mottled with gray silty clay, 14 87 H=2.0-3.5little fine to medium sand, trace coarse sand, few cobbles, contains silt seams near top of stratum, damp. 18 20-100 H=2.25-3.2514 100 H = 3.010 14 100 H=3.2525 9 100 634.4 11A H = 2.5Medium-stiff to stiff brown clayey silt, "and" fine H=0.5-1.5 to medium sand, trace coarse sand, includes sand seams, moist. 100 30 SYMBOLS USED Ţ Drill Rod Energy Ratio: 0.75 WATER LEVEL: 18.7 - Gradation-- Uncon Comp H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: Inside Well 8/2/2013 Separate W - Unit Dry Wt (pcf) T - Triax Comp C - Consol. DATE: 12/15/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME ATV 550-2

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2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. MW-BAP-4 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,880, E. 2,513,617 11/20/15 - 11/23/15 ELEVATION: 661.1 DATE: 4-1/4" I.D. Hollow-stem Auger 40.0' DRILLING METHOD: COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $\overset{\circ}{\mathsf{Z}}$ **DESCRIPTION** EL RESULTS 30-630.6 Medium-stiff to stiff brown clayey silt, "and" fine to medium sand, trace coarse sand, includes sand seams, moist. 100 13 0 SH, Very-loose brown and gray fine to medium sand, little to "and" silt (percent varies), contains zones with a trace of coarse sand, wet. SH0 67 35 15 3 67 100 SH, 621.1 40-- Encountered water at 5.5'. - Encountered cobbles at 18.5'. - Borehole converted to monitoring well upon completion - See separate well completion diagram. - Boring location and elevation surveyed by AEP. 45 - Datum: Ohio State Plane South, NAD 27/NAVD 29 (Plant Grid). 50-55 60 SYMBOLS USED ₹ Drill Rod Energy Ratio: 0.75 WATER LEVEL: 18.7 - Gradation-- Uncon Comp H - Penetrometer (tsf) WATER NOTE: Inside Well **Last Calibration Date:** 8/2/2013 Separate W - Unit Dry Wt (pcf) Triax Comp DATE: 12/15/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME

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2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,052, E. 2,513,277 11/24/15 - 11/25/15 ELEVATION: 669.2 DATE: 62.5' DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE EFFORT DEPTH FEET TEST NATURAL MOISTURE CONTENT $^{\circ}_{\rm N}$ DESCRIPTION EL RESULTS **AGGREGATE - 12 INCHES** 668.2 FILL: Medium-dense brown fine to coarse sand, 24 60 some fine to coarse gravel, some to "and" silty clay, dry. 13 2 60 13 73 • × 3 5 663.7 FILL: Hard gray and brown silty clay, "and" fine 51 4 87 H=4.5to coarse sand, little to some fine to coarse gravel, 32 damp. 39 80 H=4.5 660.7 FILL: Medium-dense brown and gray fine to 30 87 13 coarse sand, little fine to coarse gravel, some silty 659.2 11 clay, damp. 10-FILL: Hard brown silty clay, some fine to coarse H=4.5P sand, some fine to coarse gravel (shale fragments), damp. 19 80 H=4.5 655.7 10 FILL: Medium-dense to dense brown fine to 45 80 H = 3.0coarse gravel, some fine to coarse sand, some silty clay becoming trace silt at bottom of stratum, 15damp. 16 652.3 20 100 10A Medium-stiff to stiff gray mottled with dark-gray 10B and brown silty clay, trace fine to coarse sand, trace fine gravel, few roots, few silt seams, P slightly organic, moist. 20-5 100 **H**=0.5-1.25 11 646.2 Medium-stiff to very-stiff brown mottled with gray silty clay, trace to little fine to coarse sand, 12 8 100 H=3.5damp. 25 P P - 30-SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: 27.1 - Gradation-- Uncon Comp H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: Inside Well 8/2/2013 Separate W - Unit Dry Wt (pcf) - Triax Comp DATE: 12/15/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME ATV 550-2

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2010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007B

LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND MONITORING WELL INSTALLATION CARDINAL PLANT, BRILLIANT, OH



11/24/15 - 11/25/15 LOCATION: N. 820,052, E. 2,513,277 **ELEVATION:** 669.2 DATE: 4-1/4" I.D. Hollow-stem Auger 62.5' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE EFFORT DEPTH FEET **TEST** NATURAL MOISTURE CONTENT $\overset{\circ}{Z}$ DESCRIPTION EL RESULTS Medium-stiff to very-stiff brown mottled with gray silty clay, trace to little fine to coarse sand, damp. 13 100 13 H=2.0-3.5100 H=2.5-3.035 14 100 H=2.510 100 H=2.540-17 6 100 H=1.25100 H=1.2545 623.7 Stiff gray mottled with brown and dark-gray silty clay, trace fine to coarse sand, slightly organic, 19 0 100 H=0.75SH, damp. 621.2 Medium-stiff to stiff gray and dark-gray organic clayey silt, trace fine to coarse sand, damp. 100 20 0 H=0.75-1.2550 618.7 Medium-dense to dense fine to coarse gravel, some to "and" fine to coarse sand, trace to little 21 23 87 silt, wet. 22 69 87 613.8 - 55 Medium-dense to dense gray and brown fine to coarse sand, "and" fine to coarse gravel, little silt, 43 80 23 wet. 35 60 60 SYMBOLS USED Ā Drill Rod Energy Ratio: 0.75 WATER LEVEL: 27.1 - Gradation-- Uncon Comp H - Penetrometer (tsf) WATER NOTE: Inside Well **Last Calibration Date:** 8/2/2013 - Triax - Consol Separate W - Unit Dry Wt (pcf) Triax Comp DATE: 12/15/15 Curves D-Relative Dens (%) **Drill Rig Number:** S&ME

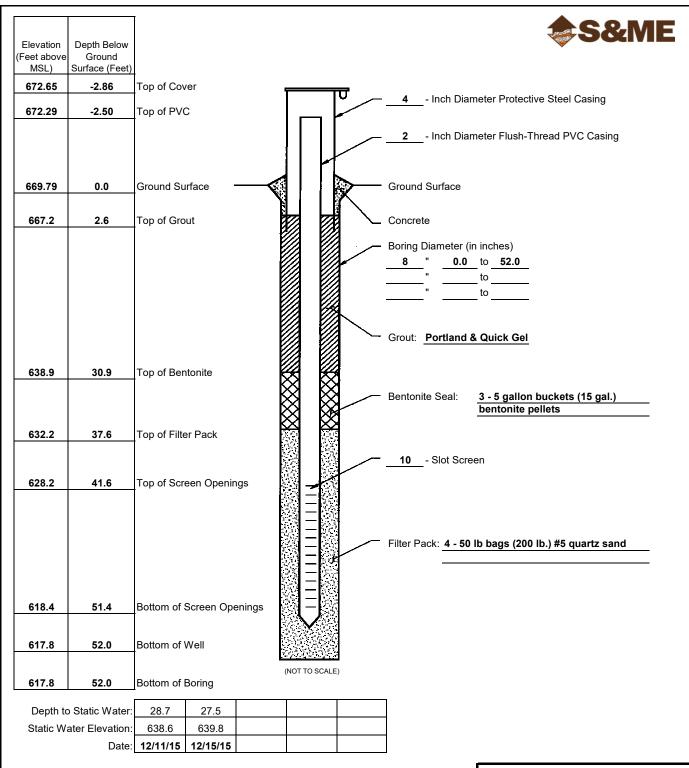
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2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND MONITORING WELL INSTALLATION



CARDINAL PLANT, BRILLIANT, OH LOCATION: N. 820,052, E. 2,513,277 11/24/15 - 11/25/15 ELEVATION: 669.2 DATE: 4-1/4" I.D. Hollow-stem Auger 62.5' DRILLING METHOD: COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT TEST NATURAL MOISTURE CONTENT $\overset{\circ}{\mathsf{Z}}$ **DESCRIPTION** EL RESULTS 60-Medium-dense to dense gray and brown fine to coarse sand, "and" fine to coarse gravel, little silt, 16 wet. 25 11 60 606.7 - Encountered water at 17.0'. - Borehole converted to monitoring well upon 65 completion. See separate well completion diagram. - Boring location and elevation surveyed by AEP. - Datum: Ohio State Plane South NAD 27/NAVD 29 (Plant Grid). 70-75 80-85 [∟] 90-SYMBOLS USED TO INDICATE TEST RESULTS ₹ Drill Rod Energy Ratio: 0.75 G - Gradation
Q - Uncon Comp
T - Triax Comp 27.1 See H - Penetrometer (tsf) WATER NOTE: Inside Well **Last Calibration Date:** 8/2/2013 Separate W - Unit Dry Wt (pcf) - Triax Comp D - Relative Dens (%) DATE: 12/15/15 Curves **Drill Rig Number:** S&ME ATV 550-2



Well Development:

12/10 - Bailed 175 gallons of water (approx. 41 well volumes) via submersible pump. Water level stayed steady during pumping. NTU = 7 at 155 gallons, but increased to NTU = 12 upon terminating pump. Bailed additional 20 gallons during which initial NTU readings were intially high but decreased to NTU = 25.4.

- -Water level measurement on 12/15 was immediately before slug testing.
- -Top cover set in 3'x3' concrete pad. Protective steel bollards placed around concrete pad.

Water Quality Readings (Horiba U-52)

Cumulative Gallons	NTU	С	ms/cm	PH	ORPmV	
175	25.4	18.09	1.31	7.15	-6	
Location: N. 820,305.3' E	E. 2,513,92	27.4'	Datum:	NAD27/N	IGVD29 OH S	

WELL COMPLETION DIAGRAM

Project Name:

AEP CD Bottom Ash Pond Monitoring Wells

Project Location:

Cardinal Plant / Brilliant, Ohio

Project Number:

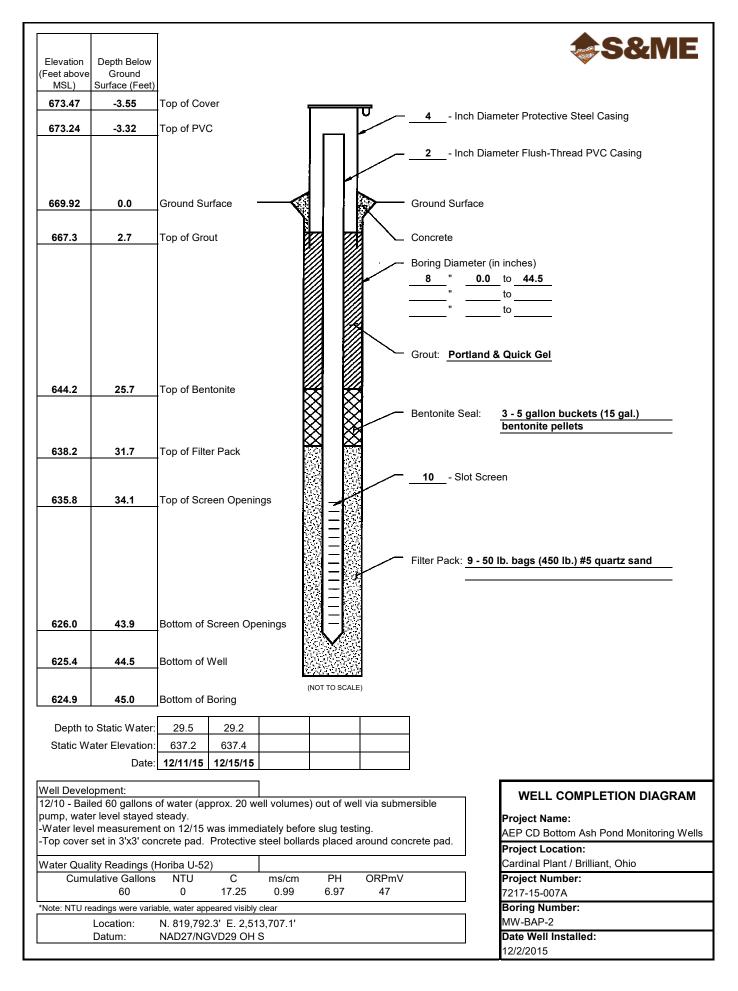
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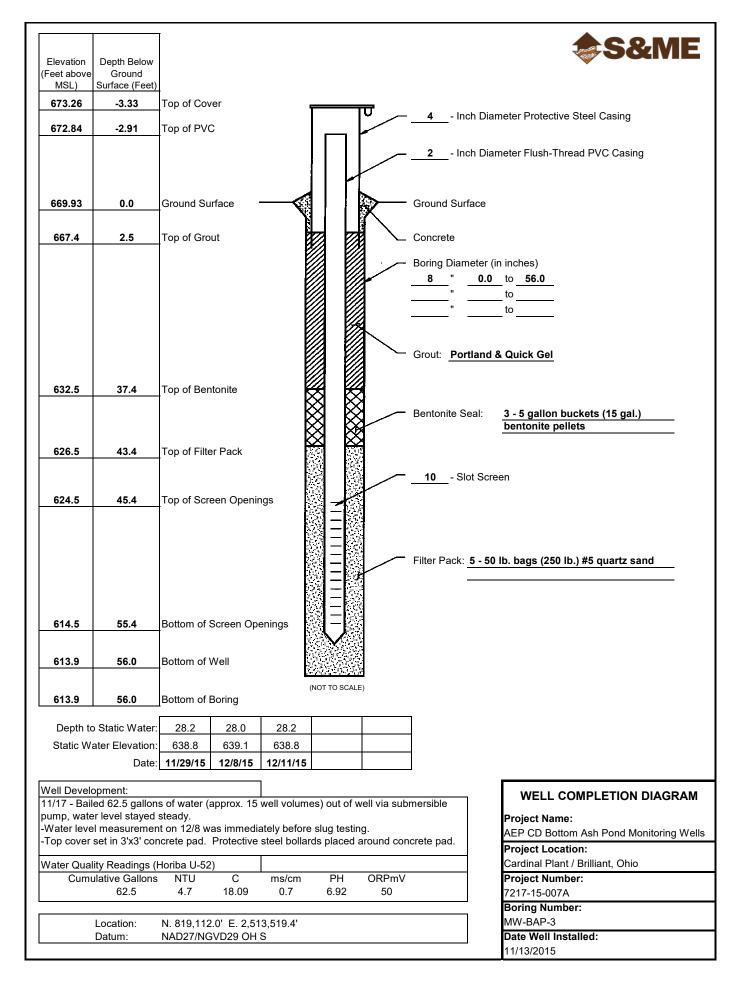
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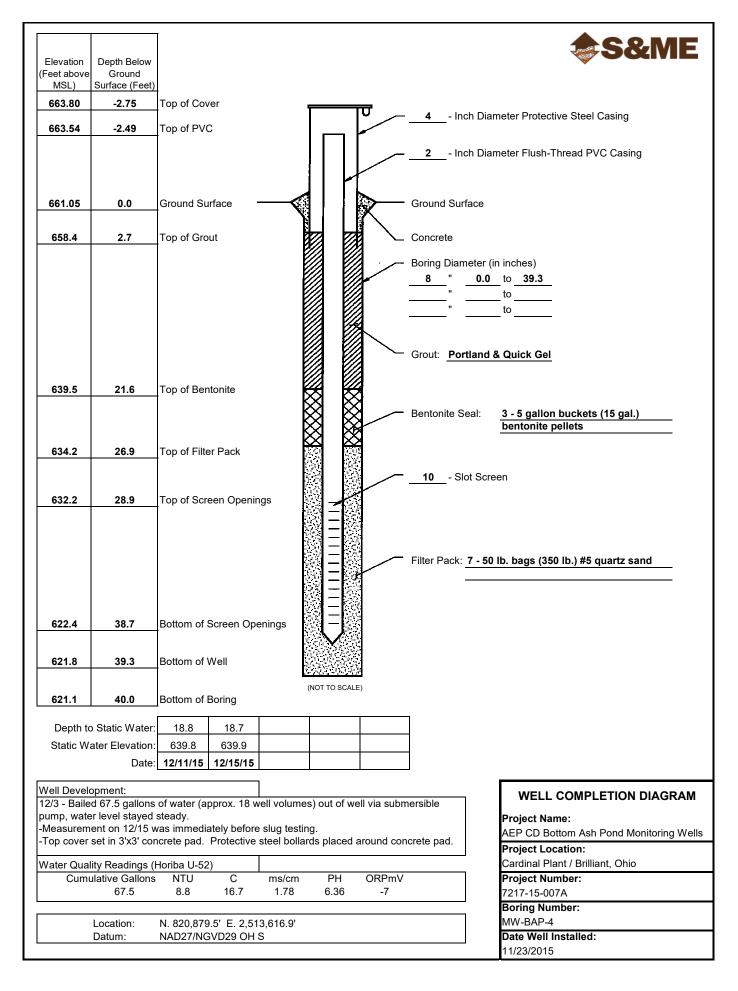
MW-BAP-1

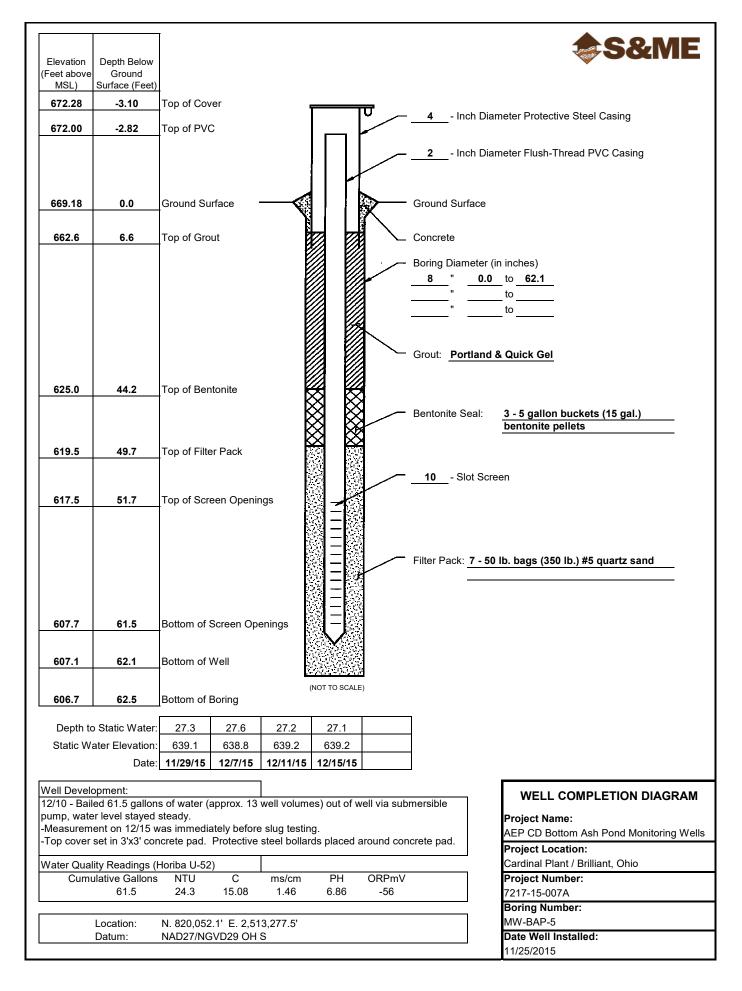
Date Well Installed:

12/10/2015



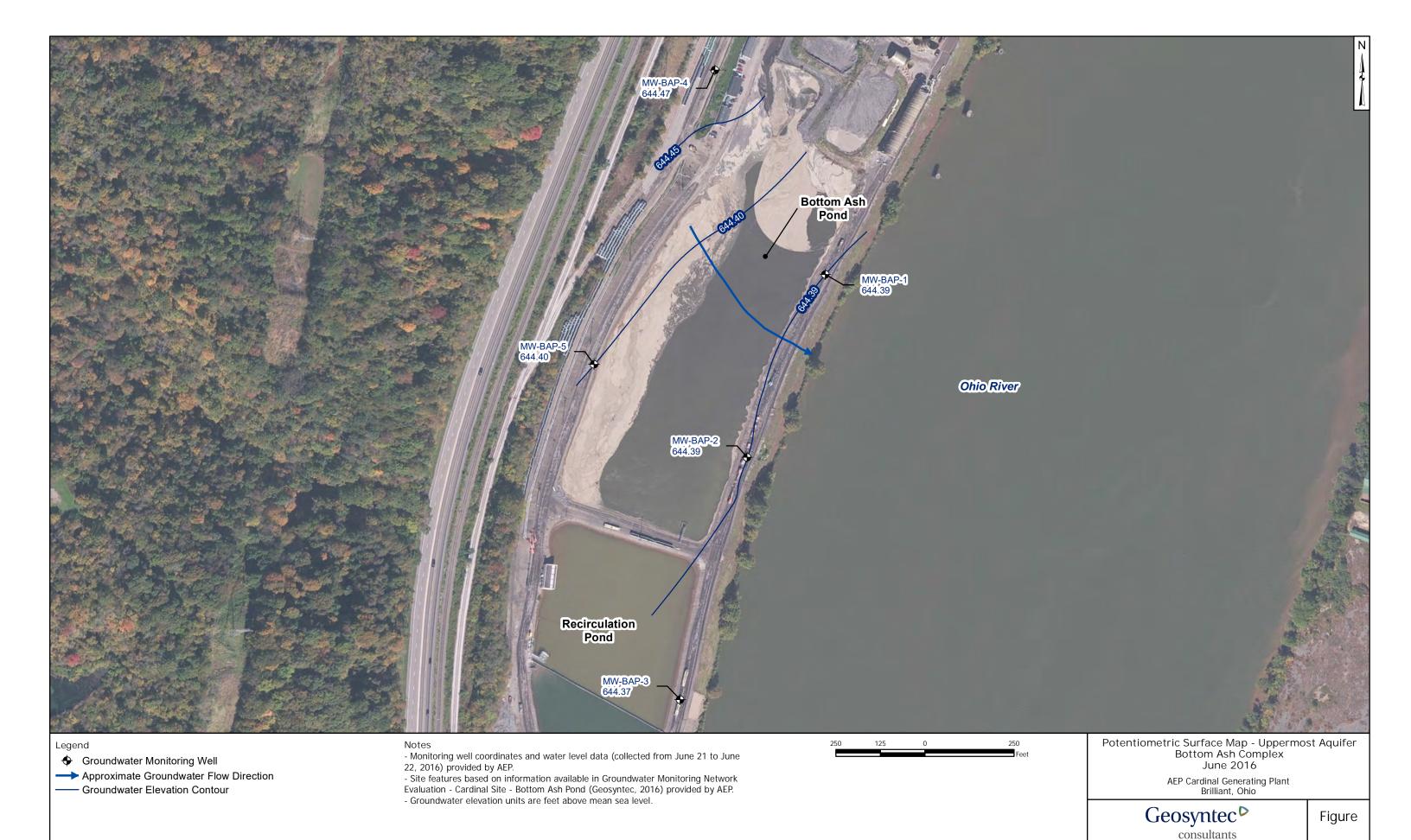






Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(2)(iii)

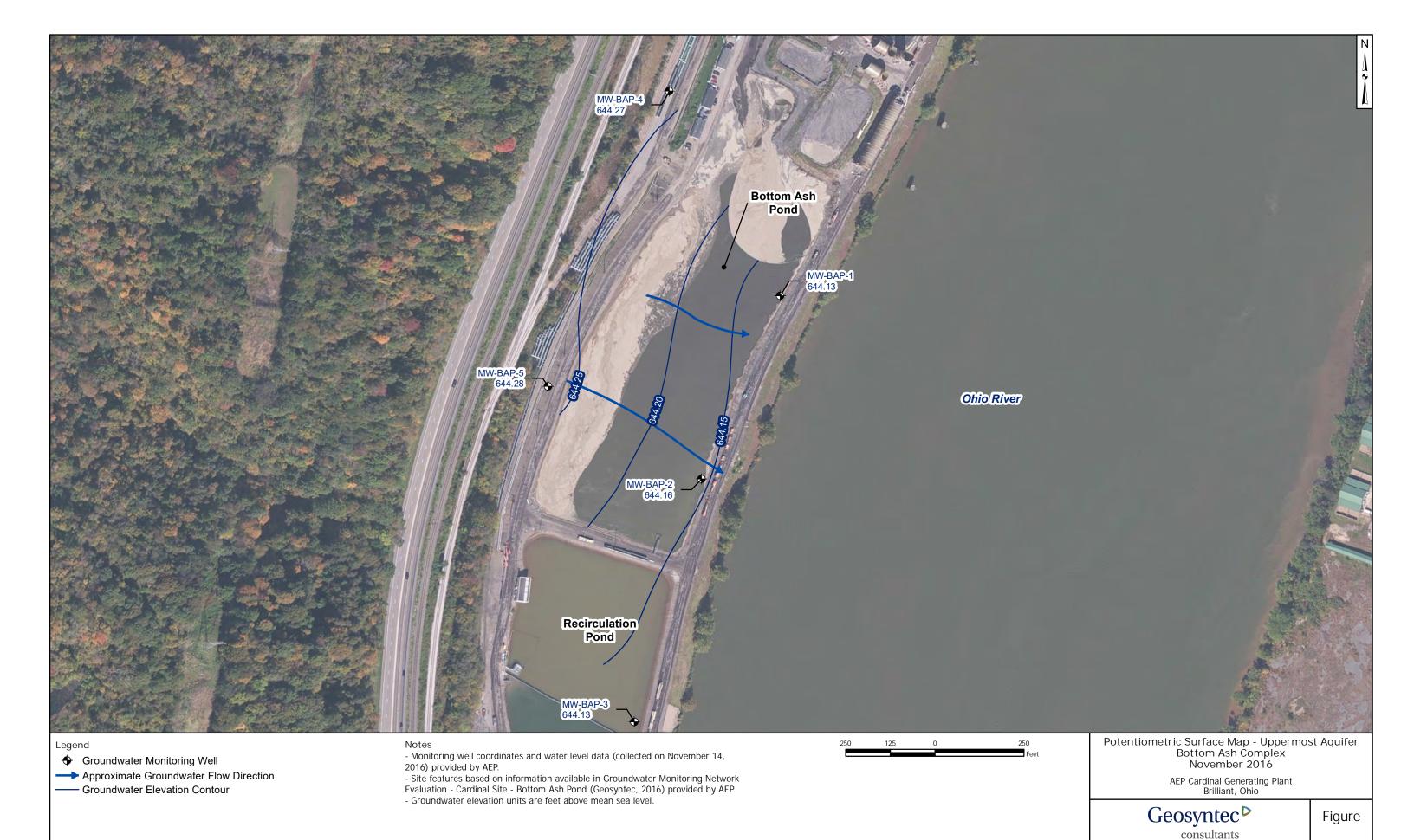
Maps that characterize the direction of groundwater flow accounting for seasonal variations



Columbus, Ohio

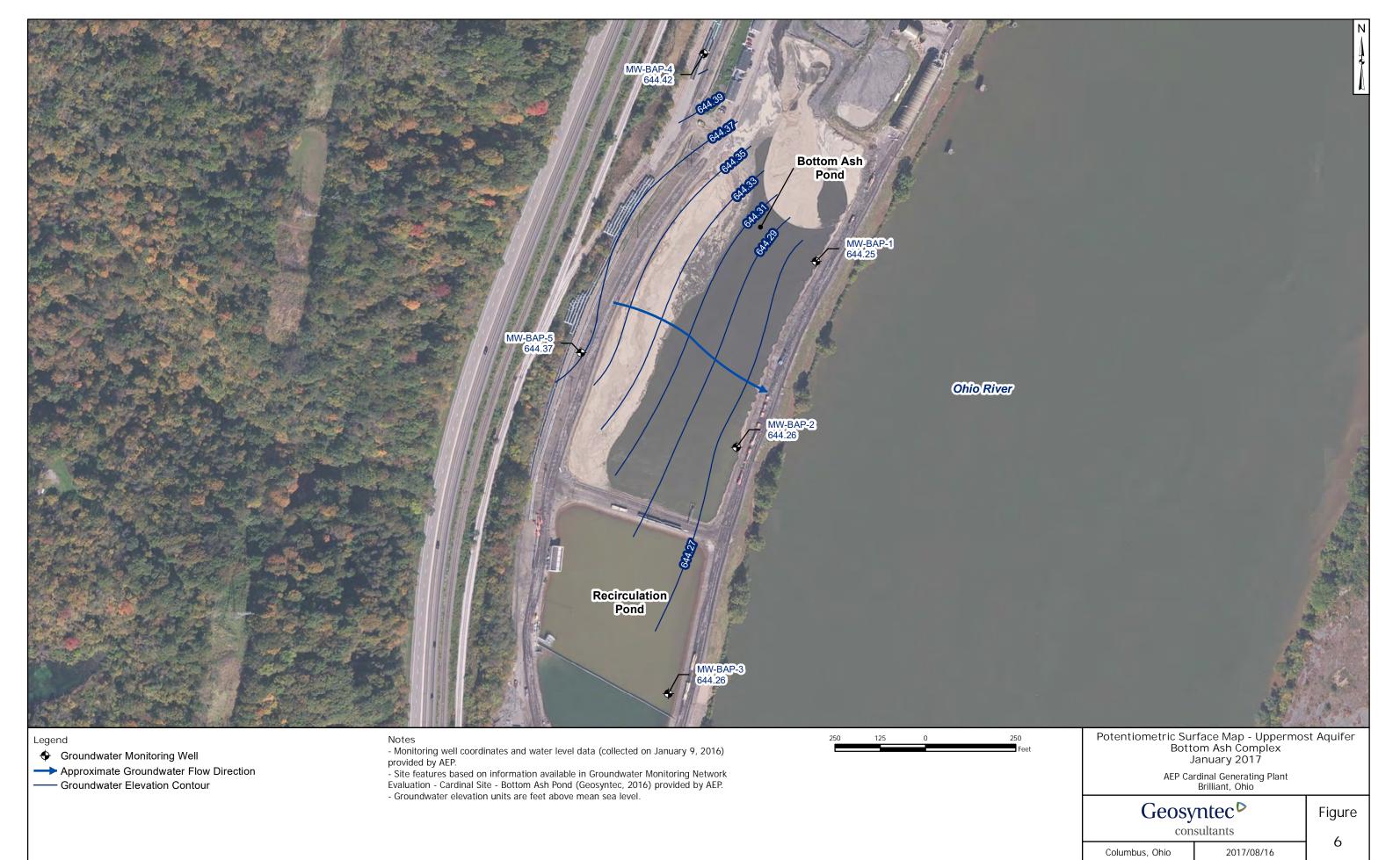
2017/08/16

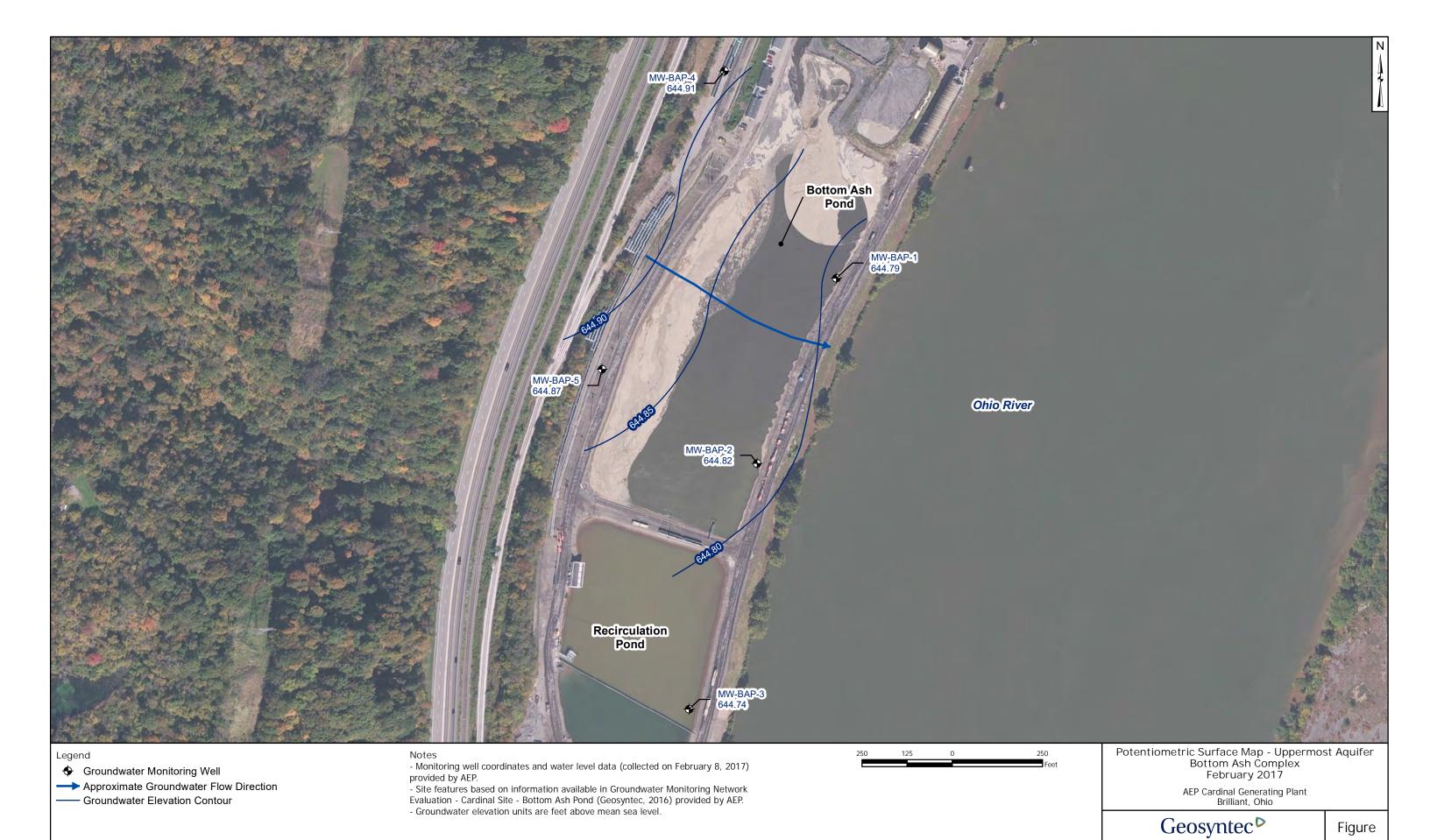




Columbus, Ohio

2017/08/16

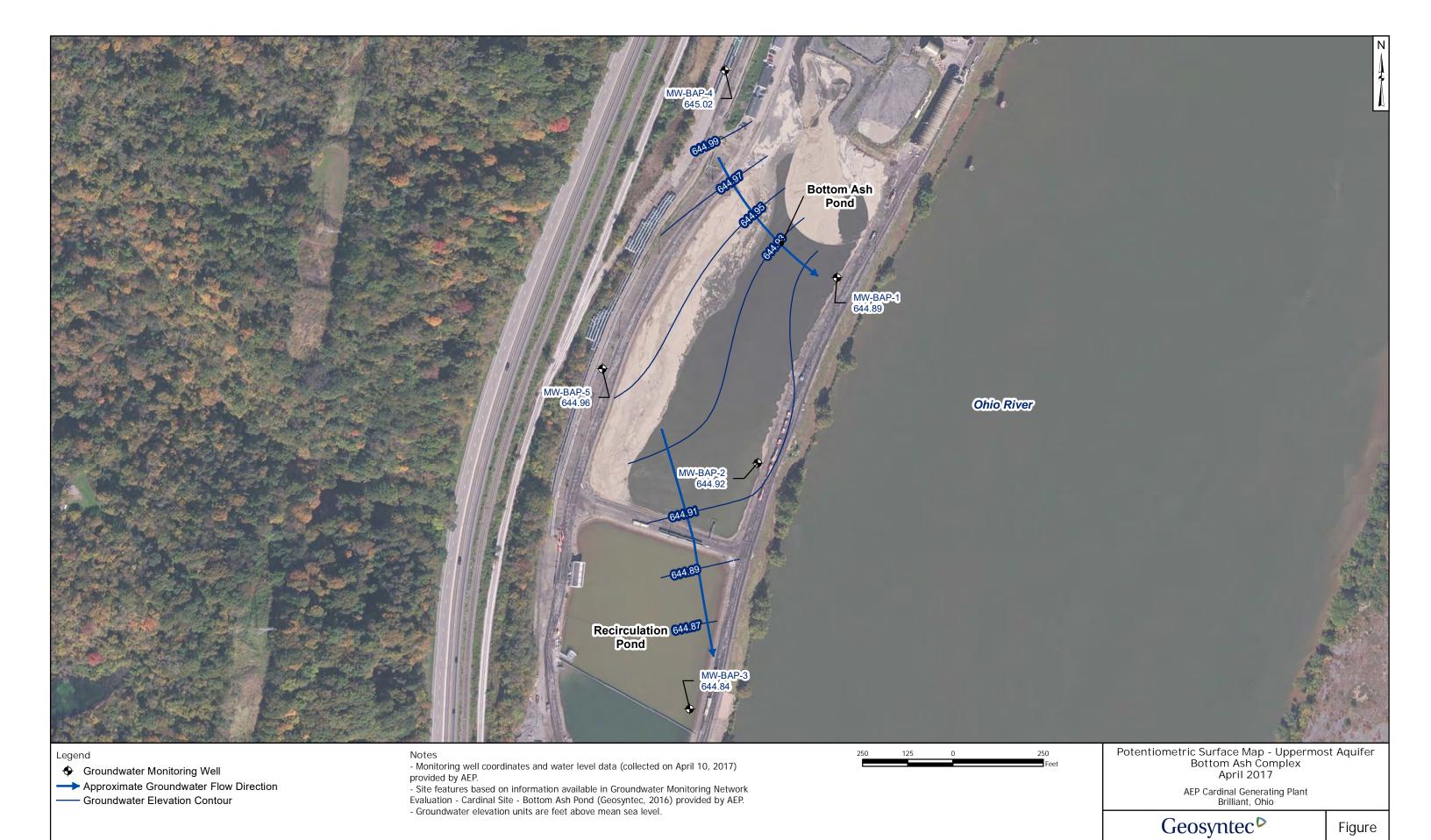




consultants

2017/08/16

Columbus, Ohio

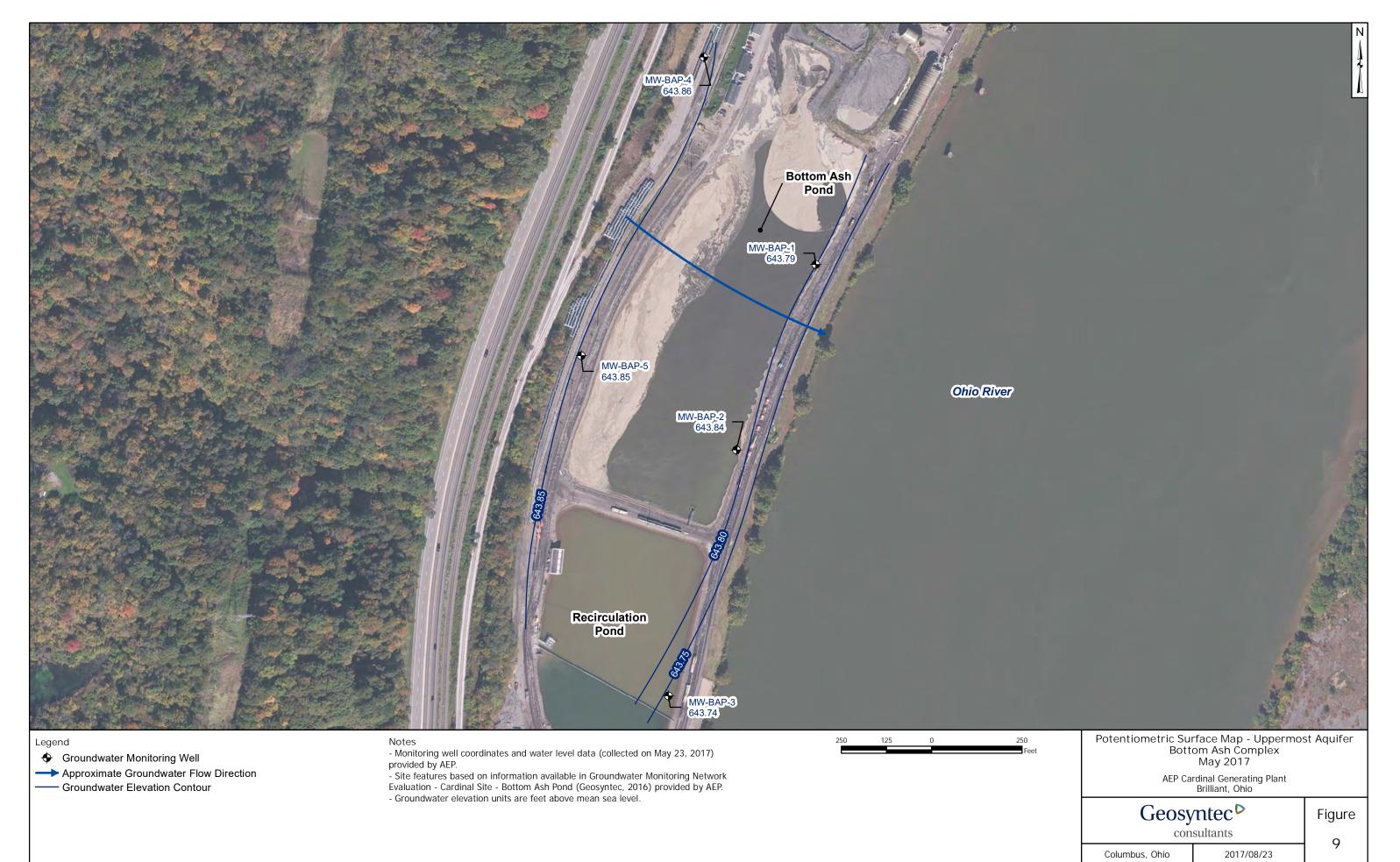


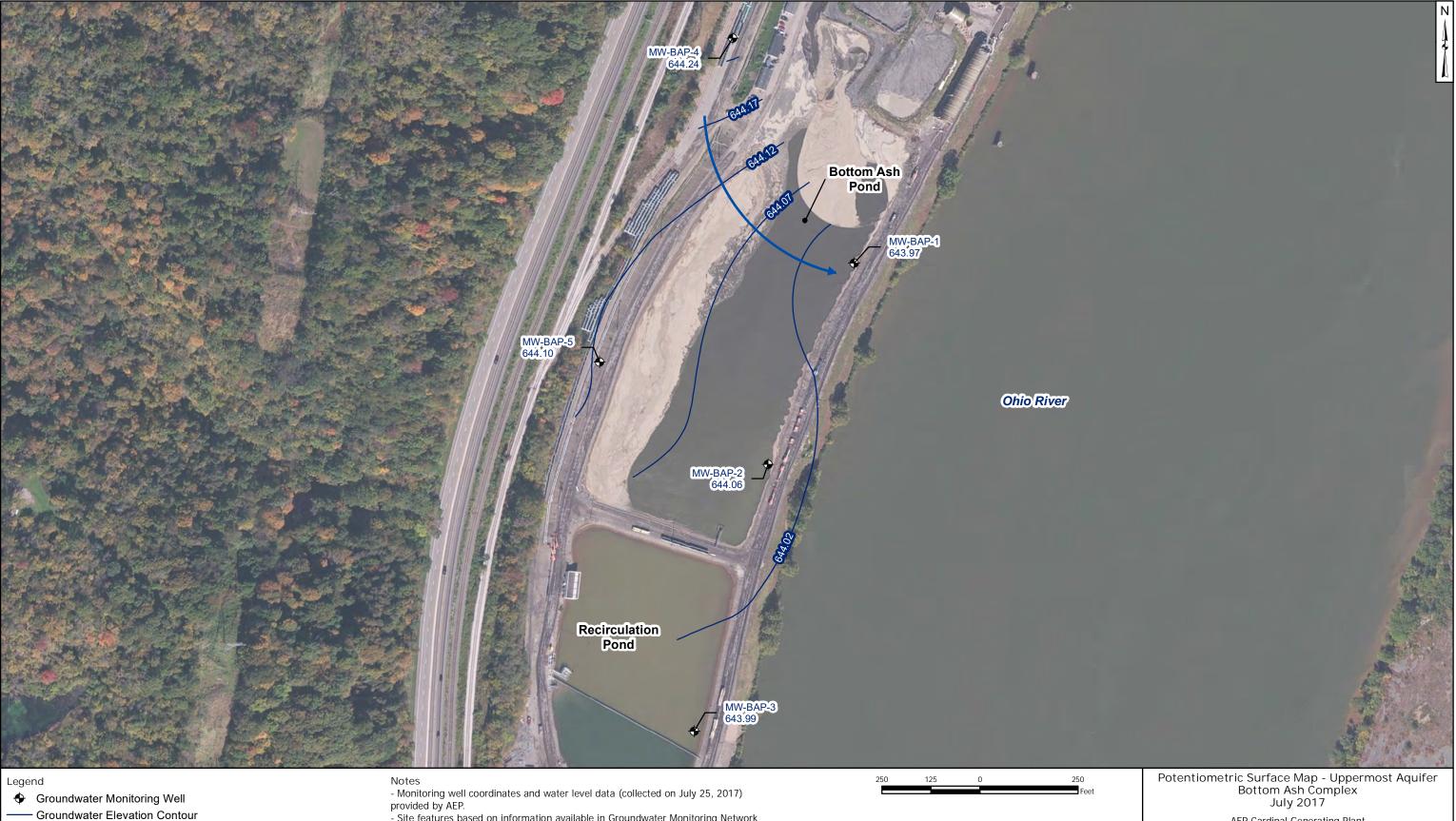
consultants

2017/08/23

Columbus, Ohio

8





→ Approximate Groundwater Flow Direction

- Site features based on information available in Groundwater Monitoring Network Evaluation - Cardinal Site - Bottom Ash Pond (Geosyntec, 2016) provided by AEP. - Groundwater elevation units are feet above mean sea level.

AEP Cardinal Generating Plant Brilliant, Ohio

Geosyntec[▶] Figure consultants 11 Columbus, Ohio 2017/08/16

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Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(3)

Constituent concentrations, summarized in table form, at each groundwater monitoring well monitored during each sampling event

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

		MW-BAP-1									
Parameter	Unit	1/12/2017	10/20/2016	5/3/2017	5/31/2017	6/20/2017	6/28/2016	8/1/2017	8/10/2016	9/26/2017	
			Background								
Antimony	μg/L	0.06	0.08	0.07	0.04J	0.04J	0.07	0.03J	0.08	-	
Arsenic	μg/L	1.13	1.6	1.56	0.78	0.53	1.45	0.4	1.05	-	
Barium	μg/L	86.5	107	85.3	72.6	63.6	93.6	61.5	107	-	
Beryllium	μg/L	0.043	0.06	0.061	0.03	0.01J	0.072	0.01J	0.037	-	
Boron	mg/L	1.95	1.73	2.27	2.11	2.4	1.71	2.69	1.83	2.7	
Cadmium	μg/L	0.13	0.11	0.15	0.12	0.1	0.12	0.09	0.11	-	
Calcium	mg/L	157	166	159	148	153	167	170	162	175	
Chloride	mg/L	96.1	94.5	95.2	94.3	95.4	98.4	100	93.4	93.7	
Chromium	μg/L	1.45	2	2.1	0.811	0.355	1.8	0.185	1.3	-	
Cobalt	μg/L	1.1	1.29	1.3	0.951	0.74	1.49	0.665	1.2	-	
Combined Radium	pCi/L	1.093	1.238	0.301	1.174	0.602	0.343	0.452	0.21	-	
Fluoride	mg/L	0.34	0.35	0.33	0.3	0.3	0.38	0.41	0.33	0.33	
Lead	μg/L	1.24	1.69	1.72	0.786	0.314	2.09	0.073	1.03	-	
Lithium	mg/L	0.021	0.015	0.02	0.017	0.029	0.035	0.022	0.019	-	
Mercury	μg/L	0.005U	0.007	0.006	0.004J	0.005U	0.01	0.003J	0.005U	-	
Molybdenum	μg/L	26.4	28.6	26.8	27.4	29	19.6	29.2	27.5	-	
Selenium	μg/L	0.2	0.4	0.3	0.1	0.06J	0.2	0.04J	0.2	-	
Total Dissolved Solids	mg/L	918	942	948	952	957	953	926	916	977	
Sulfate	mg/L	405	407	411	419	458	402	471	397	469	
Thallium	μg/L	0.071	0.226	0.058	0.059	0.05J	0.05	0.05J	0.122	-	
рH	SU	7.06	7.08	6.98	7.62	7.28	7.06	6.94	7.17	6.76	

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

		MW-BAP-2									
Parameter	Unit	1/12/2017	10/20/2016	5/3/2017	5/31/2017	6/20/2017	6/28/2016	8/1/2017	8/10/2016	9/5/2017	9/26/2017
		Background								Detection	
Antimony	μg/L	0.03J	0.1	0.05J	0.03J	0.03J	0.07	0.03J	0.04J	0.03J	-
Arsenic	μg/L	26	29.6	10.6	13.1	11.1	11.3	17.1	11.1	9.08	-
Barium	μg/L	104	123	104	106	91.5	94.3	93.8	89.5	78.4	-
Beryllium	μg/L	0.035	0.083	0.032	0.02J	0.01J	0.02J	0.02J	0.02J	0.01J	-
Boron	mg/L	2.08	1.79	2.2	2.09	2.16	2.28	1.95	2.04	1.75	1.73
Cadmium	μg/L	0.05	0.09	0.04	0.04	0.02J	0.04	0.02	0.03	0.03	-
Calcium	mg/L	86.4	92.3	82.4	87.6	84.6	98.7	86	89.5	81.6	86.8
Chloride	mg/L	72.9	79.6	72	70.7	71.9	74.1	71.4	75.9	69.1	68.2
Chromium	μg/L	0.65	1.8	0.704	0.292	0.213	0.5	0.371	0.3	0.217	-
Cobalt	μg/L	1.59	2.17	1.61	1.37	1.21	1.52	1.2	1.36	1.06	-
Combined Radium	pCi/L	0.776	0.849	0.376	1.206	0.993	0.749	1.086	0.588	0.731	-
Fluoride	mg/L	0.62	0.79	0.42	0.33	0.34	0.35	0.46	0.33	0.35	0.33
Lead	μg/L	0.965	2.16	0.77	0.325	0.234	0.439	0.33	0.307	0.197	-
Lithium	mg/L	0.016	0.006	0.013	0.009	0.02	0.011	0.01	0.01	0.013	-
Mercury	μg/L	0.002J	0.004J	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	26.2	31.9	42.1	46.6	49	37.6	46.1	38.4	42.7	-
Selenium	μg/L	0.1	0.4	0.2	0.09J	0.07J	0.09J	0.08J	0.08J	0.09J	-
Total Dissolved Solids	mg/L	583	628	557	562	563	612	560	544	538	552
Sulfate	mg/L	176	190	213	222	234	239	218	228	226	230
Thallium	μg/L	0.03J	0.075	0.03J	0.02J	0.02J	0.03J	0.02J	0.03J	0.03J	-
pН	SU	6.73	6.76	6.85	7.15	7.1	6.75	6.74	6.31	-	6.94

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

-: Not sampled

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

						MW-	BAP-3				
Parameter	Unit	1/12/2017	10/20/2016	5/3/2017	5/31/2017	6/20/2017	6/28/2016	8/1/2017	8/11/2016	9/5/2017	9/26/2017
						Background					Detection
Antimony	μg/L	0.03J	0.02J	0.02J	0.02J	0.02J	0.03J	0.02J	0.04J	0.04J	-
Arsenic	μg/L	0.99	0.69	0.39	0.36	0.32	0.42	0.31	0.75	0.74	-
Barium	μg/L	52.2	55.8	47.7	51.7	46.7	49.1	47.4	65.3	66.4	-
Beryllium	μg/L	0.009J	0.009J	0.006J	0.005J	0.02U	0.008J	0.005J	0.022	0.036	-
Boron	mg/L	1.77	1.8	1.87	1.91	2.05	1.92	2.12	2.03	1.99	2.03
Cadmium	μg/L	0.07	0.05	0.06	0.1	0.09	0.04	0.08	0.05	0.17	-
Calcium	mg/L	62.6	65.7	60.6	60.3	62.1	64.1	67	63	65.6	69.1
Chloride	mg/L	60.7	60.1	61.9	61.8	62.8	59.8	63.4	58.8	63.5	63.8
Chromium	μg/L	0.427	0.4	0.257	0.128	0.111	0.5	0.126	0.8	1.05	-
Cobalt	μg/L	0.779	0.759	0.721	0.675	0.591	0.759	0.579	0.962	0.92	-
Combined Radium	pCi/L	0.546	1.738	0.853	0.506	0.373	0.358	0.00513	0.76	0.767	-
Fluoride	mg/L	0.16	0.1J	0.16	0.1J	0.1J	0.17	0.1J	0.1J	0.1J	0.1
Lead	μg/L	0.216	0.184	0.091	0.088	0.065	0.164	0.066	0.487	0.814	-
Lithium	mg/L	0.012	0.001U	0.003	0.001U	0.013	0.018	0.005	0.005	0.007	-
Mercury	μg/L	0.003J	0.002J	0.005U	0.005U	0.007	0.002J	0.005U	0.003J	0.004J	-
Molybdenum	μg/L	2.7	2.45	3.57	2.51	2.21	2.13	1.87	5.63	1.8	-
Selenium	μg/L	0.03J	0.07J	0.06J	0.1U	0.1U	0.05J	0.1U	0.09J	0.1	-
Total Dissolved Solids	mg/L	390	396	402	410	421	418	424	400	417	421
Sulfate	mg/L	119	129	131	135	145	130	148	134	142	146
Thallium	μg/L	0.05J	0.059	0.04J	0.05J	0.05J	0.05J	0.05J	0.061	0.052	-
pН	SU	6.67	6.7	6.74	7.22	6.95	6.65	6.52	6.7	-	6.53

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

						MW-BAP-4					
Parameter	Unit	1/12/2017	10/20/2016	5/2/2017	5/31/2017	6/20/2017	6/30/2016	8/1/2017	8/10/2016	9/26/2017	
		Background Detection									
Antimony	μg/L	0.09	0.1	0.05J	0.04J	0.03J	0.06	0.05	0.07	1	
Arsenic	μg/L	44.8	42.4	41.9	35.9	42.7	36.3	43.7	42.2	1	
Barium	μg/L	59.9	69.8	44.9	51.7	41.9	54.9	49.9	54.7	-	
Beryllium	μg/L	0.176	0.227	0.071	0.111	0.046	0.119	0.092	0.117	-	
Boron	mg/L	0.02	0.064	0.16	0.024	0.038	0.115	0.034	0.062	0.033	
Cadmium	μg/L	0.14	0.18	0.05	0.1	0.03	0.11	0.06	0.1	1	
Calcium	mg/L	197	214	197	181	190	233	202	220	203	
Chloride	mg/L	27.5	28.6	27.5	27.6	27.5	30	27.6	30.6	27.1	
Chromium	μg/L	4.16	4.4	1.48	1.96	0.834	1.7	1.89	2.4	ı	
Cobalt	μg/L	20.3	19.8	19.2	20.2	18	18.7	19.9	18.2	1	
Combined Radium	pCi/L	0.703	1.17	0.377	0.599	0.645	0.535	1.069	0.722	1	
Fluoride	mg/L	0.1J	0.1J	0.1J	0.1J	0.1J	0.15	0.1J	0.16	0.1	
Lead	μg/L	4.63	5.67	1.66	2.94	0.955	3.2	2.06	2.78	1	
Lithium	mg/L	0.012	0.006	0.009	0.005	0.02	0.015	0.013	0.012	1	
Mercury	μg/L	0.005	0.007	0.005U	0.004J	0.005U	0.005U	0.005U	0.004J	1	
Molybdenum	μg/L	1.76	1.87	1.56	1	2.15	1.35	1.52	4.51	1	
Selenium	μg/L	0.7	0.9	0.3	0.4	0.2	0.5	0.4	0.5	1	
Total Dissolved Solids	mg/L	1200	1300	1250	1270	1280	1400	1330	1320	1250	
Sulfate	mg/L	620	617	584	590	655	661	631	629	618	
Thallium	μg/L	0.102	0.106	0.03J	0.03J	0.02J	0.03J	0.04J	0.063	-	
рН	SU	6.37	6.72	6.45	6.63	6.81	6.37	6.27	6.28	6.36	

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

						MW-BAP-5							
Parameter	Unit	1/12/2017	10/20/2016	5/2/2017	5/31/2017	6/20/2017	6/28/2016	8/1/2017	8/10/2016	9/26/2017			
			Background Detection										
Antimony	μg/L	0.06	0.12	0.07	0.05	0.03J	0.07	0.03J	0.09	-			
Arsenic	μg/L	8.78	16.1	11.5	11.7	9.1	11.3	10.6	12.1	-			
Barium	μg/L	87.9	118	88.2	95.3	77.7	92.7	83.1	102	-			
Beryllium	μg/L	0.061	0.157	0.095	0.075	0.045	0.068	0.039	0.112	-			
Boron	mg/L	0.043	0.058	0.116	0.073	0.05	0.072	0.043	0.043	0.059			
Cadmium	μg/L	0.02	0.06	0.04	0.03	0.02J	0.03	0.01J	0.05	-			
Calcium	mg/L	207	226	201	176	200	228	206	209	209			
Chloride	mg/L	15.3	14.3	14.8	13.3	15.7	13.4	14.7	13.5	15.3			
Chromium	μg/L	2.35	5.7	2.83	2.1	1.33	2	1.16	3.4	-			
Cobalt	μg/L	1.34	3.06	1.92	1.47	0.966	1.28	0.855	2.03	-			
Combined Radium	pCi/L	1.411	1.497	0.364	0.894	0.788	0.6516	0.686	1.026	-			
Fluoride	mg/L	0.09J	0.08	0.1J	0.06J	0.08J	0.1J	0.08J	0.09J	0.09			
Lead	μg/L	1.72	4.6	2.77	1.95	1.18	1.92	1.04	3.08	-			
Lithium	mg/L	0.008	0.007	0.01	0.012	0.016	0.02	0.012	0.01	-			
Mercury	μg/L	0.005U	0.003J	0.005U	0.005U	0.005U	0.005U	0.005U	0.003J	-			
Molybdenum	μg/L	0.74	1.15	0.62	0.94	0.52	0.8	0.52	1.22	-			
Selenium	μg/L	0.2	0.7	0.4	0.3	0.2	0.2	0.1	0.4	-			
Total Dissolved Solids	mg/L	1050	1010	1010	955	1080	1050	1050	1060	1050			
Sulfate	mg/L	474	433	418	404	472	449	448	456	442			
Thallium	μg/L	0.058	0.114	0.059	0.04J	0.03J	0.03J	0.02J	0.059	-			
рН	SU	6.6	6.59	6.6	7.07	6.94	6.6	6.55	6.7	6.72			

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

			BAP-1			BAP-2			BAP-3		BA	P-4	BA	P-5
Parameter	Unit	1/23/2018	5/17/2018	8/29/2018	1/23/2018		8/29/2018	1/23/2018		8/29/2018				
		Detection	Asses	sment	Detection	Asses	sment	Detection	Asses	sment	Asses	sment	Asses	sment
Antimony	μg/L	-	0.0400 J	0.5 U	-	0.0300 J	0.5 U	-	0.0200 J	0.5 U	0.0300 J	0.5 U	0.0400 J	0.5 U
Arsenic	μg/L	-	0.430	0.5 U	-	12.4	122	-	0.270	0.5 U	34.1	44.2	7.78	6.20
Barium	μg/L	-	56.0	57.6	-	92.3	135	-	48.1	46.8	38.8	49.7	72.1	78.7
Beryllium	μg/L	-	0.0100 J	0.1 U	-	0.0200 J	0.1 U	-	$0.00800 \mathrm{J}$	0.1 U	0.0360	0.100	0.0500	0.1 U
Boron	mg/L	2.91	2.70	3.44	1.97	1.57	1.92	1.91	1.97	2.45	0.137	0.0217	0.112	0.0956
Cadmium	μg/L	-	0.100	0.140	-	0.0200	0.1 U	-	0.110	0.1 U	0.0200	0.1 U	$0.0200 \mathrm{J}$	0.1 U
Calcium	mg/L	-	159	153	-	82.0	79.5	-	66.8	69.4	202	216	203	222
Chloride	mg/L	86.2	76.9	74.4	61.1	60.0	70.0	64.1	67.2	67.2	27.7	28.5	17.0	19.2
Chromium	μg/L	-	0.598	1 U	-	0.345	1 U	-	0.270	1 U	0.715	2.10	1.45	1 U
Cobalt	μg/L	-	0.649	0.790	-	1.16	1.30	-	0.521	0.5 U	19.1	20.1	0.950	0.770
Combined Radium	pCi/L	-	0.227	0.686	-	0.643	0.225	-	0.385	0.312	0.987	1.06	0.865	1.01
Fluoride	mg/L	0.370	0.380	0.360	0.390	0.490	0.620	-	0.130	0.110	0.160	0.140	0.0900	0.0930
Lead	μg/L	-	0.246	0.5 U	-	0.217	0.5 U	-	0.0720	0.5 U	0.601	1.70	1.19	0.540
Lithium	mg/L	-	0.0100	0.0166	-	0.00400	10 U	-	0.001 U	10 U	0.00600	10 U	0.00300	10 U
Mercury	μg/L	-	0.00300 J	0.00126	-	0.005 U	0.000930	-	0.005 U	0.5 U	0.005 U	0.00266	0.005 U	0.00123
Molybdenum	μg/L	-	27.4	30.6	-	37.4	36.3	-	1.73	1.50	1.31	1.50	0.460	0.510
рН	SU	7.09	7.04	6.96	6.90	6.81	6.86	6.71	6.48	6.59	6.26	6.32	6.48	6.56
Selenium	μg/L	-	0.100	0.5 U	-	0.100 J	0.5 U	-	0.0400 J	0.5 U	0.200	0.5 U	0.200	0.5 U
Total Dissolved Solids	mg/L	-	924	927	-	518	519	-	416	415	1260	1240	1030	974
Sulfate	mg/L	-	446	494	-	228	217	-	157	159	590	628	433	464
Thallium	μg/L	-	0.0610	0.5 U	-	0.0300 J	0.5 U	-	0.0680	0.5 U	0.0500 J	0.5 U	0.0300 J	0.5 U

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

Table 1 - Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

Donomoton	Unit	BA	P-1	BA	P-2	BA	P-3	BA	P-4	BA	P-5
Parameter	Onit	4/8/2019	10/9/2019	4/8/2019	10/9/2019	4/8/2019	10/10/2019	4/8/2019	10/10/2019	4/8/2019	10/10/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Arsenic	μg/L	0.500 U	0.500 U	122	34.9	0.500 U	0.500 U	39.0	54.8	5.20	5.80
Barium	μg/L	52.3	50.0	225	121	44.4	44.3	42.4	47.1	77.4	83.4
Beryllium	μg/L	0.100 U	0.100 U	0.260	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U
Boron	μg/L	2,680	3,050	1,960	1,560	2,020	2,100	19.8	19.5	92.0	118
Cadmium	μg/L	0.130	0.120	0.230	0.100 U	0.100 U	0.100	0.100 U	0.100 U	0.100 U	0.100 U
Calcium	μg/L	167,000	158,000	91,100	82,800	76,000	71,900	209,000	184,000	224,000	213,000
Chloride	mg/L	64.7	68.9	59.4	64.5	64.6	68.4	20.9	25.3	14.9	16.7
Chromium	μg/L	1.00 U	1.00 U	5.50	1.00 U	1.00 U	1.00 U	1.20	1.70	1.00 U	2.20
Cobalt	μg/L	1.00	0.700	4.60	1.20	0.570	0.500 U	17.8	19.1	1.00	1.10
Combined Radium	pCi/L	1.10	6.52	0.617	1.06	0.552	0.371	0.564	1.48	0.765	1.27
Fluoride	mg/L	0.380	0.370	0.800	0.560	0.140	0.110	0.150	0.140	0.0990	0.0680
Lead	μg/L	0.500 U	0.500 U	5.30	0.500 U	0.500 U	0.500 U	1.20	1.40	1.10	1.20
Lithium	μg/L	17.1	19.8	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U
Mercury	μg/L	0.000500 U	0.000500 U	0.00965	0.000670	0.000500 U	0.000500 U	0.00186	0.00117	0.00123	0.000785
Molybdenum	μg/L	30.4	32.3	36.3	40.0	1.30	1.60	1.30	1.40	0.500 U	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.570	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Sulfate	mg/L	419	416	167	202	149	164	471	560	404	433
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Total Dissolved Solids	mg/L	905	874	563	484	415	425	1,260	1,210	1,050	983
рН	SU	6.82	7.10	7.12	6.95	6.53	6.05	6.35	6.26	6.65	6.43

Notes:

mg/L: milligrams per liter $\mu g/L$: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

All samples were collected as part of the assessment monitoring program in accordance with 40 CFR 257.90(e)(3).

Spring 2020 App III & IV Parameters Cardinal Plant - Bottom Ash Pond

Parameter	Unit	BAP-1	BAP-2	BAP-3	BAP-4	BAP-5
		4/08/2020	4/08/2020	4/08/2020	4/08/2020	4/08/2020
Antimony	μg/L	0.50 U				
Arsenic	μg/L	2.4	24.2	1.1	45.1	2.3
Barium	μg/L	89.1	160	83.6	42.8	80.1
Beryllium	μg/L	0.15	0.10 U	0.10 U	0.10 U	0.10 U
Boron	μg/L	2770	1860	1940	20.7	138
Cadmium	μg/L	0.15	0.10 U	0.15	0.10 U	0.10 U
Calcium	μg/L	147000	88000	69700	186000	234000
Chloride	mg/L	73.9	83.7	77.3	29	22.1
Chromium	μg/L	4.6	1.5	3.5	1.4	1.0 U
Cobalt	μg/L	2.3	1.8	1.9	19.6	0.99
Combined Radium	pCi/L	1.63	0.736	0.641	0.552	0.794
Fluoride	mg/L	0.38	0.58	0.12	0.11	0.08
Lead	μg/L	3.3	1.1	1.5	1.1	0.50 U
Lithium	μg/L	27.5	12.1	10.0 U	12.9	11.4
Mercury	μg/L	0.0137	0.00249	0.0084	0.00223	0.000734
Molybdenum	μg/L	29.9	35.2	2.7	1.4	0.50 U
pН	SU	6.82	6.67	6.36	6.31	6.47
Selenium	μg/L	0.50 U				
Total Dissolved Solids	mg/L	825	527	430	1170	1080
Sulfate	mg/L	389	208	158	637	511
Thallium	μg/L	0.50 U				

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter pCi/L: picocuries per liter SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

2019 ANNUAL GROUNDWATER MONITORING REPORT

FEDERAL CCR RULE

CARDINAL PLANT – BOTTOM ASH POND BRILLIANT, OHIO

Submitted to



Cardinal Operating Company

306 County Road 7E Brilliant, Ohio 43913

Submitted by



engineers | scientists | innovators

941 Chatham Lane, Suite 103 Columbus, Ohio 43221

January 10, 2020

CHA8468

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LIST OF ACRONYMS AND ABBREVIATIONS

BAP Bottom Ash Pond

CCR Coal Combustion Residuals

CFR Code of Federal Regulations

ESP Electrostatic Precipitator

FGD Flue Gas Desulfurization

GWPS Groundwater Protection Standards

MCL Maximum Contaminant Level

MW Megawatt

RSW Residual Solid Waste

SCR Selective Catalytic Reduction

SSI Statistically Significant Increase

SSL Statistically Significant Level

USEPA United States Environmental Protection Agency

1. INTRODUCTION

The Federal Coal Combustion Residuals (CCR) Rule (40 Code of Federal Regulations [CFR] Part 257.90(e)) (USEPA, 2015) requires owners and or operators of existing CCR landfills and surface impoundments to prepare a Groundwater Monitoring and Corrective Action Report (Report) no later than January 31 annually. Geosyntec Consultants (Geosyntec) has prepared this Report for the Bottom Ash Pond (BAP), an existing CCR unit at the Cardinal Plant in Brilliant, Ohio (Site). This Report summarizes the groundwater monitoring activities conducted pursuant to the CCR Rule through December 31, 2019.

2. SITE DESCRIPTION

2.1 Site Description

The Site is located one-mile south of Brilliant, Ohio in Jefferson County (**Figure 1**) and is operated by Buckeye Power, Inc. (Buckeye Power). Located along the Ohio River, the generating station consists of three coal-powered units with an 1,800 megawatt (MW) capacity and annual coal use of 5.2 million tons (Geosyntec, 2016). Units 1 and 2 began operation in 1967 and Unit 3 began operation in 1977. As of 2012, all three units were equipped with an electrostatic precipitator (ESP), a selective catalytic reduction (SCR) system, and a flue gas desulfurization (FGD) system.

The BAP is situated along the Ohio River south of Cardinal Plant Unit 3. The BAP perimeter dikes enclosing the facility are approximately 6,500 feet (ft) in length with a 20-foot average height. The dikes were originally constructed in the 1960s, with major reconstruction in 1974 as part of the Unit 3 addition. The BAP receives bottom ash, pyrite, and other wastes from the coal burning process in addition to stormwater drainage and wastewater flows from the property. Site features and locations are outlined in **Figure 2**.

2.2 Regional Physiographic Setting

The Site is underlain by horizontal sequences of lower Permian and upper Pennsylvanian sedimentary rock. The Conemaugh Group, 500 ft thick in Jefferson County, consists of shale, sandstone, limestone, claystone, and coal. This group includes the Morgantown Sandstone underlain by the Elk Lick Limestone, the Skelly Limestone and Shale, the Ames Limestone, and the Cow Run Sandstone (Geosyntec, 2016). Above the current grade of the Residual Solid Waste (RSW) Landfill lies the Monongahela Group consisting of shale, sandstone, limestone, coal, claystone, and siltstone. Overlying the Monongahela Group, at approximately 1,250 feet in elevation, is the Permian-age Dunkard Group.

The uppermost aquifer at the Site consists of fine to coarse sand and gravel below a silty clay, interbedded organic clay and silt. The uppermost aquifer is hydraulically connected to the Ohio River. Groundwater in the uppermost aquifer generally flows southeast towards the Ohio River

with hydraulic conductivity ranging from 1×10^{-1} to 1×10^{-4} centimeters per second (cm/s) (Geosyntec, 2016).

3. GROUNDWATER MONITORING SYSTEM

The BAP's groundwater monitoring network was designed to comply with 40 CFR 257.91. The groundwater monitoring network utilizes monitoring wells initially installed as part of a separate site-wide hydrogeologic investigation and is used to monitor groundwater quality in the uppermost aquifer at the Site. Monitoring well construction and soil boring logs were provided in the *Groundwater Monitoring Network Design Report* (Geosyntec, 2016).

The BAP groundwater monitoring well network consists of five monitoring wells, as shown in **Figure 2**. Two upgradient monitoring wells (MW-BAP-4 and MW-BAP-5) are used to measure background conditions and three downgradient monitoring wells (MW-BAP-1, MW-BAP-2, and MW-BAP-3) are used as compliance wells.

4. CCR RULE GROUNDWATER KEY ACTIVITIES COMPLETED

4.1 2018 Statistical Evaluation Activities

A Groundwater Protection Standard (GWPS) was established for each Appendix IV parameter in accordance with the United States Environmental Protection Agency (USEPA's) *Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities – Unified Guidance* (Unified Guidance; USEPA, 2009) and the Site's Statistical Analysis Plan (Geosyntec, 2017). The established GWPSs were determined to be the greater value of the background concentration and the maximum contaminant level (MCL) or risk-based screening level for each Appendix IV parameter. GWPSs determined in 2018 are provided in the *2018 Annual Groundwater Monitoring Report* (Geosyntec, 2019a).

A statistical evaluation of the 2018 assessment monitoring data compared against the GWPSs was completed in January 2019 and is described in the *Statistical Analysis Summary – Bottom Ash Pond* (Geosyntec, 2019b). The statistical analysis report included an evaluation of significant levels (SSLs) for Appendix IV parameters and an evaluation of statistically significant increases (SSIs) for Appendix III parameters. Additionally, prediction limits for interwell tests were recalculated using data collected during the 2018 assessment monitoring events. No SSLs were identified at the BAP. SSIs for boron and chloride were identified at MW-BAP-1, MW-BAP-2, and MW-BAP-3 and SSIs for fluoride were identified at MW-BAP-1 and MW-BAP-2 (Geosyntec, 2019b). Based on these results, the CCR unit remained in assessment monitoring.

4.2 2019 Sampling and Data Evaluation Activities

4.2.1 Assessment Monitoring Program

The BAP remained in assessment monitoring throughout 2019. Assessment monitoring sampling events were conducted in April and October 2019 in accordance with 40 CFR 257.95(b) and 40 CFR 257.95(d)(1), respectively. Samples from both events were analyzed for all Appendix III and Appendix IV parameters; results are shown in **Table 1**. A revision of the GWPS and statistical evaluation of the 2019 assessment monitoring data is ongoing and will be completed outside of the timeframe of this report.

4.2.2 Groundwater Elevation and Flow Velocities

Prior to sampling, a synoptic round of groundwater level measurements was collected from compliance and background monitoring wells. Potentiometric surface maps based on groundwater elevations measured during the April and October 2019 assessment monitoring events are presented in **Figure 3** and **Figure 4**, respectively. The potentiometric maps show that groundwater near the BAP flows southeast towards the Ohio River. The groundwater residence time (inverse of velocity) at the BAP ranged from 1.4 days at well MW-BAP-3 to 6.5 days at MW-BAP-2 and MW-BAP-3. A summary of hydraulic gradients and groundwater residence times at the BAP is provided in **Table 2**.

4.2.3 Data Usability

Upon receipt of laboratory analytical reports, the data were evaluated for usability. Analytical data were checked for the following:

- Samples were analyzed within the method specified hold times;
- Samples were received within holding temperature;
- The chain of custody form was complete;
- Precision was within control limits using relative percent differences of blind duplicate samples;
- Matrix spike and matrix spike duplicate recoveries and laboratory control samples were within the control limits; and
- Potential for positive bias was evaluated using method blanks.

All data received during 2019 were considered complete and usable.

5. PROBLEMS ENCOUNTERED AND RESOLUTIONS

No problems were encountered during 2019 that were related to assessment monitoring activities at the BAP. No monitoring wells were gauged dry, abandoned, or added to the well network during 2019. All analytical data received were deemed to be of acceptable quality.

6. STATUS OF MONITORING PROGRAM

During the time period of this report, the Site has remained in assessment monitoring. Assessment monitoring events were conducted in April and October 2019. The BAP's status will be reevaluated after completion of the ongoing statistical evaluation.

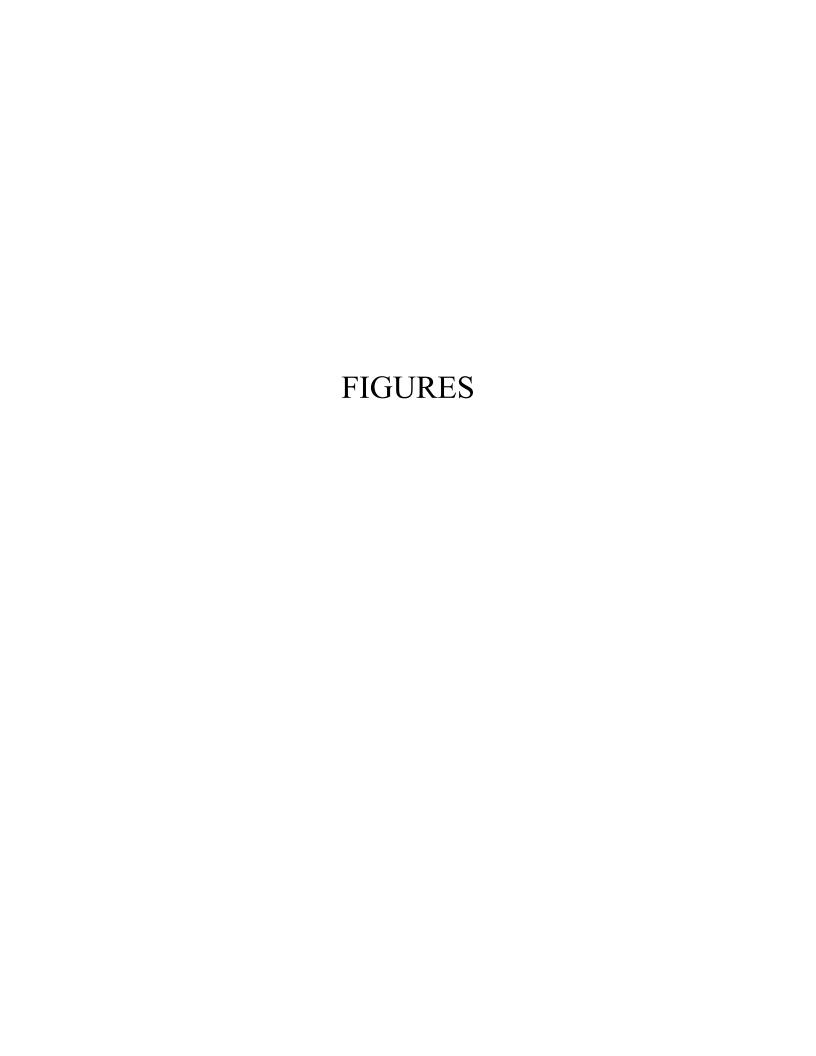
7. PLANNED KEY ACTIVITIES FOR 2020

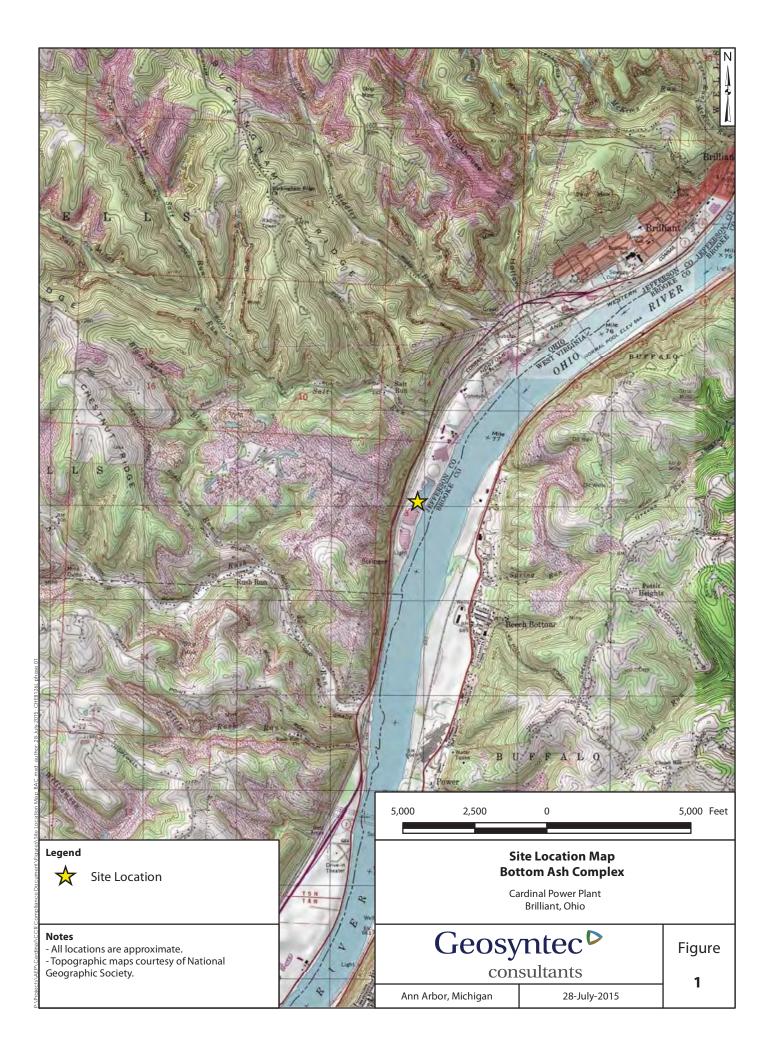
The following activities are planned for 2020 at the BAP:

- The 2019 Annual Groundwater Monitoring Report will be entered into the facility's operating record and posted to the public internet site;
- A statistical evaluation of the 2019 assessment monitoring event will be completed in January 2020, which will evaluate potential SSIs against revised GWPSs. The BAP's monitoring well status will be confirmed following the evaluation;
- Assuming the unit remains in assessment monitoring, two semi-annual groundwater assessment monitoring program events will be conducted and tested for potential SSLs and SSIs; and
- The 2020 Annual Groundwater Monitoring will be prepared for submittal in January 2021.

8. REFERENCES

- Geosyntec Consultants, Inc. 2016. Groundwater Monitoring Network Evaluation, Cardinal Site Bottom Ash Pond, July.
- Geosyntec Consultants, Inc. 2017. Statistical Analysis Plan. January.
- Geosyntec Consultants, Inc. 2019a. 2018 Annual Groundwater Monitoring Report, Federal CCR Rule, Cardinal Plant Bottom Ash Pond. January.
- Geosyntec Consultants, Inc. 2019b. Statistical Analysis Summary Bottom Ash Pond, Cardinal Plant. 2019.
- United States Environmental Protection Agency (USEPA). 2009. Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities Unified Guidance. EPA 530/R-09-007. March.
- United States Environmental Protection Agency (USEPA). 2015. Hazardous and Solid Waste Management System; Disposal of Coal Combustion Residuals from Electric Utilities (Final Rule). Fed. Reg. 80 FR 21301, pp. 21301-21501, 40 CFR Parts 257 and 261, April.







- Compliance Sampling Location
- Background Sampling Location

Bottom Ash Pond

- Monitoring well coordinates provided by Buckeye Power.
 Site features based on information available in Groundwater Monitoring Network Evaluation Cardinal Site Bottom Ash Pond (Geosyntec, 2016) provided by Buckeye Power.

Site Layout Bottom Ash Complex

Buckeye Power Cardinal Generating Plant Brilliant, Ohio

2018/01/25

Geosyntec D consultants

Columbus, Ohio

Figure 2

C:\Users\mmuenich\Documents\local_projects\AEP_GIS\Cardinal\MXDs\BAP\AEP-Cardinal_BAP_Site_Layout.mxd. MMuenich. 1/25/2018. CHA8423/01/08.



→ Approximate Groundwater Flow Direction

— Groundwater Elevation Contour

- - - Groundwater Elevation Contour (Inferred)

provided by Buckeye Power.

- Site features based on information available in Groundwater Monitoring Network Evaluation - Cardinal Site - Bottom Ash Pond (Geosyntec, 2016) provided by AEP.

- Groundwater elevation units are feet above mean sea level.

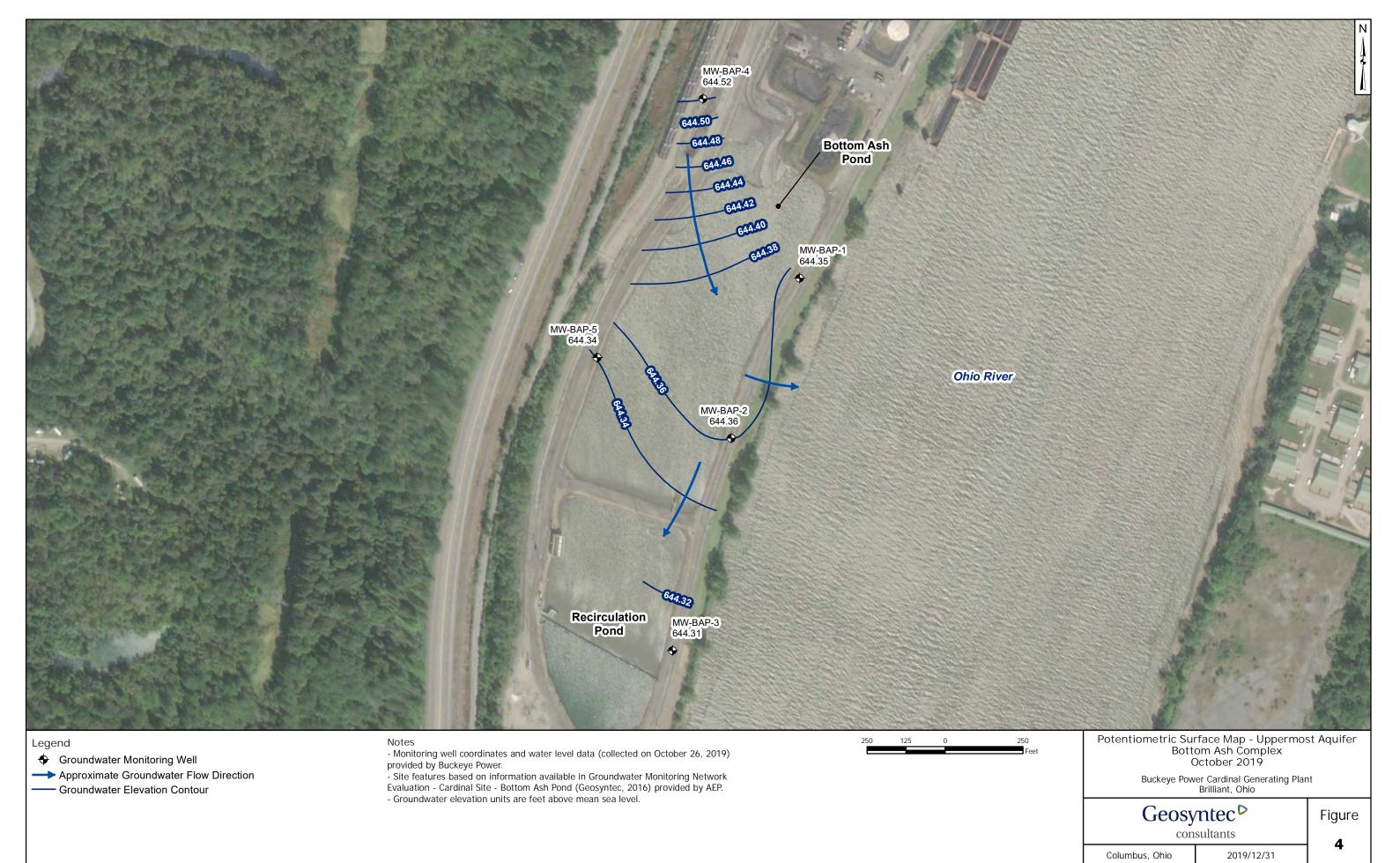
Buckeye Power Cardinal Generating Plant Brilliant, Ohio

Figure

3

Geosyntec[▶] consultants 2020/01/03 Columbus, Ohio

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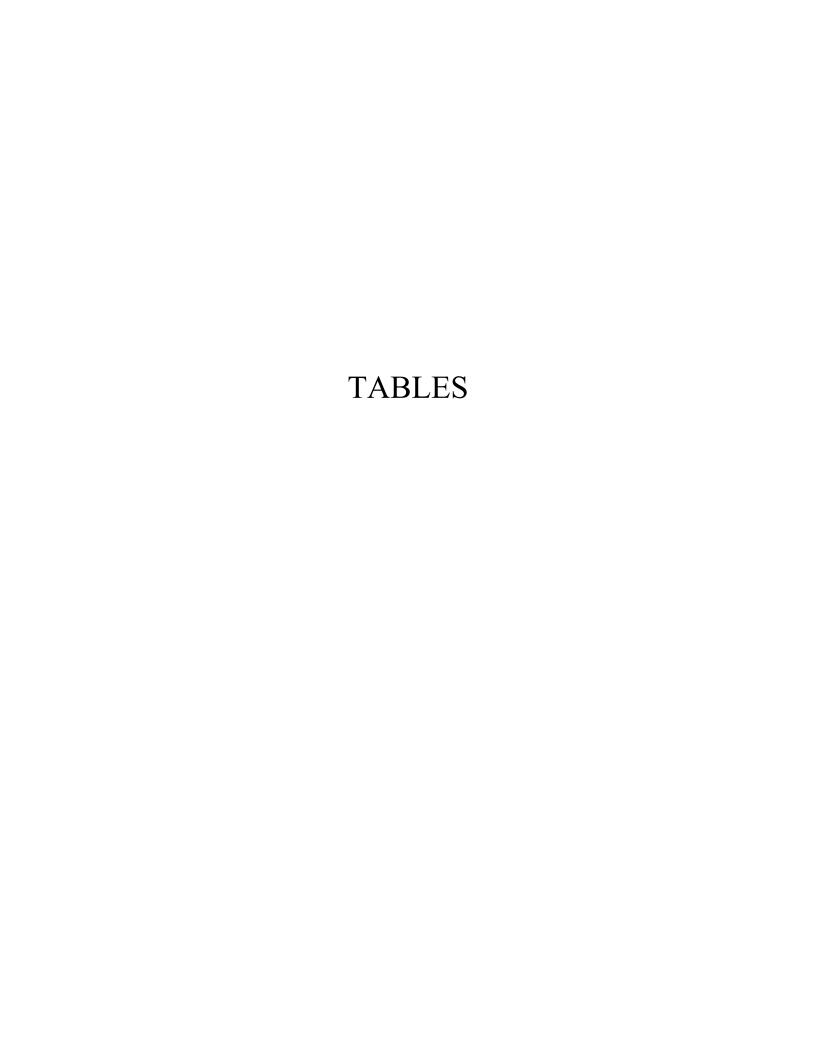


Table 1 - Groundwater Data Summary Cardinal Plant - Bottom Ash Pond

Donomoton	Unit	BA	P-1	BA	P-2	BA	P-3	BA	P-4	BA	P-5
Parameter	Onit	4/8/2019	10/9/2019	4/8/2019	10/9/2019	4/8/2019	10/10/2019	4/8/2019	10/10/2019	4/8/2019	10/10/2019
Antimony	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Arsenic	μg/L	0.500 U	0.500 U	122	34.9	0.500 U	0.500 U	39.0	54.8	5.20	5.80
Barium	μg/L	52.3	50.0	225	121	44.4	44.3	42.4	47.1	77.4	83.4
Beryllium	μg/L	0.100 U	0.100 U	0.260	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U	0.100 U
Boron	μg/L	2,680	3,050	1,960	1,560	2,020	2,100	19.8	19.5	92.0	118
Cadmium	μg/L	0.130	0.120	0.230	0.100 U	0.100 U	0.100	0.100 U	0.100 U	0.100 U	0.100 U
Calcium	μg/L	167,000	158,000	91,100	82,800	76,000	71,900	209,000	184,000	224,000	213,000
Chloride	mg/L	64.7	68.9	59.4	64.5	64.6	68.4	20.9	25.3	14.9	16.7
Chromium	μg/L	1.00 U	1.00 U	5.50	1.00 U	1.00 U	1.00 U	1.20	1.70	1.00 U	2.20
Cobalt	μg/L	1.00	0.700	4.60	1.20	0.570	0.500 U	17.8	19.1	1.00	1.10
Combined Radium	pCi/L	1.10	6.52	0.617	1.06	0.552	0.371	0.564	1.48	0.765	1.27
Fluoride	mg/L	0.380	0.370	0.800	0.560	0.140	0.110	0.150	0.140	0.0990	0.0680
Lead	μg/L	0.500 U	0.500 U	5.30	0.500 U	0.500 U	0.500 U	1.20	1.40	1.10	1.20
Lithium	μg/L	17.1	19.8	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U	10.0 U
Mercury	μg/L	0.000500 U	0.000500 U	0.00965	0.000670	0.000500 U	0.000500 U	0.00186	0.00117	0.00123	0.000785
Molybdenum	μg/L	30.4	32.3	36.3	40.0	1.30	1.60	1.30	1.40	0.500 U	0.500 U
Selenium	μg/L	0.500 U	0.500 U	0.570	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Sulfate	mg/L	419	416	167	202	149	164	471	560	404	433
Thallium	μg/L	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U	0.500 U
Total Dissolved Solids	mg/L	905	874	563	484	415	425	1,260	1,210	1,050	983
рН	SU	6.82	7.10	7.12	6.95	6.53	6.05	6.35	6.26	6.65	6.43

Notes:

mg/L: milligrams per liter $\mu g/L$: micrograms per liter

SU: standard unit

pCi/L: picocuries per liter

U: Parameter was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

All samples were collected as part of the assessment monitoring program in accordance with 40 CFR 257.90(e)(3).

Table 2: Residence Time Calculation Summary Cardinal Plant - Bottom Ash Pond

			201	9-03	2019-10		
CCR Management Unit	Monitoring Well	Well Diameter (inches)	Groundwater Velocity (ft/year)	Groundwater Residence Time (days)	Groundwater Velocity (ft/year)	Groundwater Residence Time (days)	
	MW-BAP-1 ^[2]	2.0	30.6	2.0	32.4	1.9	
D // A 1	MW-BAP-2 ^[2]	2.0	9.4	6.5	12.4	4.9	
Bottom Ash Pond	MW-BAP-3 ^[2]	2.0	20.8	2.9	9.3	6.5	
1 onu	MW-BAP-4 ^[1]	2.0	16.6	3.7	42.8	1.4	
	MW-BAP-5 ^[1]	2.0	10.1	6.0	20.1	3.0	

Notes:

- [1] Upgradient Well
- [2] Compliance Well

Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(4)

A description of site hydrogeology including stratigraphic cross-sections



2.4.2 Regional and Local Geologic Setting

The BAP is located in an area of Ohio which was unglaciated during the last ice age. The surficial geology at the BAP consists of alluvial silt, clay, and sand deposited by the Ohio River floodwaters, underlain by glacial outwash deposits of sand and gravel. The glacial outwash deposits extend to the bedrock surface, which occurs at approximately 60 feet below the natural ground surface at the pond. Bedrock consists of interbedded shale, sandstone, coal, and limestone of the Pennsylvanian-aged Conemaugh Formation (BBC&M, 2009; CHA, 2009).

2.4.3 Surface Water and Surface Water-Groundwater Interactions

The BAP is located immediately west of the Ohio River. According to United States Army Corps of Engineers records, the Ohio River elevation at this location is controlled by the Pike Island Dam, with a regular pool elevation of 644.0 ft above msl (USACE, 2003). Notes on an AEP plan drawing provide 50-year and 100-year flood elevations for the Ohio River of 664.0 ft and 666.0 ft above msl, respectively.

Surface water near the BAP enters a tributary to the Ohio River. According to USACE maps, the nearest tributary entering the Ohio River is Salt Run, located approximately 0.5 miles to the north (USACE, 2003). Riddles Run and Blockhouse Run are located approximately 1.25 and 1.5 miles to the north, respectively. Groundwater also flows towards and recharges the Ohio River. Seasonal fluctuations in the Ohio River pool stage near the BAP are expected to reflect seasonal precipitation values for Brilliant, Ohio with the highest pool elevations in the spring and summer months. The BAP is separated from the lower aquifer by a confining silt and clay layer of at least 5 feet in thickness. However, limited seepage may occur from the BAP to the near-surface zone of saturation, which drains towards the Ohio River.

2.4.4 Water Users

Based on water well records obtained from the Ohio Department of Natural Resources (ODNR, 2016) online search tools, the nearest domestic water supply wells are located approximately one mile west of the BAP. The well records indicate well depths ranging from 30 to 110 feet below



ground surface within shale and sandstone aquifers. According to the Jefferson County Water and Sewer District, there are no surface water intakes supplying water to the town of Brilliant, Ohio. Brilliant's water source comes from two groundwater wells located at a water treatment plant approximately three miles northeast of the BAP.



3. MONITORING NETWORK EVALUATION

3.1 **Hydrostratigraphic Units**

3.1.1 Horizontal and Vertical Position relative to CCR Unit

The principal regional aquifer is comprised of the alluvial sediments along the Ohio River, located below and east of the BAP. The identified uppermost aquifer in the vicinity of the BAP is the Sand and Gravel aquifer, which is hydraulically connected to the Ohio River. The BAP is lies above and is separated from the uppermost aquifer by a lower conductivity layer of silty clay and silt which thickens toward the west away from the Ohio River. The five (5) groundwater monitoring wells that make up the groundwater monitoring network around the BAP are screened to target the Sand and Gravel beneath the lower conductivity separation layer. Cross-sections illustrating the horizontal and vertical position of BAP relative to the uppermost aquifer are provided in Appendix B.

3.1.2 Overall Flow Conditions

Regionally, the most productive aquifer is the surficial aquifer, comprised of sand and gravel alluvial deposits along the Ohio River. Water supply wells within this aquifer can sustain yields of up to several hundred gallons per minute (gpm). This surficial aquifer is likely recharged through direct precipitation, infiltration from the Ohio River, and to a smaller extent, discharge from the surrounding bedrock (Geosyntec, 2006). Seasonal variation in the groundwater table beneath the BAP is expected to reflect the seasonal variation in precipitation with the highest groundwater elevations in the spring and summer months as well as the season fluctuation in the pool stage of the Ohio River.

Based on ODNR water well logs, the surficial aquifer of alluvial sediments along the Ohio River near the BAP can generally sustain yields of up to several hundred gpm.

3.2 Uppermost Aquifer

3.2.1 CCR Rule Definition

According to the 2015 CCR rule, the term "uppermost aquifer" has the same provisions as in §257.40: "The geologic formation nearest the natural ground surface that is an aquifer, as well as lower aquifers that are hydraulically interconnected with this aquifer within the facility's property boundary. This definition includes a shallow, deep, perched, confined, or unconfined aquifer, provided that it yields usable water" (40 CFR 257.60).

For purposes of this report, it is assumed that the uppermost useable aquifer has the following characteristics: (1) groundwater production rate over a 24-hour period of at least 0.1 gallons per



minute (gpm); and (2) groundwater quality with total dissolved solids (TDS) less than 10,000 milligrams per liter (mg/L).

3.2.2 Identified Onsite Hydrostratigraphic Unit

Based on boring log and monitoring well data around the BAP, the uppermost aquifer system is comprised of fine to coarse sand and gravel associated with the alluvial sediments of the Ohio River valley. The sand and gravel of the uppermost aquifer has an estimated range of hydraulic conductivity from 1 x 10-1 to 1 x 10-4 centimeters per second (cm/sec). in the area of the BAP. The direction of flow is generally to the east and southeast toward the Ohio River. Contours depicting the groundwater elevations and general direction of flow in the uppermost aquifer are shown in Figure 3-1. The uppermost aquifer is separated from an upper zone of saturation and the bottom of the BAP unit by a layer of silty clay, organic clay and silt that varies in thickness from 9.5 ft to 33.6 ft. The thicker portions of the layer are typically found along the west side of the BAP farthest from the Ohio River. Boring logs also suggest that the top of top of the uppermost aquifer ranges in elevation from approximately 619 ft to 635 ft. above mea sea level (amsl).

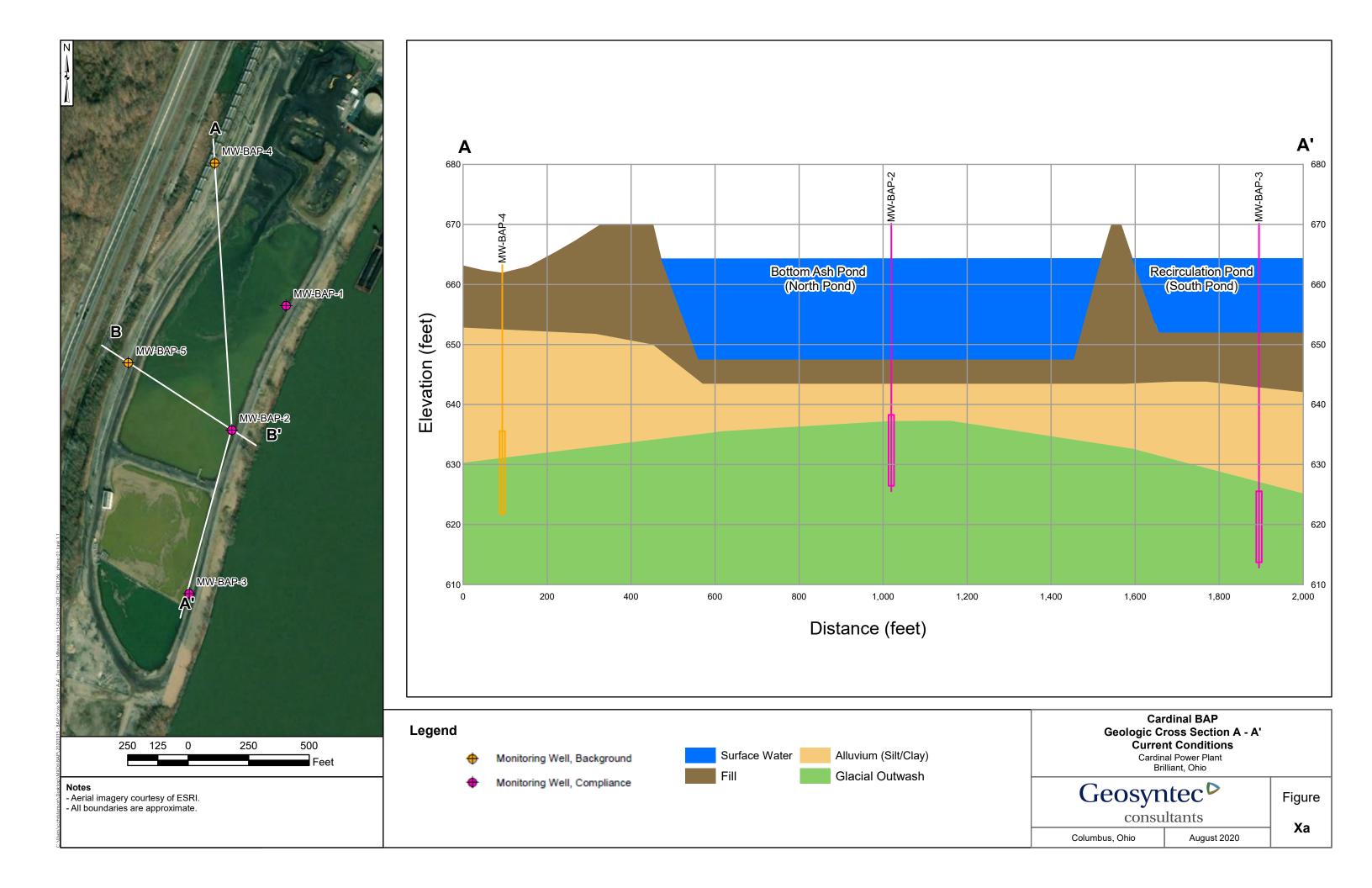
3.3 Review of Existing Monitoring Network

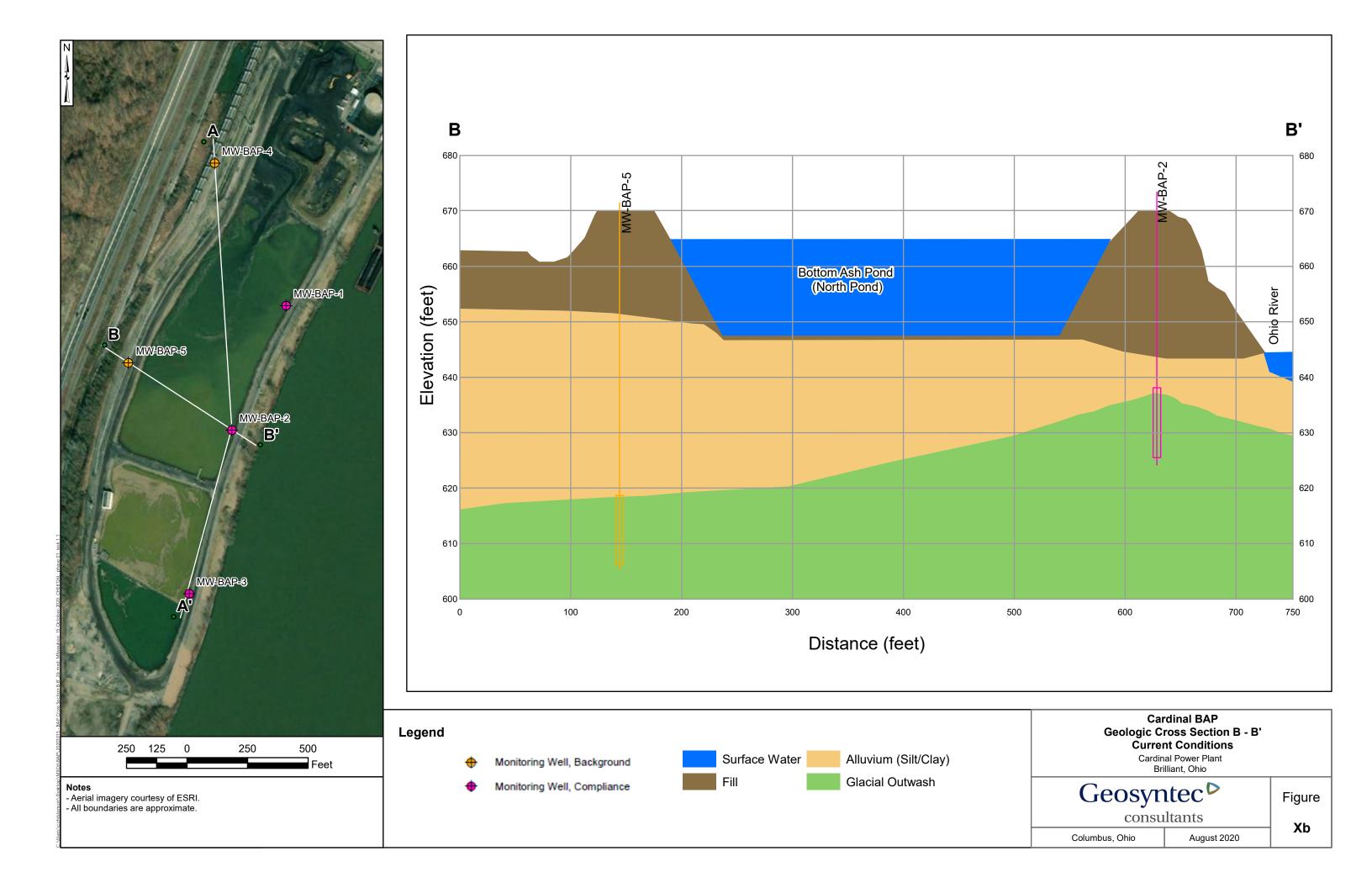
3.3.1 Overview

The groundwater monitoring network is shown on Figure 3-2 and consists of two (2) wells located upgradient (MW-BAP-4 and MW-BAP-5) and three (3) monitoring wells located downgradient (MW-BAP-1, MW-BAP-2 and MW-BAP-3) of the BAP and provide detection monitoring for the uppermost aquifer (Sand and Gravel Aquifer). The number, spacing, and depth of groundwater monitoring wells included in the groundwater monitoring network are based on site-specific geochemical, geologic and hydrogeologic information of the uppermost aquifer. Well construction details are summarized in Table 3-1. Boring and well construction logs for the groundwater monitoring well network wells are provided in Appendix C.

3.3.2 Compliance Assessment

Review of the existing groundwater monitoring well network in relation to the geologic and hydrogeologic conditions in the area of the BAP indicates that the monitoring well network consists of a sufficient number of wells installed at the appropriate depths to collect groundwater samples from the uppermost aquifer that accurately represent the groundwater quality upgradient and downgradient of the BAP. The groundwater monitoring well network is also capable of providing upgradient background groundwater quality and downgradient detection monitoring for a potential contaminant release to the uppermost aquifer (Sand and Gravel Aquifer) nearest the waste boundary. Based on the above review, the groundwater monitoring network around the Cardinal BAP meets the requirements of 40 CFR 257.91.





Bottom Ash Pond

40 CFR 257.101 (f)(1)(iv)(B)(5)

Any corrective measures assessment conducted as required at 40 CFR 257.96

Not applicable. The Bottom Ash Pond is currently in Assessment Monitoring.

Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(6)

Any progress reports on corrective action remedy selection and design and the report of final remedy selection required at 40 CFR 257.97(a)

Not applicable. The Bottom Ash Pond is currently in Assessment Monitoring.

Bottom Ash Pond

40 CFR 257.101 (f)(1)(iv)(B)(7)

The most recent structural stability assessment required at 40 CFR 257.73(d)

STRUCTURAL STABILITY ASSESSMENT

CFR 257.73(d)

Bottom Ash Pond Complex Cardinal Plant Brilliant, Ohio

October, 2016

Prepared for: Cardinal Operating Company - Cardinal Plant

Brilliant, Ohio

Prepared by: Geotechnical Engineering Services

American Electric Power Service Corporation

1 Riverside Plaza

Columbus, OH 43215



STRUCTURAL STABILITY ASSESSMENT CFR 257.73(d) BOTTOM ASH COMPLEX CARDINAL PLANT

GERS-16-135

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DATE

10/4/2016

Mohammad A. Ajlouni, Ph.D.,P.E.

REVIEWED BY

DATE

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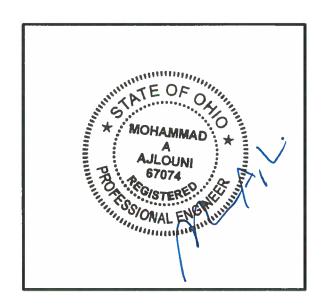
John T. Massey Norton

APPROVED BY

DATE

10/5/2016

Manager - AEP Geotechnical Engineering



I certify to the best of my knowledge, information and belief that the information contained in this structural stability assessment meets the requirements of 40 CFR 257.73(d)

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1.0 OBJECTIVE 257.73(d)

This report was prepared by AEP- Geotechnical Engineering Services (GES) section to fulfill requirements of CFR 257.73(d) and document whether the design, construction, operations, and maintenance of the CCR unit is consistent with recognized and generally accepted good engineering practices. This is the initial assessment as per the Rule.

2.0 NAME AND DESCRIPTION OF CCR SURFACE IMPOUNDMENT

The Cardinal Power Plant in Wells Township, Jefferson County, near the town of Brilliant in eastern Ohio. The Cardinal Power Plant is owned by Buckeye Power and AEP Generation Resources (GENCO) a unit of American Electric Power. is operated by Cardinal Operating Company. The facility operates two surface impoundments for storing CCR; the Bottom Ash Pond (BAP) Complex and Cardinal Fly Ash Reservoir II (FAR II) Dam. The focus of this report is the Bottom Ash Pond Complex.

The BAP complex is comprised of diked embankments on the east and west sides while the north and south sides of the BAP are incised. The complex consists of two separate ponds, the larger bottom ash pond and the smaller recirculation pond. The entire crest length is just over a mile, and the nominal crest width is 20 feet. The north end of the pond has been partially filled in with ash and the exact limits of the pond are poorly defined.

The pond complex was originally developed as part of the construction of Units 1 and 2 in the 1960s. The crest of the dikes forming the original pond was at El. 658.0. However, the pond complex was raised to a crest elevation of 970.0 and extensively modified in 1974 as part of the construction of Unit 3.

3.0 STABLE FOUNDATION AND ABUTMENTS 257.73(d)(1)(i)

[Was the facility designed for and constructed on stable foundations and abutments? Describe any foundation improvements required as part of construction.]

Based on the historical cross-sections extending through both the Bottom Ash Pond and the Recirculation Pond from the vertical expansion, the original ash pond embankments along the Ohio River ranged in height from 4 to 6 feet above the bottom of the ash pond.

A subsurface investigation was conducted in 2009 and the strength parameters of the foundation as well as the embankment were defined based on laboratory tests or correlations to known strengths based on blow counts. Table 1 lists the material properties for the foundation material.

The original ground surface at the site is generally located between El. 645 and 655. Near surface soils generally consist of a layer of alluvium silt, clay and fine sand (organic in some locations) over glacial outwash deposits of variable thickness overlying the bedrock surface. The alluvium clays and silts were deposited in the backwater of the Ohio River, while the outwash materials typically consist of sand, gravel and silt deposits deposited during the last ice age. Based on geological literature, the glacial outwash extends to the bedrock surface, estimated to be roughly 50 to 60 feet below the natural ground surface at the pond. The upper most bedrock consists of shale and/or sandstone belonging to the Conemaugh Group of Pennsylvanian Age. The soils were screened for liquefaction potential and found to be non-liquefiable.

Table 1 Strength Parameters for main Natural/constructed zones.

Lavor	$\gamma_{\rm m}$	c'	φ'
Layer	pcf	psf	degrees
Newer Embankment Fill	125	0	31
Original Embankment Fill	125	100	30
Alluvium Silt/Clay	125	0	30
Organic Clayey Silt	125	0	30
Loose Glacial Outwash Sand/Gravel	115	0	29
MDe Glacial Outwash Sand/Gravel	120	0	34

4.0 SLOPE PROTECTION 257.73(d)(1)(ii)

[Describe the slope protection measures on the upstream and downstream slopes.]

The Bottom Ash Complex was designed and constructed with soil embankment covered with a layer of bottom ash built up along the inboard slopes providing further protection. The outboard slopes primarily consist of grass vegetation with portions of the outboard slope protected by coarse riprap.

Operation and maintenance of the aggregate primarily includes periodic spraying for vegetation control. Grassed slopes are mowed regularly. Any erosion or slips that may occur is repaired within a timely period.

5.0 EMBANKMENT CONSTRUCTION 257.73 (d)(1)(iii)

[Describe the specifications for compaction and/or recent boring to give a relative comparison of density.]

The BAP complex embankments have maximum height of approximately 25 feet and are constructed of compacted clay on a slope ranging from 2.5:1 (2.5 feet horizontal, 1 foot vertical). The elevation at the top of the embankment around the perimeter of the BAP is approximately 670 feet msl, and the normal operating level is approximately 665 feet msl. The embankment fill materials dike ranged from hard silty Clay to fine and coarse gravel, overlying native material. The interior bottom elevation of the BAP Complex is approximately 645 feet msl.

The pond complex was originally developed as part of the construction of Units 1 and 2 in the 1960s. The crest of the dikes forming the original pond was at El. 658.0. However, the pond complex was raised to a crest elevation of 970.0 and extensively modified in 1974 as part of the construction of Unit 3.

No construction specifications are available for the Bottom Ash Pond. Recent borings through the embankment indicate that the embankment material is a medium stiff to very stiff sandy lean clay and representative of a compacted earthen material. A stability analysis of the diking system was also conducted which demonstrates that the facility has a factor of safety great than minimum values required by the CCR rule.

6.0 VEGETATION CONTROL 257.73 (d)(1)(iv)

[Describe the maintenance plan for vegetative cover.]

The vegetative areas are mowed to facilitate inspections and maintain the growth of the vegetative layer; and prevent the growth of woody vegetation.

7.0 SPILLWAY SYSTEM 257.73(d)(1)(v)

[Describe the spillway system and its capacity to pass the Inflow Design Flood as per its Hazard Classification.]

The Bottom Ash Complex has been determined to be a Significant Hazard potential CCR impoundment. Based on this hazard classification the design flood is determined by section 257.82(a)(3) to be the 1000-year storm. An analysis was performed for the 50% Probable Maximum Flood (PMF), which looks at 50% of the runoff from PMP storm of 33 inches in 24 hours. This produces significantly more runoff than the 1000-year storm and therefore exceeds the requirements of section 257.82(a)(3).

The Cardinal Bottom Ash Complex is comprised of diked embankments on three sides which directs storm water away from the impoundment and limits runoff to that which falls directly on the pond surface. The area of the pond is approximately 24.3 acres. The pond also receives pumped inflow from plant facilities and stormwater collection areas.

Discharge to the Ohio River is through a principal spillway located at the south end of the recirculation pond (a drop outlet and a 36"-pipe). During normal operation, there is no discharge to the river; rather all flows are re-circulated into the plant via the pump station located on the west side of the re-circulation pond.

Based on the flood routing, the calculated peak discharge from the dam is 67.7 cfs at a maximum pool elevation of 668.1 feet NGVD.

8.0 BURIED HYDRAULIC STRUCTURES 257.73 (d)(1)(vi)

[Describe the condition of the sections of any hydraulic structure that in buried beneath and/or in the embankment.]

The discharge pipe does not show any sign of corrosion or deterioration based on an exterior visual inspection.

9.0 SUDDEN DRAWDOWN 257.73 (d)(1)(vii)

[If the downstream slope is susceptible to inundation, discuss the stability due to a sudden drawdown.]

The downstream slope of the Bottom Ash Complex is not expected to be inundated from any adjacent water bodies.

Bottom Ash Pond 40 CFR 257.101 (f)(1)(iv)(B)(8)

The most recent safety factor assessment required at 40 CFR 257.73(e)

Bottom Ash Pond Initial Safety Factor Assessment Cardinal Power Plant Brilliant, Ohio S&ME Project No. 7217-15-007A



American Electric Power

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Columbus, Ohio 43215

Prepared by: S&ME, Inc. 6190 Enterprise Court Dublin, OH 43016

December 30, 2015

S&ME Project No. 7217-15-007A



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December 30, 2015



1.0 Introduction

1.1 Background

In April of 2015, the US EPA formally published national regulations for disposal of coal combustion residuals (CCR) from electric facilities. As part of the rule, the owner or operator of the CCR unit must obtain a certification from a qualified professional engineer stating that aspects of the CCR impoundments are in accordance with the rules. Based on our understanding of the Request for Fee Estimate received from AEP on April 29, 2015, AEP specifically requested P.E. certification to fulfill the requirements of 40 CFR § 257.73(e), *Periodic Safety Factor Assessments*. In the employment of BBC&M Engineering, Inc., the undersigned engineers conducted site investigations at the bottom ash pond in 2009 and 2010. Due to our familiarity with the site, S&ME was selected to perform the Safety Factor Assessment for this facility. S&ME understands that certification and/or documentation for other structural integrity criteria will be performed by AEP or other consultants.

1.2 Location and Geologic Conditions

The Cardinal Generating Plant is located along the Ohio River between Brilliant, Ohio and Tiltonsville, Ohio. The Bottom Ash Pond Complex is located along the west bank of the river just to the south of the Unit 3 area. The Bottom Ash Complex consists of two components: the Bottom Ash Pond and the Recirculation Pond. The Bottom Ash Pond is located north of the Recirculation Pond and they are separated by an earthen embankment. The crest elevation for all of the embankments has a minimum Elevation of 670 feet. The total length of the exterior embankment along the Ohio River is approximately 2,000 feet. Based on the current topography around the bottom ash complex, there is no discernable embankment on the north and south ends, thus the areas of the pond embankments are typically identified by referencing the eastern or western embankments. The bottom ash pond is operated at a constant Elevation of 664.5 feet. For comparison, the normal pool for this stretch of the Ohio River is EL. 644, as controlled by the Pike Island Dam Both ponds are isolated from exterior surface water inflow and during normal operation, all water that enters the pond is pumped back to the plant via the pump station located within the Recirculation Pond. The exception is during high rainfall events where the principal spillway may activate releasing water into the Ohio River through an NPDES outfall. The discharge is controlled by a 4-foot wide weir surveyed at Elevation 666.2. A review of the historical plans available for the bottom ash pond facility is included in Appendix V.

The original ground surface at the site is generally located between El. 645 and 655. Near surface soils generally consist of a layer of alluvium silt, clay and fine sand (organic in some locations) over glacial outwash deposits of variable thickness overlying the bedrock surface. The alluvium clays and silts were deposited in the backwater of the Ohio River, while the outwash materials typically consist of sand, gravel and silt deposits deposited during the last ice age. Based on geological literature, the glacial outwash extends to the bedrock surface, estimated to be roughly 50 to 60 feet below the natural ground surface at the pond. The upper most bedrock most likely consists of shale and/or sandstone belonging to the Conemaugh Group of Pennsylvanian Age.





Figure 1-1 - Cardinal Plant

1.3 Previous Investigations

In 2009, the undersigned engineers, when in the employment of BBC&M Engineering, Inc., completed a subsurface investigation and geotechnical assessment of the bottom ash pond embankments. The assessment, dated August 4, 2009, concluded that the embankment exhibited adequate factors of safety against slope failure under steady-state seepage and seismic loading conditions relative to typical US Army Corps of Engineers requirements. In 2010, BBC&M Engineering, Inc. performed additional geotechnical analyses and an hydrology and hydraulic evaluation of the pond. As part of this work, additional slope stability failure modes were examined, including the maximum surcharge pool and rapid drawdown load cases. A report documenting the additional geotechnical analysis, dated December 17, 2010, was submitted as an addendum to the 2009 report. The text from the 2009 report and an excerpt from the 2010 follow-up report is Appendices V and VI.



2.0 Scope of Work

In accordance with AEP's request, the following work items were performed by S&ME:

- 1. S&ME completed a cursory review of previously conducted assessment work performed by the undersigned engineers, as well as a limited number of construction documents made available by AEP.
- 2. S&ME visited the site along with personnel from AEP. The site visit was not a formal inspection, but rather served to document any significant modifications or changed conditions that may have taken place since the time of the previous investigations.
- 3. Upon completing Tasks 1 and 2, S&ME determined that there was insufficient information to certify the structural integrity of the surface impoundment in accordance with the requirements of 40 CFR § 257.73(e). To this end, S&ME was authorized to perform a supplemental investigation to support the safety factor assessment. Details regarding the investigation are described in the following sections of this report.

3.0 Information Review and Site Visit

S&ME conducted a cursory review of previous documents relating to the bottom ash pond and conducted a site visit at the facility. AEP provided S&ME with the following documents:

- Site Development Plan 1973 (Dwg. 3-3017-5 and 3-3027-3)
- Assessment of Dam Safety Final Report, Clough Harbour, & Assoc., December, 2009
- Bottom Ash Pond Subsurface Investigation & Analysis, BBC&M Engineering, Inc., August, 2009
- Addendum to Bottom Ash Pond Investigation, BBC&M Engineering, Inc., December, 2010

On August 18, 2015, the undersigned S&ME personnel met with Dr. Mohammad Ajlouni (AEP Civil Engineering) and Mr. Randy Sims (Landfill Operations) at the Cardinal Plant and conducted a site visit at the bottom ash pond. The participants discussed and observed the operations of the bottom ash and recirculation ponds, including the hydraulic structures within the ponds. During our visit, two localized possible seepage areas were observed on the outboard slope of the eastern embankment of the recirculation pond. Based on discussions with the group, it was believed that the seepage areas were relatively new.

One apparent seepage area was located immediately north of the existing riprap and the other was approximately 300 feet north of the riprap. The limits of the possible seepage areas were delineated with a handheld GPS unit. The apparent seepage areas range from 35 to 50 feet wide by 6 to 8 feet high. The seepage areas were observed to be wetter than the surrounding area and were muddy in some areas, which may be a result of mowing operations. While the ground surface has been softened as a result of seepage, there was no indication of flowing water emanating at either of the areas at the time of our visit. Additionally there was no indication of piping of soil. S&ME understands the riprap on the outboard slope of the recirculation pond to the south of the new seepage area was constructed as an inverted filter; similar seepage conditions were observed in this area resulting in construction of the filter. Based on the historical drawings, the embankments do not contain any internal drains to intercept/control the phreatic





surface within the embankment. Despite this, S&ME understands the embankments have otherwise performed well, particularly in regard to shallow sloughs along the outboard slope of the 41 years that they have been in service in the current configuration.

While no other visual observations suggested dam safety concerns, S&ME noted the following modifications to the bottom ash pond complex since the 2009 and 2010 assessments:

- The northern section of the western bottom ash pond embankment was widened on the outboard side to create additional space for construction staging.
- Crest improvements were made to raise low areas and establish a consistent top of dam Elevation of 670 feet.
- The 2009 investigation focused only on the river side embankment. Although the river side embankment is significantly taller than the west embankment, investigation of the west embankment was believed to be warranted.

4.0 Field and Laboratory Work

As part of the 2009 investigation, 7 soil borings were performed along the eastern embankment of the bottom ash pond and recirculation pond. For the 2015 supplemental investigation, S&ME performed 4 soil borings along the western embankments, as well as two additional shallow borings through the eastern embankment crest upstream from the identified seepage areas. The borings are designated as CD-BAP-1501 through B-1505 and MW-BAP-4 through MW-BAP-5. Boring CD-BAP-1503, originally planned to be located at the toe of the west embankment could not be accessed and was not performed. Boring numbers with 'MW' indicate a monitoring well was installed at this location, which were performed as part of a separate hydrogeology study. Additionally, S&ME installed three other monitoring wells, designated MW-BAP-1 through MW-BAP-3, and advanced one soil boring designated CD-BAP-1506 as part of the separate hydrogeology study at the bottom ash pond facility. Although not performed as part of this factor of safety assessment, the results from these explorations were considered in developing our understanding of the embankments and foundation soils. Locations of all explorations are shown on the Plan of Borings included as Drawing No. 1 in Appendix I.

Laboratory testing was performed on selected representative soil samples obtained during the field investigations to determine natural moisture content (ASTM D2216), liquid and plastic limits (S&ME adjustment to ASTM D4318), and grain size analyses (ASTM D422). The results of these and other tests permit an evaluation of the strength, compressibility and permeability characteristics of the soils encountered at this site.

The results of the moisture content testing and of the liquid and plastic limits are graphically displayed on the individual boring logs presented in Appendix I. All laboratory test results, including a summary of laboratory test results and grain size analyses are presented in Appendix II.



5.0 Subsurface Conditions

5.1 Stratigraphy

Borings CD-BAP-1501,CD-BAP-1502, and MW-BAP-5 were performed from the crest of the western embankment, while Boring MW-BAP-4 was performed from the toe of the western embankment. Based on the descriptions of the samples recovered in the borings and laboratory testing, the subsurface stratigraphy for each section can generally be described in descending order from the top of the western embankment as follows:

- Borings CD-BAP-1502 and MW-BAP-5 were performed from the crest of the embankment encountered 15 inches of aggregate at the ground surface overlying 10 to 13 feet of embankment fill consisting of medium-dense to dense fine to coarse sand and gravel and hard clayey silt. SPT N-values (corrected for 60% energy) ranged from 13 to 60 while hand penetrometer measurements on samples exhibiting cohesion ranged from __ to 4.5+ tons per square foot (tsf).
- Boring CD-BAP-1501 was performed from the widened crest area. The boring encountered 15 inches aggregate underlain by 11.5 feet of embankment fill consisting of a thin stratum of medium-stiff clayey silt over of loose to medium dense fine to coarse sand.
- Underlying the embankments, the borings encountered alluvial soils consisting of

Borings CD-BAP-1504 and CD-BAP-1505 were performed from the crest of the eastern embankment adjacent to the observed seepage areas. The main purpose of these boring was to identify potential anomalies within the embankments that would suggest a unique circumstance which could be contributing to the observed seepage. Both borings were advanced to a depth of 16 feet within the embankment fill. For reference, the seepage areas were observed to begin approximately 6 to 8 feet below the crest. These borings, along with results from the sampling from monitoring wells MW-BAP-1, MW-BAP-2 and MW-BAP-3 did not reveal any appreciable differences from the crest borings performed during the 2009 investigation, such as a layer or zone of clean sand, as the embankment fill was already known to contain soils of a varying degree.

The stratigraphy of the eastern embankments is summarized in the text from the 2009 Investigation included as Appendix V.

5.2 Groundwater Conditions

Groundwater observations were made as each boring was being advanced and measurements were made at the completion of drilling. The groundwater observations are graphically displayed on the boring logs and also noted at the bottom of the log, and are referenced from the ground surface. Groundwater was encountered within the crest borings at a depth of approximately 15 feet. Groundwater in Boring MW-BAP-4 was encountered at a depth of 5.5 feet. The groundwater readings correlate to an approximate Elevation of 655 feet.

Temporary open standpipe piezometers were installed in Borings CD-BAP-1504 and CD-BAP-1505 to obtain groundwater information in relation to the observed seepage area. Unfortunately, owing to the presence of overhead electric along the outboard side of the crest, the borings had to be performed near the inboard side of the crest. Several longer term groundwater readings were taken during the course of

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the field work. The readings are summarized on the individual well logs, and generally range between Elevation 661 and Elevation 663. The readings indicate a small decrease in water level from the recirculation pond operating pool. It should be noted that all of the wells positioned within the crest are located on the inboard side to avoid blocking the road as well as the overhead power lines.

5.3 Shear Strength and Permeability

The laboratory testing results for the 2015 investigation were compared to laboratory testing completed as part of the 2009 investigation. The comparison of the index testing was performed to determine if there was any justification for developing different shear strength and permeability values for the subsurface materials encountered in the western side of the complex than had been previously been estimated for cross-sections on the eastern side in 2009. As the results of the 2009 laboratory index testing are very similar to the new index testing results, S&ME is of the opinion that the strength parameters used to characterize the eastern embankment and foundation soils in 2009 are applicable to the supplemental investigation of the western embankment and foundation soils.

The shear strength parameters used in the slope stability analysis are shown in Table 5-1.

Effective Ywet **Material Description** (pcf) c' (psf) Reference SPT and Index Testing Newer Embankment Fill 125 31° 0 Correlations 125 Original Embankment Fill 30° 100 **Index Testing Correlations** Alluvium Silt and Clay 125 30° 0 **Index Testing Correlations Index Testing Correlations and** Organic Clayey Silt 125 30° 0 CU Triaxial Test (BBCM 2009) Very Loose to Loose Glacial 29° 0 SPT and Grain Size Correlations 115 Outwash Sand and Gravel Medium Dense Glacial 34° SPT and Grain Size Correlations 120 0 Outwash Sand and Gravel Granular 30° SPT and Grain Size Correlations 115 0 Embankment Fill⁽¹⁾

Table 5-1 – Shear Strength Parameters

6.0 Safety Factor Assessment

As part of the safety factor assessment, S&ME completed Parts 1 and 2 of Section 257.73(e) of the Final Rules for the Disposal of Coal Combustion Residuals from Electric Utilities published on April 17, 2015 in the Federal Register. In accordance with the Rule, the analysis was performed for the critical cross-sections(s) that are anticipated to be most susceptible of all cross-sections to structural failure based on appropriate engineering considerations. The Rule specified the following loading conditions for analysis:

⁽¹⁾Applies only to widened crest area on the northwestern side of bottom ash pond



- i. Static Factor of Safety under the long-term, maximum storage pool loading condition must equal or exceed 1.50.
- ii. Calculated static factor of safety under the maximum surcharge pool loading condition must equal or exceed 1.50.
- iii. The calculated seismic factor of safety must equal or exceed 1.00.
- iv. For dikes constructed of soils susceptible to liquefaction, the calculated liquefaction factor of safety must equal or exceed 1.20.

6.1 Limit Equilibrium Analyses

The 2009 Investigation Report and the 2010 Addendum discuss in detail the subsurface investigation, laboratory testing, parameter justification, seepage analyses and limit equilibrium slope stability analyses that were performed to develop safety factors for the bottom ash pond embankments. As mentioned previously, engineering parameters developed as part of the 2009 and 2010 investigations were utilized for the new analyses associated with the western embankment as the laboratory testing and subsurface investigation did not encounter soil properties that differed greatly from the soils encountered in the previous investigations.

In summary, four sections along the eastern (river-side) embankment and two sections along the western embankment were studied. Both cross-sections through the western embankment are located within the bottom ash pond as the embankment adjacent to the recirculation pond is only 4 to 6 feet high and access to the toe was not readily available. Subsurface information for each section was obtained by performing borings through the crest and toe of the embankment. Based on a review of all six sections explored, three were selected for detailed limit equilibrium stability analysis (two on the eastern embankment and one on the western embankment).

Prior to performing the limit equilibrium stability analyses as part of the 2009 assessment, seepage analyses were performed to develop a better understanding of the likely phreatic surface within the embankment and foundation. The models were calibrated by adding additional total head boundary conditions within the subsurface to best model the groundwater table as observed in the observation wells. Although a classically shaped phreatic surface extending from the ash pond level to the Ohio River was generated by the seepage analyses, much of the seepage emanating from the ponds appears to be moving downward through the newer embankment fill and thin stratum of alluvium soils and into the glacial outwash sand and gravel stratum which essentially serves as a drain.

Results of the slope stability analysis indicate that the critical cross-section occurs through the eastern embankment of the bottom ash pond (referred to as Section D in the 2009 and 2010 assessments). The design cross-section does not vary along the eastern embankment, but Section D yielded the lowest factors of safety due to slight variations in the outboard slope. All load cases performed for the Safety Factor Assessment as well as additional load cases evaluated for typical US Army Corps of Engineer's requirements met the minimum factor of safety for global stability.

One observed seepage area is located just north of Section B and the other is located approximately 200 feet south. Comparison of boring logs for CD-BAP-1504 and CD-BAP-1505 with the log for boring CD-PZ-BAP-0902 located at Section B do not reveal any key differences in the embankment fill. In fact, Boring CD-PZ-BAP-0902 exhibited a larger zone of granular embankment fill located within the observed





elevation of seepage on the outboard slope, but no seepage was observed adjacent to this boring. The fill soils are believed to vary laterally through the embankment as much as it was observed to vary vertically at the boring locations, suggesting that the granular layers observed in the borings are unlikely to extend all the way through the embankment. Considering this, it is the opinion of S&ME that at this time, the seepage areas are representative of localized pockets of more permeable soils within the overall embankment matrix. As such, it is not believed that the phreatic surface intercepts the outboard face, but rather that there are narrow zones of seepage with unsaturated soils beneath. Nonetheless, these areas should be addressed, as further discussed below.

As noted, the seepage observed during our August, 2015 site visit appeared to occur in two isolated areas. With time, the outboard slope at these locations may weaken due to the presence of groundwater within close proximity to the ground surface resulting in reduced shear strength and shallow slope failures. Though such a failure would typically be minor in extent, S&ME recommends these areas be addressed in the near future before they lead to more significant issues over time. Construction of an inverted filter may be suitable given the performance of the existing inverted filter on the south end. S&ME also recommends continued monitoring of these areas to ensure soils particles are not being carried from inside the embankment.

6.2 Liquefaction Potential of Embankment Soils

S&ME evaluated the potential of the embankment soils to liquefy during a seismic event. The embankment material is classified as a fined grained material and the recovered samples with gradation testing were evaluated following guidelines presented in the 2003 NEHRP (National Earthquake Hazards Reduction Program) Recommended Provisions for Seismic Regulations for New Buildings and Other Structures. The provisions in Chapter 7 indicate that liquefaction potential in fine grained soils should be assessed provided the following criteria are met (Seed and Idriss 1982; Seed et al., 1983): the weight of the soil particles finer than 0.005 mm is less than 15 percent of the dry unit weight of a specimen of the soil; the liquid limit of soil is less than 35 percent; and the moisture content of the in-place soil is greater than 0.9 times the liquid limit. If all of these criteria are not met, the soils may be considered non-liquefiable.

Laboratory testing results from 16 fine grained samples that were available from the 2009 and 2015 investigations for evaluation of the screening criteria. Of the 16 samples, 8 samples contained data to check all three screening criteria, and 7 samples contained data to check two screening criterion. Based on the results of the screening, no sample met all 3 criteria; therefore, these fine grained embankment fill can be considered non-liquefiable. A table depicting this evaluation is included in Appendix IV.

The potential for the coarse grained embankment soils to resist liquefaction was evaluated. The fine grained (cohesive) and coarse grained (granular) embankment soils appear to be from the same borrow source as there are no well-defined layers and often only minor variations in the percent by weight of the recovered sample change the main description from fine grained to coarse grained. Although construction records were not available, the density of the coarse grained samples and consistency of the fine grained samples within the embankment fill suggest they were well compacted. Based on the controlled manner in which the fill was placed, the coarse grained embankment soils can be considered non-liquefiable.

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6.3 Summary of Results

A summary of the computed safety factors for the critical cross-section is provided in Table 5-2. Also included in the table are the minimum values defined in 40 CFR § 257.73(e)(1) subparts (i) through (iv). Graphical output corresponding to the analysis cases are presented in Appendix IV along with additional slope stability load cases evaluated during the course of the bottom ash pond assessments.

Table 6-1 – Safety Factor Summary

Analysis Case	Minimum Safety Factor	Computed Safety Factor
Long-term, maximum storage pool	1.50	1.52
Maximum surcharge pool	1.40	1.52
Pseudo-static seismic loading	1.00	1.09
Embankment Liquefaction	1.20	Non-liquefiable

7.0 Certification

Based on our previous investigations and current assessment of the Bottom Ash Pond facility, S&ME certifies that this assessment meets the requirements of paragraphs (e)(1) and (e)(2) of Part 257.73 for the critical cross-section of the embankment.

We appreciate having been given the opportunity to be of service on this project. If you have any questions, please do not hesitate to contact this office.

Sincerely,

S&ME, Inc.

Michael T. Romanello, P.E.

Project Engineer

Registration No. 74384

Michael G. Rowland, P.E.

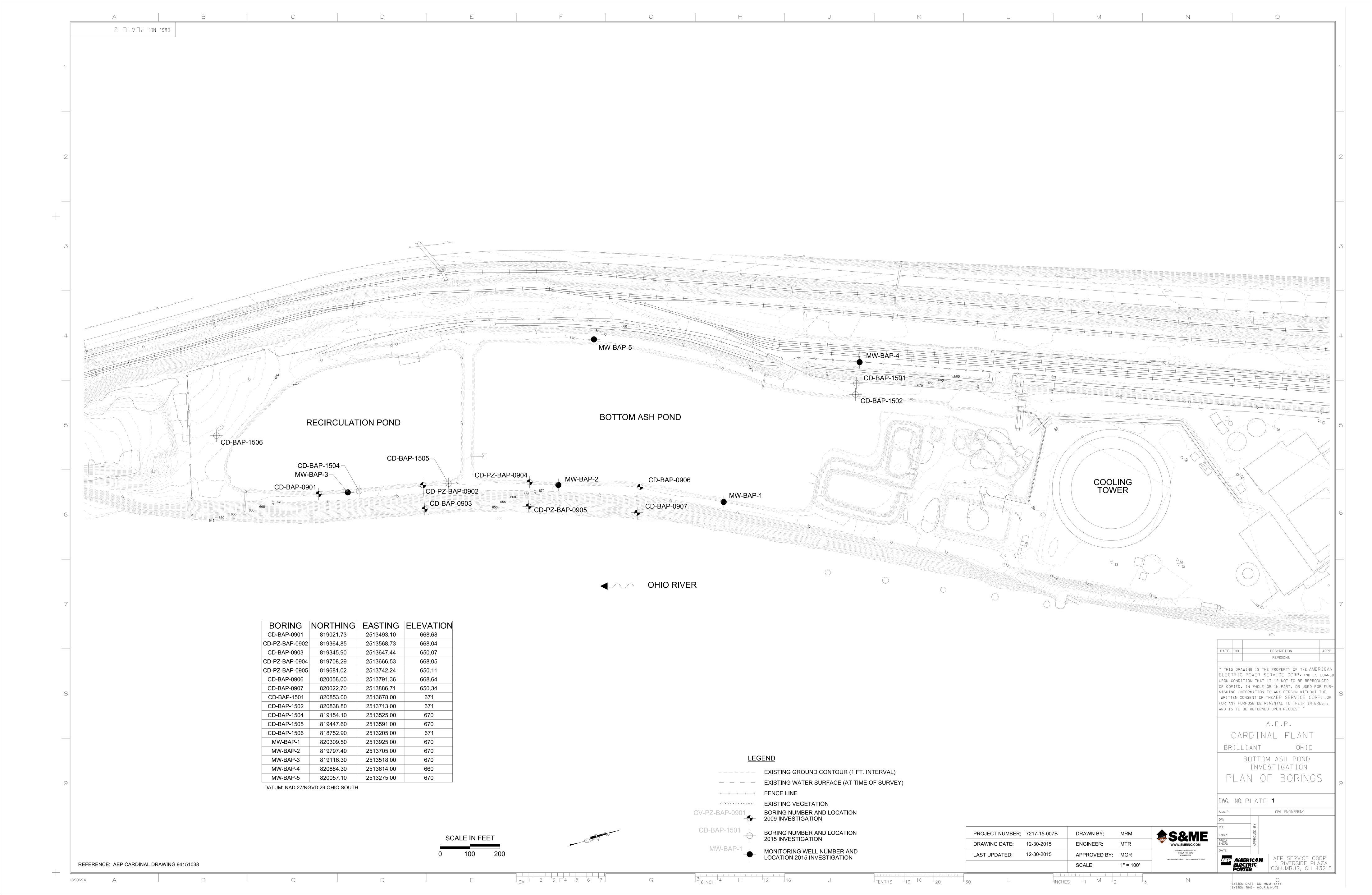
Michael A. La

Senior Engineer

Registration No. 65559



Appendix I – 2009 & 2015 Site Investigation Figures



EXPLANATION OF SYMBOLS AND TERMS USED ON BORING LOGS FOR SAMPLING AND DESCRIPTION OF SOIL

SAMPLING DATA



- Blocked-in "SAMPLES" column indicates sample was attempted and recovered within this depth interval.



- Sample was attempted within this interval but not recovered.

2/5/9

- The number of blows required for each 6-inch increment of penetration of a "Standard" 2-inch O.D. split-barrel sampler, driven a distance of 18 inches by a 140-pound hammer freely falling 30 inches. Addition of one of the following symbols indicates the use of a split-barrel other than the 2" O.D. sampler:

2S

- 2½"O.D. split-barrel sampler

3S

- 3" O.D. split-barrel sampler

P -

- Shelby tube sampler, 3" O.D., hydraulically pushed.

R -

- Refusal of sampler in very-hard or dense soil, or on a resistant surface.

50-2"

- Number of blows (50) to drive a split-barrel sampler a certain number of inches (2), other than the normal 6-inch increment.

S/D

- Split-barrel sampler (S) advanced by weight of drill rods (D),

S/H

- Split-barrel sampler (S) advanced by combined weight of rods and drive hammer (H).

SOIL DESCRIPTIONS

All soils have been classified basically in accordance with the Unified Soil Classification System, but this system has been augmented by the use of special adjectives to designate the approximate percentages of minor components as follows:

<u>Adjective</u>	Percent by Weight
trace	1 to 10
little	11 to 20
some	21 to 35
"and"	36 to 50

The following terms are used to describe density and consistency of soils:

<u>Term (Granular Soils)</u>	Blows per foot
Very-loose	Less than 5
Loose	5 to 10
Medium-dense	11 to 30
Dense	31 to 50
Very-dense	Over 50
Term (Cohesive Soils)	Qu (tsf)
Very-soft	Less than 0.25
Soft	0.25 to 0.5
Medium-stiff	0.5 to 1.0
Stiff	1.0 to 2.0
Very-stiff	2.0 to 4.0
Hard	Over 4.0

2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. CD-BAP-1501 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,853, E. 2,513,678 11/17/15 - 11/18/15 ELEVATION: 671 DATE: 4-1/4" I.D. Hollow-stem Auger 16.0' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT DESCRIPTION $\overset{\circ}{N}$ **RESULTS AGGREGATE - 15 INCHES** 669.8 FILL: Medium-stiff gray clayey silt, "and" fine to 87 45 coarse sand, little fine gravel, intermixed with 668.2 silty clay, damp. 13 2 67 FILL: Loose to medium-dense brown and gray 6 fine to coarse sand, little to some silty fine to coarse gravel, little to some silt, damp. 8 53 3 5 4 6 53 10 80 18 80 10-0 50-1"R 659.5 FILL: Dense brown fine to coarse sand, trace fine 40 73 8 gravel, some to "and" clayey silt, damp. 658.0 13 9A 43 100 FILL: Stiff to very-stiff gray silty clay, some to H=1.75-2.25 "and" fine to coarse sand, little fine to coarse 9B H=3.0-4.0656.5 gravel, damp. 15 FILL: Dense brown and gray fine to coarse sand, 10 34 67 little fine to coarse gravel, some silt, damp. 655.0 - Boring backfilled with cement bentonite grout. - Boring location recorded with a hand-held GPS unit. Elevation estimated from March, 2015 plant 20-- Datum: Ohio State Plane South NAD 27/ NAVD 29 (Plant Grid). 25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: - Gradation - Uncon Comp See H - Penetrometer (tsf) Last Calibration Date: 2/20/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) T - Triax Comp Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

2010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007A

LOG OF BORING NO. CD-BAP-1502 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,839, E. 2,513,713 11/18/15 ELEVATION: 671 DATE: 41.5' 4-1/4" I.D. Hollow-stem Auger COMPLETION DEPTH: DRILLING METHOD: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT DESCRIPTION $^{09}_{80}$ **RESULTS AGGREGATE - 12 INCHES** 670.0 FILL: Dense brown and gray fine to coarse 38 53 gravel, some fine to coarse sand, little silt, damp. 668.5 18 FILL: Hard brown and gray clayey silt, "and" fine 60 2 80 H=4.5to coarse sand, little fine gravel, damp. 667.2 FILL: Medium-dense to very-dense brown and gray fine to coarse sand, little to some fine to 51 80 3 5 coarse gravel, little to some silt, silty clay, or clayey silt (varies), damp. 31 80 4 26 93 662.5 FILL: Hard gray and brown clayey silt, some to 33 87 H=4.5 "and" fine to coarse sand, little fine to coarse 15 gravel, damp. 10-41 53 H=4.5 P 657.5 FILL: Medium-dense gray and brown fine to 14 67 coarse sand, some fine to coarse gravel, some silty clay, moist becoming wet. 15 P 654.0 FILL: Medium-dense gray fine to coarse sand, 9 19 87 some fine to coarse gravel, some clayey silt, wet. 652.7 Stiff gray clayey silt, some fine to coarse sand, some fine gravel, moist. 10 11 100 H=1.2520-H=1.25 649.2 Stiff brown silty clay, some fine to coarse sand, little to some fine to coarse gravel, moist. 13 73 H=2.5 11 0 12 33 H=1.25 25 645.5 Very-stiff red-brown mottled with gray silty clay, trace to little fine to coarse sand, contains silt H=3.0-3.7593 13 16 seams, damp. 93 13 H=3.5- 30 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: Gradation -See H - Penetrometer (tsf) - Uncon Comp Last Calibration Date: 2/20/2013 WATER NOTE: W - Unit Dry Wt (pcf) Triax Comp Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE: ATV 550-2

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LOG OF BORING NO. CD-BAP-1502 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



CARDINAL PLANT, BRILLIANT, OH LOCATION: N. 820,839, E. 2,513,713 11/18/15 ELEVATION: 671 DATE: 4-1/4" I.D. Hollow-stem Auger 41.5' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE REC-% SAMPLE DEPTH, FEET EFFORT ELEV **TEST** NATURAL MOISTURE CONTENT N_{60} DESCRIPTION **RESULTS** Very-stiff red-brown mottled with gray silty clay, trace to little fine to coarse sand, contains silt seams, damp. 15 87 15 H=3.5638.5 Stiff to very-stiff brown mottled with gray silty clay, some to "and" from to medium sand, trace H=1.5-2.25 coarse sand, damp. 636.5 Loose red-brown from to medium sand, trace 35-8 100 16 coarse sand, "and" silt, damp. 2010 NEW DEFAULT BORING LOG-W/ N60 634.0 Stiff red-brown silty clay, "and" fine to medium sand, trace coarse sand, trace fine gravel, damp. 632.7 100 17 6 H=1.75Very-loose brown fine to medium sand, "and" silt, damp. 40-5 67 629.5 - Encountered water at 15.0' - Boring backfilled with cement bentonite grout. - Boring location surveyed with a hand-held GPS unit. Elevation estimated from March 2015 plant 45 survey. - Datum: Ohio State Plane South NAD 27/NAVD 29 (Plant Grid). 50-55 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 G - Gradation Q - Uncon Comp T - Triax Comp C - Consol. WATER LEVEL: See H - Penetrometer (tsf) Last Calibration Date: 2/20/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. CD-BAP-1504 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



11/16/15 LOCATION: N. 819,154, E. 2,513,525 ELEVATION: 670 DATE: DRILLING METHOD: 4-1/4" I.D. Hollow-stem Auger 18.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX EFFORT DEPTH, FEET **TEST** ELEV NATURAL MOISTURE CONTENT DESCRIPTION $\overset{\circ}{N}$ RESULTS **AGGREGATE - 16 INCHES** 668.7 FILL: Hard gray and brown silty clay, some fine 40 87 18 to coarse sand, brown fine gravel, dry. 667.5 FILL: Medium-dense dark-brown fine to coarse 24 2 80 H=4.010 sand, trace fine gravel, trace silt, dry. 666.0 FILL: Hard gray and brown silty clay, "and" fine 49 93 3 H=4.020 to coarse sand, little fine gravel, dry. 5 664.5 FILL: Dense dark-gray and brown fine to coarse 53 100 H=4.0sand, little to some fine to coarse gravel, some 663.0 silty clay, dry. FILL: Hard brown silty clay, some fine to coarse ∇ 5 39 67 sand, little fine gravel, dry. 661.5 FILL: Medium-dense to dense brown and 44 33 dark-gray fine to coarse sand, little to some fine to coarse gravel (sandstone fragments), little to 10-"and" silty clay, dry. 34 67 23 8 27 54 47 15 14 0 654.0 100 H=1.5-2.0FILL: Medium-stiff to stiff brown and gray silty clay, some fine to coarse sand, little fine to coarse 10 12 H=0.75-1.5gravel, damp becoming wet. 652.0 - No seepage encountered. 20-- Encountered water at 16.5'. - Borehole converted to temporary piezometer upon completion - See Separate Well Log. - Boring backfilled with cement bentonite grout. - Boring location surveyed with a hand-held GPS unit. Elevation estimated from March 2015 plant - Datum: Ohio State Plane South NAD 27/NAVD 29 (Plant Grid). 25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: 7.9 - Gradation - Uncon Comp See H - Penetrometer (tsf) In Well Last Calibration Date: 2/20/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) T - Triax Comp C - Consol. 12/10/15 Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. CD-BAP-1505 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



CARDINAL PLANT, BRILLIANT, OH 11/17/15 LOCATION: N. 819,448, E. 2,513,591 ELEVATION: 670 DATE: 4-1/4" I.D. Hollow-stem Auger 17.5' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT N_{60} **DESCRIPTION** RESULTS **AGGREGATE - 16 INCHES** 668.7 FILL: Medium-dense to dense brown and gray 31 60 fine to coarse sand, some fine to coarse gravel, little silt, dry. 65 53 2 12 666.0 FILL: Medium-dense brown fine to coarse gravel, 24 3 53 some fine to coarse sand, little to some silt, dry. 5 29 13 28 80 661.5 FILL: Very-stiff to hard brown clayey silt, "and" 15 53 H=3.5fine to coarse sand, little to some fine to coarse gravel, damp to moist. 7A 23 100 H=1.5659.2 FILL: Medium-dense brown and gray fine to 7B coarse sand, some fine to coarse gravel, little silty clay, dry. 8 18 73 657.0 FILL: Hard brown and gray silty clay, some fine 16 67 H=4.5 to coarse sand, little fine to coarse gravel, moist. 15 10 16 87 H=4.5 654.0 FILL: Medium-stiff brown and gray silty clay, 11 11 53 H=0.5-1.0some fine to coarse sand, little fine to coarse 652.5 gravel, moist. - No seepage encountered. - Encountered water at 14.5'. 20-- Borehole converted to temporary piezometer well upon completion - See Separate Well Log. - Boring backfilled with cement bentonite grout. - Boring location surveyed with a hand-held GPS unit. Elevation estimated from March 2015 plant - Datum: Ohio State Plane South NAD 27/NAVD 29 (Plant Grid). 25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: 8.8 Gradation Gradation See H - Penetrometer (tsf In Well Last Calibration Date: 2/20/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) T - Triax Comp 12/10/15 Curves D - Relative Dens (%) Drill Rig Number: S&ME DATE:

LOG OF BORING NO. MW-BAP-4 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,884, E. 2,513,614 11/20/15 - 11/23/15 ELEVATION: 660 DATE: 4-1/4" I.D. Hollow-stem Auger 40.0' DRILLING METHOD: COMPLETION DEPTH: SAMPLER(S): 2" O.D. Split-barrel Sampler SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT N_{60} DESCRIPTION **RESULTS AGGREGATE - 12 INCHES** 20 659.0 FILL: Medium-dense to dense gray and brown 39 fine to coarse gravel, some to "and" fine to coarse 87 H=4.25-4.5 sand, little to some silt, dry. 2 18 53 2010 NEW DEFAULT BORING LOG-W/ N60 3 20 67 5 654.7 FILL: Very-soft brown and gray silty clay, "and" 654.2 fine to coarse sand, little fine to coarse gravel. 31 87 FILL: Dense bown fine to coarse sand, little fine to coarse gravel, "and" clayey silt, cobbles, moist. 20 5 50-3"R 652.5 Stiff to very-stiff dark-brown mottled with dark-gray silty clay, little fine to coarse sand, trace fine gravel, slightly organic, damp. 3, 9 87 H=2.0-3.010 P H=1.25-2.515 643.8 Very-stiff brown mottled with gray silty clay, 14 87 H=2.0-3.5little fine to medium sand, trace coarse sand, few cobbles, contains silt seams near top of stratum, damp. 100 H=2.25-3.2520 100 H=3.014 100 H=3.25SYMBOLS USED TO INDI Drill Rod Energy Ratio: 0.75 WATER LEVEL: - Gradation - Uncon Comp See H - Penetrometer (tsf) Last Calibration Date: 8/2/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) Triax Comp Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE: ATV 550-2

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LOG OF BORING NO. MW-BAP-4 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



LOCATION: N. 820,884, E. 2,513,614 11/20/15 - 11/23/15 **ELEVATION:** 660 DATE: 4-1/4" I.D. Hollow-stem Auger 40.0' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT DESCRIPTION N_{60} RESULTS Very-stiff brown mottled with gray silty clay, 20 30 40 little fine to medium sand, trace coarse sand, few cobbles, contains silt seamsnear top of stratum, 9 100 H=2.5633.3 3, 11B H=0.5-1.5Medium-stiff to stiff brown clayey silt, "and" fine to medium sand, trace coarse sand, includes sand seams, moist. [/]2 4 100 2010 NEW DEFAULT BORING LOG-W/ N60 30 629.5 Very-loose brown and gray fine to medium sand, little to "and" silt (percent varies), contains zones with a trace of coarse sand, wet. 13 0 100 0 67 35 3 67 0 100 16 620.0 40 - Encountered water at 5.5'. - Encountered cobbles at 18.5'. - Borehole converted to monitoring well upon completion - See separate well log. - Boring elevation recorded with a hand held GPS unit. Elevation estimated from March 2015 survey. - Datum: Ohio State Plane South, NAD 45 27/NAVD 29 (Plant Grid). SYMBOLS USED TO INDI Drill Rod Energy Ratio: 0.75 G - Gradation
Q - Uncon Comp
T - Triax Comp
C - Consol. WATER LEVEL: See H - Penetrometer (tsf) Last Calibration Date: 8/2/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

2010 NEW DEFAULT BORING LOG-W/ N60

LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH



11/24/15 - 11/25/15 LOCATION: N. 820,057, E. 2,513,275 ELEVATION: 670 DATE: 62.5' 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT **TEST** ELEV NATURAL MOISTURE CONTENT N_{60} DESCRIPTION RESULTS 0 **AGGREGATE - 12 INCHES** 20 30 669.0 FILL: Medium-dense brown fine to coarse sand, 8 some fine to coarse gravel, some to "and" silty 24 60 11 clay, dry. 2 13 60 3 13 73 5 664.5 FILL: Hard gray and brown silty clay, "and" fine to coarse sand, little to some fine to coarse gravel, 4 51 87 H=4.532 damp. 5 39 80 H=4.5661.5 FILL: Medium-dense brown and gray fine to 30 87 coarse sand, little fine to coarse gravel, some silty clay, damp. 660.0 FILL: Hard brown silty clay, some fine to coarse sand, some fine to coarse gravel (shale H=4.5fragments), damp. 19 80 H=4.5656.5 FILL: Medium-dense to dense brown fine to coarse gravel, some fine to coarse sand, some 80 8 45 H=3.0silty clay becoming trace silt at bottom of stratum, 15damp. 16 653.1 10A 20 100 Medium-stiff to stiff gray mottled with dark-gray 10B and brown silty clay, trace fine to coarse sand, trace fine gravel, few roots, few silt seams, slightly organic, moist. 20-SH 5 100 H=0.5-1.2511 647.0 Medium-stiff to very-stiff brown mottled with gray silty clay, trace to little fine to coarse sand, 8 100 damp. 12 H=3.5SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: Gradation Gradation See H - Penetrometer (tsf) **Last Calibration Date:** WATER NOTE: 8/2/2013 Separate W - Unit Dry Wt (pcf) Triax Comp Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

Page 2 of 3

2010 NEW DEFAULT BORING LOG-W/ N60

JOB: 7217-15-007B

LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT. BRILLIANT. OH



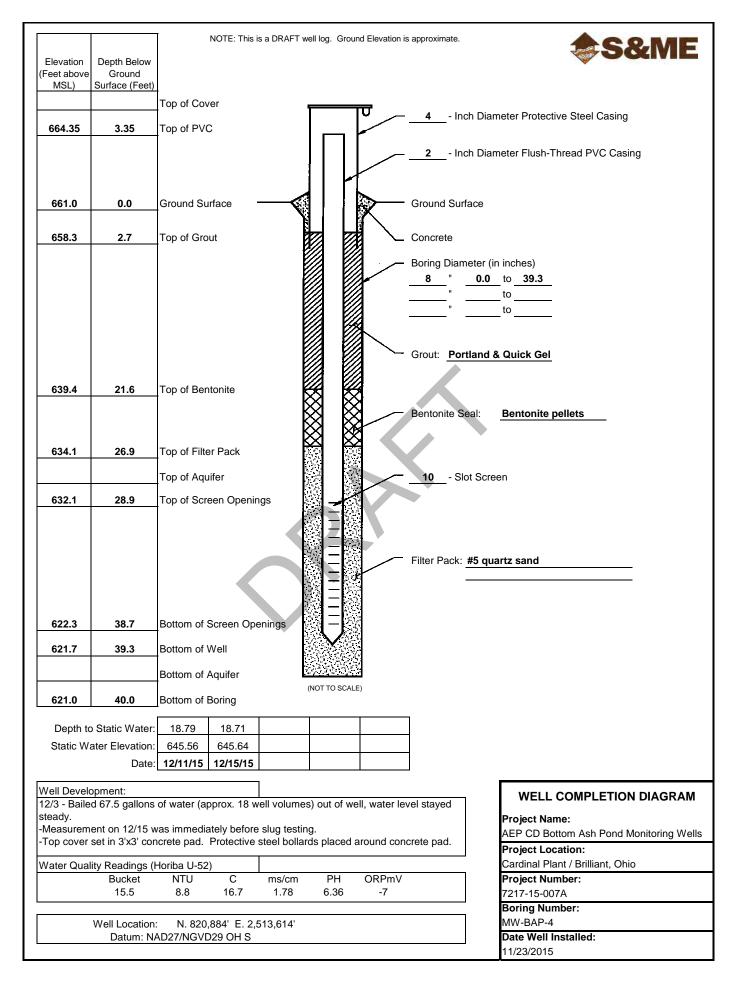
CARDINAL PLANT, BRILLIANT, OH LOCATION: N. 820,057, E. 2,513,275 11/24/15 - 11/25/15 **ELEVATION:** 670 DATE: 4-1/4" I.D. Hollow-stem Auger 62.5' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE REC-% NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET EFFORT ELEV **TEST** NATURAL MOISTURE CONTENT N_{60} DESCRIPTION RESULTS Medium-stiff to very-stiff brown mottled with 20 30 gray silty clay, trace to little fine to coarse sand, damp. P P 30 13 13 100 H=2.0-3.5100 H=2.5-3.011 35 100 H=2.5100 10 H=2.540 17 6 100 H=1.25SH SH, 0 100 H=1.25 624.5 Stiff gray mottled with brown and dark-gray silty clay, trace fine to coarse sand, slightly organic, SH SH, damp. 19 0 100 H=0.75622.0 Medium-stiff to stiff gray and dark-gray organic clayey silt, trace fine to coarse sand, damp. 20 SH, 0 100 H=0.75-1.25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.75 WATER LEVEL: - Gradation-- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: **Last Calibration Date:** 8/2/2013 T - Triax C - Consol Separate W - Unit Dry Wt (pcf) Triax Comp Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:

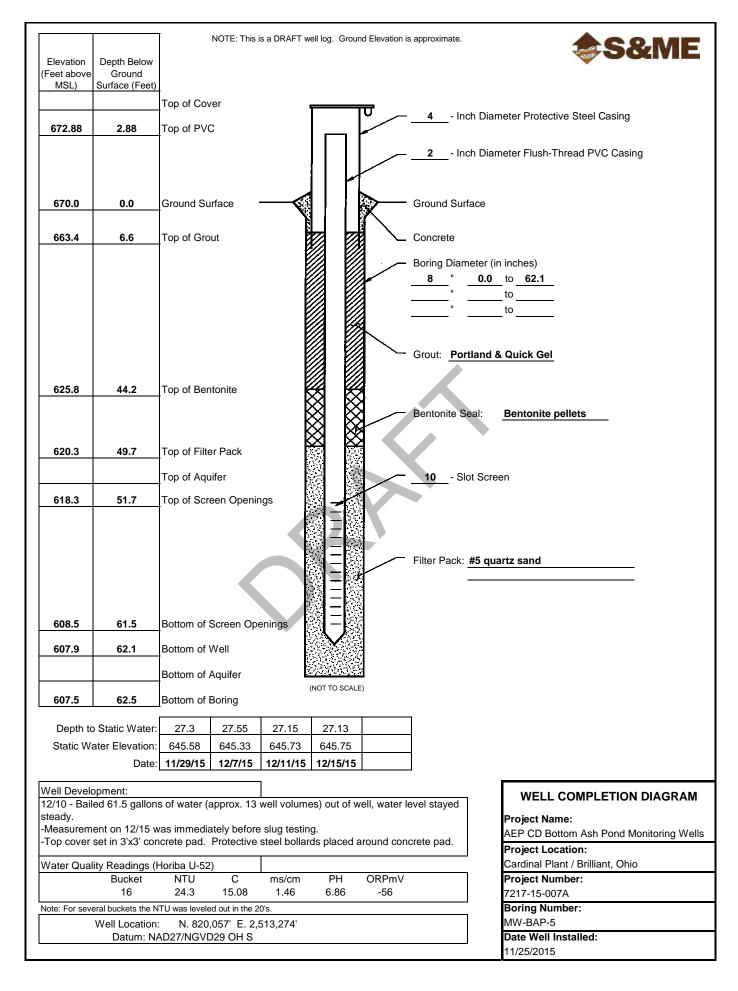
Page 3 of 3

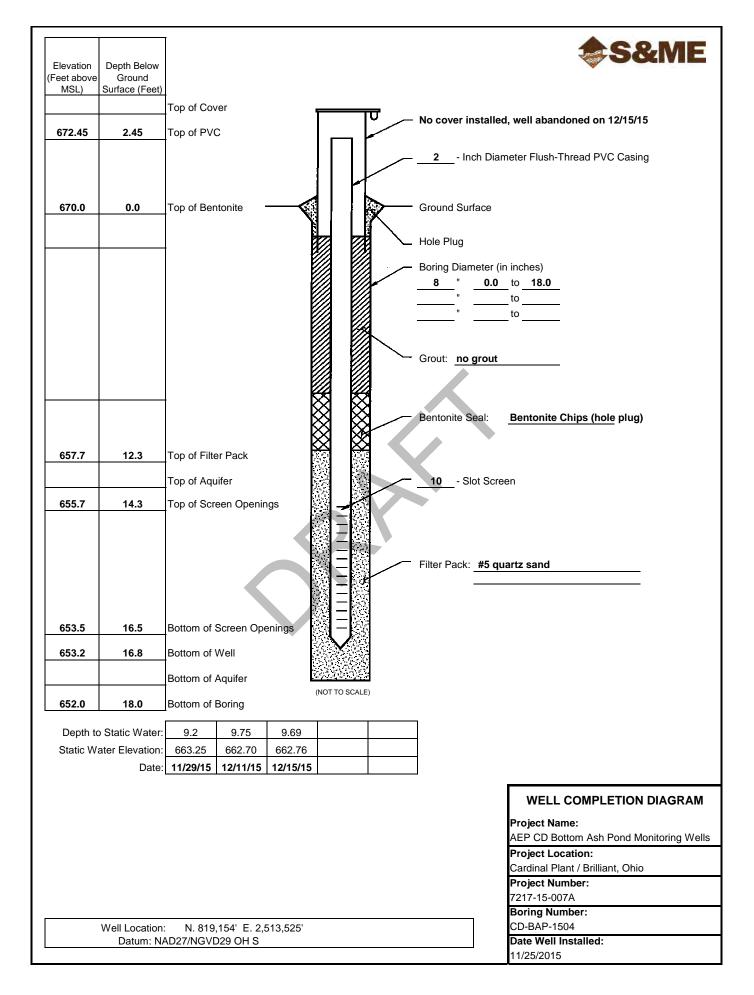
LOG OF BORING NO. MW-BAP-5 BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION CARDINAL PLANT, BRILLIANT, OH

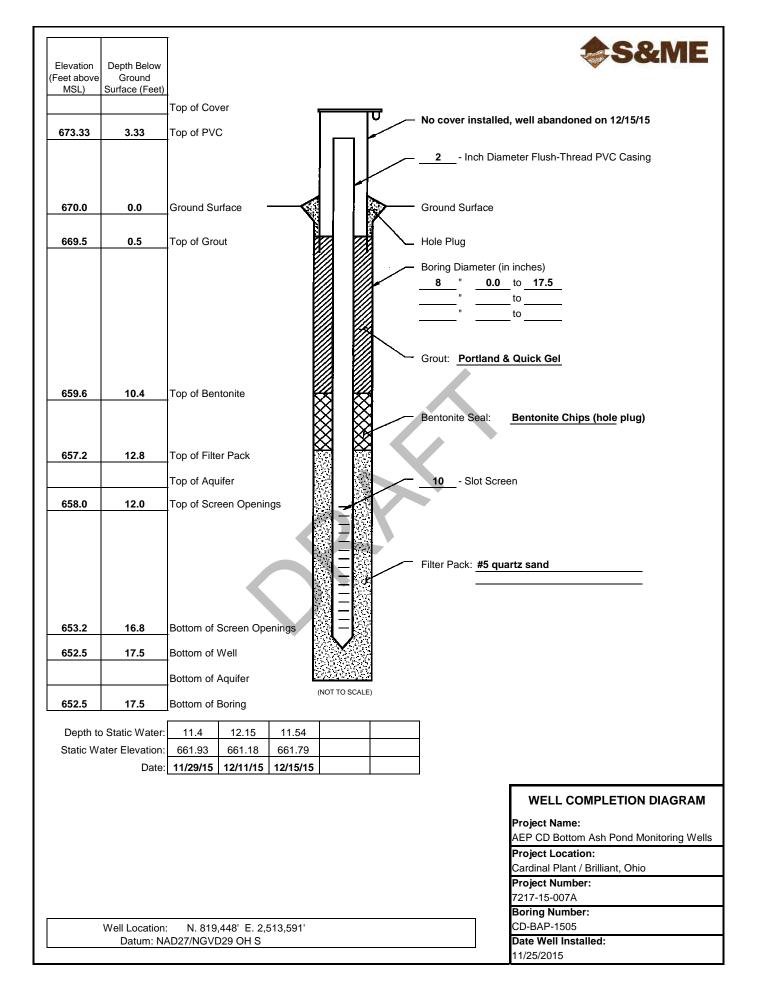


CARDINAL PLANT, BRILLIANT, OH LOCATION: N. 820,057, E. 2,513,275 11/24/15 - 11/25/15 **ELEVATION:** 670 DATE: 4-1/4" I.D. Hollow-stem Auger 62.5' DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE REC-% SAMPLE EFFORT DEPTH, FEET **TEST** ELEV NATURAL MOISTURE CONTENT DESCRIPTION N_{60} RESULTS 50-619.5 Medium-stiff to stiff gray and dark-gray organic 20 30 clayey silt, trace fine to coarse sand, damp. Medium-dense to dense fine to coarse gravel, 9 23 some to "and" fine to coarse sand, trace to little 21 87 silt, wet. [/]21 69 87 2010 NEW DEFAULT BORING LOG-W/ N60 55 614.6 Medium-dense to dense gray and brown fine to coarse sand, "and" fine to coarse gravel, little silt, wet. 23 43 80 35 24 60 60 11 60 607.5 - Encountered water at 17.0'. - Borehole converted to monitoring well upon 65 completion. See separate well log. - Boring location recorded with a hand-held GPS unit. Elevation estimated from March 2015 plant - Datum: Ohio State Plane South NAD 27/NAVD 29 (Plant Grid). 70-SYMBOLS USED TO INDI Drill Rod Energy Ratio: 0.75 G - Gradation
Q - Uncon Comp
T - Triax Comp
C - Consol. WATER LEVEL: See H - Penetrometer (tsf) Last Calibration Date: 8/2/2013 WATER NOTE: Separate W - Unit Dry Wt (pcf) Curves D - Relative Dens (%) **Drill Rig Number:** S&ME DATE:









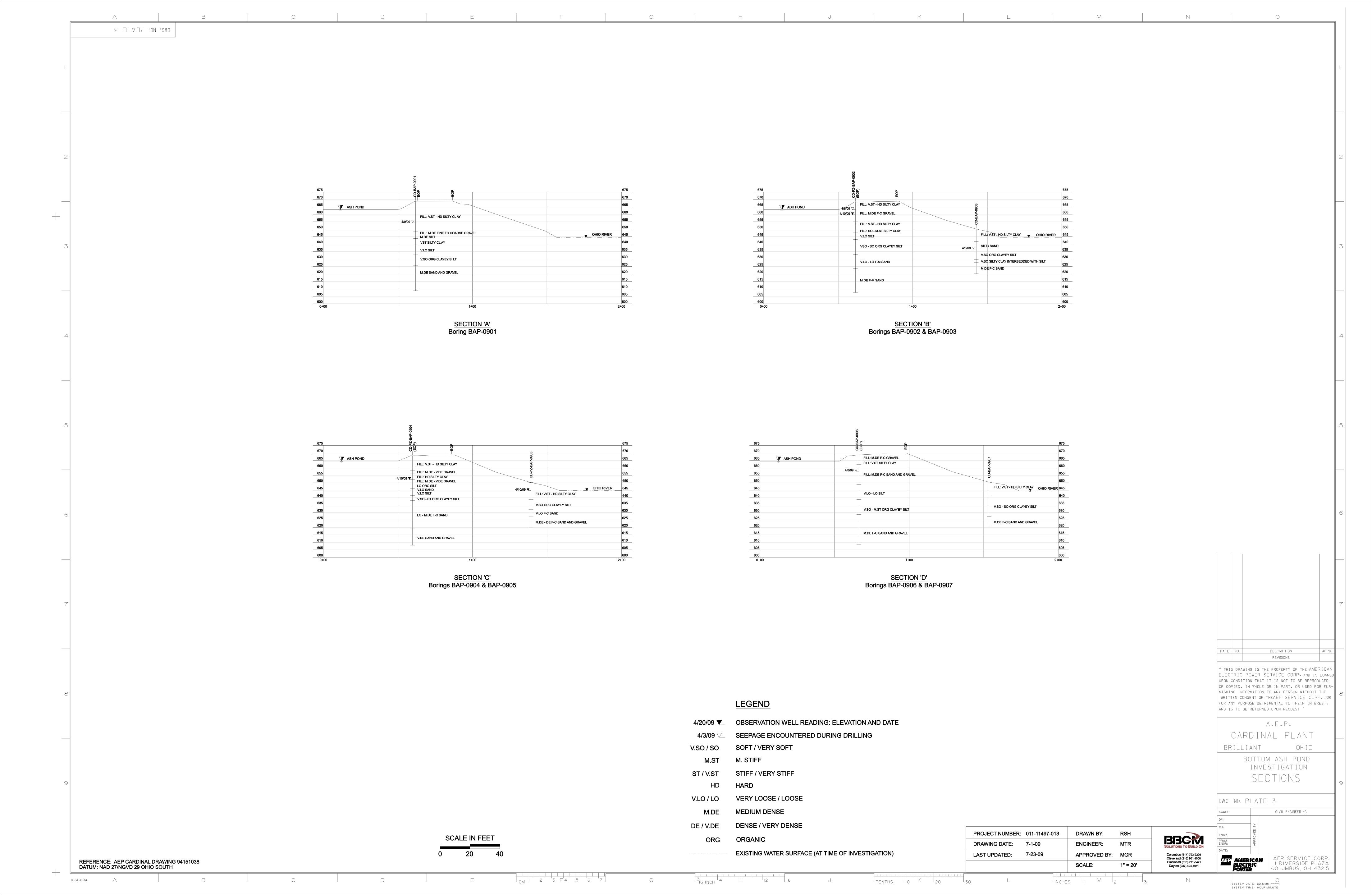
2009 SITE INVESTIGATION

Last Updated:

7-6-2009

Scale: 1" = 3000'

1.1



EXPLANATION OF SYMBOLS AND TERMS USED ON BORING LOGS FOR SAMPLING AND DESCRIPTION OF SOIL

SAMPLING DATA



- Blocked-in "SAMPLES" column indicates sample was attempted and recovered within this depth interval.



- Sample was attempted within this interval but not recovered.

2/5/9

- The number of blows required for each 6-inch increment of penetration of a "Standard" 2-inch O.D. split-barrel sampler, driven a distance of 18 inches by a 140-pound hammer freely falling 30 inches. Addition of one of the following symbols indicates the use of a split-barrel other than the 2" O.D. sampler:

2S

- 2½"O.D. split-barrel sampler

3S

- 3" O.D. split-barrel sampler

~ ~1

- Shelby tube sampler, 3" O.D., hydraulically pushed.

P -

- Refusal of sampler in very-hard or dense soil, or on a resistant surface.

50-2"

R

- Number of blows (50) to drive a split-barrel sampler a certain number of inches (2), other than the normal 6-inch increment.

S/D

- Split-barrel sampler (S) advanced by weight of drill rods (D),

S/H

- Split-barrel sampler (S) advanced by combined weight of rods and drive hammer (H).

SOIL DESCRIPTIONS

All soils have been classified basically in accordance with the Unified Soil Classification System, but this system has been augmented by the use of special adjectives to designate the approximate percentages of minor components as follows:

<u>Adjective</u>	Percent by Weight
trace	1 to 10
little	11 to 20
some	21 to 35
"and"	36 to 50

The following terms are used to describe density and consistency of soils:

Term (Granular Soils)	Blows per foot
Very-loose	Less than 5
Loose	5 to 10
Medium-dense	11 to 30
Dense	31 to 50
Very-dense	Over 50
Term (Cohesive Soils)	Qu (tsf)
Very-soft	Less than 0.25
Soft	0.25 to 0.5
Medium-stiff	0.5 to 1.0
Stiff	1.0 to 2.0
Very-stiff	2.0 to 4.0
Hard	Over 4.0

LOG OF BORING NO. CD-BAP-0901 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A DATE: ELEVATION: **668.7** 3-1/4" I.D. Hollow-stem Auger 60.0 DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE SAMPLE REC-% DEPTH FEET **TEST** ELEV -NATURAL MOISTURE CONTENT 09 Z DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 **GRAVEL FILL - 0.9 FEET** 10 20 30 40 667.8 FILL: Hard gray and brown silty clay, some fine 111497013.GPJ BBCM.GDT to coarse sand, some fine to coarse gravel 13 30 80 H=4.5+(sandstone, siltstone, and shale fragments), dry. 666.2FILL: Medium-dense to dense brown and gray fine to coarse gravel (sandstone, siltstone, and 2 16 67 H=2.5-3.5shale fragments), some fine to coarse sand, "and" silty clay, dry. 3 60 100 H = 2.52008 NEW DEFAULT BORING LOG-W/ N60 5 4 60 80 H=4.5+661.7 FILL: Hard gray clayey silt, some fine to coarse 10, 37 sand, some fine to coarse gravel (sandstone, 5 93 H=4.5+16 siltstone and shale fragments), dry. 660.2FILL: Very-stiff brown and gray silty clay, some fine to coarse sand, some fine to coarse gravel 34 87 H=3.0-4.06 16 (sandstone, siltstone, and shale fragments), dry. 658.7 10-FILL: Medium-dense to dense gray and brown fine to coarse gravel (sandstone, siltstone, and 7 70 100 H = 4.5 +shale fragments), some fine to coarse sand, some silty clay becoming "and" clayey silt with depth, dry. 8 20 67 29 73 • × H=4.5+× 654.2 FILL: Very-stiff to hard brown and gray silty 15 clay, some fine to coarse sand, some fine to 10 32 80 H=4.0coarse gravel (sandstone, siltstone, and shale 4.5 +fragments), medium-dense gray and brown fine to coarse gravel (shale fragments) seam from 20 67 H=3.8-11 17.5' to 18.3', moist to wet. 4.5 +12 22 53 G 26 53 H=4.5 13 20-648.2 FILL: Medium-dense gray fine to coarse gravel (shale fragments), little fine to coarse sand, little 14 32 67 H=4.5silty clay, moist to wet. 646.7 Medium-dense gray silt, trace clay, trace fine to medium sand, moist to wet. 80 15 27 G 10 16A SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: 0.86 13.8 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA Last Calibration Date: 02/17/09 -Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp 4/9/09 Drill Rig Number: TRUCK 55 DATE: Curves - Consol

LOG OF BORING NO. CD-BAP-0901 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.7 DATE: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE SAMPLE REC-% -25 FEET **TEST** -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT Very-stiff brown mottled with gray silty clay, 10 20 30 40 16B. 111497013.GPJ BBCM.GDT 8/4/09 trace fine sand, damp. 6 17 22 67 H=2.5-3.510 100 \times H=1.6-2.5 638.7 2008 NEW DEFAULT BORING LOG-W/ N60 Gray mottled with dark-gray and brown clayey silt, some fine sand, trace medium to coarse sand, few seams and lenses of silty clay and fine sand, H=1.0-1.5damp. \bullet × X 19 G 635.9 Very-loose dark-brown and gray organic silt, some fine sand, moist to wet. 100 H=0.735-633.2 Soft to medium-stiff gray mottled with dark-gray organic clayey silt, little to some fine sand, trace medium to coarse sand, few lenses of fine sand 21 6 100 H = 0.4interbedded with organic silt near top of stratum, moist to wet. 22 100 H=0.5-0.8× 40-H=0.3-0.767 625.7 Medium-dense to dense brown and gray fine to coarse gravel, some fine to coarse sand, trace silt, wet. 24 34 53 45 2.5 40 53 53 50-SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio : <u>0.86</u> WATER LEVEL: 13.8 - Gradation -- Uncon Comp See Penetrometer (tsf) WATER NOTE: Inside HSA **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp 4/9/09 Drill Rig Number: TRUCK 55 DATE: Curves - Consol

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LOG OF BORING NO. CD-BAP-0901 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



BRILLIANT, OHIO LOCATION: See Plate 2 of Appendix A DATE: ELEVATION: 668.7 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE REC-% NATURAL CONSISTENCY INDEX SAMPLE DEPTH, FEET **TEST** -NATURAL MOISTURE CONTENT 09 N **DESCRIPTION** RESULTS PLASTIC LIMIT LIQUID LIMIT 50-Medium-dense to dense brown and gray fine to 10 20 30 40 2008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 coarse gravel, some fine to coarse sand, trace silt, 16 53 33 28 29 33 55 29 30 40 610.7 Medium-dense brown fine to medium sand, trace coarse sand, trace fine gravel, trace silt, wet. 30 40 608.7 60-- Seepage encountered at 14.5'. - Borehole grouted upon completion. - Boring location and elevation surveyed by AEP. 65 70-75 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio : <u>0.86</u> 13.8 - Gradation -- Uncon Comp G Q T C See H - Penetrometer (tsf) WATER NOTE: Inside HSA **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) - Triax Comp 4/9/09 Drill Rig Number: TRUCK 55 DATE: Curves

LOG OF BORING NO. CD-PZ-BAP-0902 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.0 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.5 COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE REC-% DEPTH FEET ELEV. **TEST** -NATURAL MOISTURE CONTENT 09 Z DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 **GRAVEL FILL - 1.0 FEET** 10 20 30 40 667.0FILL: Very-stiff to hard brown silty clay, some 111497013.GPJ BBCM.GDT fine to coarse sand, some fine to coarse gravel 17 87 H=3.5-4.0(sandstone, siltstone, and shale fragments), dry. 665.5FILL: Medium-dense brown and gray fine to 20 80 coarse gravel (sandstone, siltstone, and shale H=3.75fragments), some fine to coarse sand, some silty 4.25 clay, cobbles near top of stratum, dry. 27 73 H=4.0-3 2008 NEW DEFAULT BORING LOG-W/ N60 5 10 4.5 +4 17 73 • × H=3.0-4.25 23 53 H=3.5-4.027 H=3.75-16 4.0 10-<u>⊽</u> 7 29 60 H=4.0-4.5+ H=3.0-3.75 8 17 73 G 655.0 FILL: Very-stiff to hard brown and gray silty 9 10 33 clay, some fine to coarse sand, trace to some fine H=3.75gravel (siltstone and shale fragments), damp to 4.5 +wet. 15 7 10 40 H=2.5-2.75 652.0 FILL: Soft to medium-stiff brown and gray silty clay, some fine to coarse sand, trace to some fine 13 H=1.0-2.011 67 gravel (siltstone and shale fragments), brown and H=1.5gray fine to coarse gravel, some near middle of 2.25 stratum, wet. 12 6 40 \times G H=0.0-0.25 1 20 13 X 20-G 647.5 Very-loose gray and dark-gray silt, little to some clay, trace becoming some with depth fine sand, 4 100 G wet. 3 53 15 G SH, 53 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: 0.86 10.7 8.4 - Gradation -- Uncon Comp See Penetrometer (tsf) WATER NOTE: Inside HSA Inside Well **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp 4/8/09 4/10/09 Drill Rig Number: TRUCK 55 DATE: - Consol Curves

LOG OF BORING NO. CD-PZ-BAP-0902 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.0 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.5' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE REC-% NATURAL CONSISTENCY INDEX SAMPLE TEET -25 **TEST** -NATURAL MOISTURE CONTENT DESCRIPTION 09 RESULTS LIQUID LIMIT LIMIT Very-soft to soft gray mottled with dark-gray 10 20 30 40 2008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 17 4 80 organic clayey silt, trace fine sand, few lenses of H=0.3organic silt near bottom of stratum, wet. LOI=10.49 18 4 80 >>**\end{array}**H=0.0-0.1 MC=54 0.0 = H19 100 G 30-3 73 G 633.1 35-Very-loose to loose brown and gray fine to 21B medium sand, trace coarse sand, trace to little silt interbedded with organic clayey silt, wet. 73 G 23 80 40 G 627.0 Medium-dense brown fine to medium sand, trace coarse sand, trace silt, trace to some fine gravel, trace coarse gravel, trace silt, wet. 19 100 G 25A 26 45 25B 26 33 67 40 33 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio : <u>0.86</u> WATER LEVEL: 10.7 8.4 - Gradation -- Uncon Comp See Penetrometer (tsf) Q T C WATER NOTE: Inside HSA Inside Well **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp Triax 4/8/09 4/10/09 Drill Rig Number: TRUCK 55 DATE: Curves - Consol.

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LOG OF BORING NO. CD-PZ-BAP-0902 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.0 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.5' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE REC-% DEPTH, FEET **TEST** -NATURAL MOISTURE CONTENT **DESCRIPTION** 09 Z RESULTS PLASTIC LIMIT LIQUID LIMIT 50-Medium-dense brown fine to medium sand, trace 10 20 30 40 :008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 coarse sand, trace silt, trace to some fine gravel, trace coarse gravel, trace silt, wet. 28 26 80 67 55 14 67 31 14 607.9 - 60-- Cobbles encountered from 4.0' to 7.0'. - Seepage encountered at 5.5'. - Groundwater encountered at 13.0'. - Borehole converted to observation well upon completion. See separate well log. - Boring location and elevation surveyed by AEP. 65 70-75 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: 0.86 10.7 8.4 - Gradation -- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** <u>02/17/09</u> WATER NOTE: Inside HSA **Inside Well** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Triax Comp 4/10/09 4/8/09 Drill Rig Number: TRUCK 55 DATE: Curves

LOG OF BORING NO. CD-BAP-0903 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 650.1 DATE: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 30.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE SAMPLE REC-% DEPTH FEET ELEV. **TEST** -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 -649.7 **TOPSOIL - 0.4 FEET** 10 20 30 40 8/4/09 FILL: Very-stiff to hard brown mottled with gray and dark-brown silty clay, trace fine to 111497013.GPJ BBCM.GDT 15 67 medium sand, few roots, damp. H=3.6-3.816 53 H=3.3-4.5 646.1 FILL: Very-stiff to hard brown mottled with gray silty clay, trace fine sand, damp. 15 80 H=2.6-4.13 2008 NEW DEFAULT BORING LOG-W/ N60 5 4 33 80 H = 4.5643.1 FILL: Very-stiff to hard brown mottled with dark-gray and gray silty clay, little fine to coarse 16 H=3.5-4.5641.8 sand, trace fine gravel, few lenses of dark-gray 6, Medium-stiff dark-gray organic clayey silt, trace 16 67 ● H=0.6 fine sand, many lenses of fine sand, few decayed 10roots, damp to moist. 636.6 Very-soft gray mottled with dark-gray organic 3 67 clayey silt interbedded with organic silt, little fine H = 0.0sand, few seams and lenses of silt and fine sand, 15 moist to wet. 67 H = 0.03 73 × H = 0.020-629.6 Very-soft gray silty clay interbedded with silt, trace fine sand, few seams of fine sand, few roots, moist to wet. 10 8 60 H = 0.2627.6 Medium-dense brown and gray fine to coarse sand, trace medium to coarse sand, trace fine to coarse gravel, little silt, few seams of silty clay, wet. 47 G 25 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: <u>0.82</u> 16.5 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA Last Calibration Date: 11/19/07 -Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp Triax 4/8/09 Drill Rig Number: <u>D50</u> DATE: - Consol. Curves

LOG OF BORING NO. CD-BAP-0903 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



BRILLIANT, OHIO LOCATION: See Plate 2 of Appendix A ELEVATION: 650.1 DATE: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 30.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE SAMPLE REC-% NATURAL CONSISTENCY INDEX DEPTH, FEET ELEV. **TEST** NATURAL MOISTURE CONTENT 09 N DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 25-624.6 10 20 30 40 111497013.GPJ BBCM.GDT 8/4/09 Medium-dense brown and gray fine to coarse gravel, some fine to coarse sand, trace silt, wet. 10, 30 33 19 13 47 620.130-2008 NEW DEFAULT BORING LOG-W/ N60 - Seepage encountered at 13.5'. - Groundwater encountered at 22.5'. - At 26.0', 1.8' heave, shook augers to sample. - Borehole grouted upon completion. - Boring location and elevation surveyed by AEP. 35-40-45 └ 50-SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $\overline{igspace}$ Drill Rod Energy Ratio: 0.82 16.5 - Gradation -- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** <u>11/19/07</u> WATER NOTE: Inside HSA -Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) - Triax Comp 4/8/09 Drill Rig Number: <u>D50</u> DATE: Curves

LOG OF BORING NO. CD-PZ-BAP-0904 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.1 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0 COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE SAMPLE REC-% DEPTH FEET **TEST** ELEV -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS LIQUID LIMIT PLASTIC LIMIT 0 · **GRAVEL FILL - 1.0 FEET** 10 20 30 40 111497013.GPJ BBCM.GDT 8/4/09 667.1 FILL: Very-stiff to hard brown and gray silty 6 clay, some fine to coarse sand, some fine to 20 100 H=4.25coarse gravel (sandstone, siltstone, and shale 4.5 +fragments), fine to coarse gravel seams near middle of stratum, dry. 27 53 H=4.5+11 33 93 H=3.5-4.03 3008 NEW DEFAULT BORING LOG-W/ N60 5 12 4 46 7 23 13 H=2.75-3.5 36 80 6 G 658.1 10-FILL: Very-dense brown and gray fine to coarse 50-3"R gravel (sandstone, siltstone, and shale fragments), 7 33 little fine sand, trace silt, dry. 656.6 FILL: Dense brown and gray fine to coarse 44 8A 655.9 gravel (sandstone fragments), cobbles, "and" fine 8B H=4.5+to medium sand, trace coarse sand, trace silt, dry. FILL: Hard brown with gray silty clay, little to some fine to coarse sand, trace fine gravel, dry. 9 20 73 H=2.5-40 $\dot{\times}$ 15 10 19 80 H=3.0-4.25 652.1 FILL: Medium-dense brown and gray fine to coarse gravel (very-soft shale fragments), some 26 60 11 G fine to coarse sand, some silty clay, cobbles, 20 12A 12B 649.1 Loose gray and dark-gray organic silt, little clay, little to some fine to medium sand, wet. 6 87 13 20-G 14 10 47 646.1 Very-loose gray and dark-gray fine to medium 15 47 sand, trace coarse sand, little fine gravel, some G organic silt, wet. 644.6 SH Very-loose gray silt, little clay, little fine sand, 0 53 wet. 16 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: <u>0.86</u> WATER LEVEL: 15.9 16.0 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA Inside Well Q T C Last Calibration Date: 02/17/09 Separate W - Unit Dry Wt (pcf) Triax Comp 4/7/09 4/10/09 D - Relative Dens (%) Drill Rig Number: TRUCK 55 DATE: Curves - Consol

LOG OF BORING NO. CD-PZ-BAP-0904 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 668.1 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE REC-% DEPTH FEET **TEST** -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT Very-loose gray silt, little clay, little fine sand, 10 20 30 40 17 4 53 wet. • G 641.6 111497013.GPJ BBCM.GDT Medium-stiff to stiff gray mottled with dark-gray organic clayey silt, interbedded with organic silt, 18 6 100 H=0.75little fine to coarse sand, trace fine gravel, wet. F.25 640.1 Very-soft to soft gray mottled with dark-gray 19 6 87 organic clayey silt, trace fine sand, wet. H=0.0-0.5638.1 2008 NEW DEFAULT BORING LOG-W/ N60 30-Loose to medium-dense brown and gray fine to medium sand, trace coarse sand, trace to some silt, few seams of gray mottled with dark-gray silty clay near bottom of stratum, contains zones interbedded with silt, wet. 20A 17 20B 35-21 11 93 G 22 10 100 40 100 24 29 100 45 40 621.4 Medium-dense brown and gray fine to coarse 25B gravel, "and" fine to coarse sand, trace silt, wet. 619.1 93 See description on the following page. 50-SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio : <u>0.86</u> WATER LEVEL: 15.9 16.0 - Gradation -- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** <u>02/17/09</u> WATER NOTE: Inside Well Inside HSA Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp Triax 4/7/09 4/10/09 Drill Rig Number: TRUCK 55 DATE: - Consol. Curves

Page 3 of 3

LOG OF BORING NO. CD-PZ-BAP-0904 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A DATE: ELEVATION: 668.1 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE REC-% SAMPLE DEPTH, FEET **TEST** NATURAL MOISTURE CONTENT 09 N DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT -50-Very-dense brown and gray fine to coarse sand, 10 20 30 40 2008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 some fine to coarse sand, trace silt, zones of fine to coarse gravel, wet. 40. 87 28 33 55 67 50-3"R 50-5"R 30 67 608.1 60-- Cobbles encountered at 10.0', 11.5' and 13.0'. - Groundwater encountered at 16.0'. - Borehole converted to observation well upon completion. See separate well log. - Boring location and elevation surveyed by AEP. 65 70--75 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio : <u>0.86</u> 15.9 16.0 - Gradation -- Uncon Comp G Q T C See H - Penetrometer (tsf) WATER NOTE: Inside HSA Inside Well **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Triax Comp 4/10/09 4/7/09 Drill Rig Number: TRUCK 55 DATE: - Consol. Curves

LOG OF BORING NO. CD-PZ-BAP-0905 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A ELEVATION: 650.1 DATE: 4-1/4" I.D. Hollow-stem Auger 30.0 DRILLING METHOD: COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE SAMPLE REC-% DEPTH FEET **TEST** ELEV -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 -649.6 ROOTMAT - 0.5 FEET 10 20 30 40 111497013.GPJ BBCM.GDT 8/4/09 FILL: Very-stiff to hard brown mottled with gray silty clay, trace fine sand, few lenses of 11 67 dark-gray silt and fine sand near bottom of H=4.0-4.5stratum, moist. 19 100 H=4.0-4.523 100 H=3.0-4.52008 NEW DEFAULT BORING LOG-W/ N60 5 48 100 H=4.5+∑ 5 34 100 H=4.0-•× 4.5+ 6A 5 21 H=3.5-4.5640.4 10 6B H=1.5-3.0FILL: Stiff to very-stiff brown mottled with gray 10-639.6 silty clay interbedded with dark-gray organic silt, little fine to coarse sand, trace fine gravel, moist. 638.9 7A 5 H=3.5-FILL: Very-stiff brown mottled with gray silty 3.75 7B clay, trace fine to coarse sand, trace fine gravel, H=0.0moist. Very-soft gray mottled with dark-gray organic clayey silt, trace fine to coarse sand, trace fine gravel, moist becoming wet. 0 100 × H = 0.015 LOI=8.4% 100 H = 0.0632.1 Very-loose brown and gray fine to coarse gravel, some fine to coarse sand, little silt, contains decayed wood, wet. 3 33 H = 0.520-629.6 Very-soft gray mottled with brown silty clay, little fine to medium sand, few seams of fine to 27 medium sand, wet. 11 16 X G 627.1 Medium-dense to dense brown and gray fine to coarse sand, trace to little fine to coarse gravel, trace silt, contains roots near top of stratum, 12 30 contains zones of fine to coarse gravel, wet. 12 SYMBOLS US
G - Gradation
Q - Uncon Comp
T - Triax Com
C - Com 25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: <u>0.82</u> WATER LEVEL: 8.0 See H - Penetrometer (tsf) WATER NOTE: Inside Well Inside Well Last Calibration Date: 11/19/07 -Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) 4/7/09 4/10/09 Drill Rig Number: <u>D50</u> DATE: Curves

LOG OF BORING NO. CD-PZ-BAP-0905 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



BRILLIANT, OHIO LOCATION: See Plate 2 of Appendix A ELEVATION: 650.1 DATE: 4-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 30.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE REC-% -25 FEET **TEST** NATURAL MOISTURE CONTENT **DESCRIPTION** 09 N RESULTS LIQUID LIMIT PLASTIC LIMIT Medium-dense to dense brown and gray fine to 10 20 30 40 111497013.GPJ BBCM.GDT 8/4/09 coarse sand, trace to little fine to coarse gravel, trace silt, contains roots near top of stratum, contains zones of fine to coarse gravel, wet. 48 67 G 51 73 620.130-2008 NEW DEFAULT BORING LOG-W/ N60 - Groundwater encountered at 18.0'. - Encountered decayed wood at 18.5'. - Borehole converted to observation well upon completion. See separate well log. - Boring location and elevation surveyed by AEP. 35-40-45 - 50-SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: <u>0.82</u> 5.4 8.0 - Gradation -- Uncon Comp See H - Penetrometer (tsf) **Last Calibration Date:** <u>11/19/07</u> WATER NOTE: Inside Well Inside Well Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) - Triax Comp 4/10/09 4/7/09 Drill Rig Number: <u>D50</u> DATE: Curves

LOG OF BORING NO. CD-BAP-0906 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A DATE: ELEVATION: 668.6 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE SAMPLE SAMPLE REC-% DEPTH FEET **TEST** -NATURAL MOISTURE CONTENT 09 Z DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 FILL: Medium-dense brown and gray fine to 10 20 30 40 8/4/09 coarse gravel (shale and siltstone fragments), some fine to coarse sand, some silty clay, dry. 111497013.GPJ BBCM.GDT 12 42 20 666.1 FILL: Medium-dense dark-gray fine to medium 2A 23 665.3 sand, trace coarse sand, little fine gravel, some 2B H=2.5-3.5 clayey silt, dry to damp. FILL: Very-stiff brown and gray silty clay and clayey silt, some fine to coarse sand, little fine 3 13 33 H = 2.32008 NEW DEFAULT BORING LOG-W/ N60 5 gravel (sandstone, siltstone, and shale fragments), damp. 662.1 4 24 40 H=2.3-3.3FILL: Medium-dense brown and gray fine to coarse gravel "and" fine to coarse sand, some silty clay (sandstone and siltstone fragments), 20 67 stiff brown silty clay seam at 13.5', damp. 6A -10-_▽6B 7 33 60 12 8 33 67 G 50 60 H=2.215-32 40 10 652.1 FILL: Very-stiff brown silty clay, some fine to coarse sand, some fine to coarse gravel, damp to 19 53 • H = 2.211 650.6 Very-loose to loose gray silt, trace to some fine 12A sand, trace to little fine to medium sand, trace fine gravel, few seams of gray fine to medium 12B sand, damp becoming wet at 20'. 20-13 6 80 0 0 67 100 25 SYMBOLS USED TO INDICATE TEST RESULTS Drill Rod Energy Ratio: 0.86 WATER LEVEL: 10.3 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA Last Calibration Date: 02/17/09 Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp 4/10/09 Drill Rig Number: TRUCK 55 DATE: - Consol Curves

LOG OF BORING NO. CD-BAP-0906 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A 668.6 DATE: ELEVATION: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER NATURAL CONSISTENCY INDEX SAMPLE REC-% SAMPLE SAMPLE -25 FEET **TEST** -NATURAL MOISTURE CONTENT 09 Z DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT G Very-loose to loose gray silt, trace to some fine 10 20 30 40 sand, trace to little fine to medium sand, trace fine gravel, few seams of gray fine to medium 13 67 111497013.GPJ BBCM.GDT G sand, damp becoming wet at 20'. G 17 80 6 67 18 638.6 2008 NEW DEFAULT BORING LOG-W/ N60 Very-soft to medium-stiff gray organic clayey silt, trace fine to coarse sand, trace fine gravel, contains seams of silty clay, silt and fine to medium sand, wet. 6 67 H = 0.9H=0.0-0.25 60 G LOI=7.9% 35-21 47 \times H = 0.022 20 53 H = 0.9628.6 40 Medium-dense brown and gray fine to coarse gravel, some fine to coarse sand, trace to little silt, contains zones of fine to coarse sand, wet. 19 40 24 23 47 G 45 70 34 67 - 50-SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio : <u>0.86</u> 10.3 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp 4/10/09 Drill Rig Number: TRUCK 55 DATE: Curves - Consol

Page 3 of 3

LOG OF BORING NO. CD-BAP-0906 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



LOCATION: See Plate 2 of Appendix A 668.6 DATE: ELEVATION: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 60.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE SAMPLE REC-% NATURAL CONSISTENCY INDEX DEPTH, FEET **TEST** -NATURAL MOISTURE CONTENT 09 N **DESCRIPTION** RESULTS PLASTIC LIMIT LIQUID LIMIT - 50-Medium-dense brown and gray fine to coarse 10 20 30 40 :008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 gravel, some fine to coarse sand, trace to little silt, contains zones of fine to coarse sand, wet. 10, 37 47 28 32 60 55 29 33 5 23 608.660-- Groundwater encountered at 20.0'. - Cobbles encountered throughout the borehole. - Borehole grouted upon completion. - Boring location and elevation surveyed by AEP. 65 70--75 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio : <u>0.86</u> 10.3 - Gradation -- Uncon Comp G Q T C See H - Penetrometer (tsf) WATER NOTE: Inside HSA **Last Calibration Date: 02/17/09** Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) - Triax Comp 4/10/09 Drill Rig Number: TRUCK 55 DATE: Curves

LOG OF BORING NO. CD-BAP-0907 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO

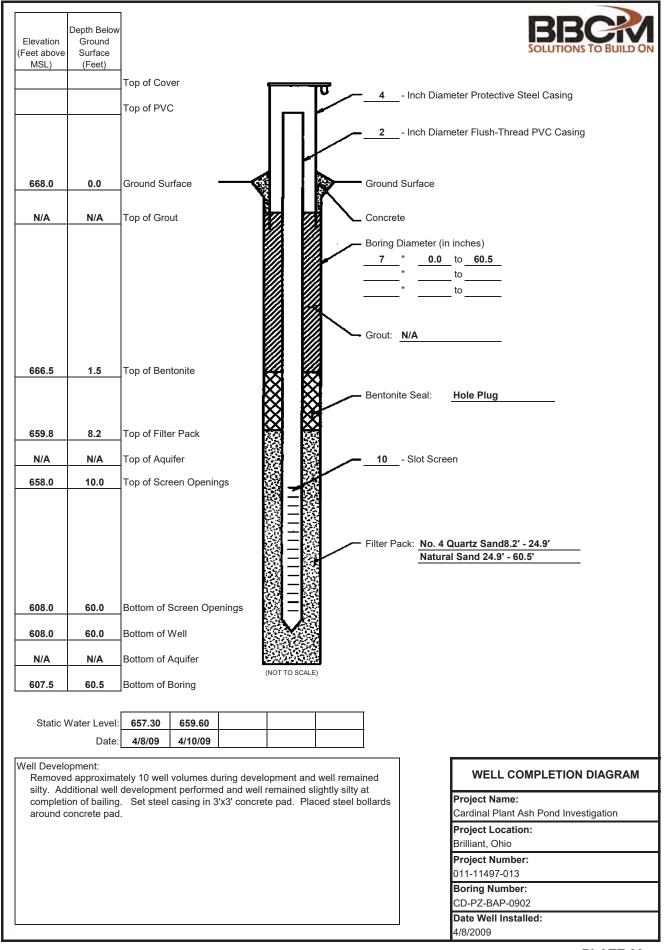


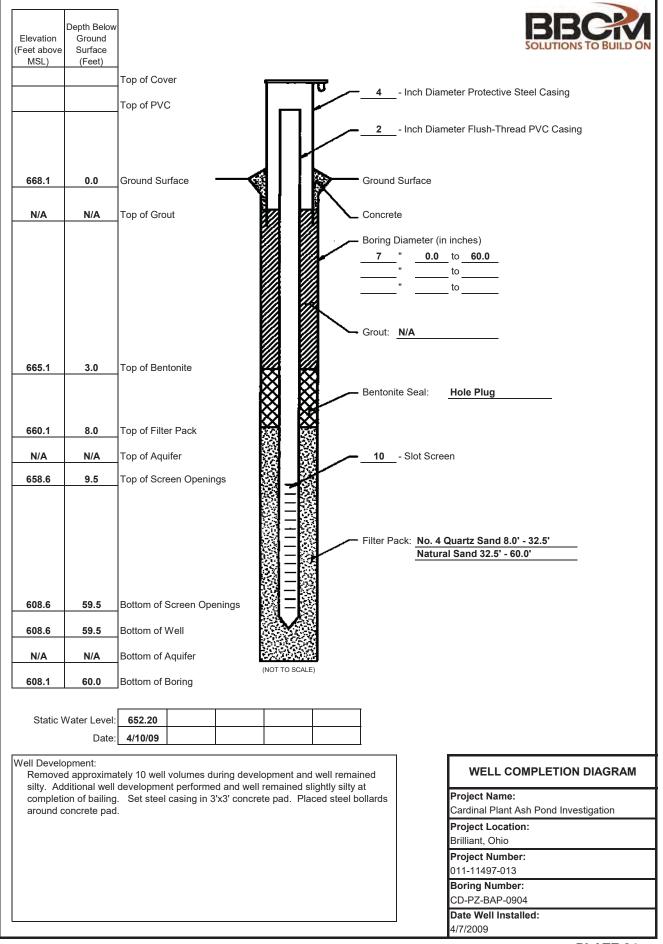
LOCATION: See Plate 2 of Appendix A ELEVATION: 650.3 DATE: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 30.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE NATURAL CONSISTENCY INDEX SAMPLE REC-% DEPTH FEET ELEV. **TEST** -NATURAL MOISTURE CONTENT 09 DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT 0 -649.9 **TOPSOIL - 0.4 FEET** 10 20 30 40 2008 NEW DEFAULT BORING LOG-W/ N60 111497013.GPJ BBCM.GDT 8/4/09 FILL: Very-stiff to hard brown mottled with gray silty clay, trace to little fine to coarse sand, 14 47 trace fine gravel, few roots near top of stratum, H=2.2-2.4contains fine to medium sand lenses and seams near middle of stratum, damp. 21 73 H=3.9-4.23 18 80 H = 4.55 4 36 100 H = 4.521 67 • × ×H=4.1-4.5 641.8 FILL: Hard brown, gray and dark-gray silty clay intermixed with organic silt, little fine to coarse H=4.56A 640.6 G sand, trace fine gravel, damp. 10-6B Stiff gray organic silt, little fine to medium sand, H = 2.2639.6 few lenses of fine sand, damp to moist. Very-soft to soft gray organic clayey silt, little SH, 0 fine to medium sand, trace fine gravel, damp to 67 H = 0.0SH moist. 0 73 × H = 0.015 H=0.0-0.25 SH, 67 G H=0.0-0.25 0 10 73 G 20-H=0.0-0.50 11 4 67 G 627.3 Medium-dense gray-brown and gray fine to coarse gravel, "and" fine to coarse sand, trace to little silt, wet. 19 25 SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: $^{
abla}$ Drill Rod Energy Ratio: 0.82 16.3 - Gradation -- Uncon Comp See H - Penetrometer (tsf) WATER NOTE: Inside HSA **Last Calibration Date: 11/19/07** -Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) Comp Triax 4/8/09 Drill Rig Number: <u>D50</u> DATE: Curves - Consol.

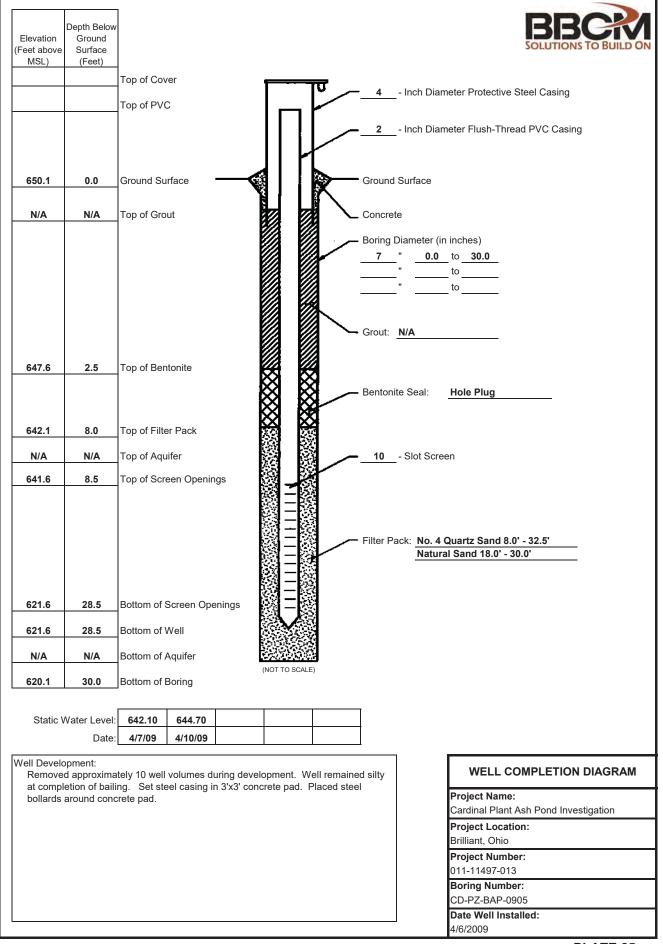
LOG OF BORING NO. CD-BAP-0907 CARDINAL PLANT ASH POND INVESTIGATION BRILLIANT, OHIO



BRILLIANT, OHIO LOCATION: See Plate 2 of Appendix A ELEVATION: 650.3 DATE: 3-1/4" I.D. Hollow-stem Auger DRILLING METHOD: 30.0' COMPLETION DEPTH: 2" O.D. Split-barrel Sampler 3" O.D. Shelby Tube Sampler SAMPLER(S): SAMPLE NUMBER SAMPLE SAMPLE REC-% NATURAL CONSISTENCY INDEX -25 FEET **TEST** NATURAL MOISTURE CONTENT 09 N DESCRIPTION RESULTS PLASTIC LIMIT LIQUID LIMIT Medium-dense gray-brown and gray fine to coarse gravel, "and" fine to coarse sand, trace to 10 20 30 40 111497013.GPJ BBCM.GDT 8/4/09 little silt, wet. 12 34 40 G 7/9 22 47 620.3 30-2008 NEW DEFAULT BORING LOG-W/ N60 - Seepage encountered at 11.0'. - Groundwater encountered at 23.0'. - Borehole grouted upon completion. - Boring location and elevation surveyed by AEP. 35-40-45 └ 50-SYMBOLS USED TO INDICATE TEST RESULTS WATER LEVEL: ▽ Drill Rod Energy Ratio: <u>0.82</u> 16.3 - Gradation -- Uncon Comp G Q T C See H - Penetrometer (tsf) **Last Calibration Date:** <u>11/19/07</u> WATER NOTE: Inside HSA Separate W - Unit Dry Wt (pcf) D - Relative Dens (%) - Triax Comp 4/8/09 Drill Rig Number: <u>D50</u> DATE: Curves







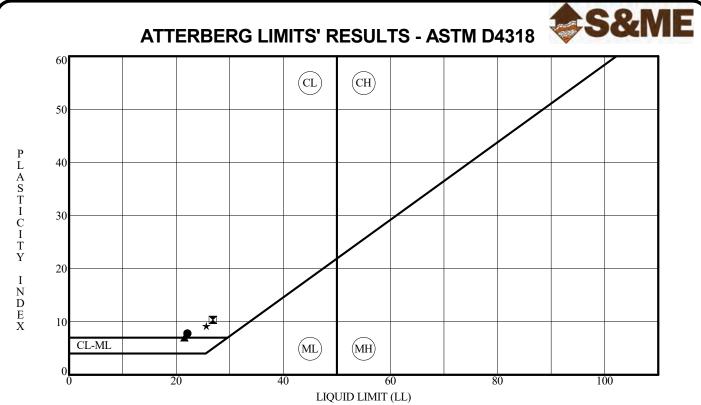
Appendix II – 2009 & 2015 Laboratory Testing Results

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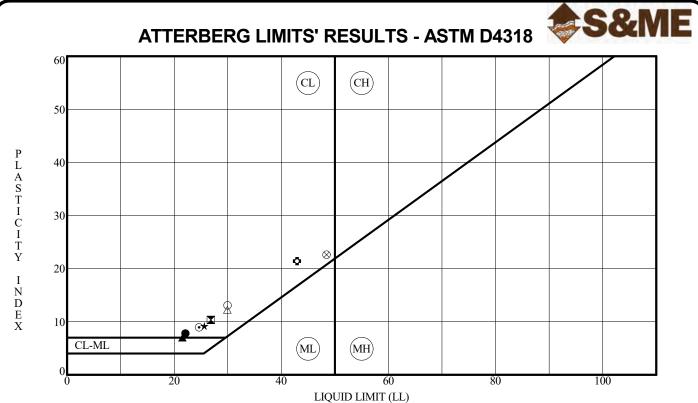
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				MW-BAP-4	MW-BAP-4	MW-BAP-4	MW-BAP-4	MW-BAP-4	MW-BAP-5				S									





Specimen Id.	Depth	MC	LL	PL	PΙ	Fines	ASTM Classification						
CD-BAP-1501	12.25	14	22	14	8								
CD-BAP-1502	6.25	9	27	16	11	28.6	CLAYEY SAND with GRAVEL SC						
CD-BAP-1502	2 11.25	9	21	14	7								
CD-BAP-1502	17.75	13	26	16	10	27.9	CLAYEY SAND with GRAVEL SC						
		POTT	NA 40:	LDONG	CHDD:		TAL INVESTIGATION						
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JOB NO.	•	CARDINAL PLANT, BRILLIANT, OH 7217-15-007A DATE 12/30/15											



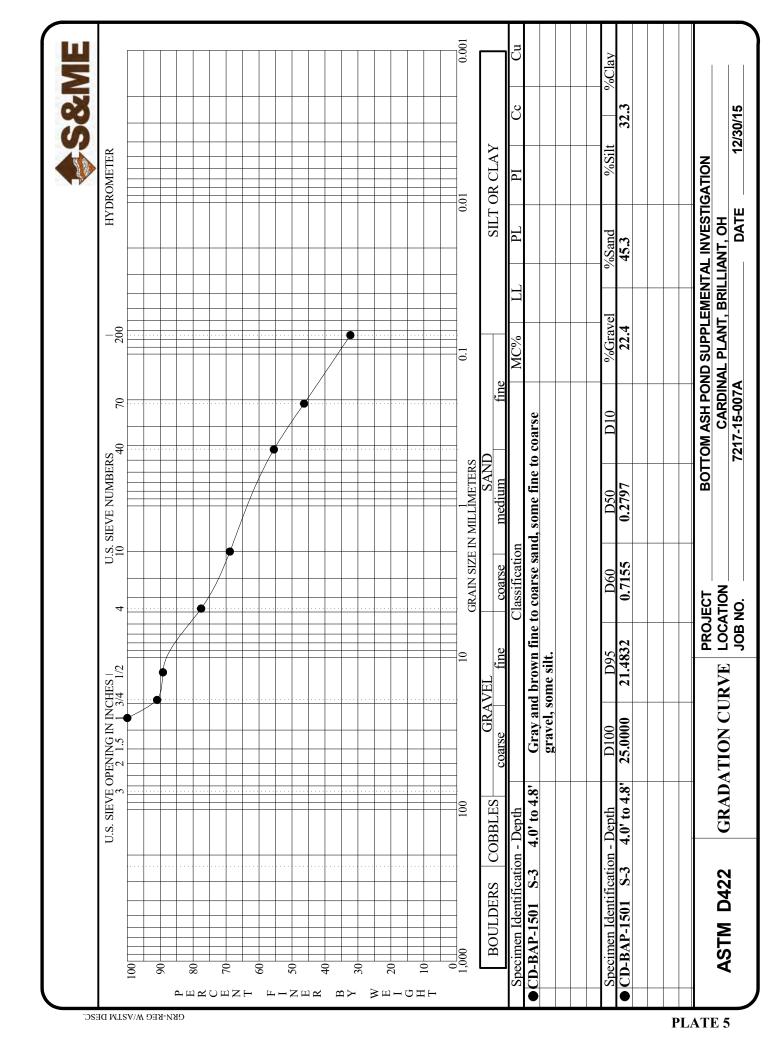


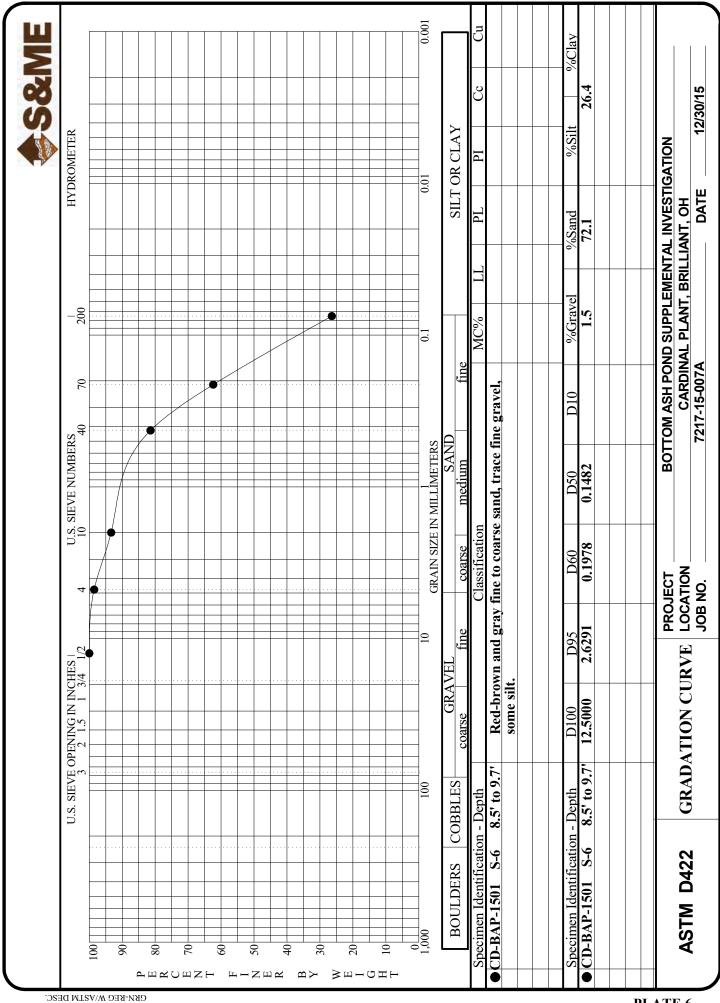
	Specimen Id.	Depth	MC	LL	PL	PI	Fines	ASTM Classification
•	CD-BAP-1501	12.25	14	22	14	8		
X	CD-BAP-1502	6.25	9	27	16	11	28.6	CLAYEY SAND with GRAVEL SC
A (CD-BAP-1502	11.25	9	21	14	7		
*(CD-BAP-1502	17.75	13	26	16	10	27.9	CLAYEY SAND with GRAVEL SC
•	MW-BAP-4	6.25	15	25	16	9	41.2	CLAYEY SAND SC
o	MW-BAP-4	9.25	24	43	21	22		
0	MW-BAP-5	4.75	13	30	17	13	38.8	CLAYEY SAND with GRAVEL SC
Δ	MW-BAP-5	9.25	10	30	18	12		
\otimes	MW-BAP-5	21.75	40	48	26	22	94.9	LEAN CLAY CL
	PROJECT		BOTTO	OM ASH	I POND	SUPP	I FMFN	TAL INVESTIGATION
	OCATION							LIANT, OH

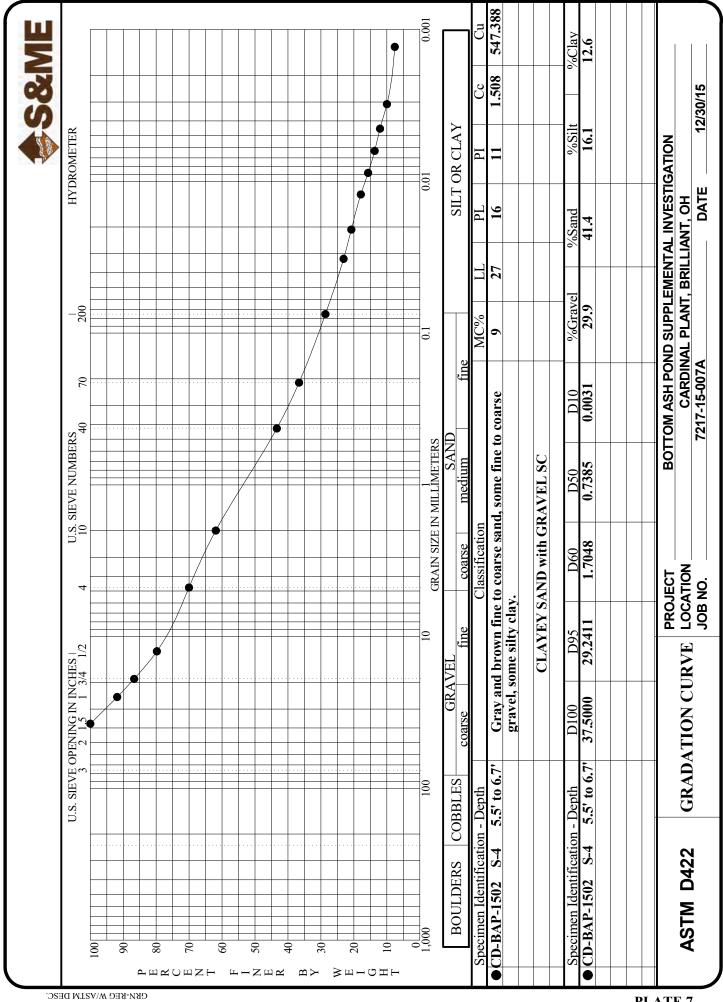
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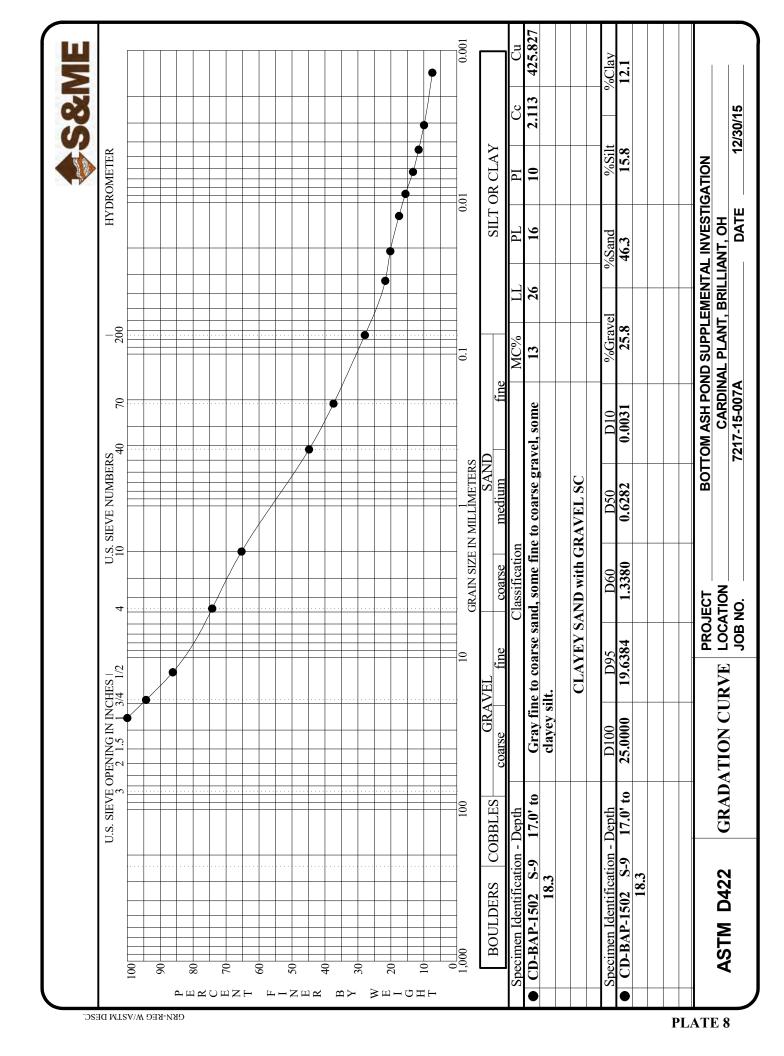
7217-15-007A

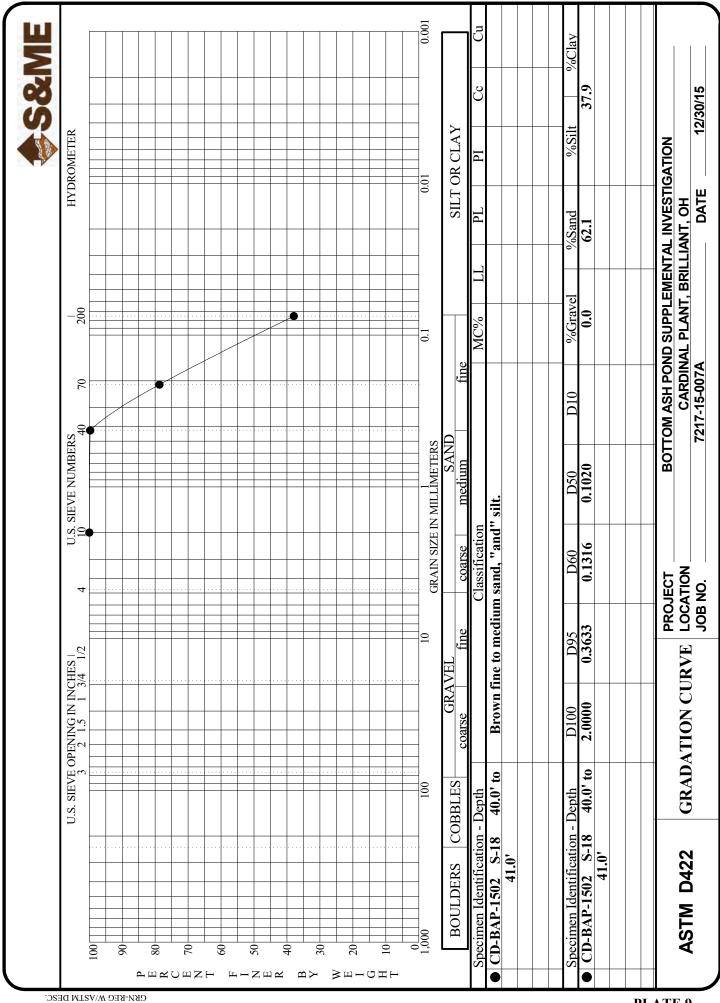
JOB NO.

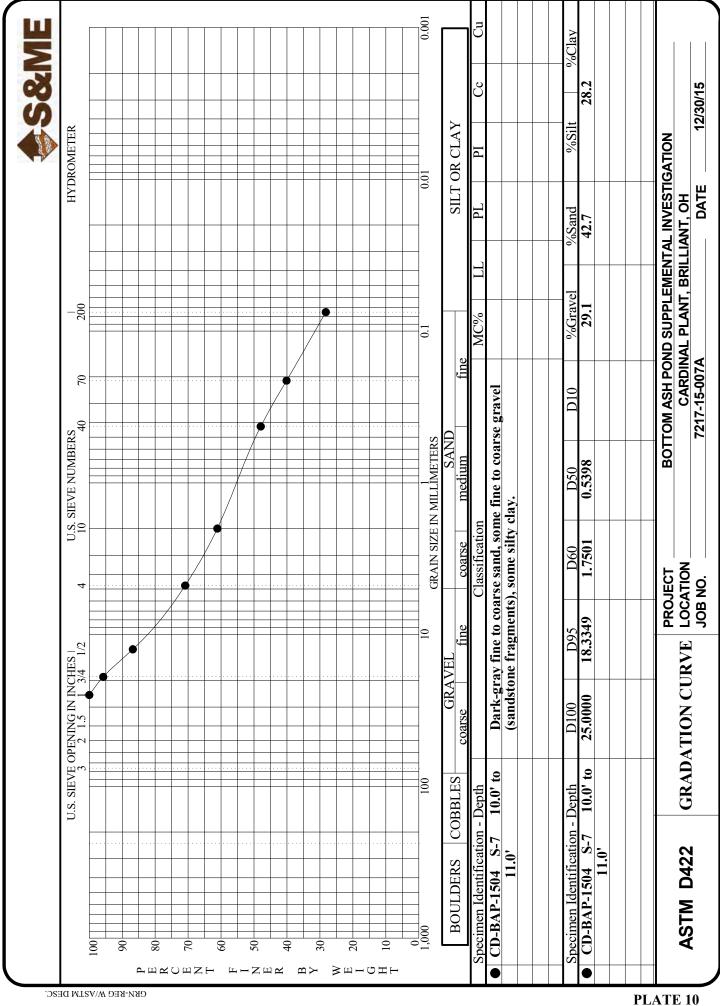


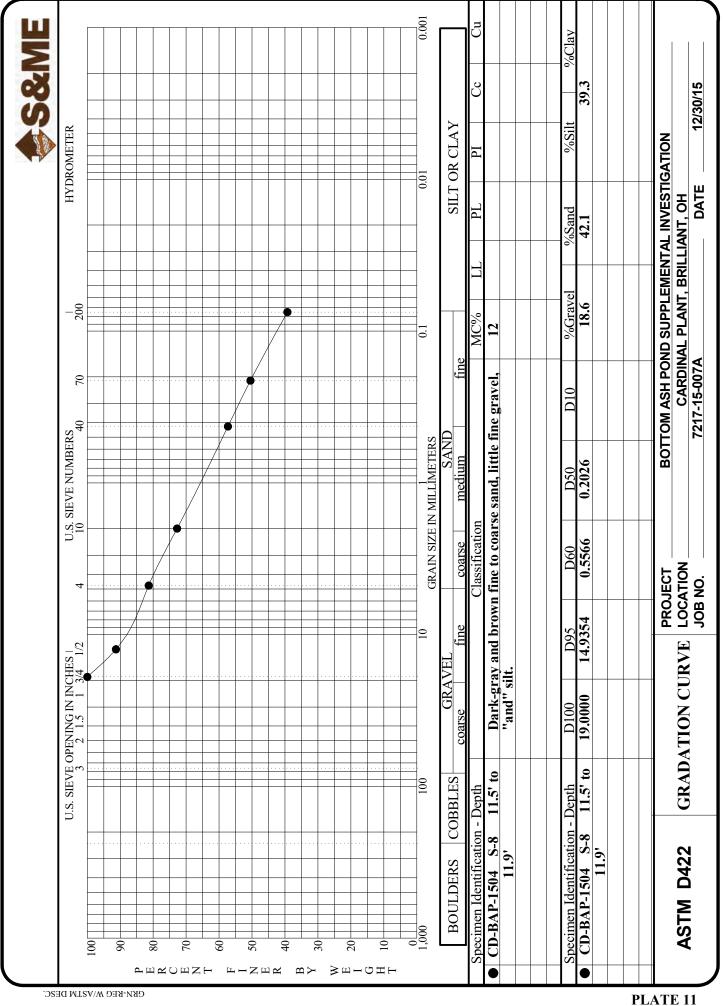


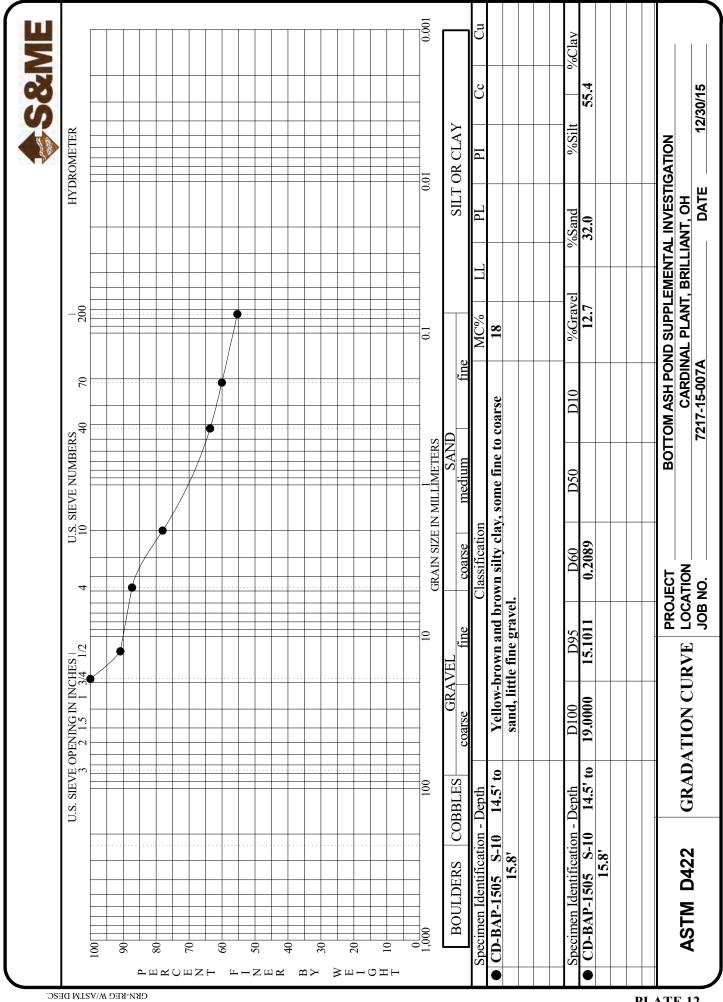


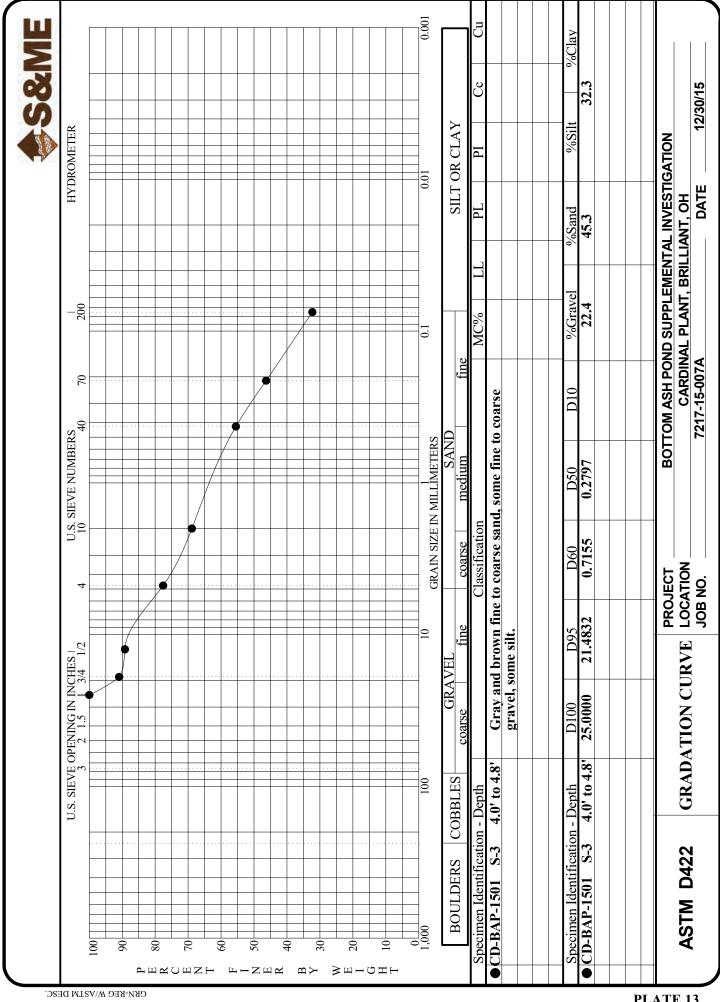


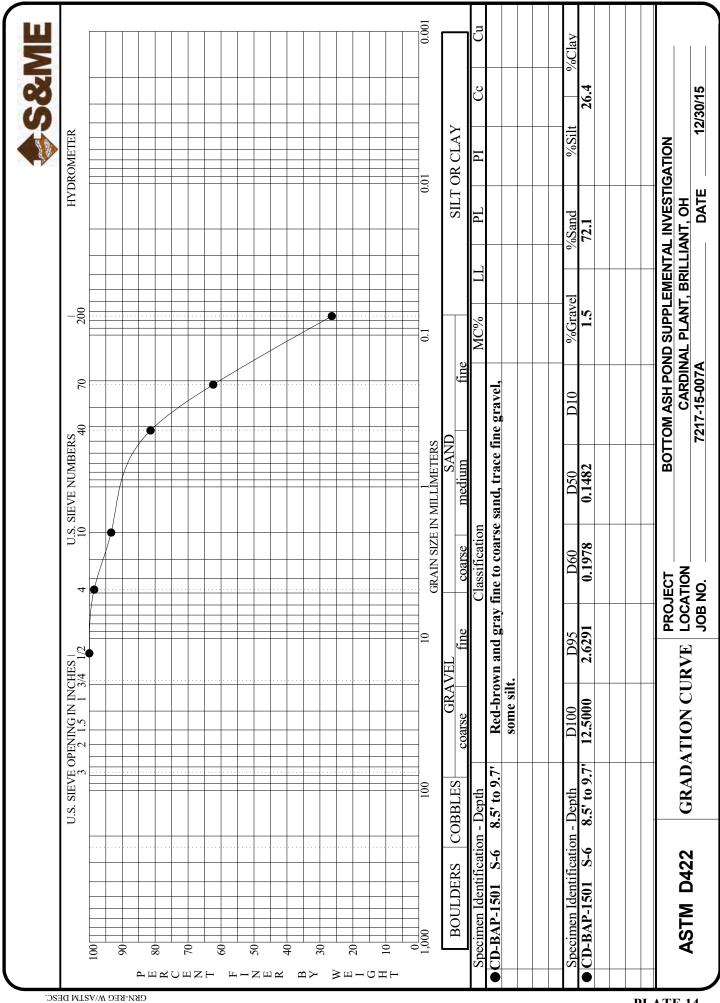


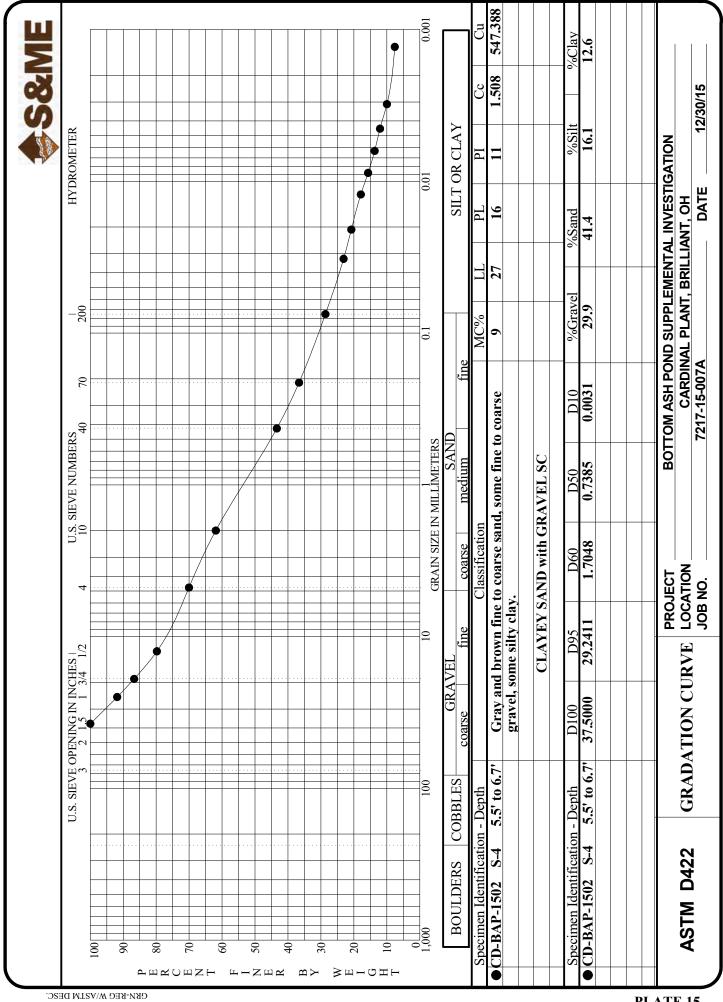


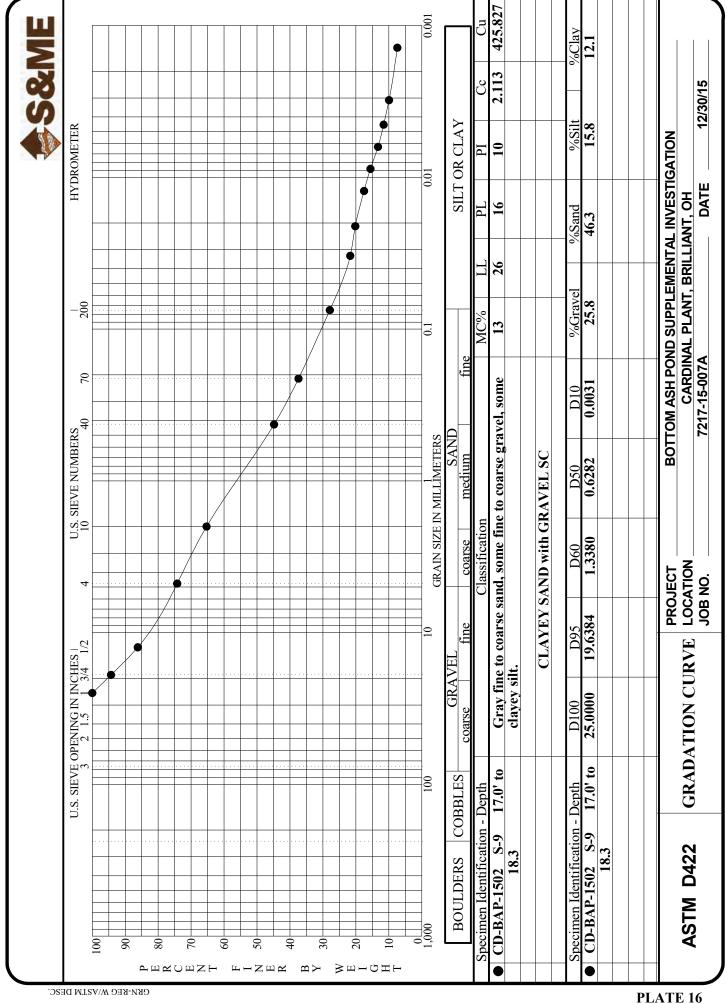


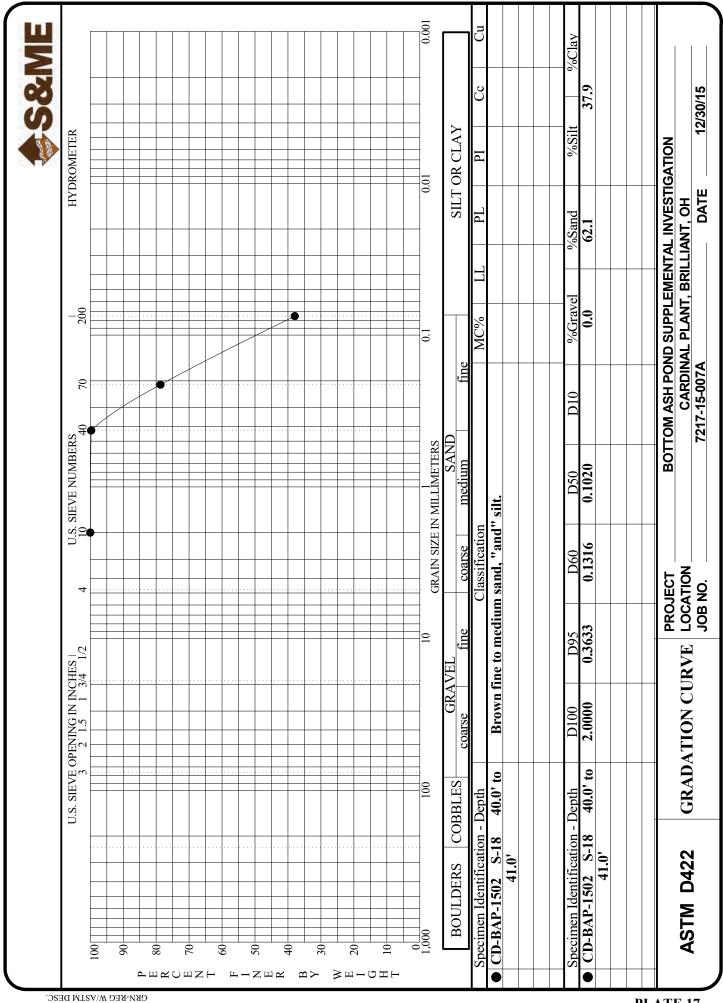


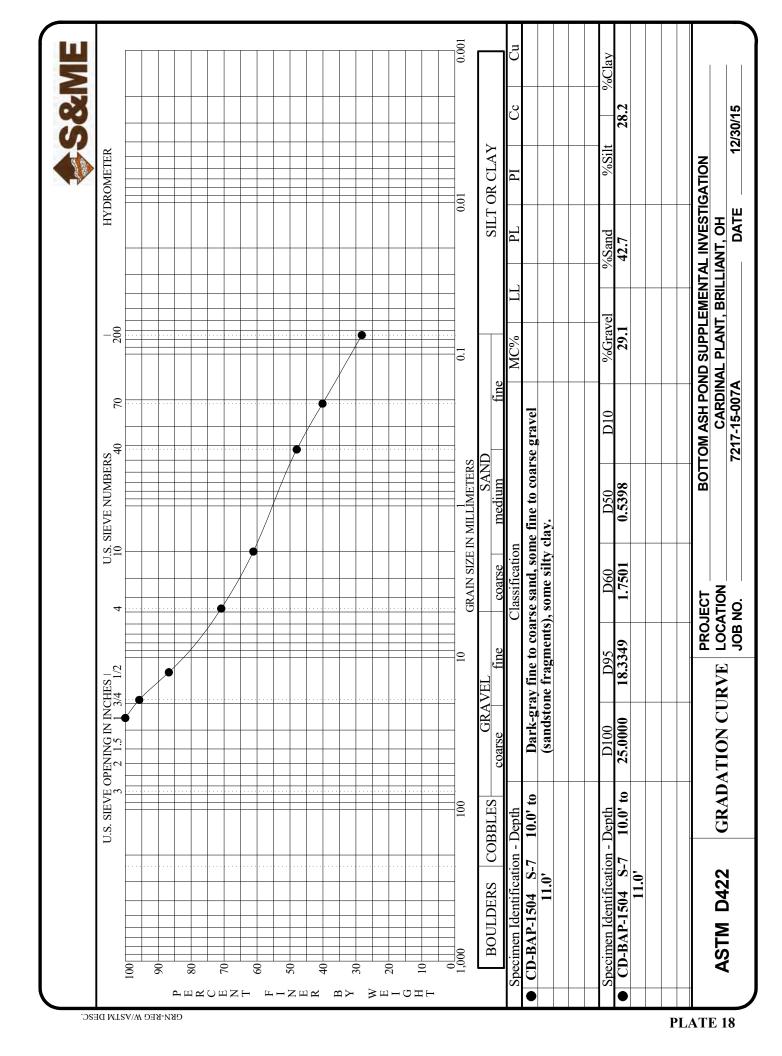


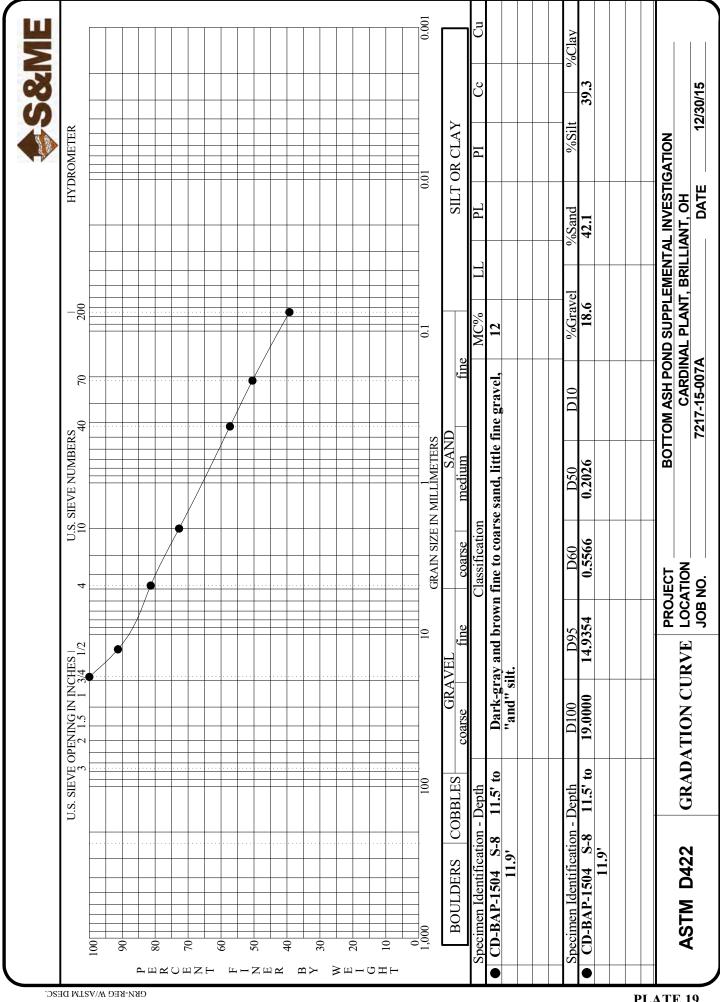


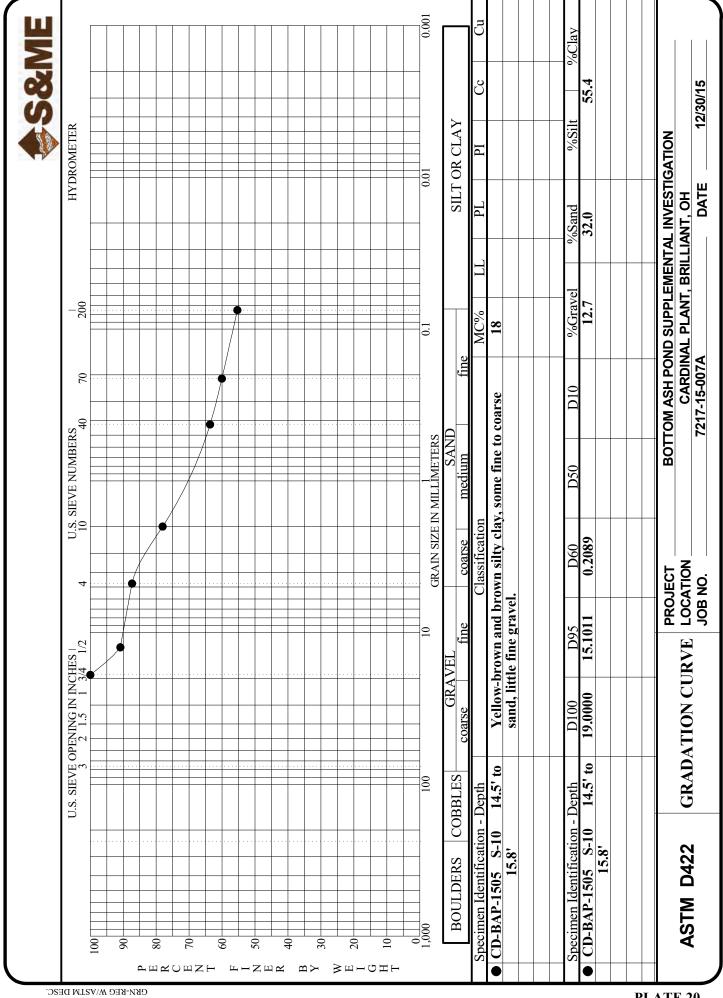


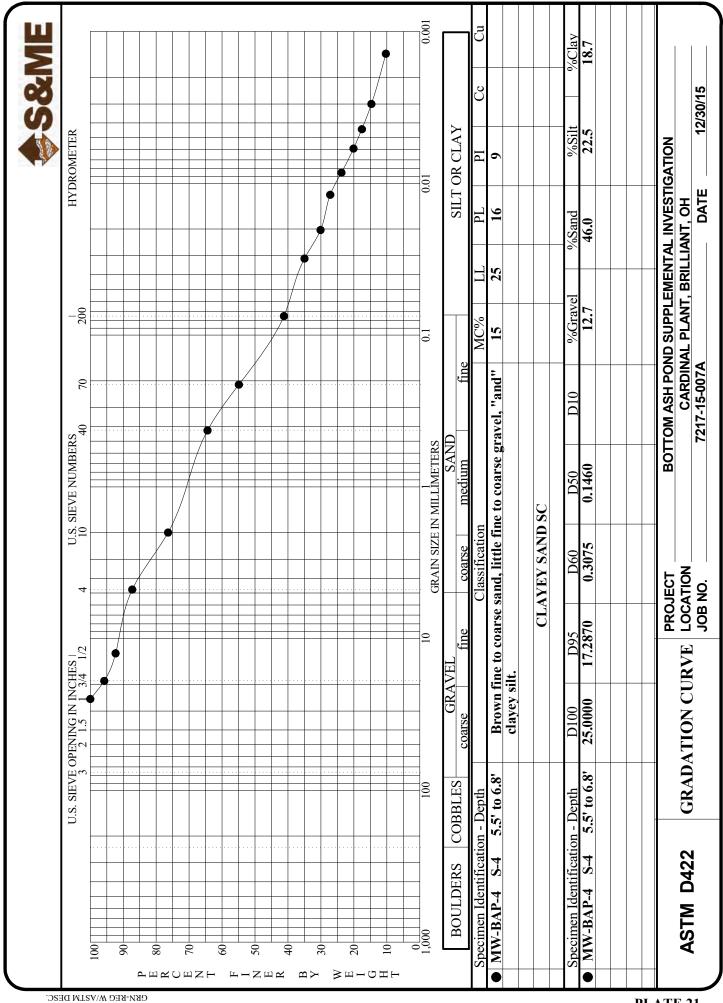


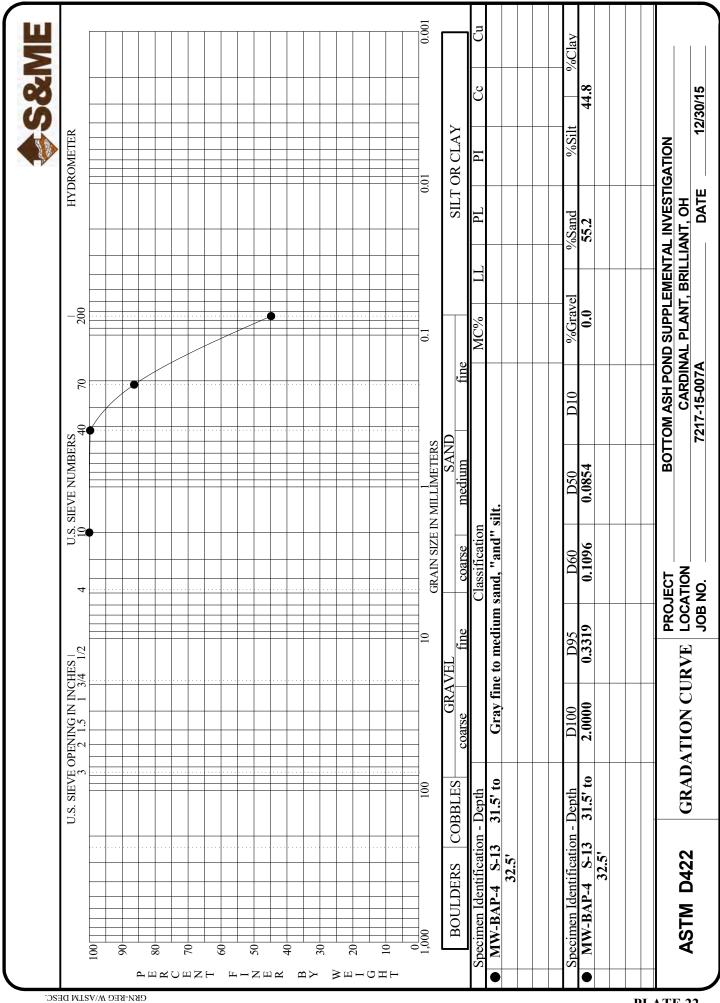


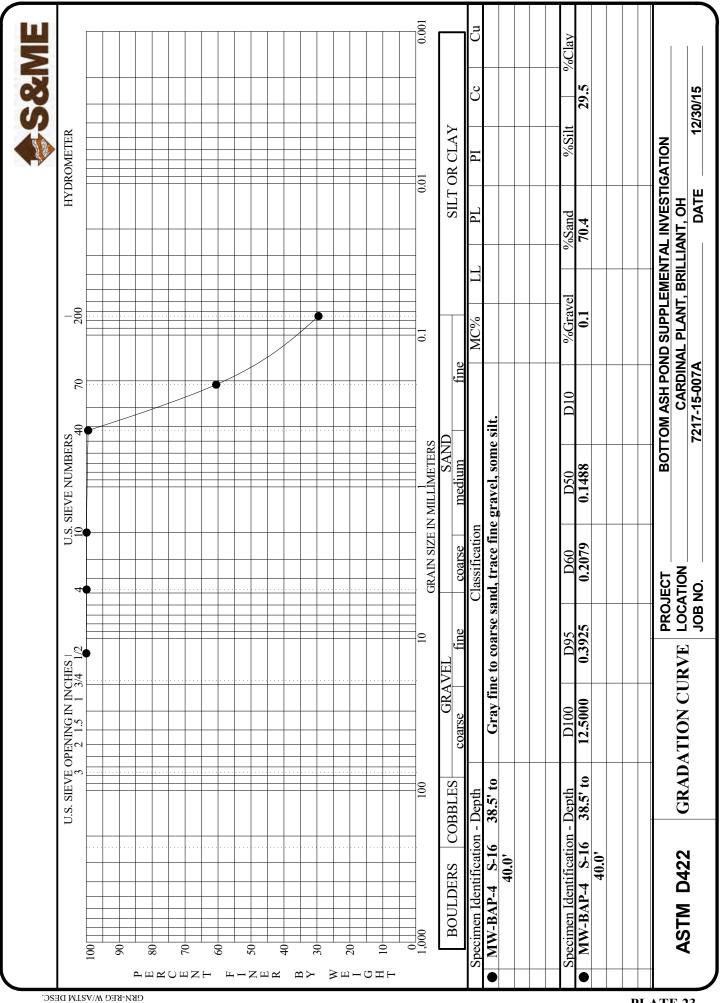


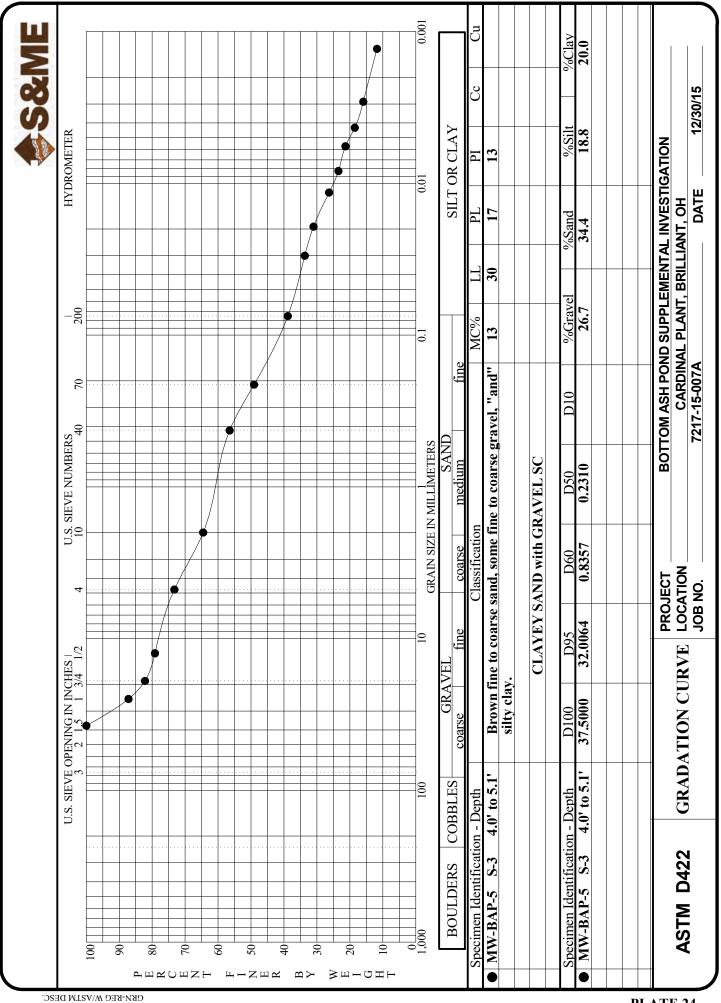


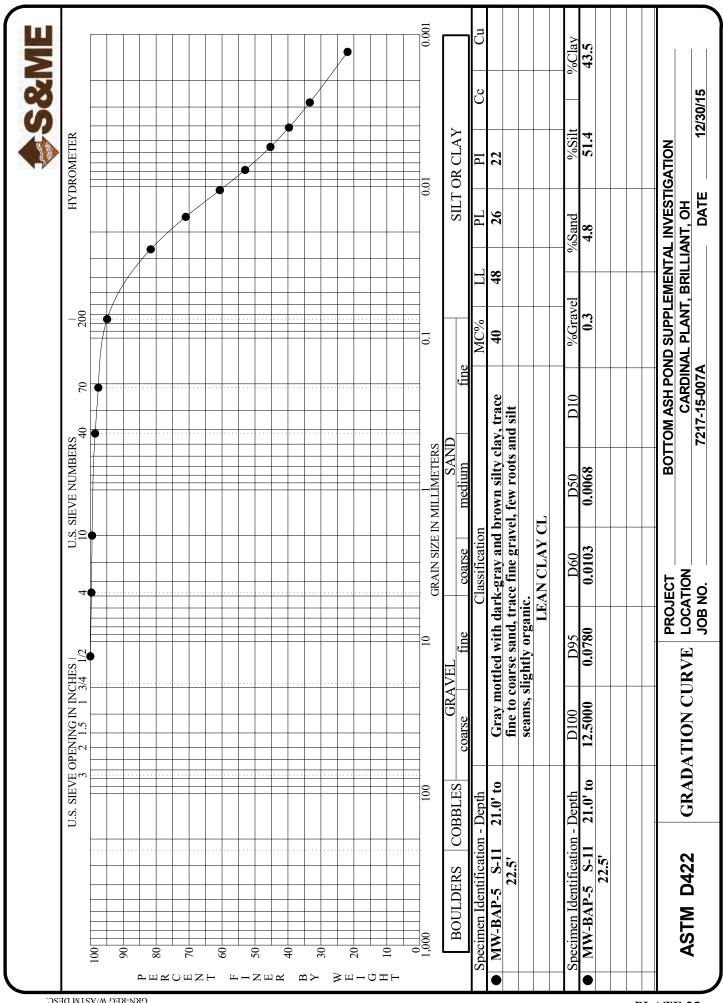


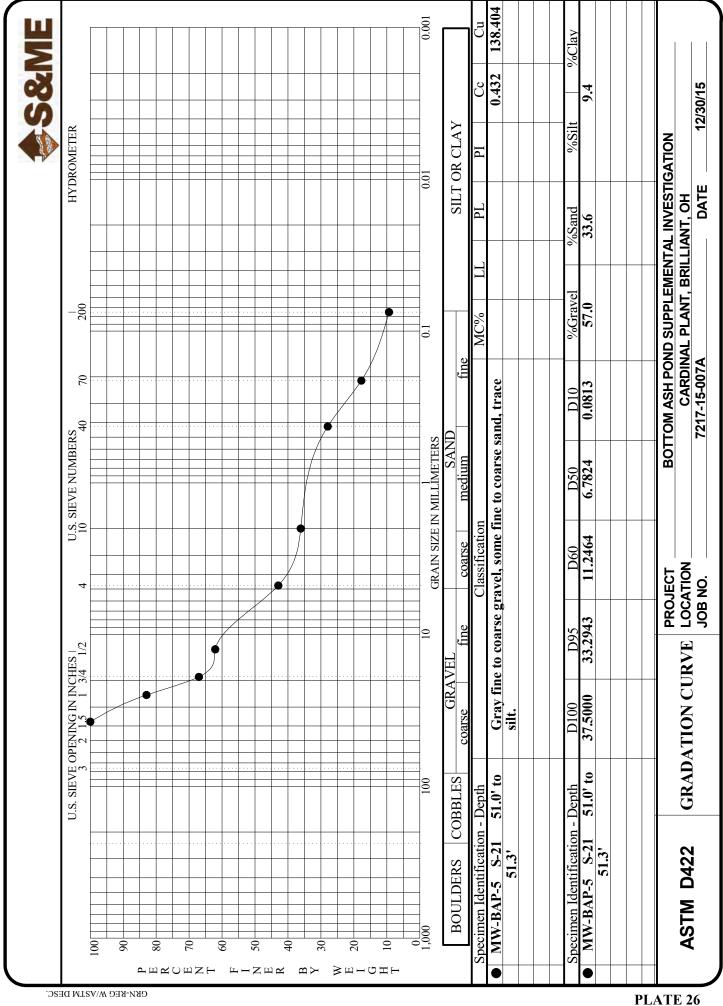


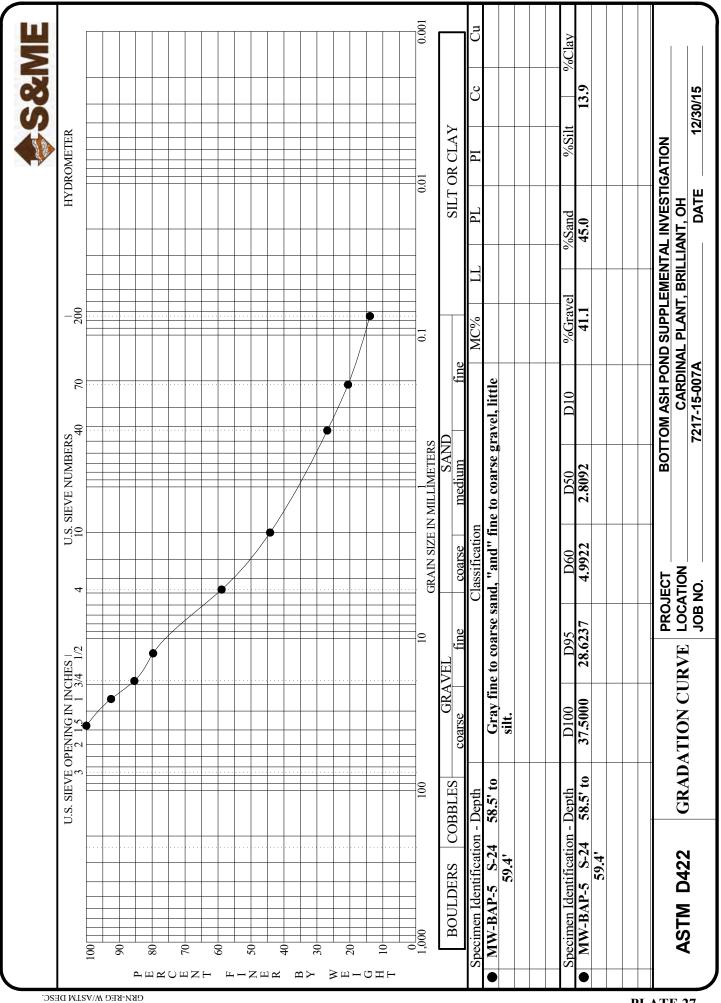












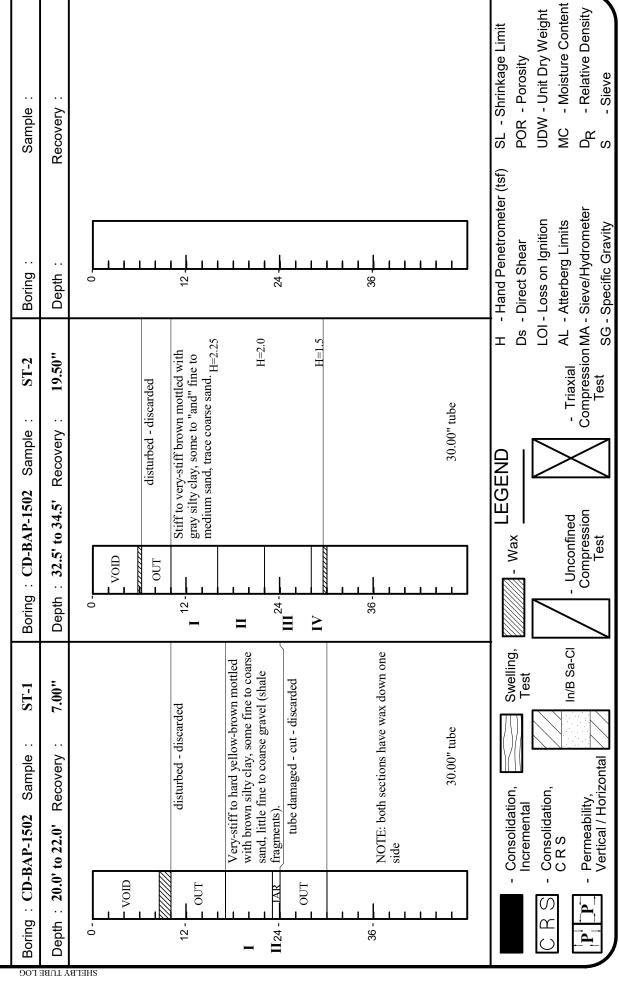
7217-15-007A JOB NUMBER

BOTTOM ASH POND SUPPLEMENTAL INVESTIGATION **PROJECT** LOCATION

CARDINAL PLANT, BRILLIANT, OH



LABORATORY LOG OF SHELBY TUBES



2009 SITE INVESTIGATION

SUMMARY OF LABORATORY TEST RESULTS UNCOP COP NES NS GRADATION COMPACTION TRIAXIAL DIRECT SHEAR PERMEABILITY CONSOLID REMOLDED Ο r w i a g l i l d f w l a u u c u w d Hydrometer u n d r a i n r e s i d u a l c o h e s i v n o n c o h o n / n d p s r o $\begin{array}{c} n & n \\ c & d \end{array}$ r G'int e l x l a BORING PΙ o r MC LL PL h o a p 0 n a n Id. e V O n a g . n i n e e % % % * SEE INDIVIDUAL TEST CURVES **PCF** BAP-0901 4.75 16 BAP-0901 7.75 28 18 10 16 BAP-0901 13.75 13 27 17 10 * BAP-0901 18.25 14 37 24 13 BAP-0901 22.75 * 30 NP NP NP BAP-0901 24.50 BAP-0901 29.25 27 37 22 15 * * BAP-0901 31.25 * * BAP-0901 31.75 33 35 28 7 BAP-0901 32.25 BAP-0901 34.25 7 42 34 27 BAP-0901 36.75 40 45 29 16 BAP-0901 39.25 42 40 23 17 * * BAP-0902 6.25 13 27 17 10 BAP-0902 10.75 20 9 * BAP-0902 12.25 10 26 17 BAP-0902 16.75 24 37 19 18 BAP-0902 18.25 21 35 17 18 * BAP-0902 19.75 31 29 17 12

BBC SOLUTIONS TO BUILD ON

26

NP

NP

NP

BAP-0902 21.25

TESTING SUMMARY - STANDARD

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PROJECT	CARDINAL	PLANT ASH PC	OND INVESTIGATION	
LOCATION		BRILLIANT,	ОНЮ	
JOB NO.	011-11497-013	DATE _	7/6/09	

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SUMMARY OF LABORATORY TEST RESULTS U C NO NCO P N E N S GRADATION COMPACTION TRIAXIAL DIRECT SHEAR **PERMEABILITY** CONSOLID REMOLDED Ο r w i a g l i l d f w l a e l x l u u |c u w | d Hydrometer u n d r a i n r e s i d u a l c o h e s i v n o n c o h o n / n d p s r o r $\begin{array}{c} n & n \\ c & d \end{array}$ G'int a BORING PΙ s h o r MC LL PL 0 o a p l i r n ạ Id. e V 0 n a S i g i n e . n e % % % * SEE INDIVIDUAL TEST CURVES **PCF** BAP-0902 22.75 * BAP-0902 27.25 54 NP NP NP 10.4 * BAP-0902 28.75 43 NP NP NP * * BAP-0902 32.25 38 28 8 36 BAP-0902 37.25 22 * BAP-0902 39.75 24 BAP-0902 42.25 * BAP-0903 3.25 24 48 24 24 BAP-0903 4.75 22 * BAP-0903 7.75 20 36 20 16 BAP-0903 9.25 3 49 41 38 BAP-0903 14.25 43 NP NP NP * BAP-0903 16.75 43 37 24 13 * * BAP-0903 19.25 44 35 24 11 * BAP-0903 21.75 34 21 35 13 BAP-0903 24.25 BAP-0904 4.75 13 BAP-0904 9.25 9 14 25 16 BAP-0904 13.75 21 16 35 14

BBC SOLUTIONS TO BUILD ON

BAP-0904 16.75

TESTING SUMMARY - STANDARD

PROJECT CARDINAL PLANT ASH POND INVESTIGATION
LOCATION BRILLIANT, OHIO
JOB NO. 011-11497-013 DATE 7/6/09

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SUMMARY OF LABORATORY TEST RESULTS UNCOP COP NES NS GRADATION COMPACTION TRIAXIAL DIRECT SHEAR PERMEABILITY CONSOLID REMOLDED r w i a g l i l d f w l a u u | c u w | d Hydrometer u n d r a i n r e s i d u a l c o h e s i v e n o n c o h o n / n d p s r o r $\begin{array}{c} n & n \\ c & d \end{array}$ G'int e l x l a BORING PΙ s h o r MC LL PL o a p 0 n a Id. e V O n g i n e . n e % % % * SEE INDIVIDUAL TEST CURVES **PCF** BAP-0904 19.75 28 NP NP NP * BAP-0904 22.75 26 NP NP NP * BAP-0904 25.75 22 NP NP NP * BAP-0904 27.25 38 24 38 14 BAP-0904 28.75 42 30 47 12 BAP-0904 36.75 BAP-0905 4.75 17 32 18 14 * BAP-0905 7.75 22 48 24 24 BAP-0905 9.85 33 * BAP-0905 14.25 45 43 27 16 BAP-0905 16.75 42 40 25 15 BAP-0905 21.75 38 38 23 15 * BAP-0905 26.75 BAP-0906 2.90 11 BAP-0906 4.75 15 27 17 10 * BAP-0906 12.75 BAP-0906 17.25 14 31 19 12 BAP-0906 24.75 NP 31 NP NP BAP-0906 26.25

BBCK1
SOLUTIONS TO BUILD ON

22

NP

NP

NP

BAP-0906 27.25

TESTING SUMMARY - STANDARD

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 PROJECT
 CARDINAL PLANT ASH POND INVESTIGATION

 LOCATION
 BRILLIANT, OHIO

 JOB NO.
 011-11497-013
 DATE
 7/6/09

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60/							5	SUM	MAR	Y OF		BO	RA	ΓORY	TES	ST R	E	SUL	_TS										
DT 7/6						GR	RADATI	ION	COMPA	ACTION		TRIAX	IAL	DIREC	Г SHEA	R U C	C	S G P R	U W	R E	PEI	RME	EABII	LITY	R D E E	L	R	S	С
BORING BOUNG	G'int Id.	MC	LL	PL	PI	S 1 e V e	S h o r	l o n g	s t a n d a r d	m o d i f i e d	u u n n c d o r n a s i	c u w o n / n d p s r o o a p l i r i n e d s	r a i n e	r a i n e	r e d s i d d i u a l	U CO UNM OP NR FE INS	CONSOLID .	S G P R A C V I F T C	U W N E T G H D T R	REMOLDED	c o h e s i v e	o h e	r w i a g l i l d	f w l a e l x l b l e	RELATITYE	I	ROCK CORE	SHELBY TUBE	B R
		%	%	%	%			* SEI	E INDI	VIDU	AL TI	EST C	URV	ES	·	•			PCF			S			%	%			
BAP-0906	31.75	34	33	22	11	*		*																					
[™] BAP-0906	34.25	43	50	30	20	*		*																		7.9			
BAP-0906	36.75	38	43	26	17	*		*																					
BAP-0906	44.25					*																							
BAP-0907	3.25	21																											
BAP-0907	6.25	15																											
BAP-0907	7.75	23	49	26	23																								
BAP-0907	9.25	28	47	29	18	*		*													*			*				*	
BAP-0907	11.75					*		*																					
BAP-0907	14.25	43	44	28	16	*		*																					
BAP-0907	16.75	44	45	29	16	*		*																					
BAP-0907	19.25	40	48	29	19	*																							
BAP-0907	21.75	39	30	24	6	*		*																					
BAP-0907	26.75					*																							

BBC SOLUTIONS TO BUILD ON

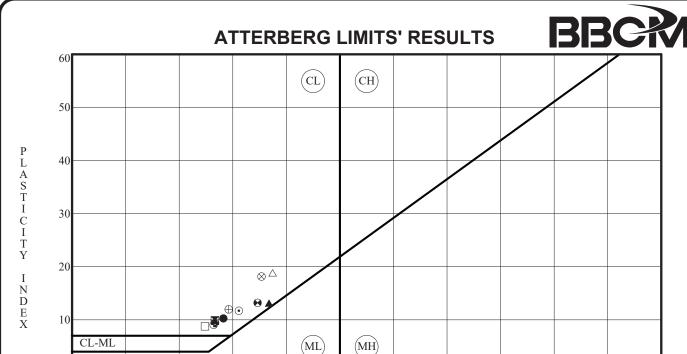
TESTING SUMMARY - STANDARD

 PROJECT
 CARDINAL PLANT ASH POND INVESTIGATION

 LOCATION
 BRILLIANT, OHIO

 JOB NO.
 011-11497-013
 DATE
 7/6/09

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011-11497-013

JOB NO.

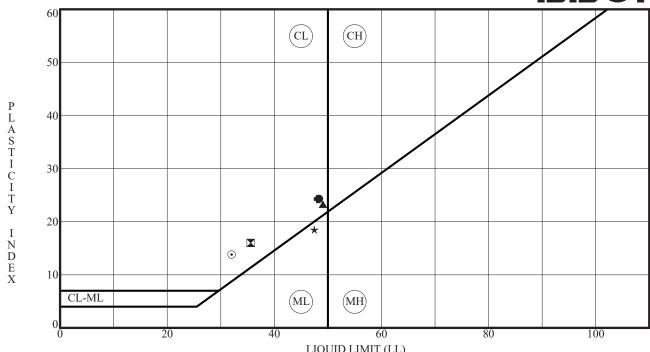
Specim	en Id.	Depth	MC	LL	PL	PI	Fines	ASTM Classification
BAP	-0901	7.75	16	28	18	10		
BAP-	-0901	13.75	13	27	17	10		
BAP-	-0901	18.25	14	37	24	13	60.6	SANDY LEAN CLAY CL
* BAP	-0906	4.75	15	27	17	10		
• BAP	-0906	17.25	14	31	19	12	38.0	CLAYEY SAND with GRAVEL SC
BAP	-0902	6.25	13	27	17	10	23.6	CLAYEY GRAVEL with SAND GC
) BAP	-0902	12.25	10	26	17	9	28.8	CLAYEY SAND with GRAVEL SC
\(\BAP \)	-0902	16.75	24	37	19	18		
⊗ BAP	-0902	18.25	21	35	17	18	54.2	SANDY LEAN CLAY CL
BAP-	-0902	19.75	31	29	17	12	78.8	LEAN CLAY with SAND CL
□ BAP	-0904	9.25	14	25	16	9	30.3	CLAYEY SAND with GRAVEL SC
BAP	-0904	13.75	16	35	21	14		
PROJE	O.T.			DDINA	I DI A	NT AC		DINVESTIGATION

DATE _____7/6/09

LIQUID LIMIT (LL)



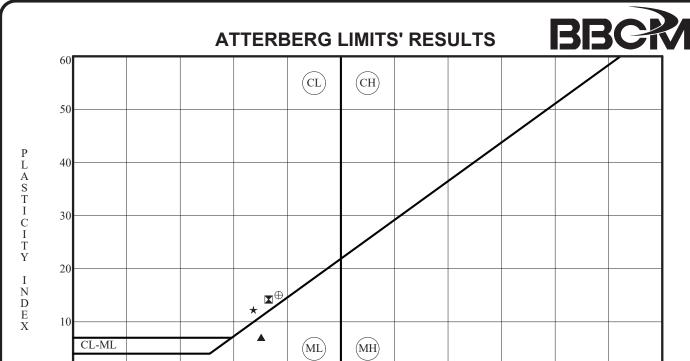




ORIGINAL EMBANKMENT FILL

LIQUID LIMIT (LL)	

5	Specimen Id.	Depth	MC	LL	PL	PI	Fines	ASTM Classification
•	BAP-0903	3.25	24	48	24	24	91.6	LEAN CLAY CL
X	BAP-0903	7.75	20	36	20	16	86.2	LEAN CLAY CL
A	BAP-0907	7.75	23	49	26	23		
*	BAP-0907	9.25	28	47	29	18	95.2	SILT ML
•	BAP-0905	4.75	17	32	18	14	75.2	LEAN CLAY with SAND CL
٥	BAP-0905	7.75	22	48	24	24		
\dashv								
_	DO 1507			DDINA	I DI A	NT AC	LL DON'	D INVESTIGATION
	ROJECT _ OCATION		CA	KUINA			H PONI NT, OH	D INVESTIGATION
	OB NO.	0	11-114	197-01			DATE	



ALLUVIUM SILT AND CLAY

LIOUID	I INAIT	(II)
LIOUID	LIMII	LLL

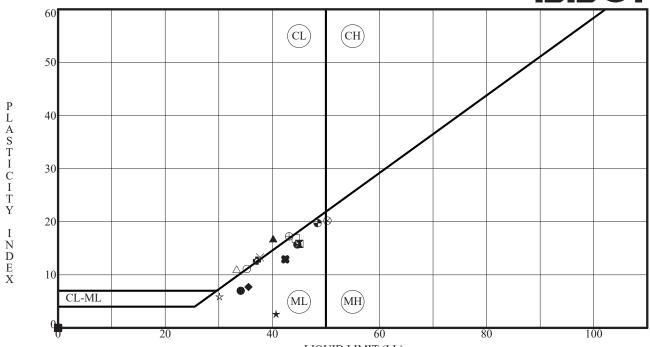
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,	Specimen Id.	Depth	MC	LL	PL	PI	Fines	ASTM Classification
•	BAP-0901	22.75	30	NP	NP	NP	95.3	SILT ML
X	BAP-0901	29.25	27	37	22	15	91.0	LEAN CLAY CL
A	BAP-0901	31.75	33	35	28	7	73.9	SILT with SAND ML
*	BAP-0903	21.75	35	34	21	13	70.1	LEAN CLAY with SAND CL
•	BAP-0906	24.75	31	NP	NP	NP	95.2	SILT ML
٥	BAP-0906	27.25	22	NP	NP	NP	75.2	SILT with SAND ML
0	BAP-0902	21.25	26	NP	NP	NP	86.8	SILT ML
Δ	BAP-0904	22.75	26	NP	NP	NP	47.4	SILTY SAND SM
\otimes	BAP-0904	25.75	22	NP	NP	NP	91.4	SILT ML
\oplus	BAP-0905	21.75	38	38	23	15	62.0	SANDY LEAN CLAY CL
P	ROJECT		CA	RDINA	L PLA	NT AS	H PONI	D INVESTIGATION
	OCATION _		44 44	107.01		RILLIA	NT, OH	
J	OB NO	U	11-114	+97-01	3		DATE	7/6/09

ATTERBERG LIMITS' RESULTS



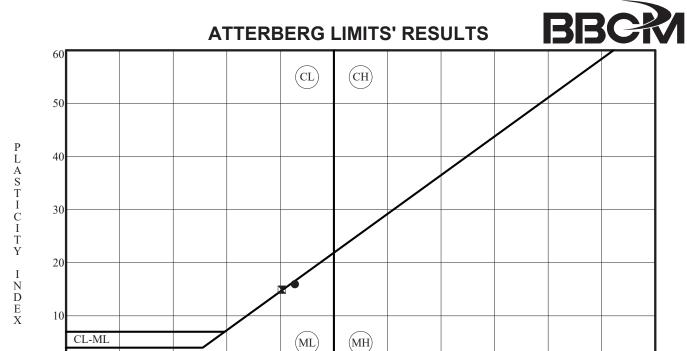


ORGANIC CLAYEY SILT

LIQUID	LIMII	(LL)

Specimen Id.	Depth	MC	LL	PL	PI	Fines	ASTM Classification
BAP-0901	34.25	42	34	27	7	78.2	ORGANIC SILT with SAND OL
BAP-0901	36.75	40	45	29	16	59.2	SANDY ORGANIC SILT OL
BAP-0901	39.25	42	40	23	17	81.5	ORGANIC CLAY with SAND OL
BAP-0903	9.25	49	41	38	3	66.6	SANDY ORGANIC SILT OL
BAP-0903	14.25	43	NP	NP	NP	71.4	ORGANIC SILT with SAND OL
BAP-0903	16.75	43	37	24	13	75.9	ORGANIC CLAY with SAND OL
BAP-0903	19.25	44	35	24	11	61.3	SANDY ORGANIC CLAY OL
BAP-0906	31.75	34	33	22	11	81.3	ORGANIC CLAY with SAND OL
BAP-0906	34.25	43	50	30	20	96.9	ORGANIC SILT OH
BAP-0906	36.75	38	43	26	17	91.1	ORGANIC CLAY OL
BAP-0907	14.25	43	44	28	16	84.7	ORGANIC SILT with SAND OL
BAP-0907	16.75	44	45	29	16	84.9	ORGANIC SILT with SAND OL
BAP-0907	19.25	40	48	29	19	90.9	ORGANIC SILT OL
BAP-0907	21.75	39	30	24	6	56.3	SANDY ORGANIC SILT OL
BAP-0902	27.25	54	NP	NP	NP	85.3	ORGANIC SILT OL
BAP-0902	28.75	43	NP	NP	NP	74.9	ORGANIC SILT with SAND OL
BAP-0902	32.25	38	36	28	8	75.4	ORGANIC SILT with SAND OL
BAP-0904	19.75	28	NP	NP	NP	92.1	ORGANIC SILT OL
BAP-0904	27.25	38	38	24	14	79.2	ORGANIC CLAY with SAND OL
BAP-0904	28.75	47	42	30	12	78.4	ORGANIC SILT with SAND OL
	BAP-0901 BAP-0901 BAP-0901 BAP-0903 BAP-0903 BAP-0903 BAP-0906 BAP-0906 BAP-0906 BAP-0907 BAP-0907 BAP-0907 BAP-0907 BAP-0902 BAP-0902 BAP-0904	BAP-0901 34.25 BAP-0901 36.75 BAP-0901 39.25 BAP-0903 9.25 BAP-0903 14.25 BAP-0903 16.75 BAP-0906 31.75 BAP-0906 34.25 BAP-0906 36.75 BAP-0907 14.25 BAP-0907 16.75 BAP-0907 19.25 BAP-0907 21.75 BAP-0907 21.75 BAP-0902 27.25 BAP-0902 28.75 BAP-0904 19.75 BAP-0904 27.25	BAP-0901 34.25 42 BAP-0901 36.75 40 BAP-0901 39.25 42 BAP-0903 9.25 49 BAP-0903 14.25 43 BAP-0903 16.75 43 BAP-0906 31.75 34 BAP-0906 34.25 43 BAP-0906 36.75 38 BAP-0907 14.25 43 BAP-0907 16.75 44 BAP-0907 19.25 40 BAP-0907 21.75 39 BAP-0907 21.75 39 BAP-0902 27.25 54 BAP-0902 28.75 43 BAP-0904 19.75 28 BAP-0904 27.25 38	BAP-0901 34.25 42 34 BAP-0901 36.75 40 45 BAP-0901 39.25 42 40 BAP-0903 9.25 49 41 BAP-0903 14.25 43 NP BAP-0903 16.75 43 37 BAP-0904 31.75 34 35 BAP-0906 31.75 34 33 BAP-0906 36.75 38 43 BAP-0907 14.25 43 44 BAP-0907 16.75 44 45 BAP-0907 19.25 40 48 BAP-0907 21.75 39 30 BAP-0902 27.25 54 NP BAP-0902 32.25 38 36 BAP-0904 19.75 28 NP BAP-0904 27.25 38 38	BAP-0901 34.25 42 34 27 BAP-0901 36.75 40 45 29 BAP-0901 39.25 42 40 23 BAP-0903 9.25 49 41 38 BAP-0903 14.25 43 NP NP BAP-0903 16.75 43 37 24 BAP-0903 19.25 44 35 24 BAP-0906 31.75 34 33 22 BAP-0906 34.25 43 50 30 BAP-0906 36.75 38 43 26 BAP-0907 14.25 43 44 28 BAP-0907 16.75 44 45 29 BAP-0907 19.25 40 48 29 BAP-0907 21.75 39 30 24 BAP-0902 27.25 54 NP NP BAP-0902 32.25 38 36 28 BAP-0904 19.75 28 NP NP BAP-0904 </td <td>BAP-0901 34.25 42 34 27 7 BAP-0901 36.75 40 45 29 16 BAP-0901 39.25 42 40 23 17 BAP-0903 9.25 49 41 38 3 BAP-0903 14.25 43 NP NP NP BAP-0903 16.75 43 37 24 13 BAP-0903 19.25 44 35 24 11 BAP-0906 31.75 34 33 22 11 BAP-0906 34.25 43 50 30 20 BAP-0906 36.75 38 43 26 17 BAP-0907 14.25 43 44 28 16 BAP-0907 16.75 44 45 29 16 BAP-0907 19.25 40 48 29 19 BAP-0902 27.25 54 NP NP NP BAP-0902 32.25 38 36 28 8 <td>BAP-0901 34.25 42 34 27 7 78.2 BAP-0901 36.75 40 45 29 16 59.2 BAP-0901 39.25 42 40 23 17 81.5 BAP-0903 9.25 49 41 38 3 66.6 BAP-0903 14.25 43 NP NP NP 71.4 BAP-0903 16.75 43 37 24 13 75.9 BAP-0903 19.25 44 35 24 11 61.3 BAP-0906 31.75 34 33 22 11 81.3 BAP-0906 34.25 43 50 30 20 96.9 BAP-0906 36.75 38 43 26 17 91.1 BAP-0907 14.25 43 44 28 16 84.7 BAP-0907 16.75 44 45 29 16 84.9 BAP-0907 21.75 39 30 24 6 56.3 BAP-0902 27.25 54 NP NP NP NP BAP-0902 32.25 38 36 28 8 <td< td=""></td<></td></td>	BAP-0901 34.25 42 34 27 7 BAP-0901 36.75 40 45 29 16 BAP-0901 39.25 42 40 23 17 BAP-0903 9.25 49 41 38 3 BAP-0903 14.25 43 NP NP NP BAP-0903 16.75 43 37 24 13 BAP-0903 19.25 44 35 24 11 BAP-0906 31.75 34 33 22 11 BAP-0906 34.25 43 50 30 20 BAP-0906 36.75 38 43 26 17 BAP-0907 14.25 43 44 28 16 BAP-0907 16.75 44 45 29 16 BAP-0907 19.25 40 48 29 19 BAP-0902 27.25 54 NP NP NP BAP-0902 32.25 38 36 28 8 <td>BAP-0901 34.25 42 34 27 7 78.2 BAP-0901 36.75 40 45 29 16 59.2 BAP-0901 39.25 42 40 23 17 81.5 BAP-0903 9.25 49 41 38 3 66.6 BAP-0903 14.25 43 NP NP NP 71.4 BAP-0903 16.75 43 37 24 13 75.9 BAP-0903 19.25 44 35 24 11 61.3 BAP-0906 31.75 34 33 22 11 81.3 BAP-0906 34.25 43 50 30 20 96.9 BAP-0906 36.75 38 43 26 17 91.1 BAP-0907 14.25 43 44 28 16 84.7 BAP-0907 16.75 44 45 29 16 84.9 BAP-0907 21.75 39 30 24 6 56.3 BAP-0902 27.25 54 NP NP NP NP BAP-0902 32.25 38 36 28 8 <td< td=""></td<></td>	BAP-0901 34.25 42 34 27 7 78.2 BAP-0901 36.75 40 45 29 16 59.2 BAP-0901 39.25 42 40 23 17 81.5 BAP-0903 9.25 49 41 38 3 66.6 BAP-0903 14.25 43 NP NP NP 71.4 BAP-0903 16.75 43 37 24 13 75.9 BAP-0903 19.25 44 35 24 11 61.3 BAP-0906 31.75 34 33 22 11 81.3 BAP-0906 34.25 43 50 30 20 96.9 BAP-0906 36.75 38 43 26 17 91.1 BAP-0907 14.25 43 44 28 16 84.7 BAP-0907 16.75 44 45 29 16 84.9 BAP-0907 21.75 39 30 24 6 56.3 BAP-0902 27.25 54 NP NP NP NP BAP-0902 32.25 38 36 28 8 <td< td=""></td<>

PROJECT	CARDINAL PLANT A	SH POND INV	'ESTIGATION	
LOCATION	BRILL	IANT, OHIO		
JOB NO.	011-11497-013	DATE _	7/6/09	



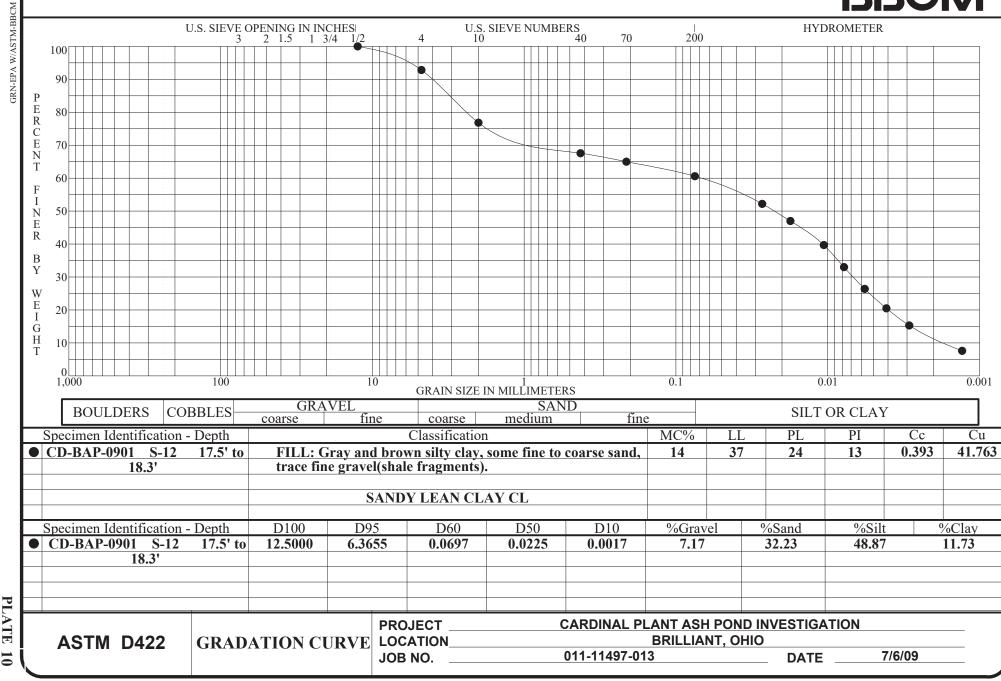
ORGANIC CLAYEY SILT

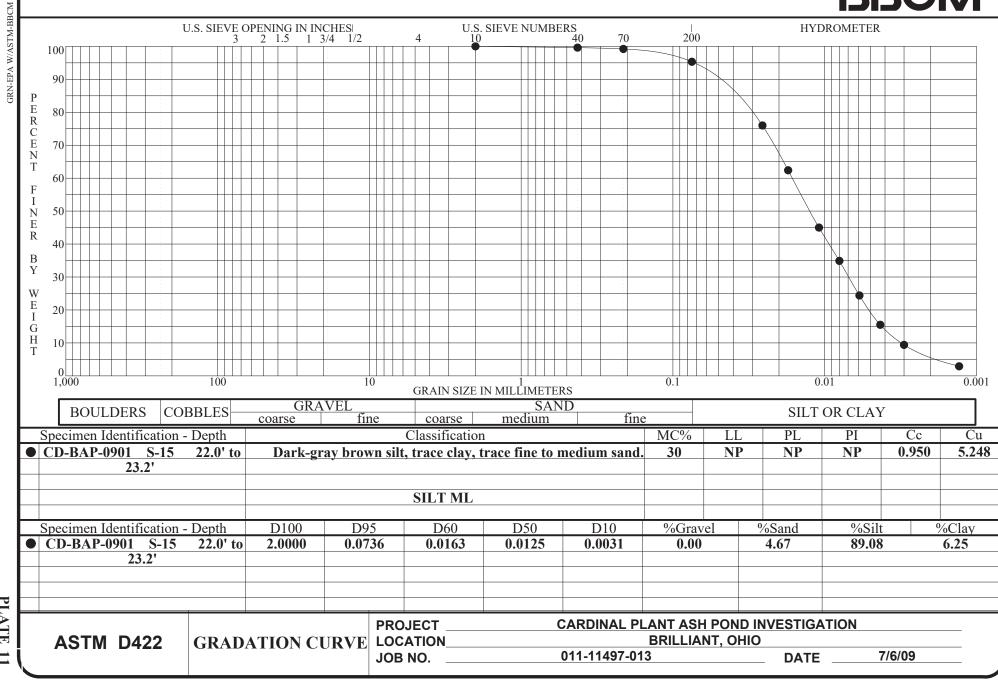
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LIQUID	LIMII	(LL)

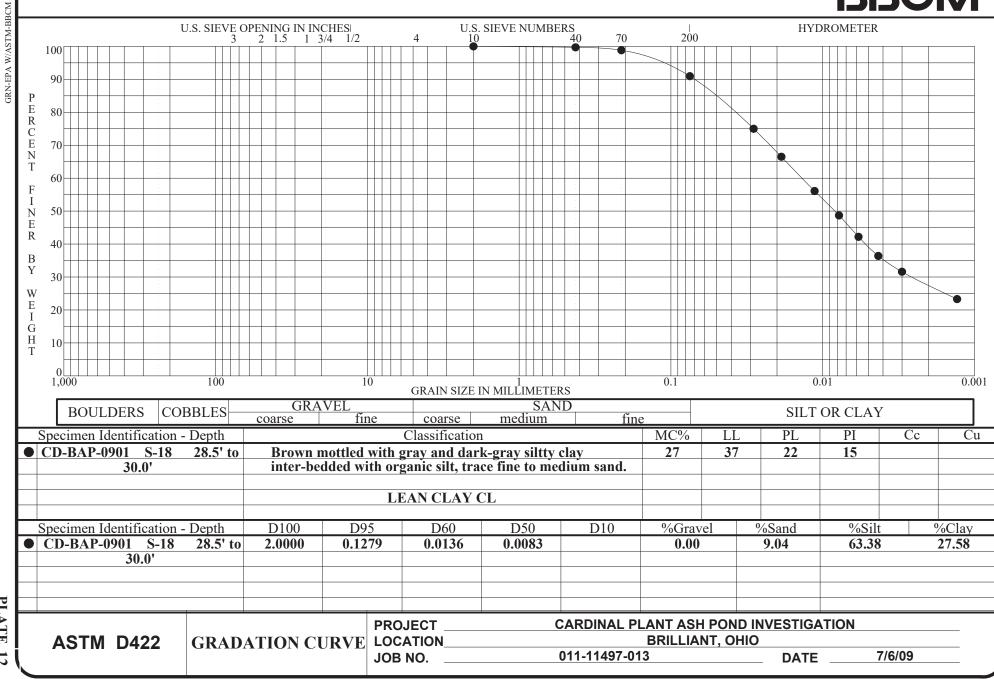
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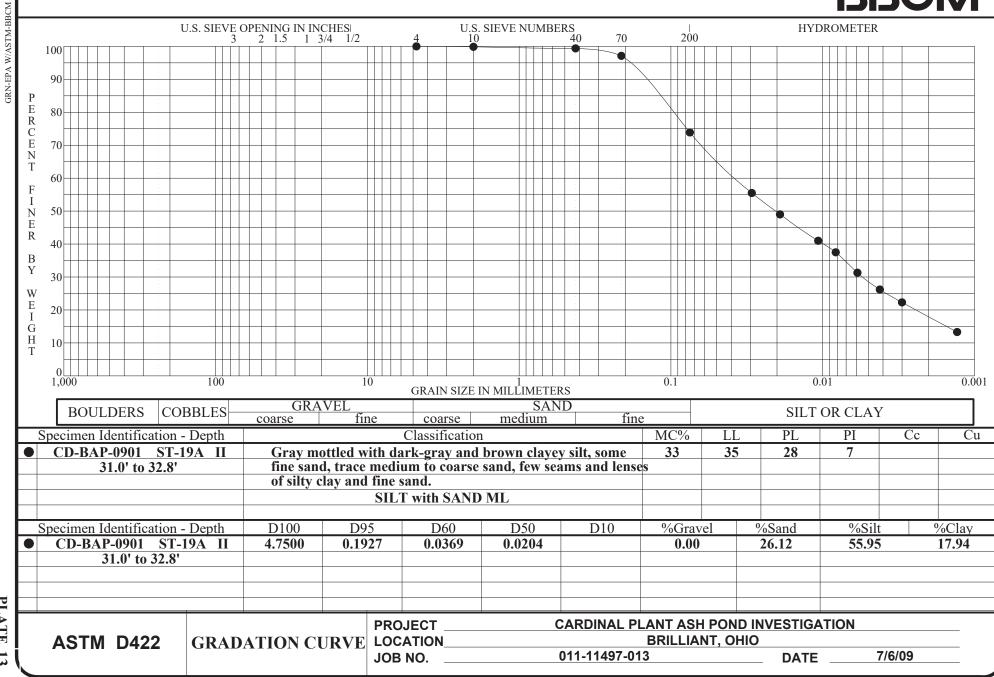
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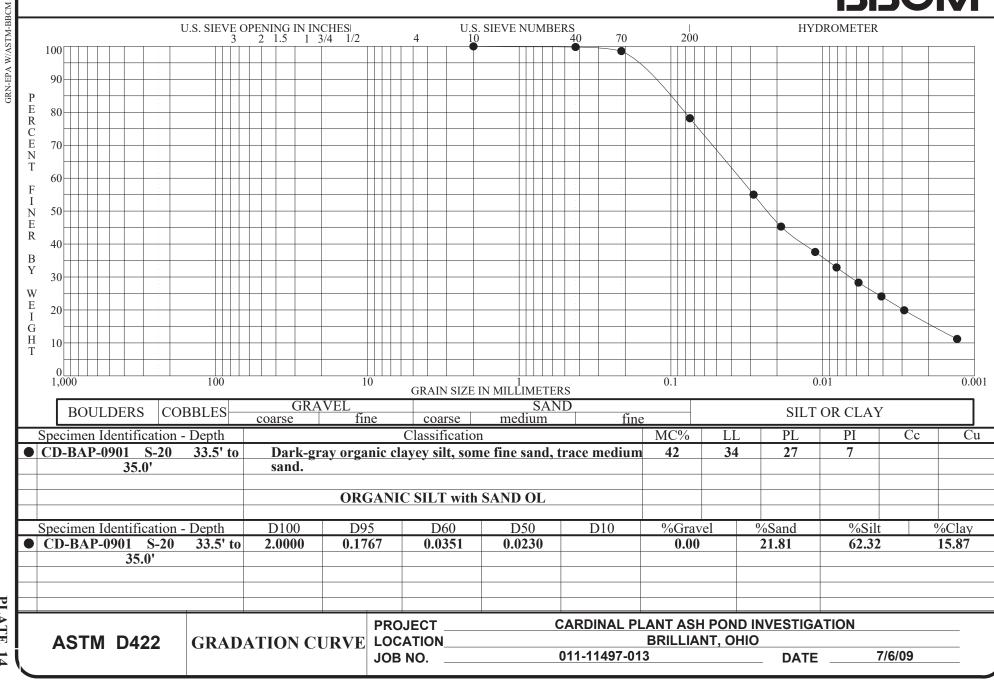
pecimen Id.	Depth	MC	LL	PL	PΙ	Fines	ASTM Classification
BAP-0905	14.25	45	43	27	16	80.5	ORGANIC SILT with SAND OL
BAP-0905	16.75	42	40	25	15	84.5	ORGANIC CLAY with SAND OL
		CA	RDINA				INVESTIGATION
OCATION _		11-114	197_01		RILLIA	NT, OH DATE	

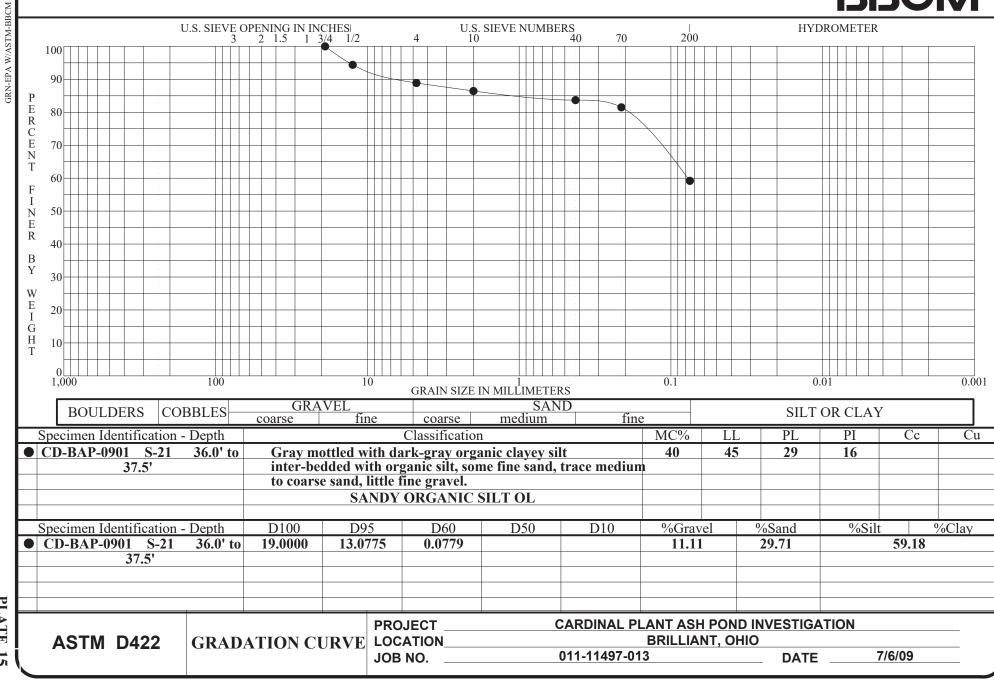


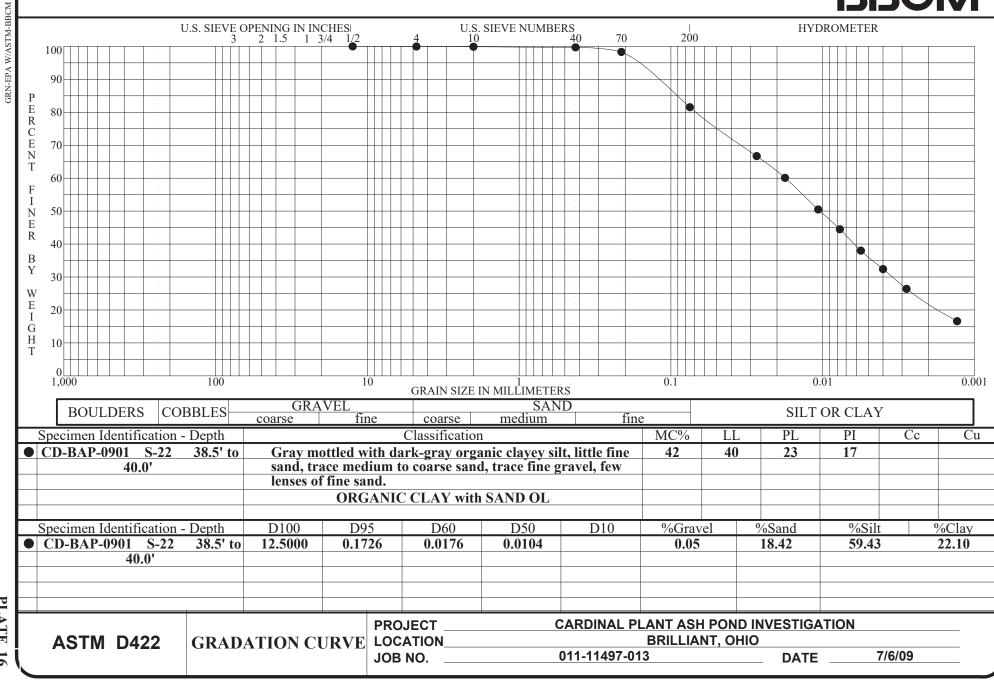


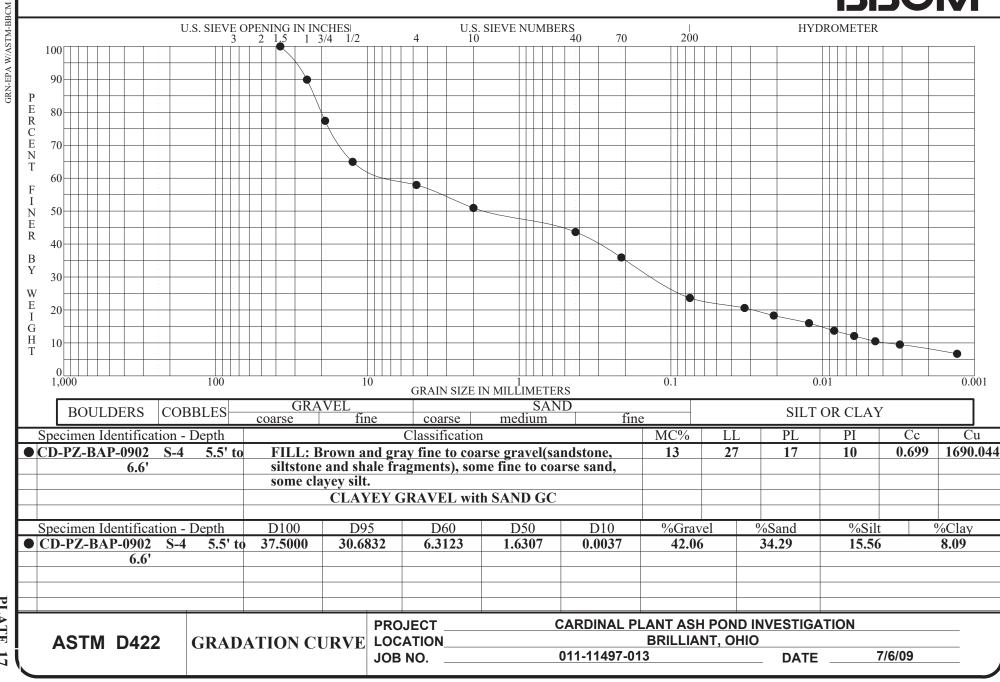


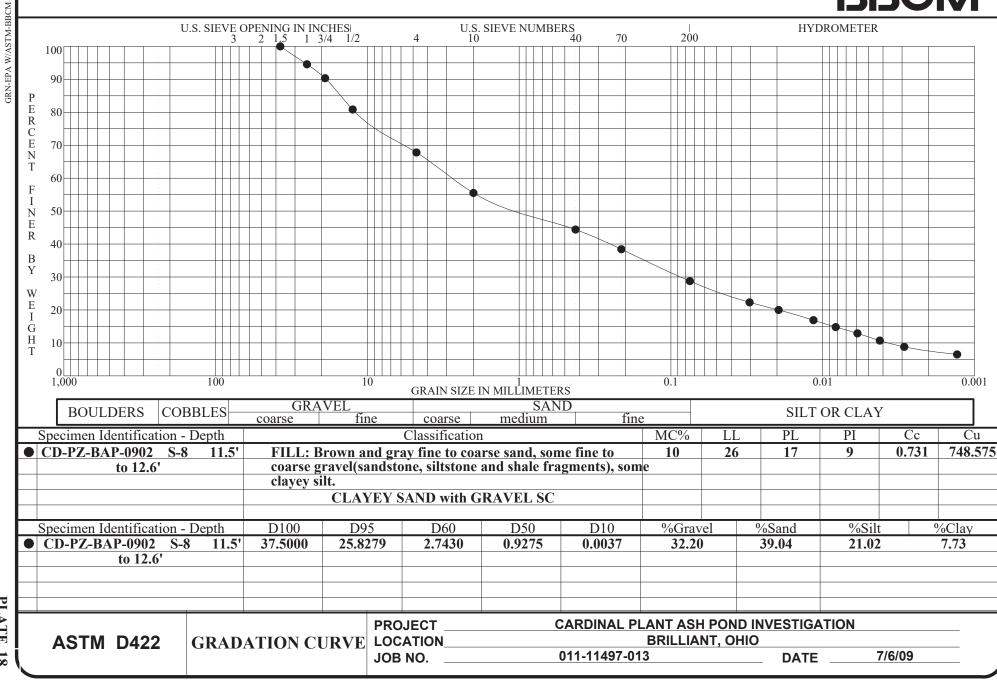


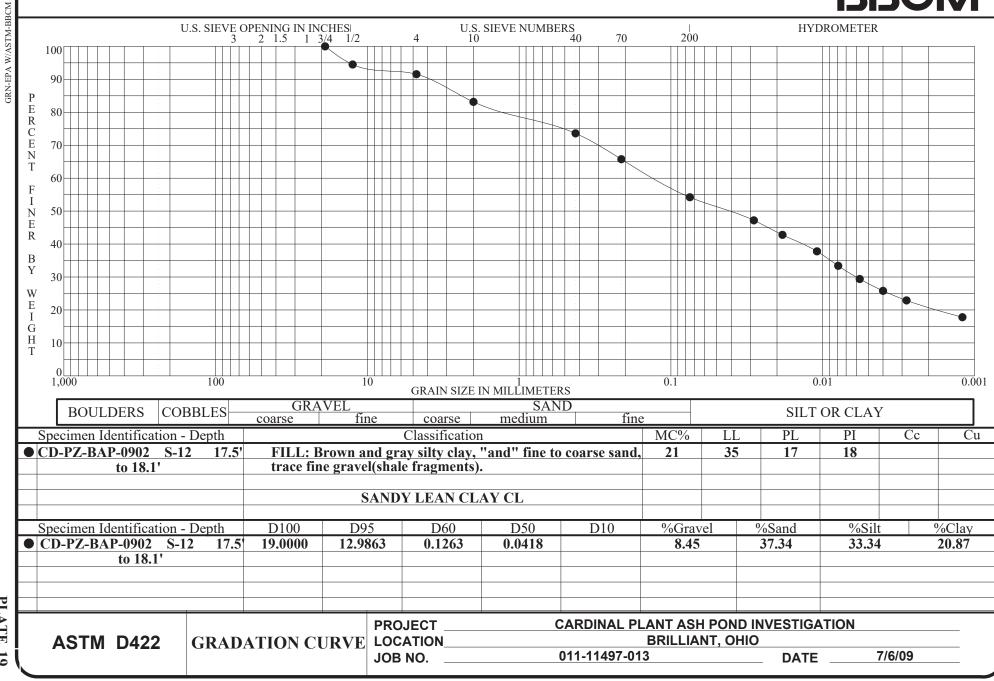


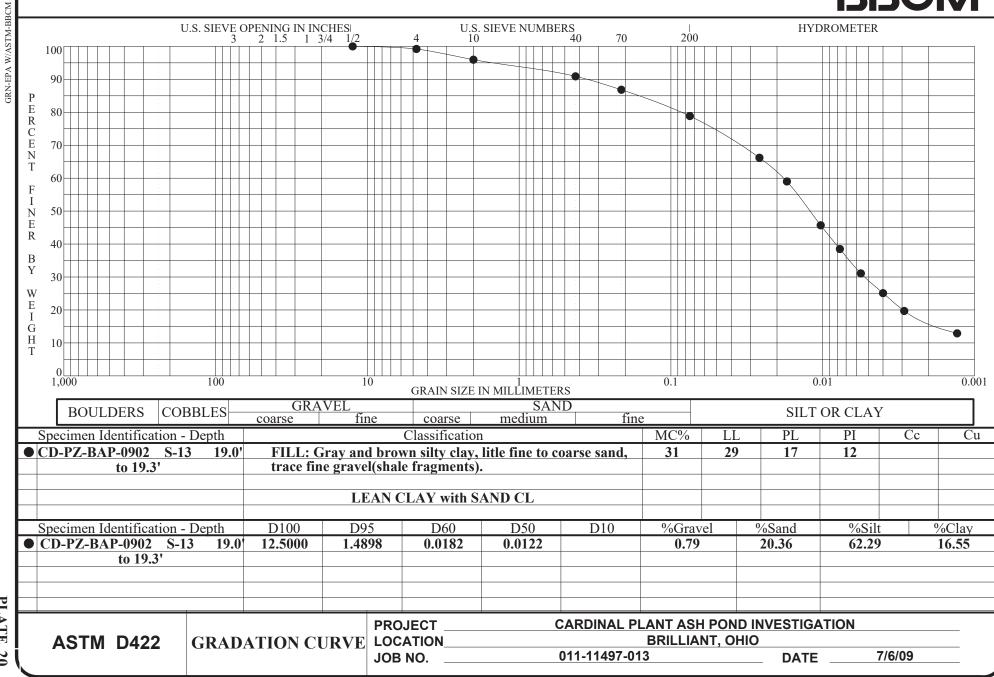




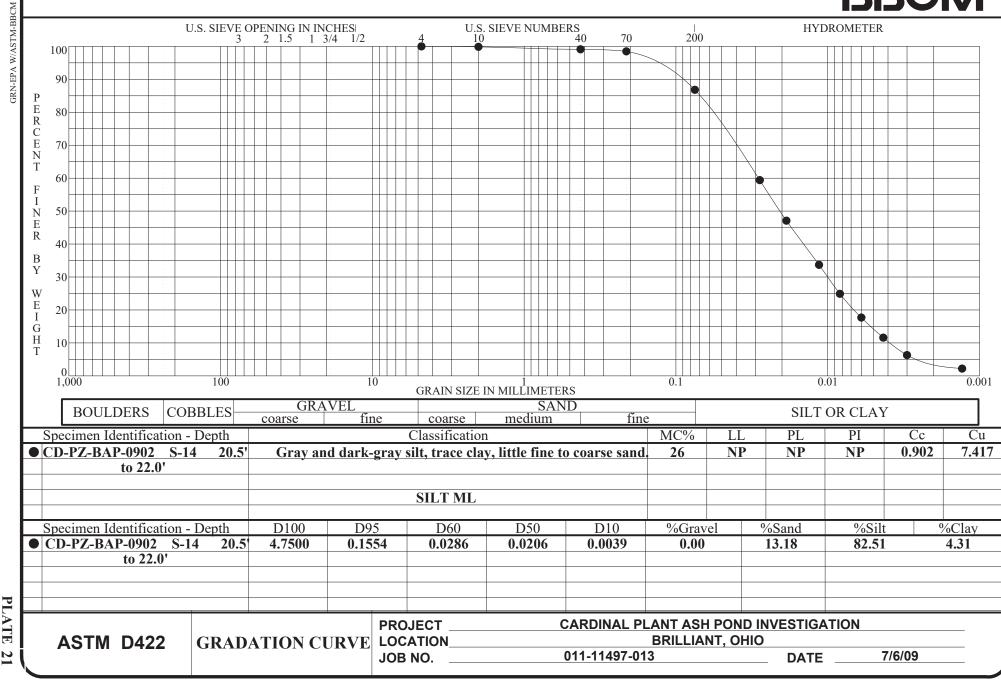




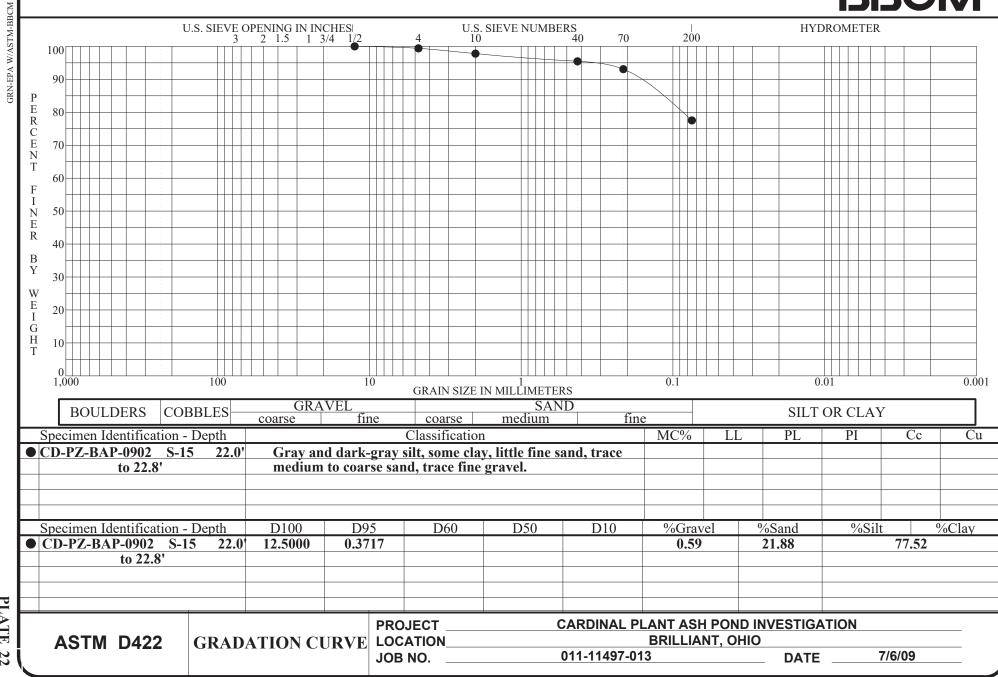


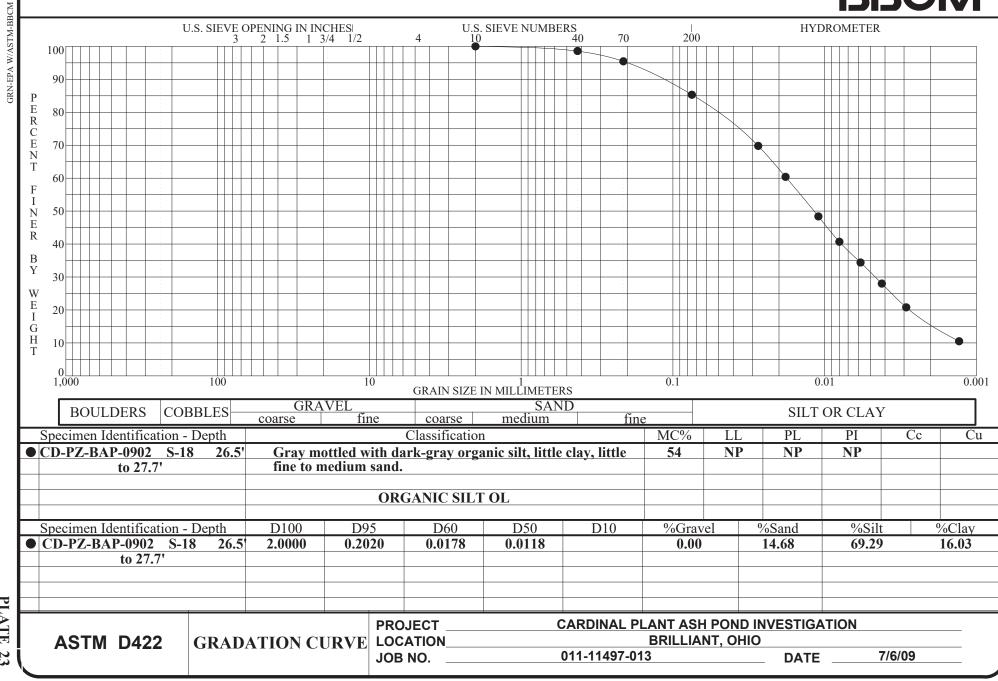


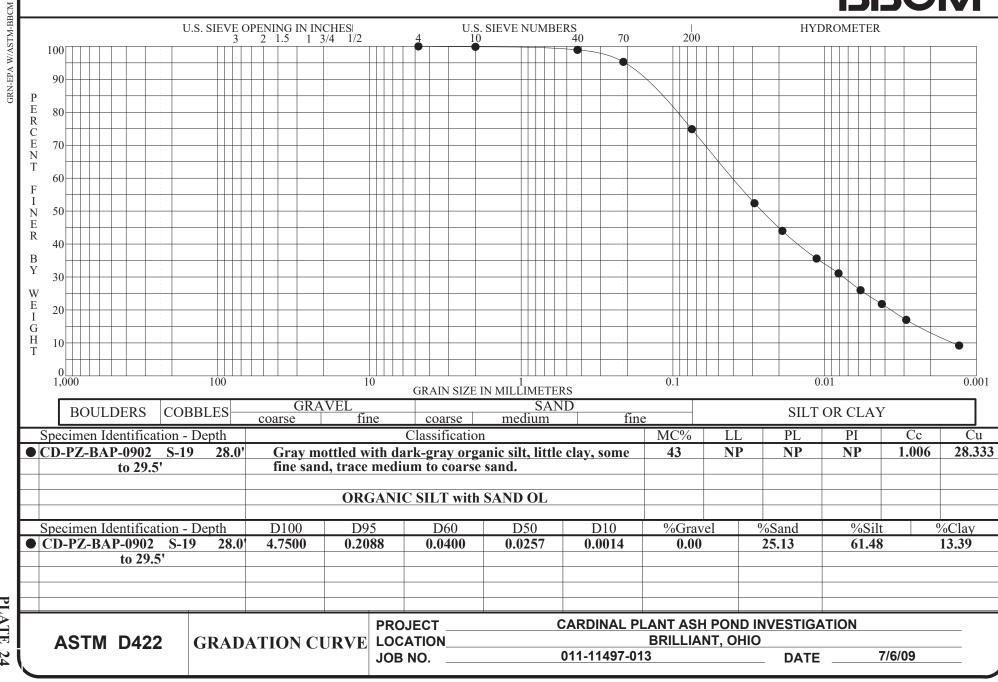
PLAIE 2

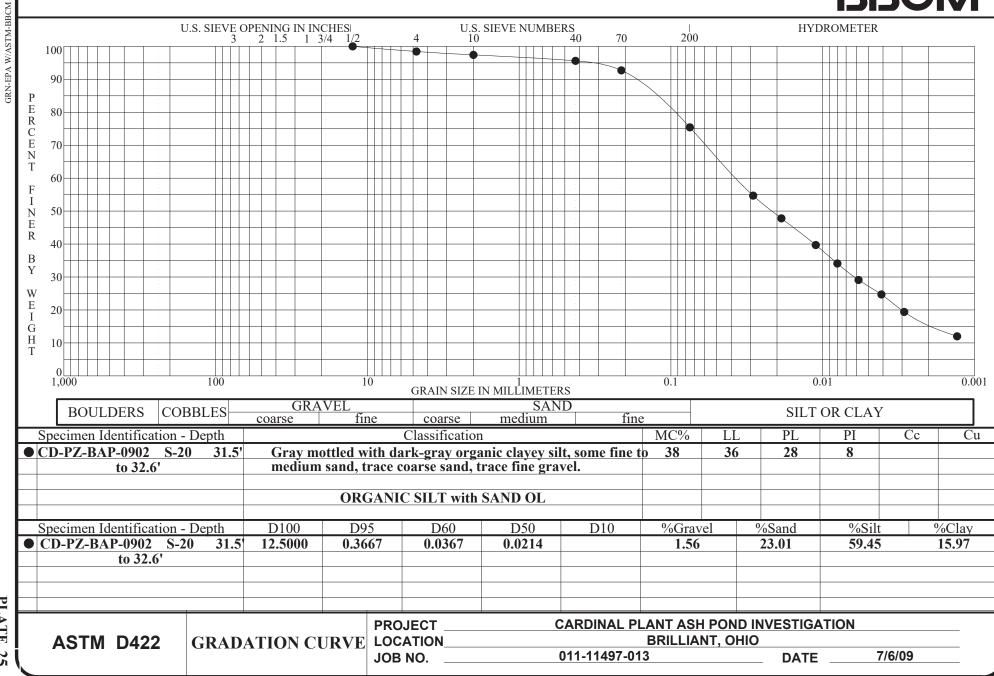


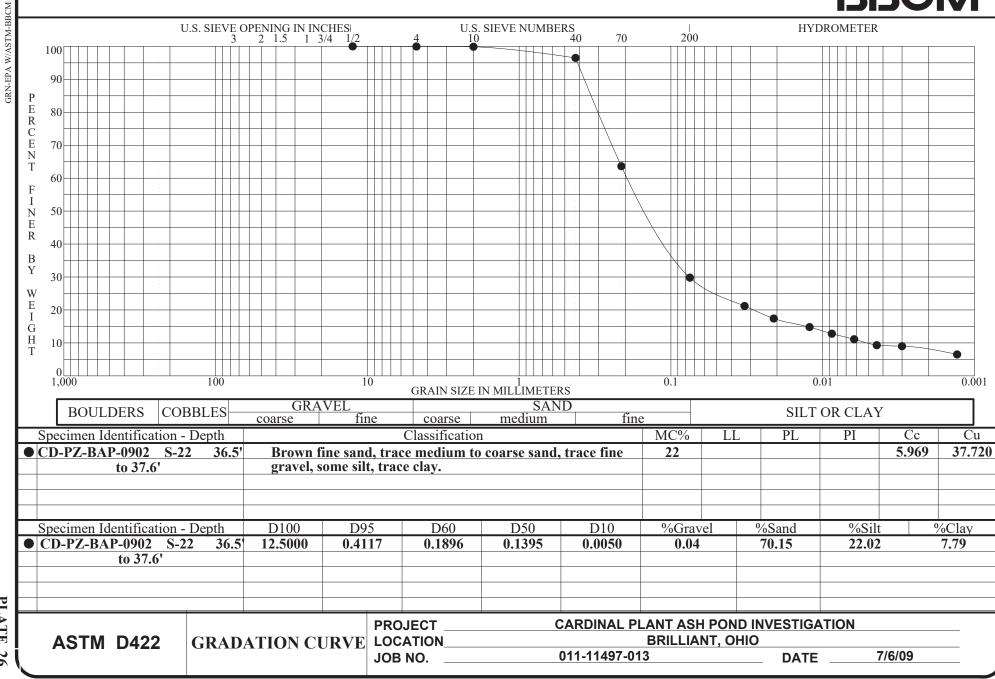


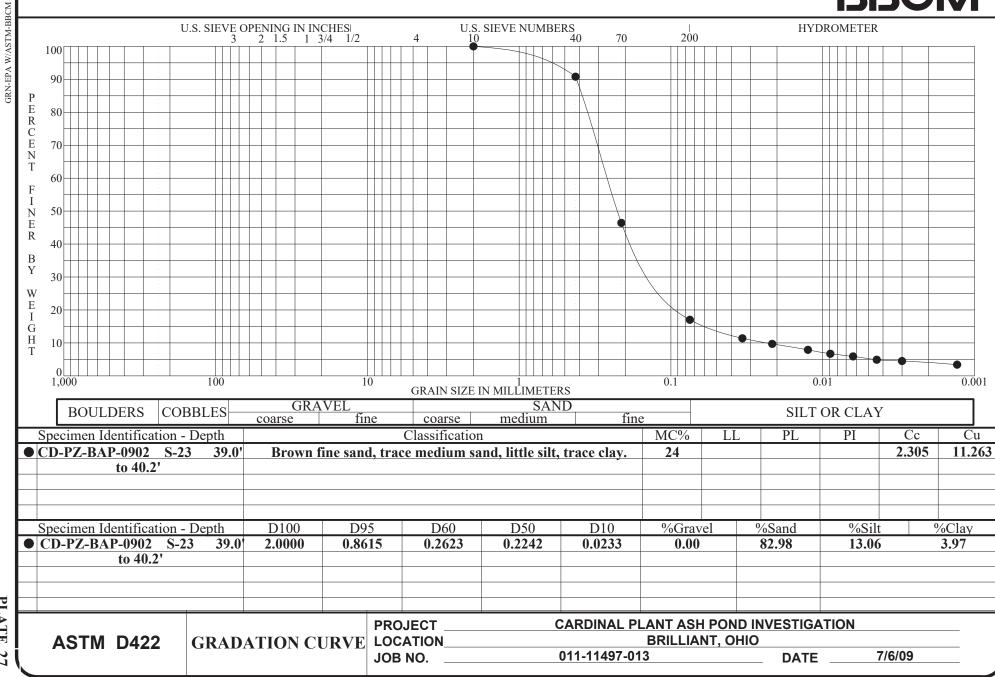


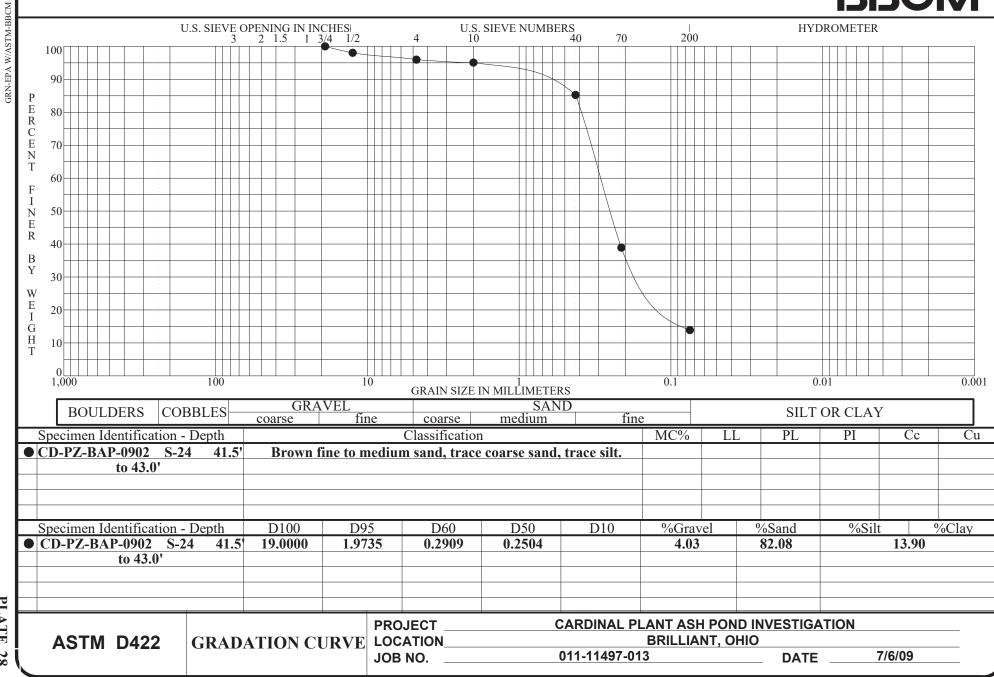






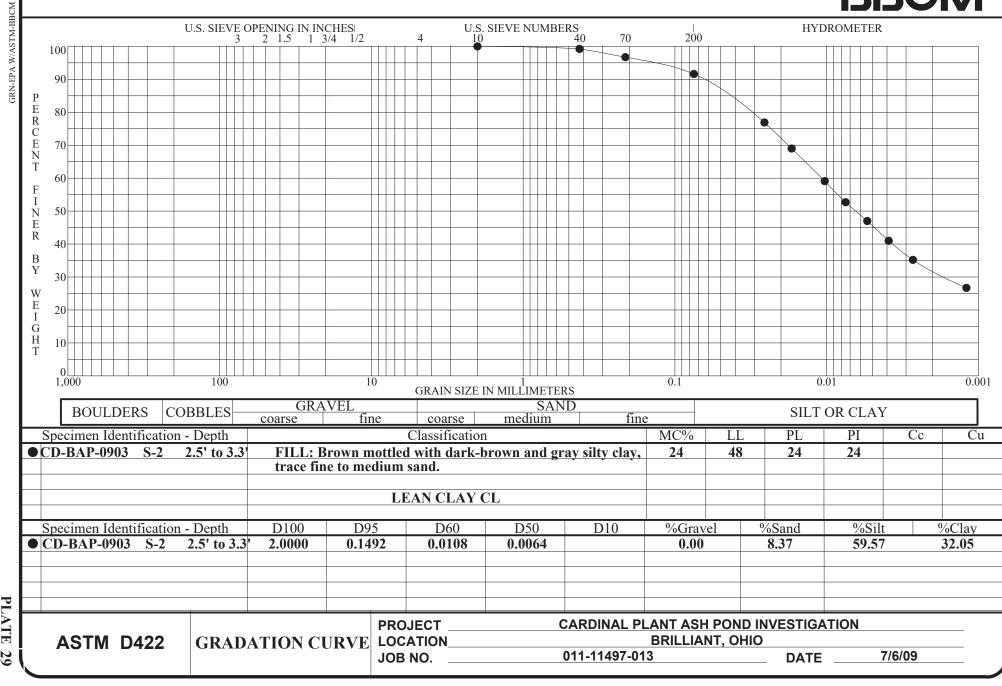




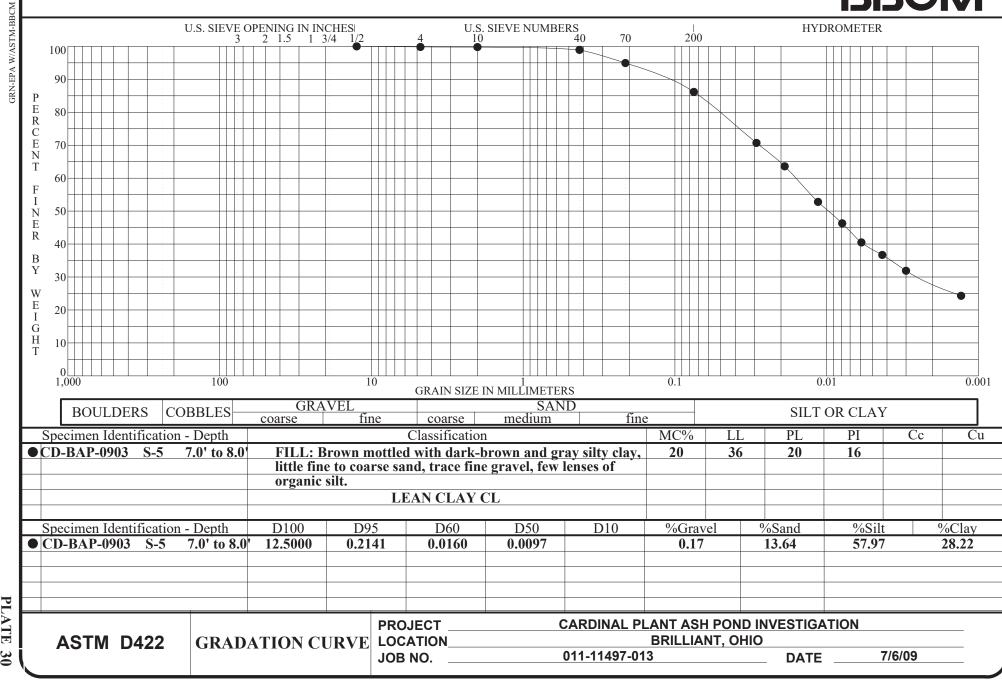


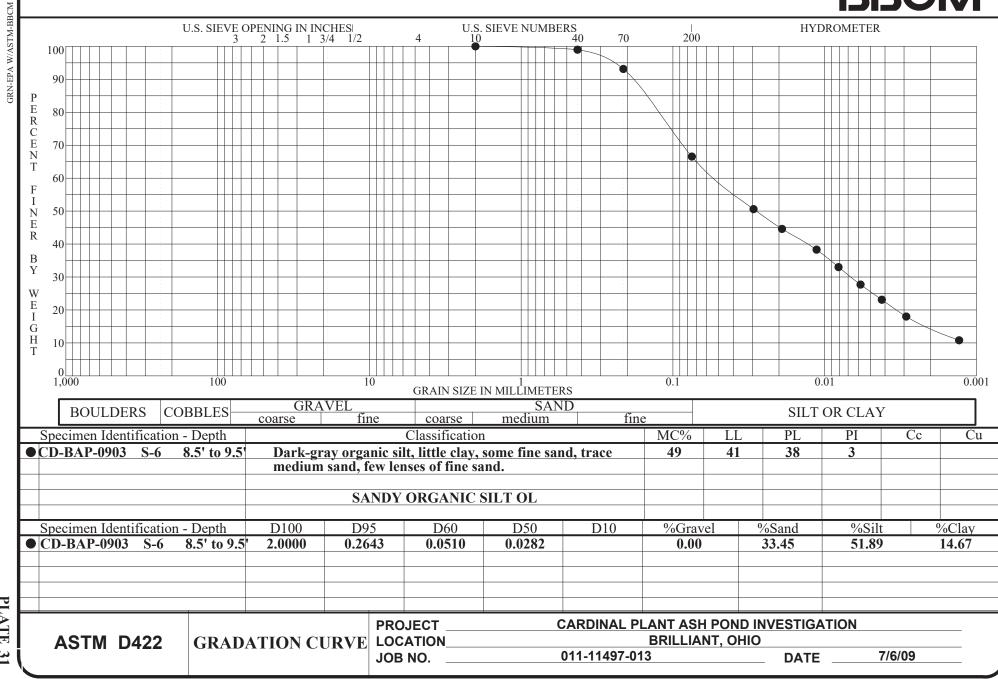
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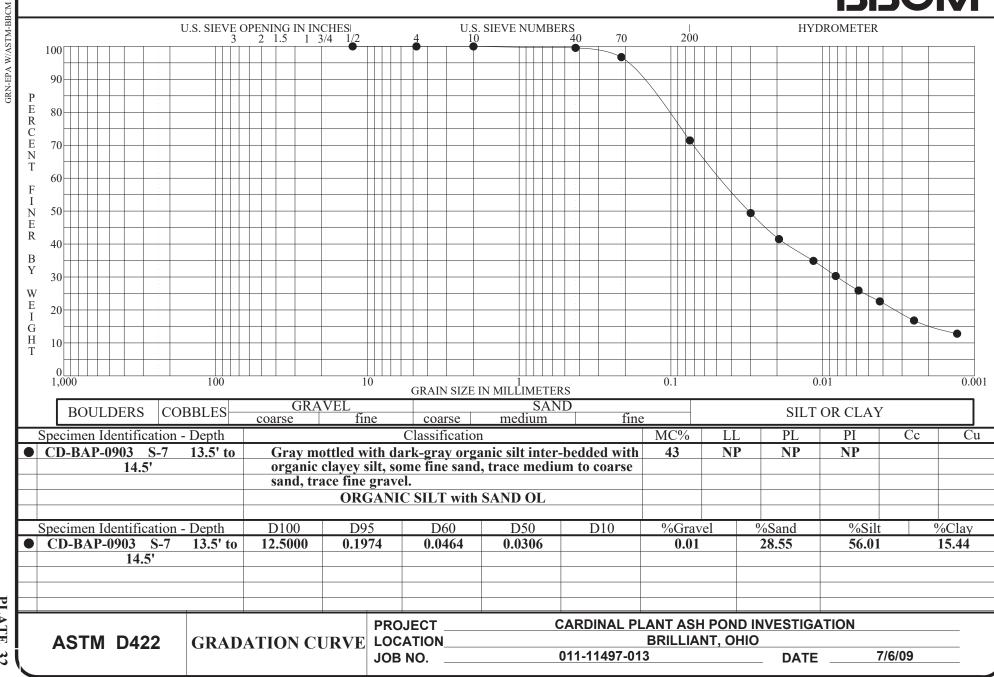


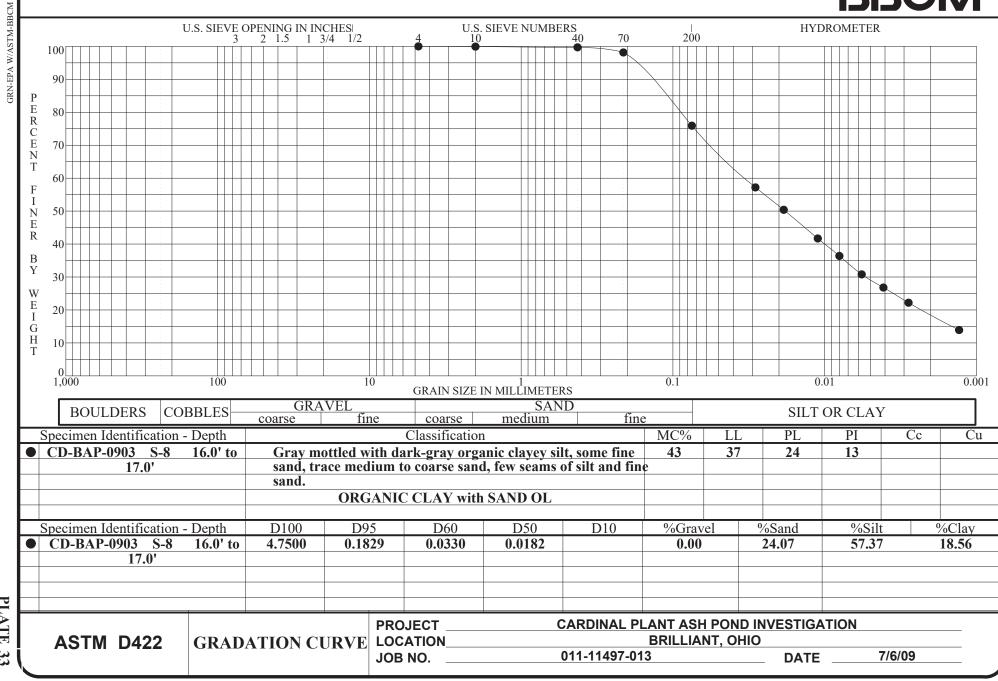


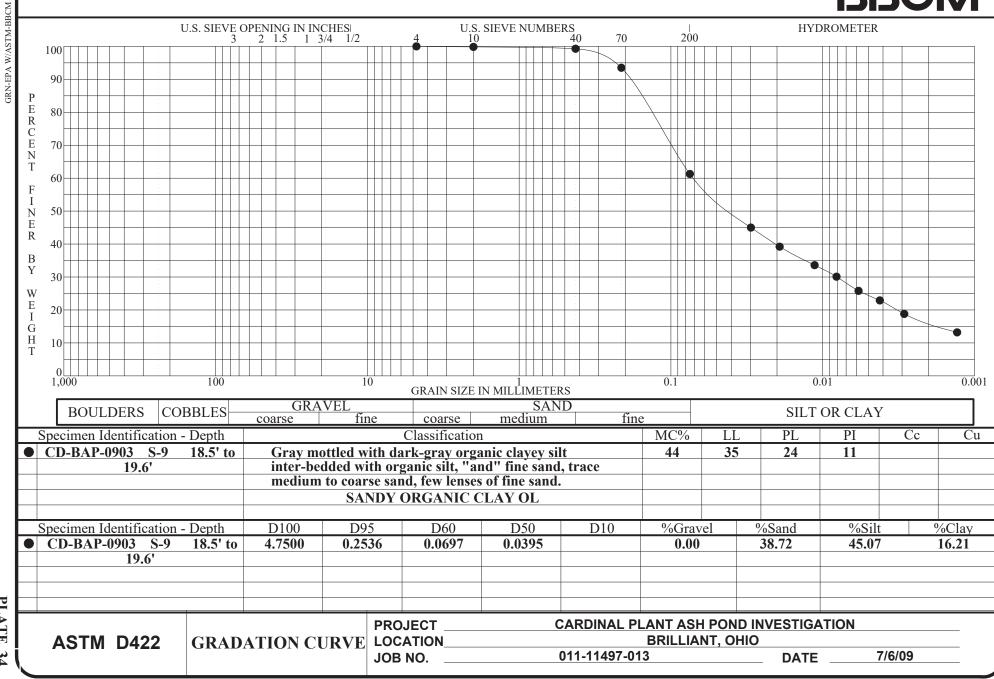


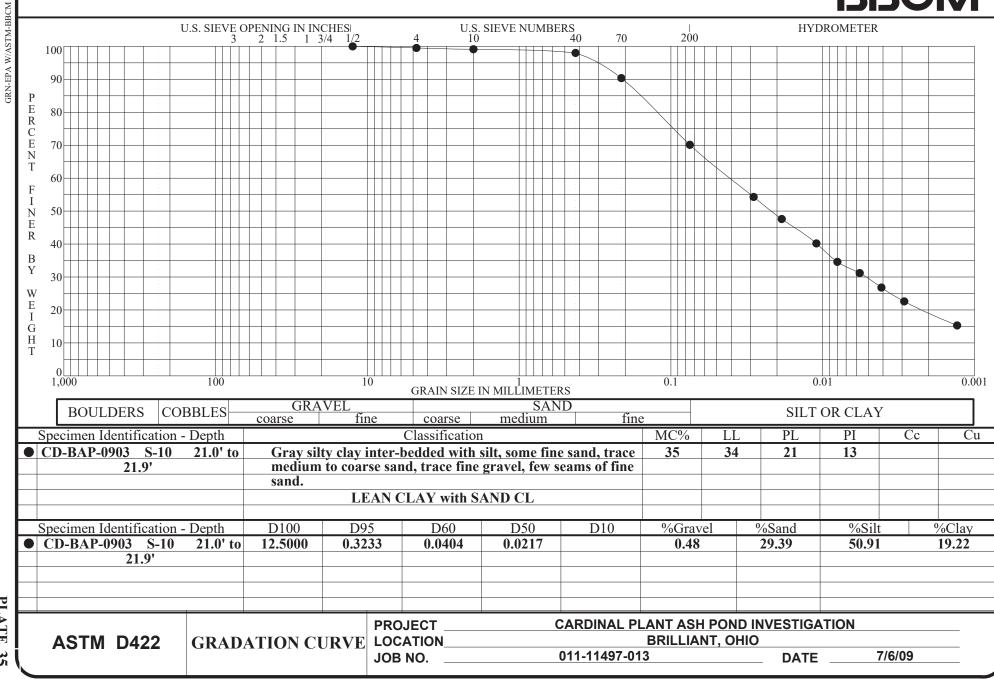


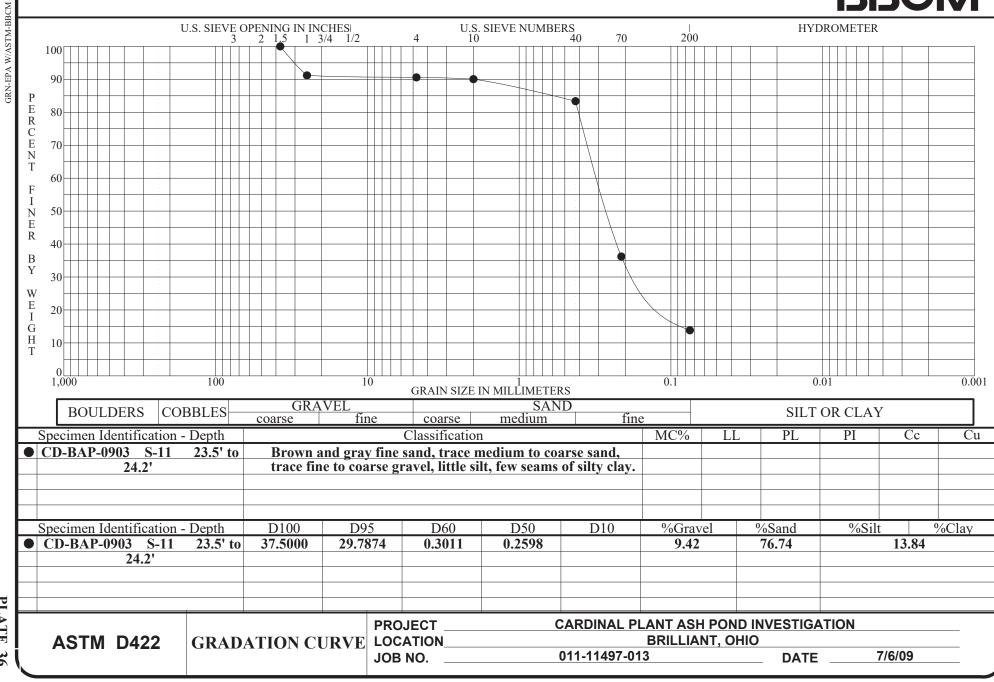


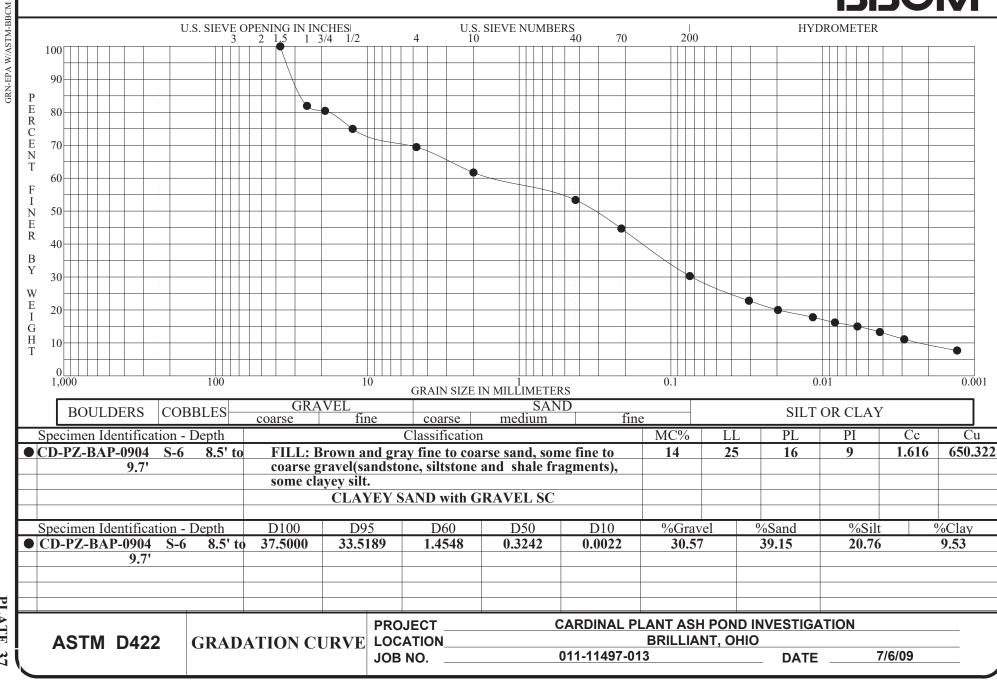


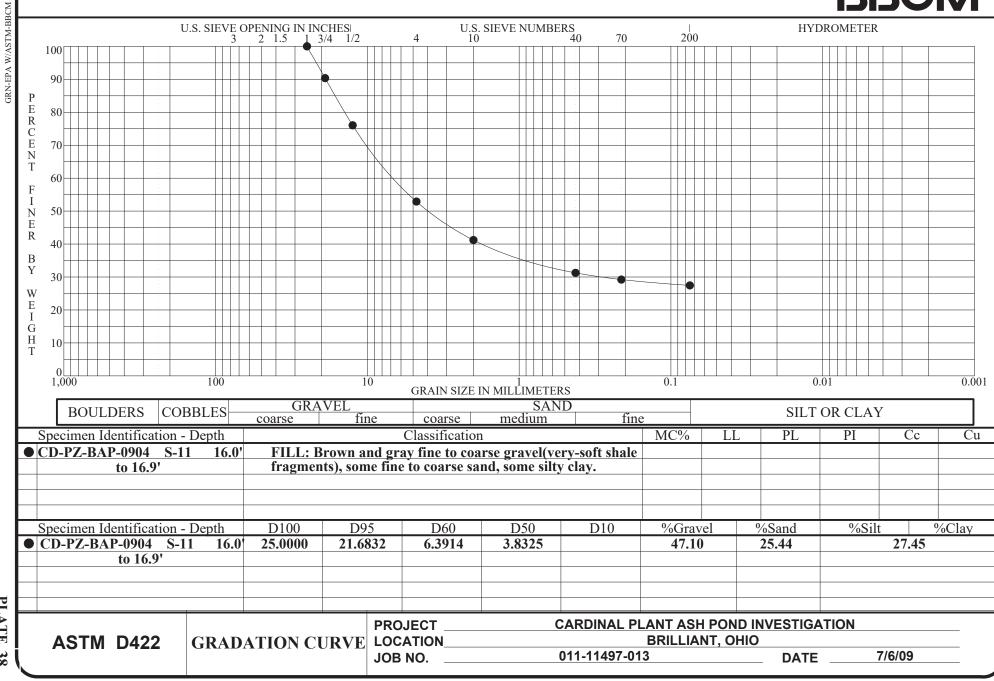


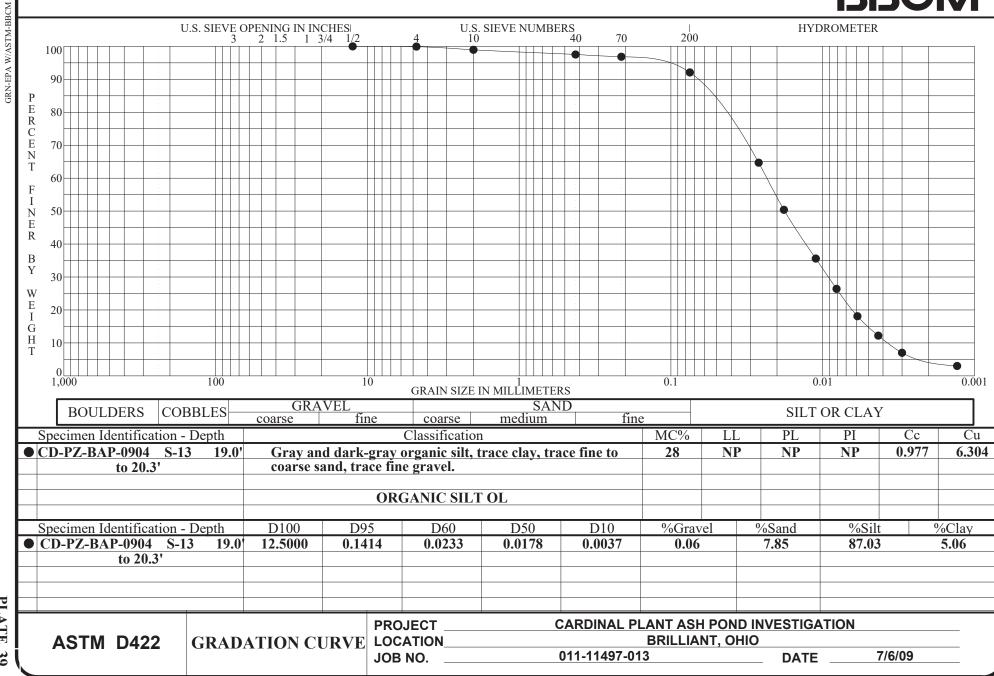


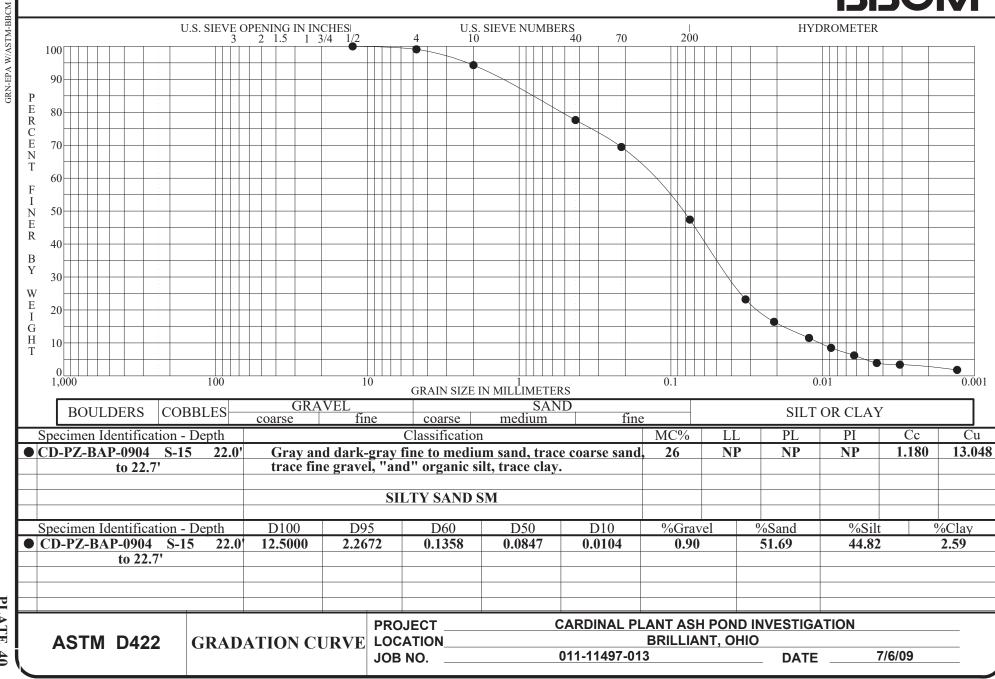


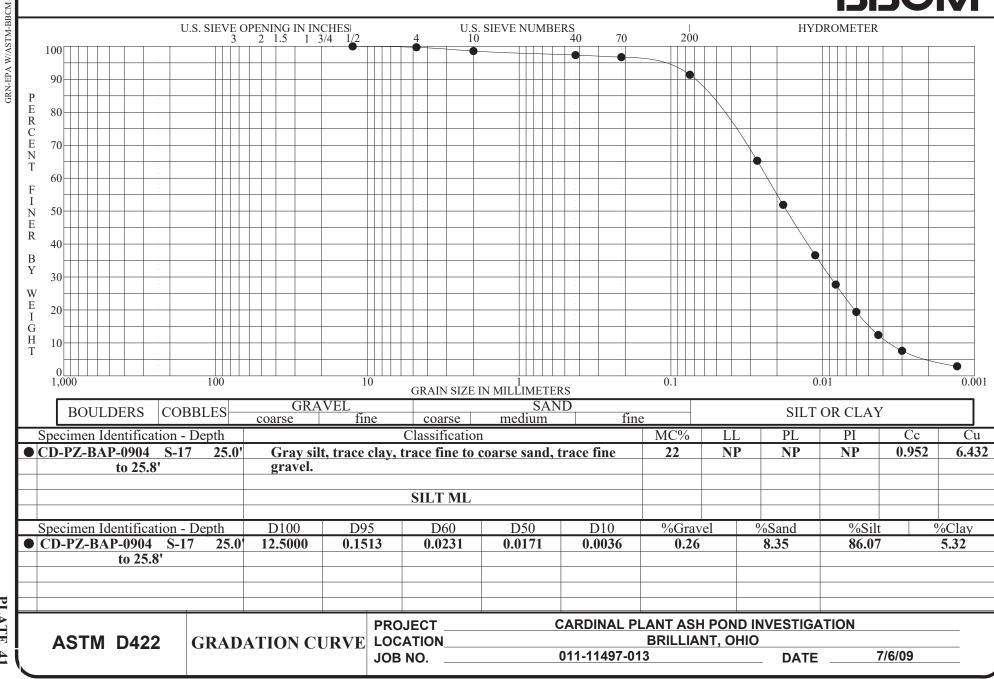


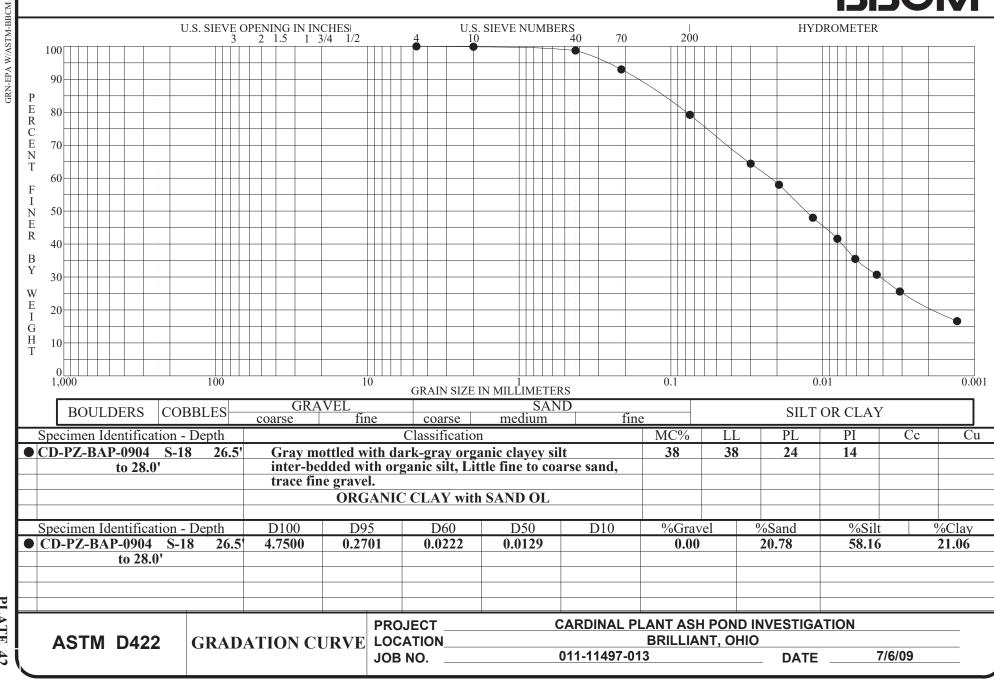


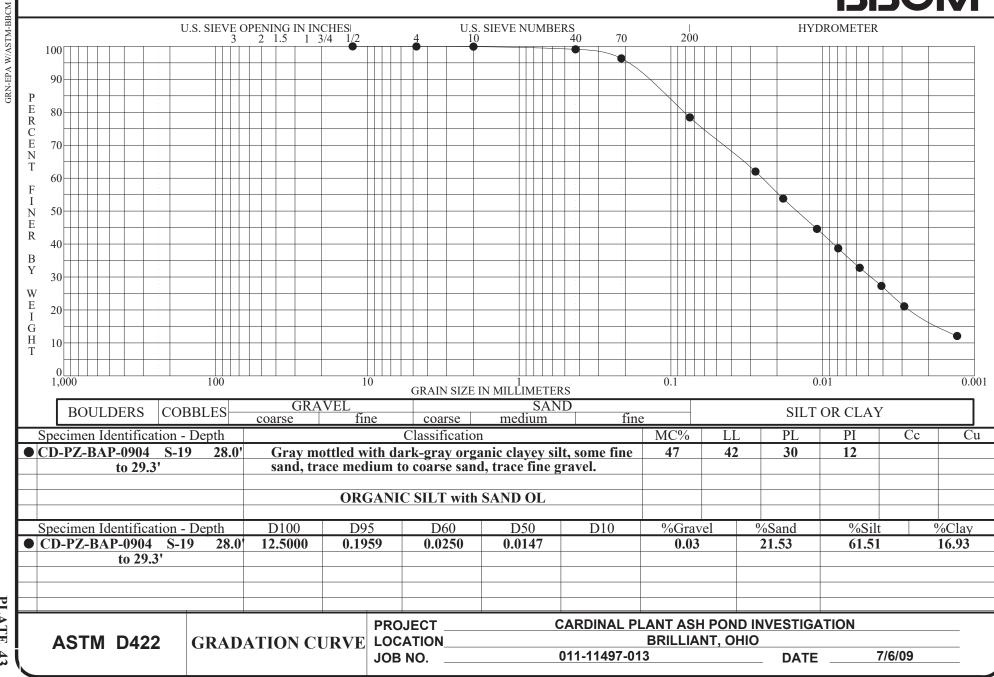


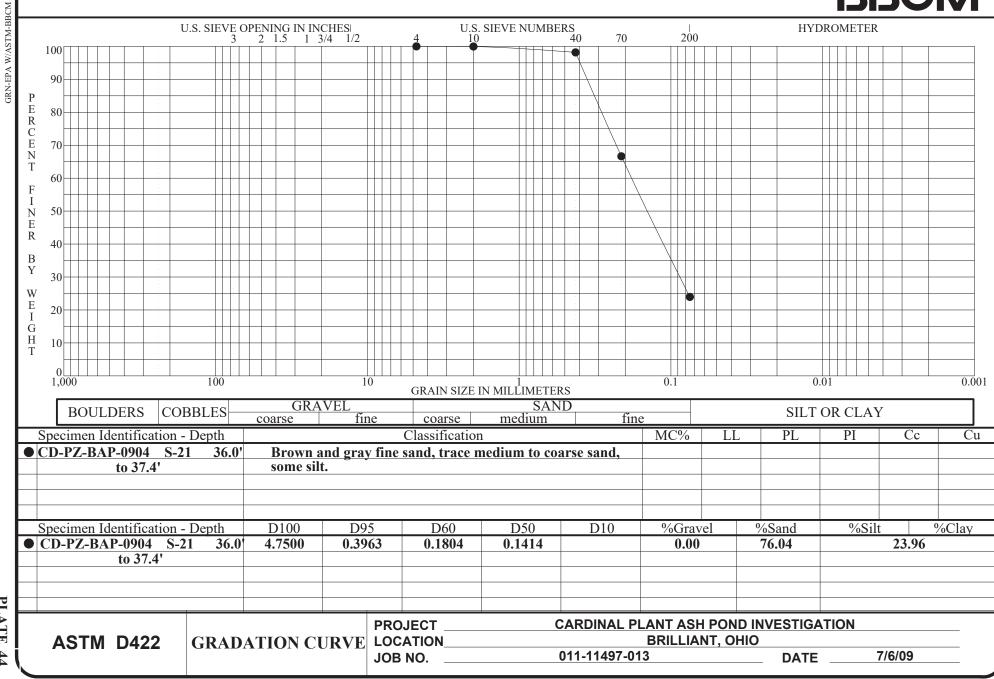


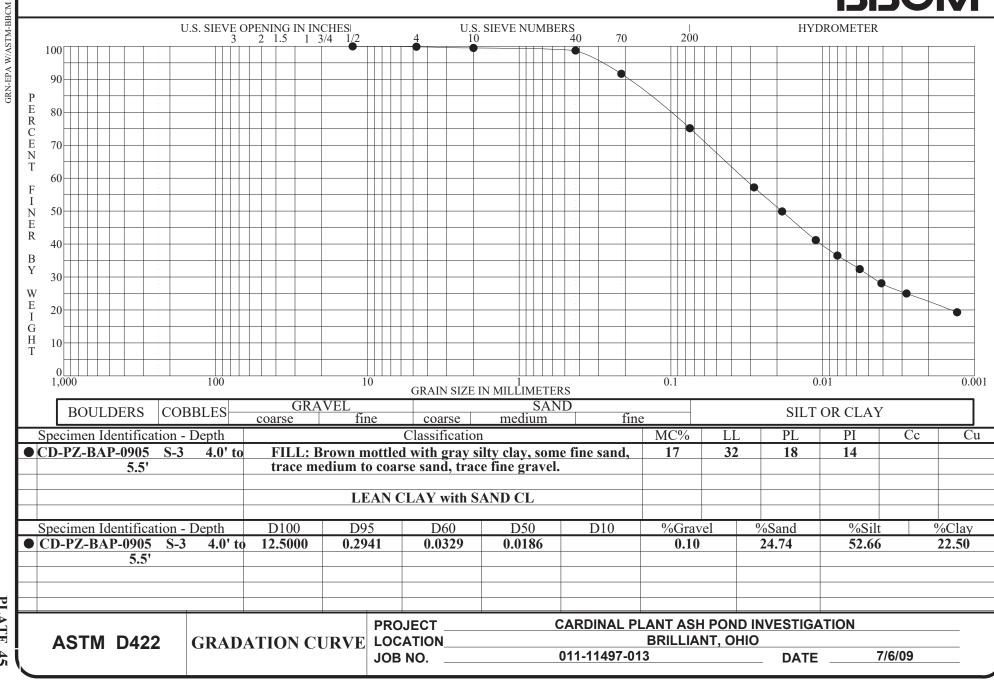


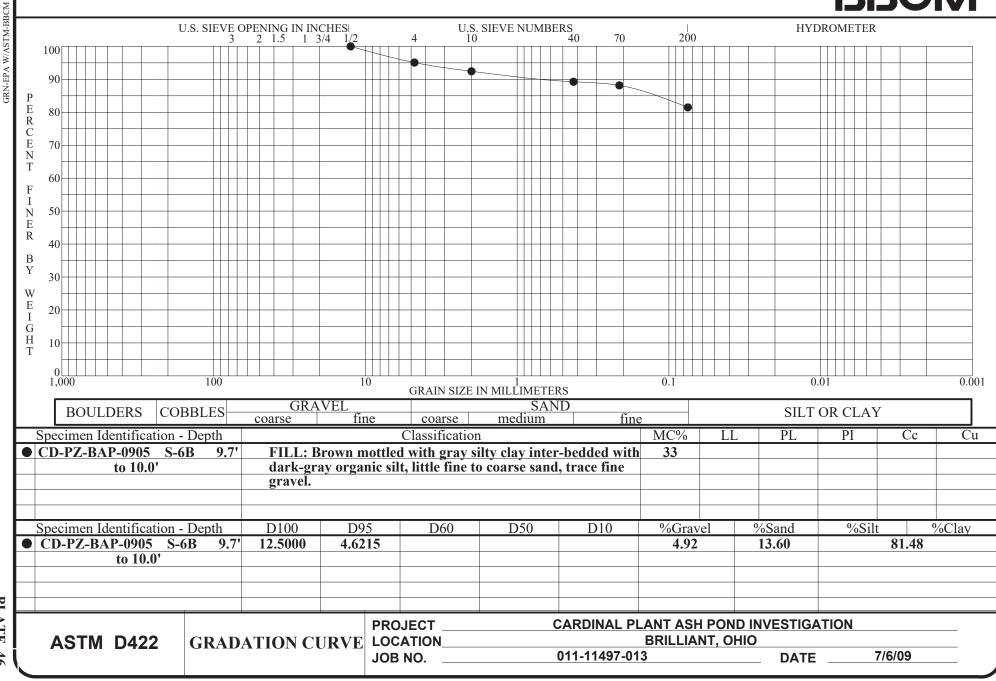


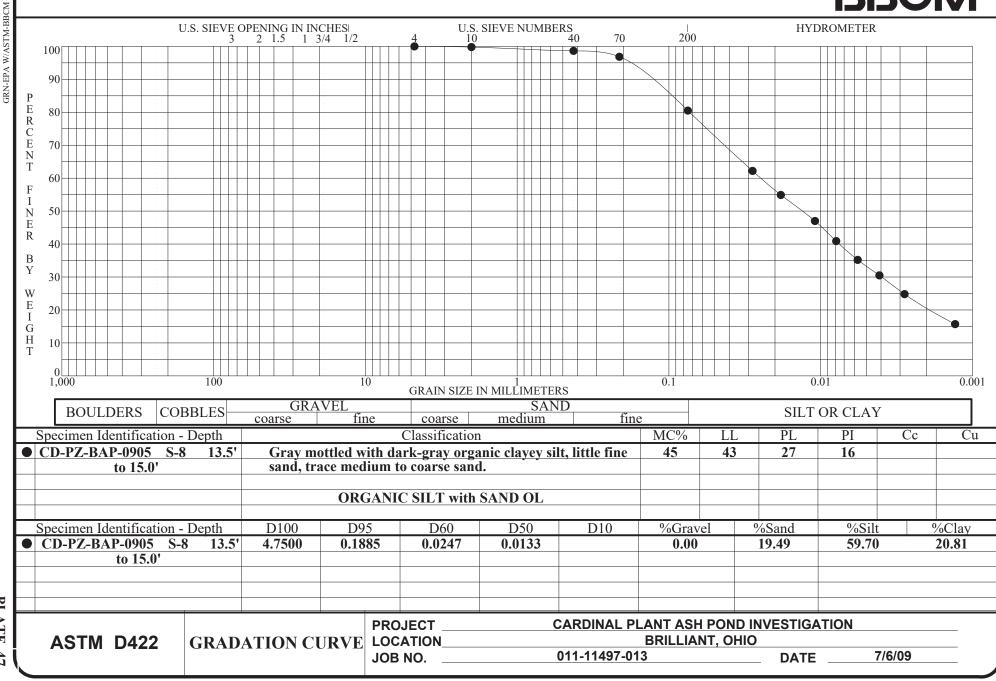


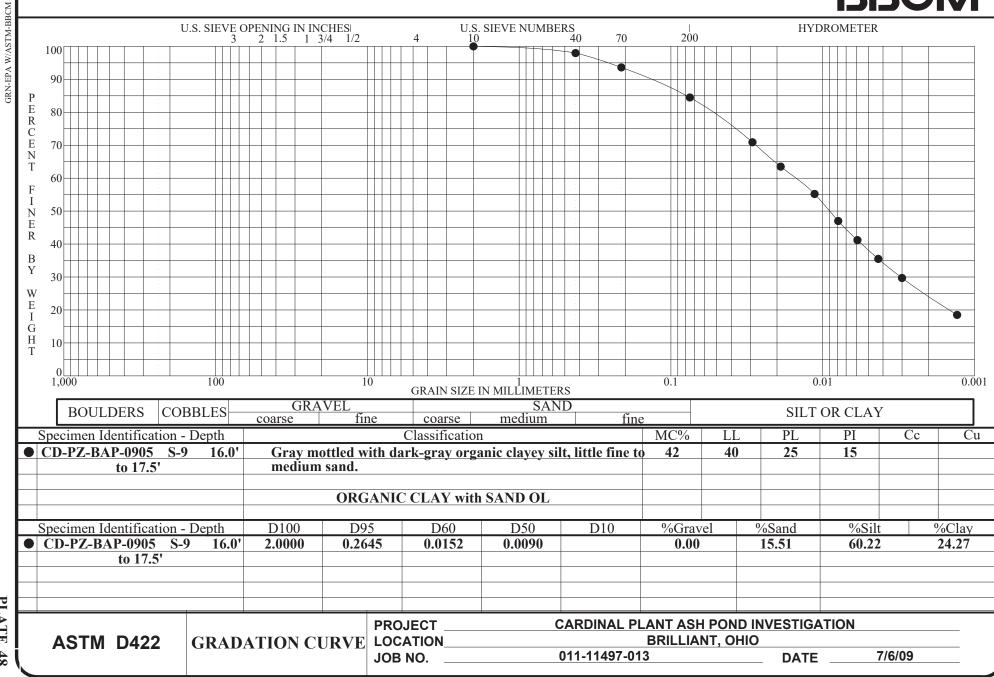


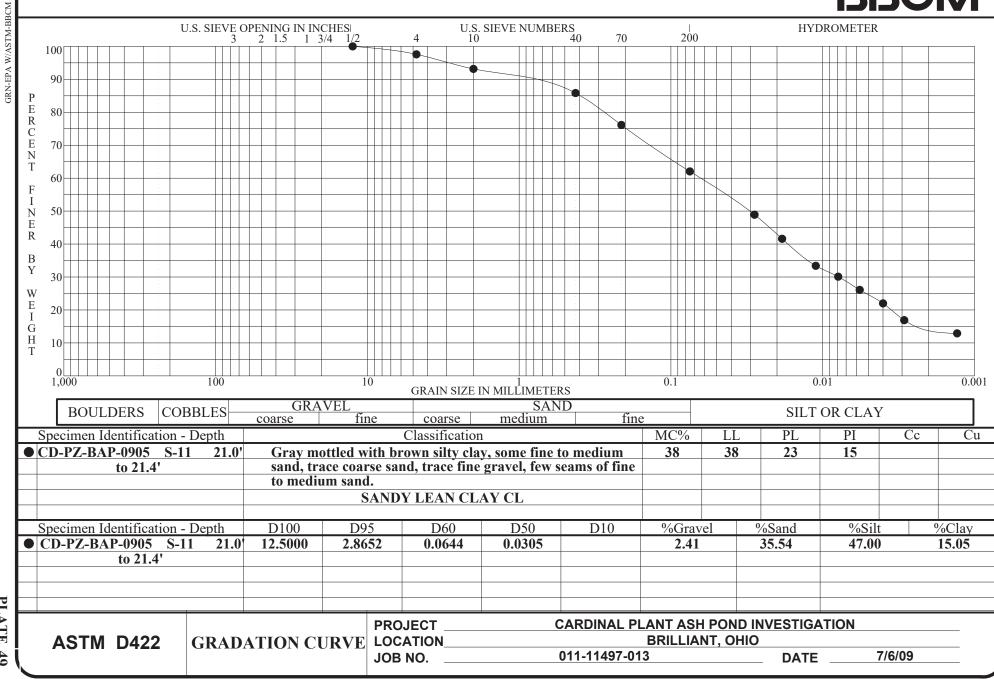


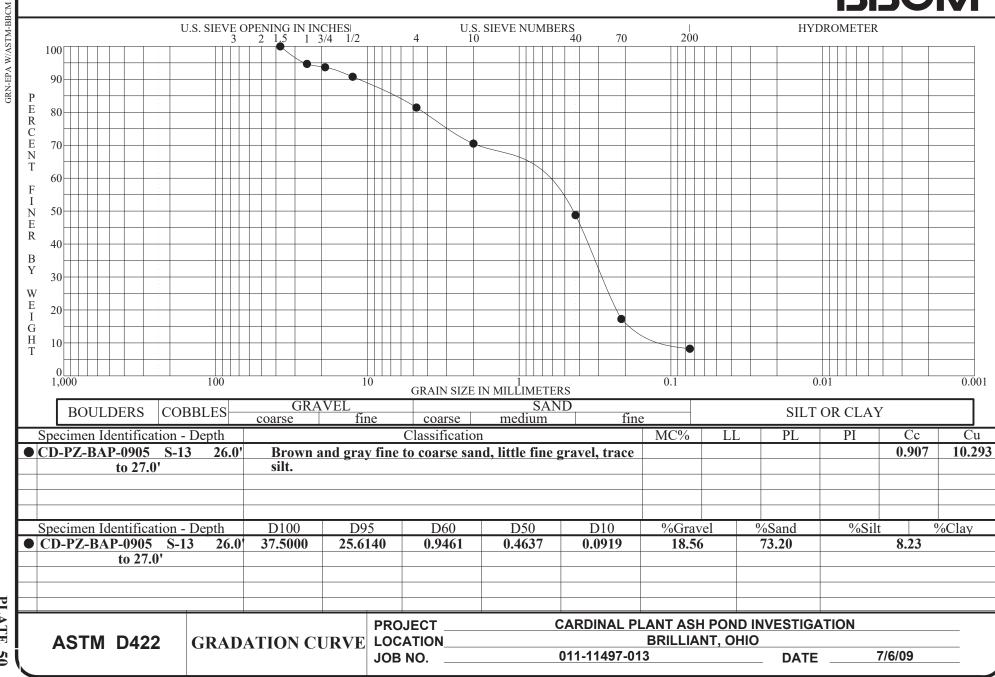


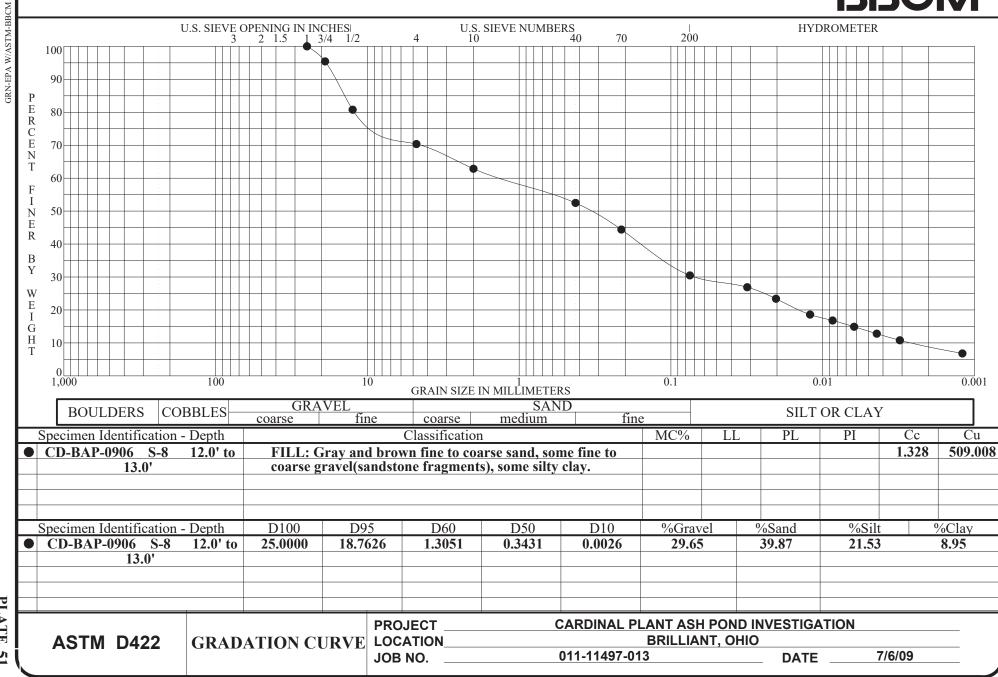


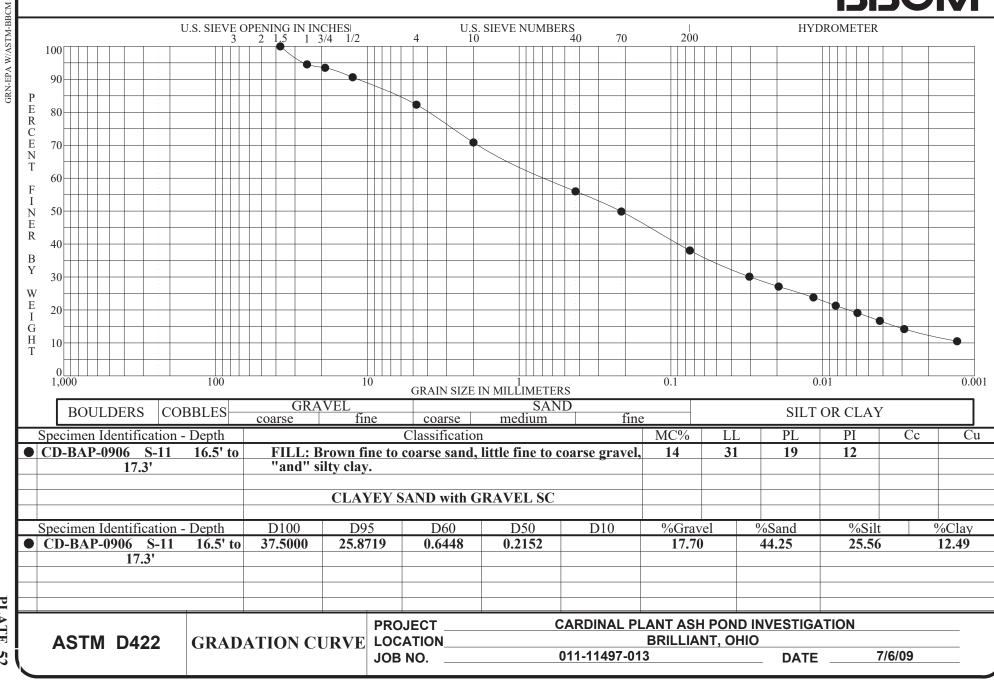


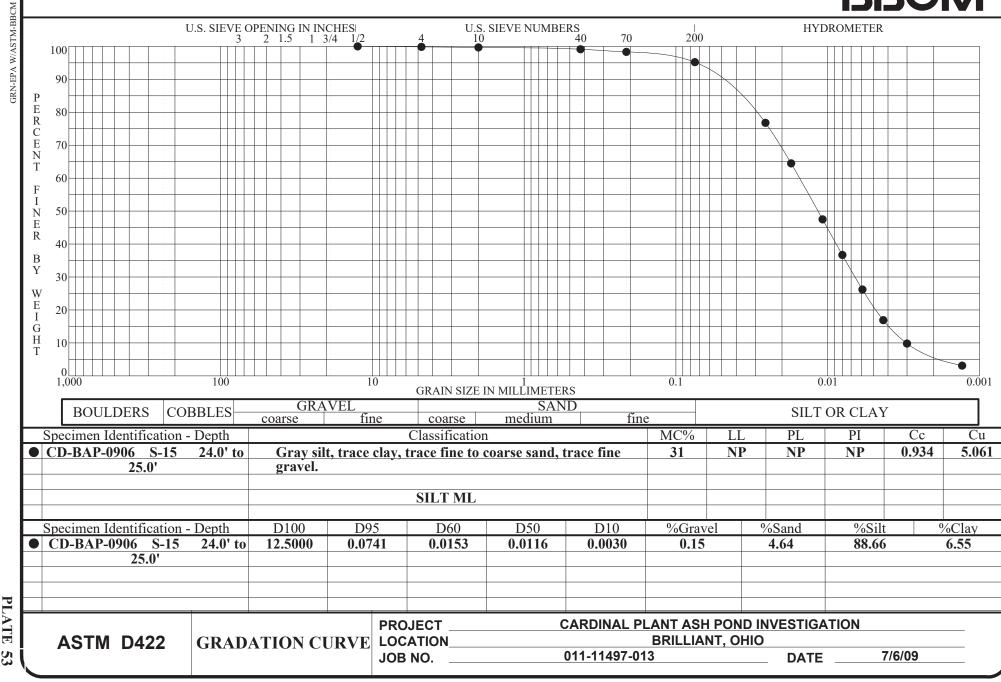


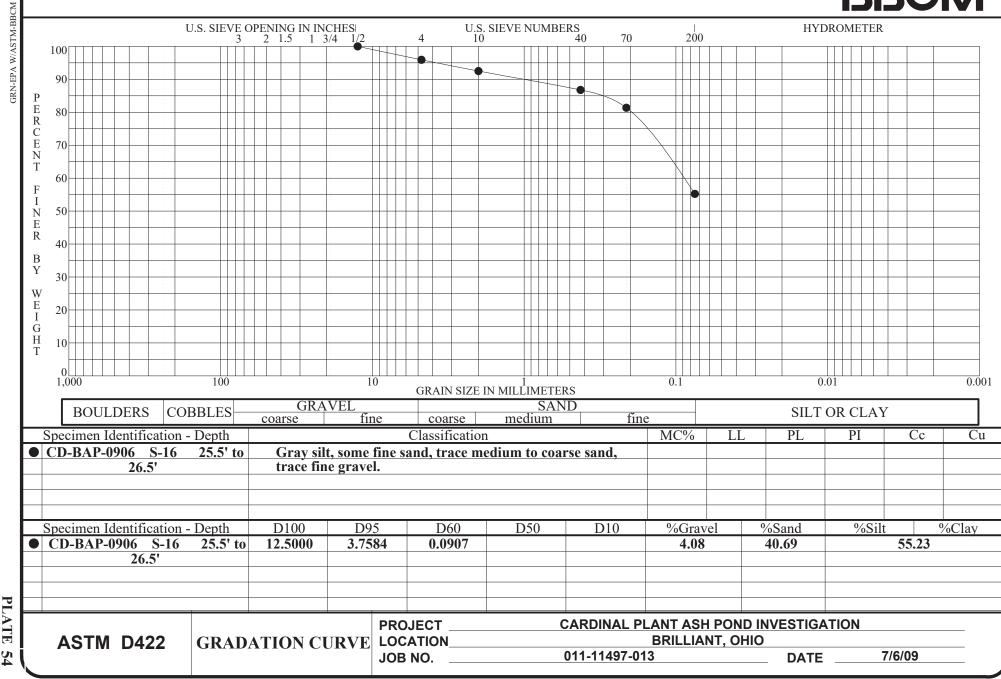


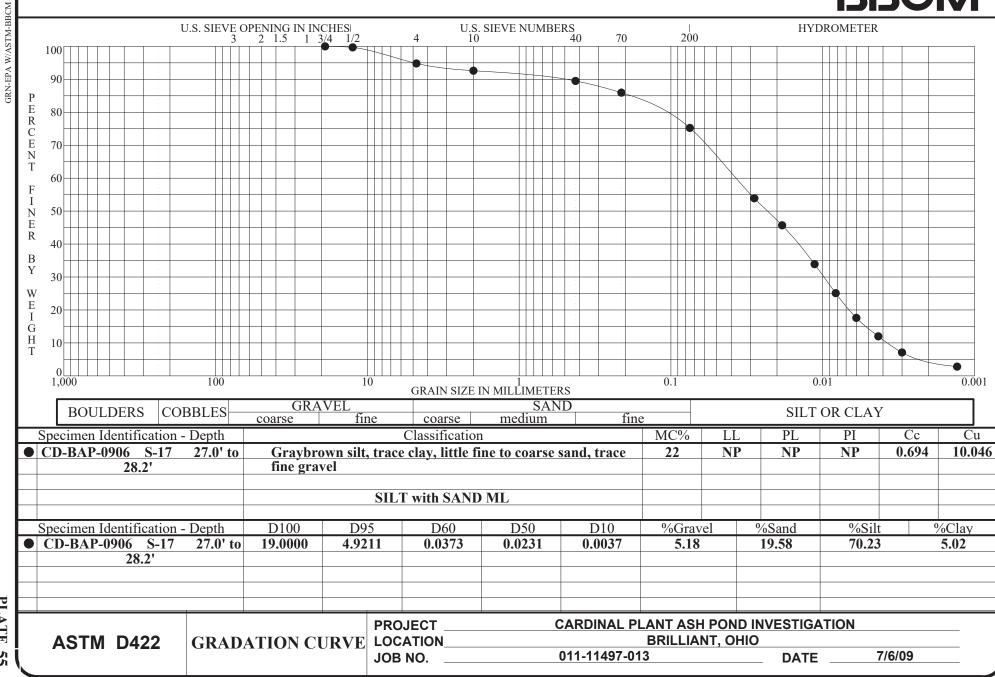


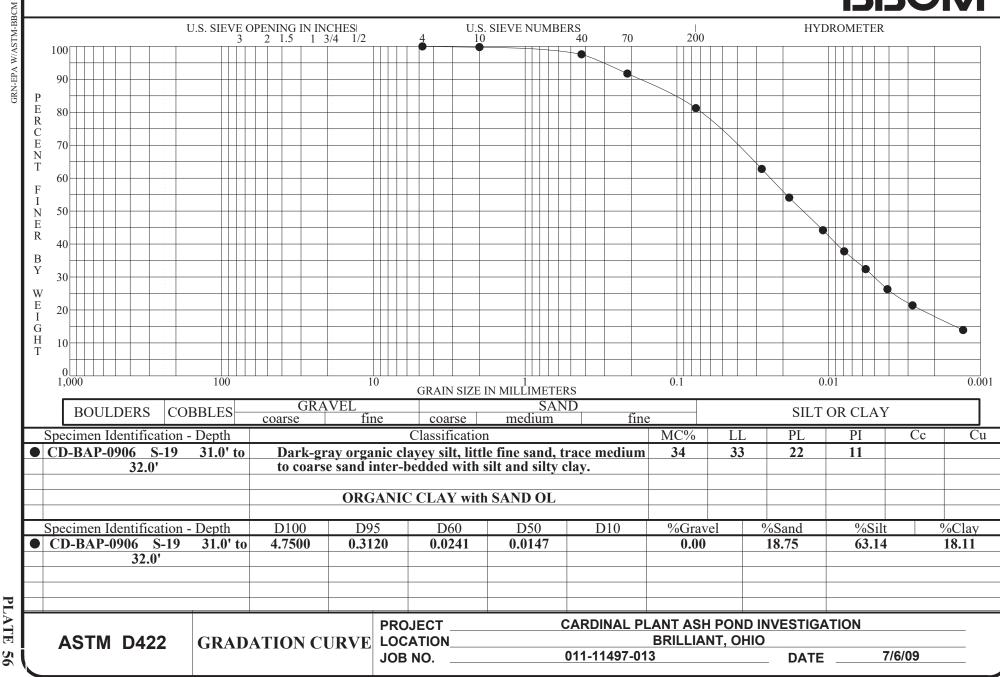


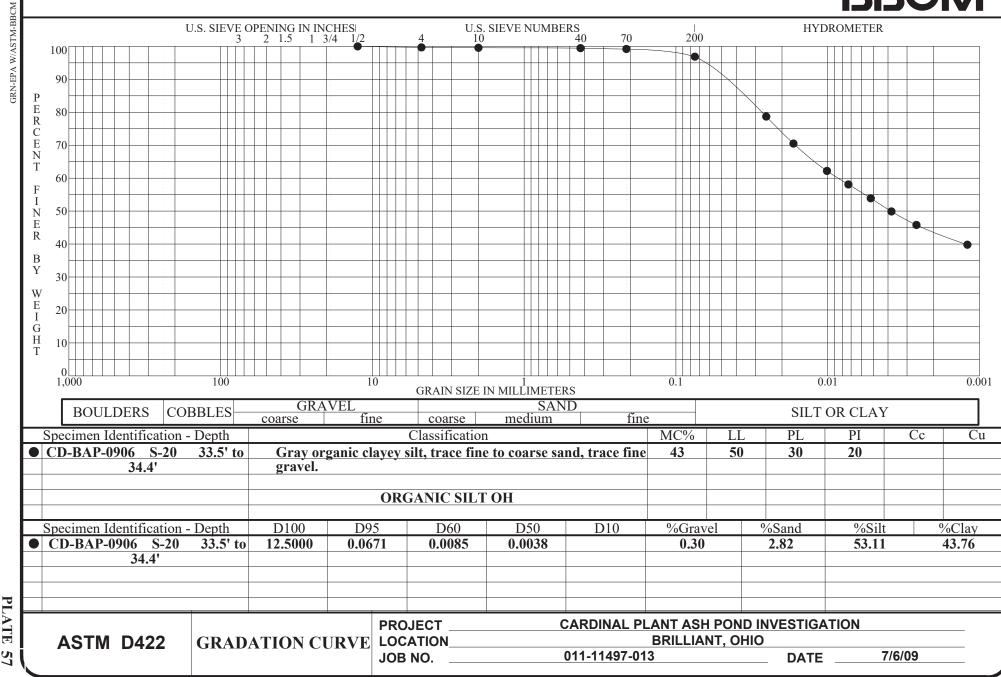


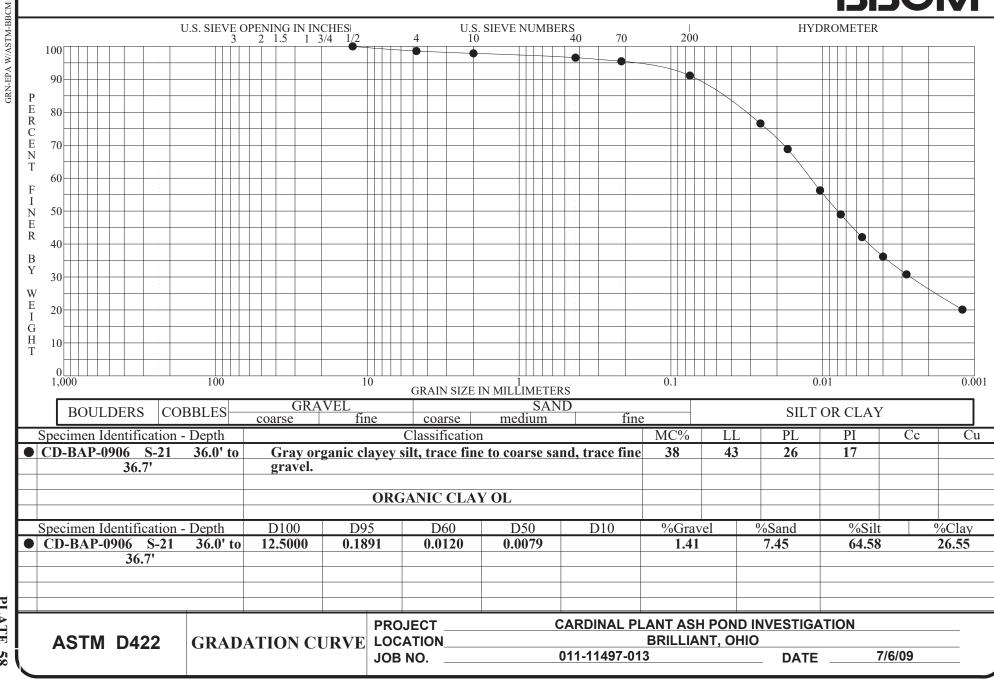


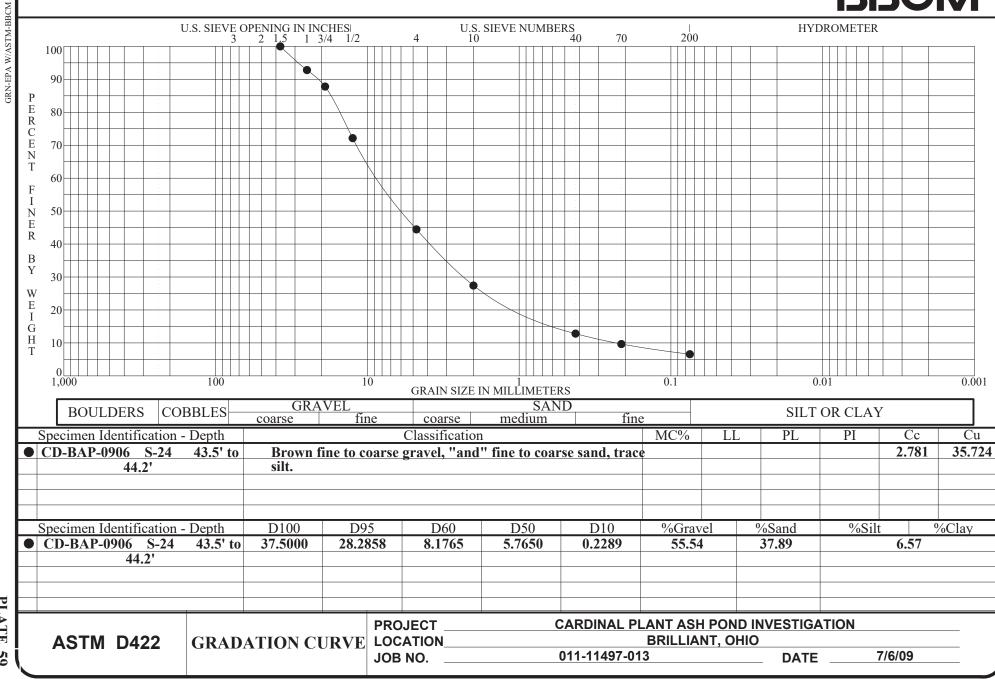


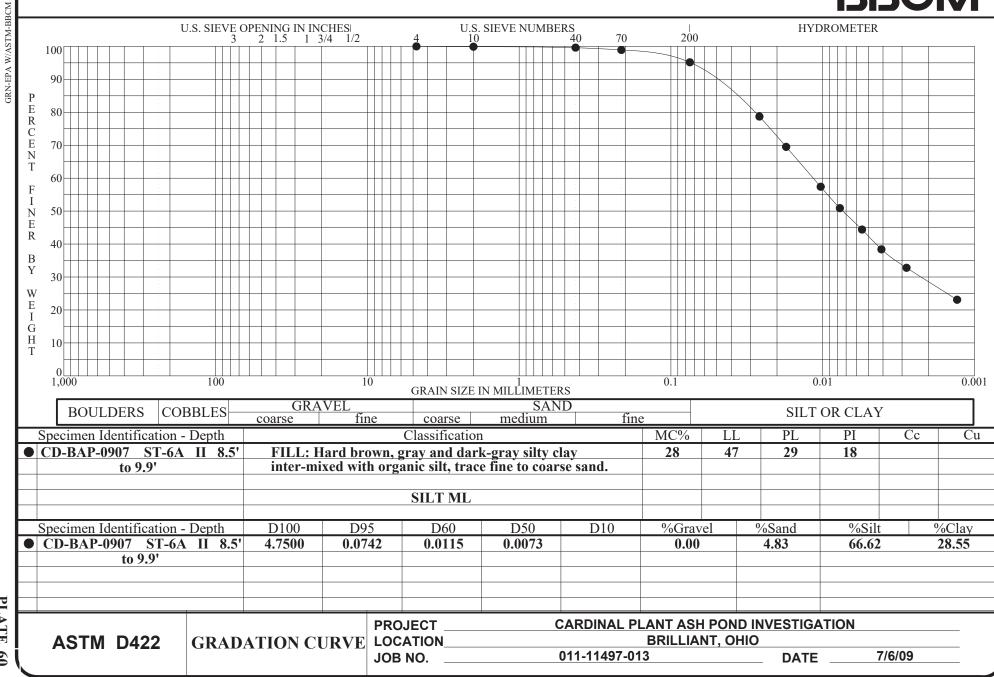


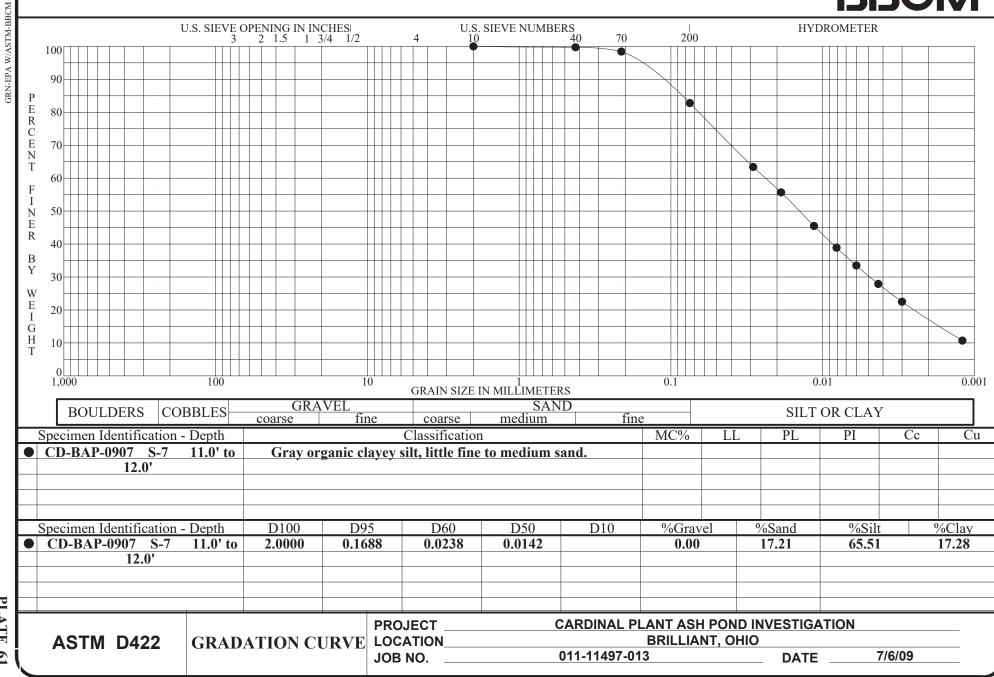




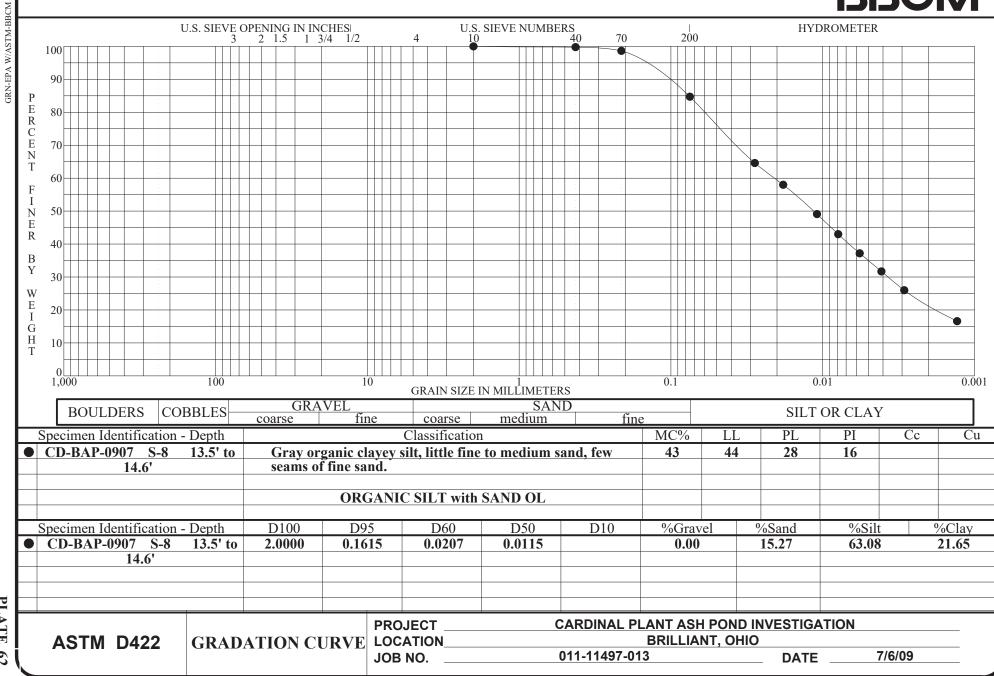


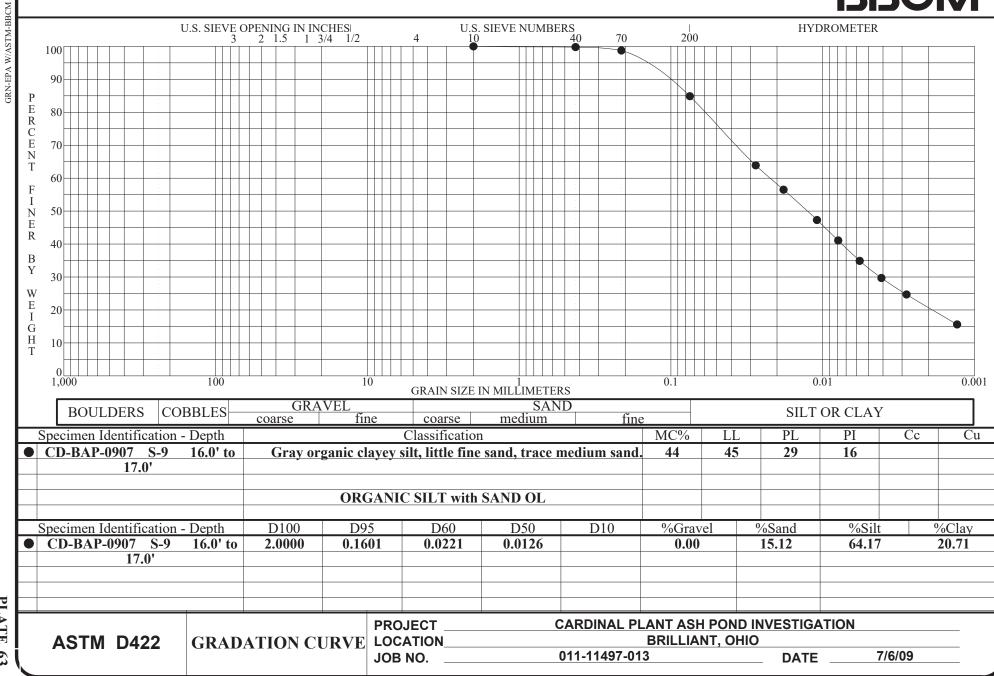


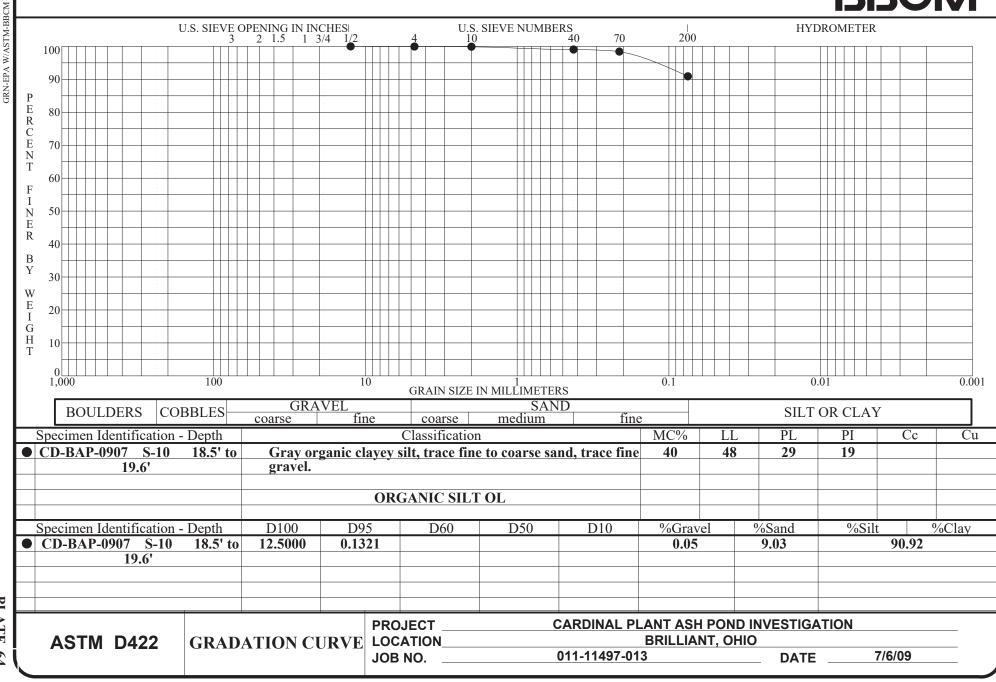


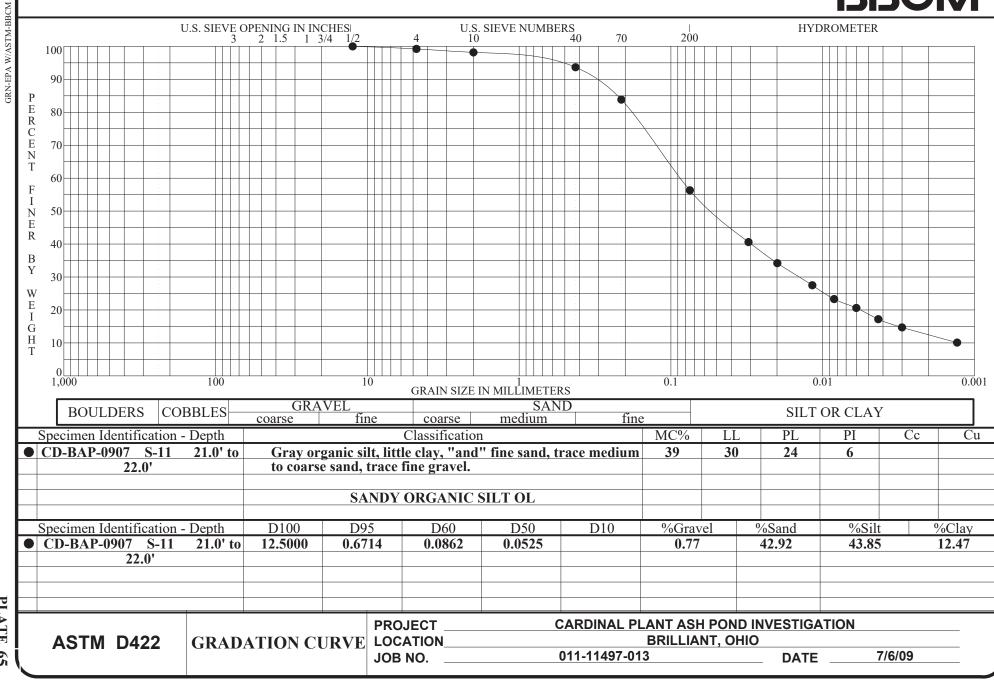


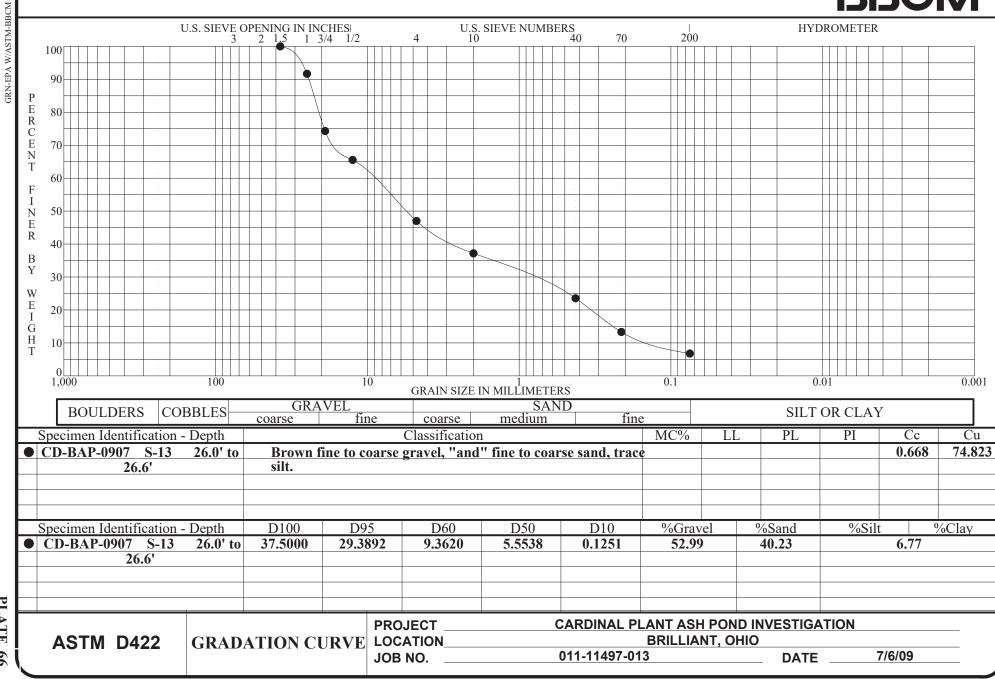
PLAIE 0











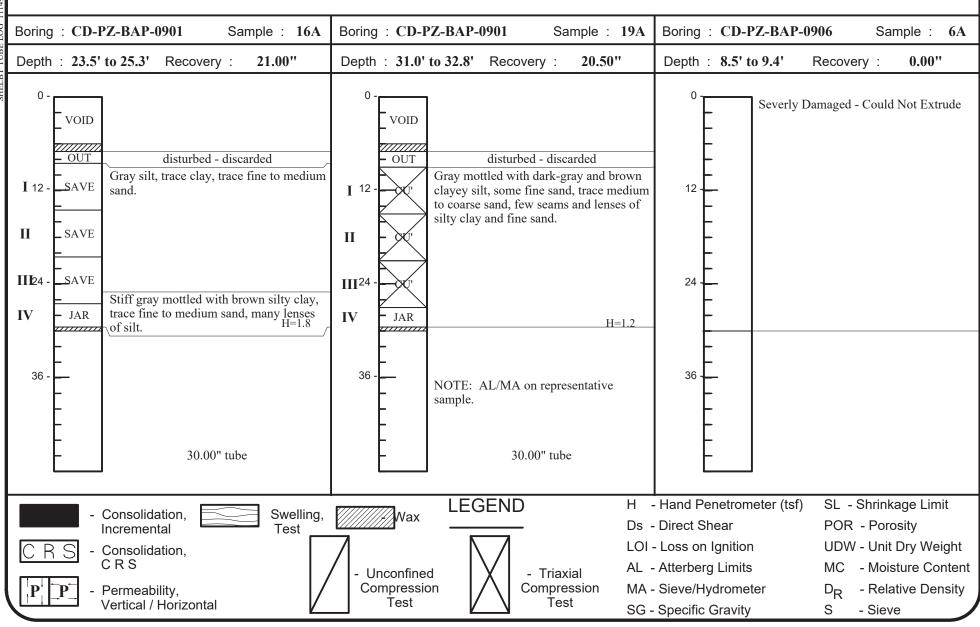
JOB NUMBER : 011-11497-013

PROJECT: CARDINAL PLANT ASH POND INVESTIGATION

LOCATION: BRILLIANT, OHIO



LABORATORY LOG OF SHELBY TUBES



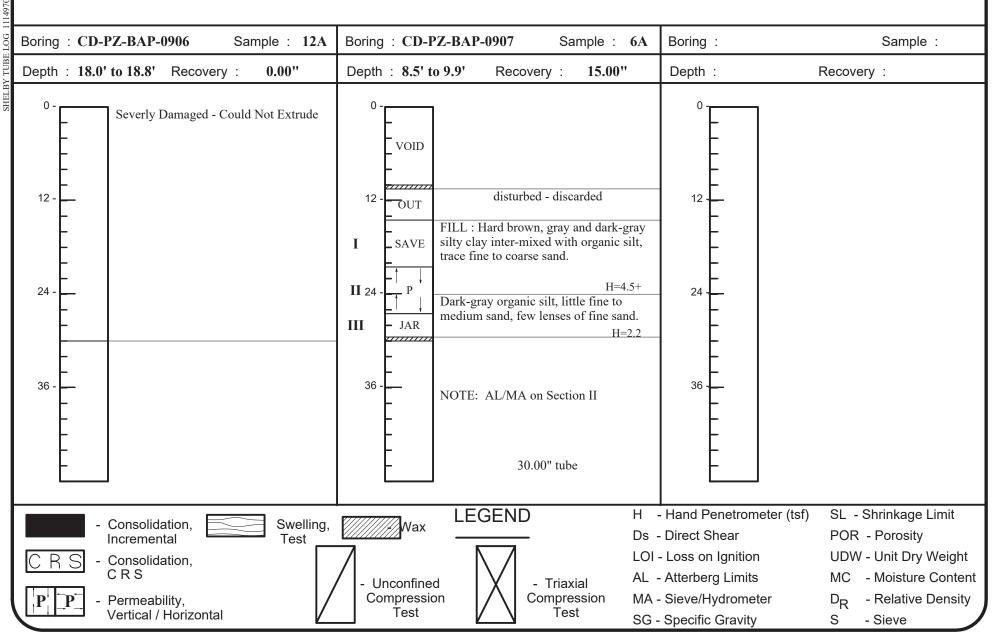
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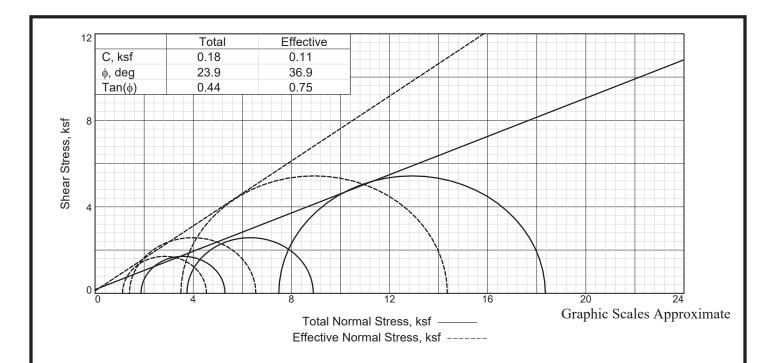
PROJECT: CARDINAL PLANT ASH POND INVESTIGATION

LOCATION: BRILLIANT, OHIO

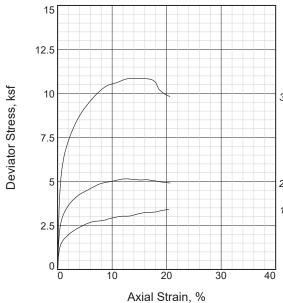


LABORATORY LOG OF SHELBY TUBES





Sample No.



	Jai	Tiple No.		_	3	
		Water Content, % Dry Density, pcf	35.1 83.0	43.8 76.2	31.9 85.0	
			92.2	97.7	87.6	
	Initial	Saturation, %				
	드	Void Ratio	1.0297	1.2123	0.9833	
3		Diameter, in.	2.90	2.85	2.90	
		Height, in.	5.59	5.59	5.59	
		Water Content, %	33.3	38.9	31.0	
	#	Dry Density, pcf	86.9	82.6	90.3	
	At Test	Saturation, %	95.6	101.0	96.5	
	Ę	Void Ratio	0.9402	1.0401	0.8674	
2	4	Diameter, in.	2.86	2.78	2.85	
		Height, in.	5.49	5.42	5.43	
1	Stra	ain rate, in./min.	0.00	0.00	0.00	
	Bad	ck Pressure, psi	40.00	40.00	40.00	
	Cel	l Pressure, psi	53.00	66.00	92.00	
	Fai	I. Stress, ksf	3.4	5.1	10.9	
	Т	otal Pore Pr., ksf	6.5	8.1	9.8	
	Ult.	Stress, ksf	3.4	4.9	9.8	
	Т	otal Pore Pr., ksf	6.5	8.0	9.9	
_	$\overline{\sigma}_1$	Failure, ksf	4.5	6.6	14.4	
	$\overline{\sigma}_{\text{3}}$	Failure, ksf	1.1	1.4	3.5	

Type of Test:

CU with Pore Pressures **Sample Type:** Shelby Tube

Description: Gray mottled with dark-gray and brown clayey silt, some fine sand, trace medium to

LL= 35 **PL=** 28 **PI=** 7

Assumed Specific Gravity= 2.7

Remarks:

Project: Cardinal Plant Ash Pond Investigation

Brilliant, Ohio

Location: CD-PZ-BAP-0901

Sample Number: ST-19A Depth: 31.0' to 32.8'

Proj. No.: 011.11497.013

Date Sampled: 5/1/09

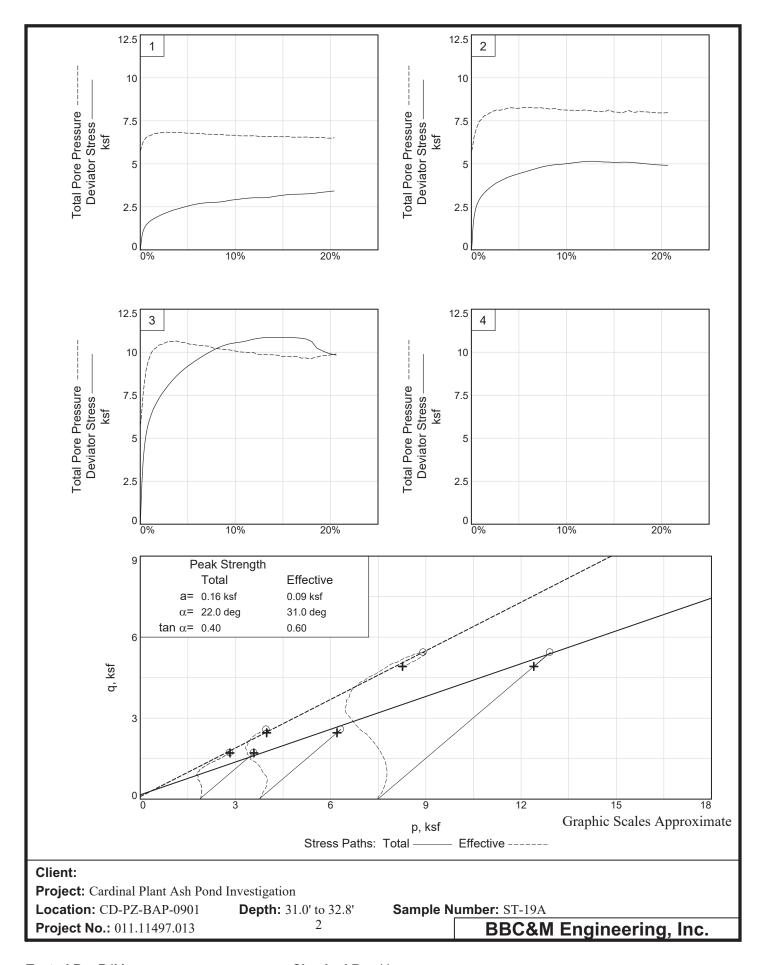
2

3

TRIAXIAL SHEAR TEST REPORT

BBC&M Engineering, Inc.

Tested By: PJM Checked By: JJ



PERMEABILITY TEST DATA AND COMPUTATION SHEET

((ASTM D-5084) FALLING HEAD, METHOD C)



Job Number:	011.11497.013	Date:	5/6-7/2009	Maximum [Dry Density:		_
Project Name:	Cardinal Ash Pond Investigation	Boring:	CD-PZ-BAP-0907	Optimum Moistu	ıre Content:		
Project Location:	Brilliant, Ohio	Sample:	ST-6A Sec. II	% C	ompaction.:		
Tested By:	PJM	Depth:	8.5' to 9.9'	0	ptimum +/-:		
Remarks:				_	Natural:	X	
Material:	FILL: Hard brown, gray and dark-gray	v eilty clay	inter-mixed with organ	ic silt traco	Pemolded:		

fine to coarse sand.

Sample:

Initial Length: 5.5945 in = 14.210 cm Final Ave. Length (L): _____ 5.6042 in = 14.235 cm

> Diameter: 2.8765 in = 7.31 cm Area (A): 6.499 sq in = 41.93 sq cm

Volume (V): 36.356 cu in = 595.77 cu cm

Wet Wt.: 1144.17 grams Unit Wet Wt.: 119.90 pcf Unit Dry Wt.: 93.99 pcf

Test Conditions:

Chamber Pressure: ____ 62 psi Back Pressure: 58 psi Confining Pressure: ____ 4 psi Temp. @ Start: 22.5 °C

Temp. @ End: 22.5 °C Average Temp.: 22.5 °C B Parameter: 0.96

Top Pipette: 60 psi

Bottom Pipette: 58 psi = 4079.6 cm

Moisture Content: Before Test After Test Pan No. = D D Wet Wt. + Pan = 1144.17 1157.03 Dry Wt. + Pan = 896.92 896.92 Wt. of Pan = 0.00 0.00 Wt. of Dry Soil = 896.92 896.92

247.25

27.57

260.11

29.00

98.30

Pipette Pressures During Test:

% SATURATION 93.80

= 4220.3 cm S.G.(est) = 2.7000

Pipette:

Area (a): 0.3435 sq in = 0.8725 sq cm

Calculations:

 $2 \cdot A \cdot \Delta t$

where:

k = Hydraulic Conductivity

a = Pipette Cross-Sectional Area

A = Sample Cross-Sectional Area

L = Length of Sample

 Δt = Time Interval (t_2 - t_1)

h₁ = Head Loss Across Permeameter/Specimen at t₁ h₂ = Head Loss Across Permeameter/Specimen at t₂

Wt. of Water =

% Moisture =

In = Natural Logarithm (Base e = 2.71828)

				Hydraulic Head		Hydraulic Head			Temp. Corr.
		Time Interval	Тор	Headwater	Bottom	Tailwater	Head Loss		Permeability
	Time	Δt	Pipette	H ₁	Pipette	H ₂	$h = H_1 - H_2$		k
Date	Readings	Seconds	СС	cm	сс	cm	cm	ℓn (h ₁ /h ₂₎	cm/sec
5/6/2009	9:45 AM	0.00	48.45	4092.08	14.20	4272.01	-179.93	-	_
5/6/2009	10:51 AM	3,960	48.40	4092.14	14.45	4271.73	-179.59	0.00191	6.740E-08
5/6/2009	12:15 PM	5,040	48.20	4092.36	14.65	4271.50	-179.13	0.00256	7.077E-08
5/6/2009	1:45 PM	5,400	48.05	4092.54	15.00	4271.09	-178.56	0.00320	8.280E-08
5/6/2009	3:17 PM	5,520	47.85	4092.77	15.25	4270.81	-178.04	0.00289	7.312E-08
5/7/2009	8:21 AM	61,440	45.60	4095.34	18.00	4267.66	-172.31	0.03272	7.431E-08

Time Weighted Average, k [cm/sec] = 7.423E-08

PLATE 71 2007 falling Head Perm.xls, 011.11497.013 B-0907

Appendix III – Shear Strength Parameter Justification

 $lmages; \ \, \hbox{$\sim$ Cardinal Plant Aerial,jpg \sim Cardinal Plant Aerial2,jpg \sim Scan71615,JPG \sim Scan71616,tiff are the context of the con$

REFERENCE: SITE DEVELOPMENT PLANS - ASH STORAGE AREA SECTIONS, 1973

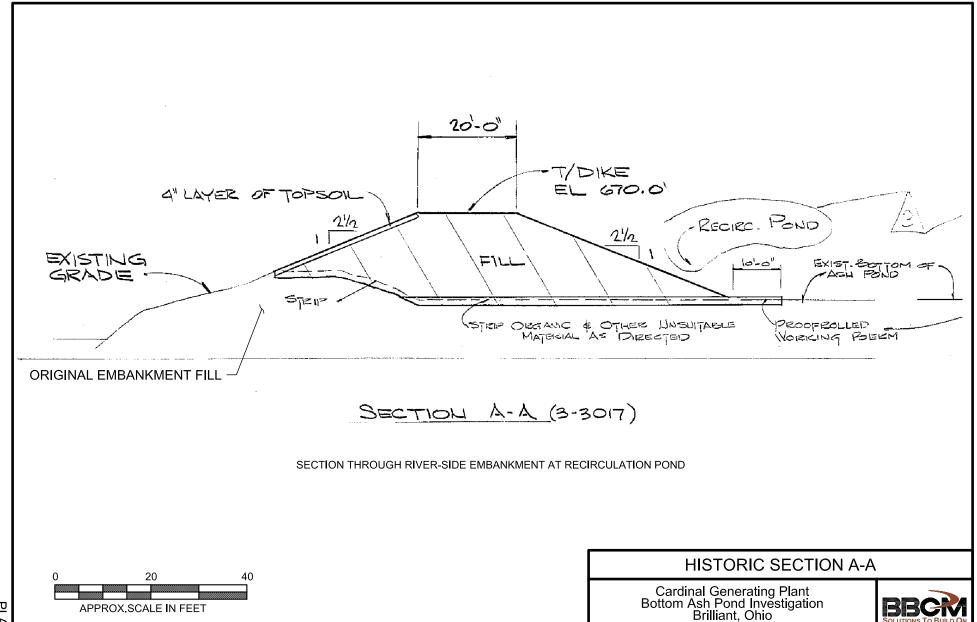
DRAWING NO. 3-3027-3

Xrefs: ~94151038.dwg

File Last Updated: Jul 06, 2009

Plot Info; 7-6-2009 @ 9;20am By; MRomanello

BBC&M Filename: I:\DEPTS\CADD\Drawings\Projects\011-11497-013\EaglePoint\011-11497-013 BASE.dwg Layout: Hist Sec A-A



Project: 011-11497-013

Drawing Date: 6-16-2009

Last Updated: 7-6-2009

Drawn By: MTR

MGR

Approved By:

Scale: 1" = 20'

Cleveland (216) 901-1000

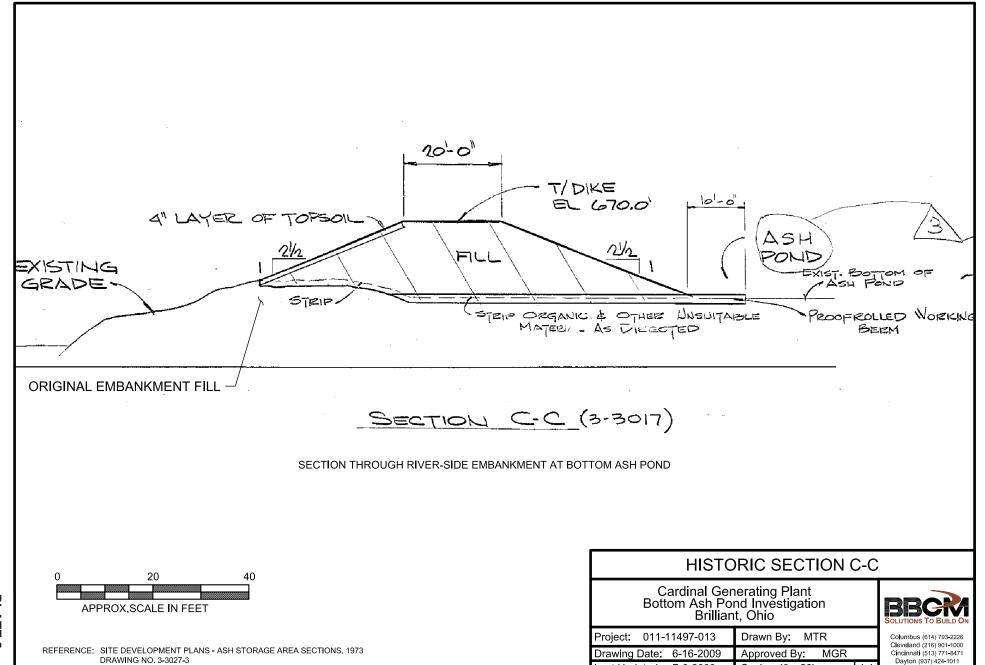
Cincinnati (513) 771-8471

Dayton (937) 424-1011

File Last Updated: Jul 06, 2009

Plot Info; 7-6-2009 @ 9;2lam By; MRomanello

BBC&M Filename: I:\DEPTS\CADD\Drawings\Projects\011-11497-013\EaglePoint\011-11497-013 BASE.dwg Layout: Hist Sec C-C



Last Updated: 7-6-2009

Scale: 1" = 20'

PLAIE 3

,												
BORING NUMBER	SAMPLE NUMBER	SAMPLE DEPTH	NATURAL MOISTURE	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX		SAND	SILT	CLAY .002 mm	SILT/CLAY	USCS CLASSIFICATION
			CONTENT	%	%	%	%	%	%	%	%	
BAP-0901	S-3	4.75	16									
BAP-0901	S-5	7.75	16	28	18	10						
BAP-0901	S-9	13.75	13	27	17	10						
BAP-0901	S-12	18.25	14	37	24	13	7	32	49	12	61	SANDY LEAN CLAY CL
BAP-0902	S-4	6.25	13	27	17	10	42	34	16	8	24	CLAYEY GRAVEL with SAND GC
BAP-0902	S-7	10.75	20									
BAP-0902	S-8	12.25	10	26	17	9	32	39	21	8	29	CLAYEY SAND with GRAVEL SC
BAP-0902	S-11	16.75	24	37	19	18						
BAP-0902	S-12	18.25	21	35	17	18	8	37	33	21	54	SANDY LEAN CLAY CL
BAP-0902	S-13	19.75	31	29	17	12	1	20	62	17	79	LEAN CLAY with SAND CL
BAP-0904	S-3	4.75	13									
BAP-0904	S-6	9.25	14	25	16	9	31	39	21	10	31	CLAYEY SAND with GRAVEL SC
BAP-0904	S-9	13.75	16	35	21	14						
BAP-0904	S-11	16.75					47	25			27	
BAP-0906	S-2A	2.9	11									
BAP-0906	S-3	4.75	15	27	17	10						
BAP-0906	S-8	12.75					30	40	22	9	31	
BAP-0906	S-11	17.25	14	31	19	12	18	44	26	12	38	CLAYEY SAND with GRAVEL SC
Samp	ole Size	18	16	12	12	12	9	9	8	8	9	1
	imum	3	10	25	16	9	1	20	16	8	24	1
Max	(imum	20	31	37	24	18	47	44	62	21	79	1
M	ean	11.7	16.3	30.3	18.3	12.1	24.0	34.4	31.3	12.1	41.6	1
Me	edian	13	15	29	17	11	30	37	24	11	31	1
M	ode	5	16	27	17	10	#N/A	39	21	12	31	1
Std	l Dev	-	5.4	4.5	2.3	3.2	16.2	7.7	16.1	4.6	18.9	1

Layer: NEWER EMBANKMENT FILL

PLAIE 4

SAMPLE NUMBER	SAMPLE DEPTH	NATURAL MOISTURE	LIQUID LIMIT			GRAVEL	SAND	SILT	CLAY	SILT/CLAY	USCS CLASSIFICATION
		CONTENT	%	%	%	%	%	%	%	%	
S-2	3.25	24	48	24	24	0	8	60	32	92	LEAN CLAY CL
S-3	4.75	22									
S-5	7.75	20	36	20	16	0	14	58	28	86	LEAN CLAY CL
S-3	4.75	17	32	18	14	0	25	53	23	76	LEAN CLAY with SAND CL
S-5	7.75	22	48	24	24						
S-6B	9.85	33				5	14			81	
S-2	3.25	21									
S-4	6.25	15									
S-5	7.75	23	49	26	23						
S-6A	9.25	28	47	29	18	0	5	67	29	96	SILT ML
e Size	10	10	6	6	6	5	5	4	4	5	1
mum	3	15	32	18	14	0	5	53	23	76	1
mum	10	33	49	29	24	5	25	67	32	96	1
ean	6.5	22.5	43.3	23.5	19.8	1.0	13.2	59.5	28.0	86.2	1
dian	7	22	48	24	21	0	14	59	29	86	
ode	8	22	48	24	24	0	14	#N/A	#N/A	#N/A	
Dev	-	5.1	7.4	4.0	4.4	2.2	7.7	5.8	3.7	8.1	
	S-2 S-3 S-5 S-6B S-2 S-4 S-5 S-6A e Size mum mum an dian de	S-2 3.25 S-3 4.75 S-5 7.75 S-3 4.75 S-5 7.75 S-6B 9.85 S-2 3.25 S-4 6.25 S-5 7.75 S-6A 9.25 e Size 10 mum 3 mum 10 an 6.5 dian 7 de 8	NUMBER DEPTH MOISTURE CONTENT S-2 3.25 24 S-3 4.75 22 S-5 7.75 20 S-3 4.75 17 S-5 7.75 22 S-6B 9.85 33 S-2 3.25 21 S-4 6.25 15 S-5 7.75 23 S-6A 9.25 28 e Size 10 10 mum 3 15 mum 10 33 an 6.5 22.5 dian 7 22 de 8 22	NUMBER DEPTH MOISTURE CONTENT LIMIT % S-2 3.25 24 48 S-3 4.75 22 36 S-5 7.75 20 36 S-3 4.75 17 32 S-5 7.75 22 48 S-6B 9.85 33 3 S-2 3.25 21 54 S-4 6.25 15 55 S-5 7.75 23 49 S-6A 9.25 28 47 e Size 10 10 6 mum 3 15 32 mum 10 33 49 an 6.5 22.5 43.3 dian 7 22 48 de 8 22 48	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % S-2 3.25 24 48 24 S-3 4.75 22 2 S-5 7.75 20 36 20 S-3 4.75 17 32 18 S-5 7.75 22 48 24 S-6B 9.85 33 3 24 24 S-6B 9.85 33 3 3 3 3 3 3 3 4 24 3 3 4 4 24 3 3 3 3 3 4 3 4 3 4 3 4 3 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2 4 4 2	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % S-2 3.25 24 48 24 24 S-3 4.75 22 2 2 2 S-5 7.75 20 36 20 16 36 36 20 16 36 </td <td>NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % % S-2 3.25 24 48 24 24 0 S-3 4.75 22 </td> <td>NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % % S-2 3.25 24 48 24 24 0 8 S-3 4.75 22 </td> <td>NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % %<td>NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % %<td>NUMBER DEPTH MOISTURE CONTENT LIMIT % INDEX % % % .002 mm % % S-2 3.25 24 48 24 24 0 8 60 32 92 S-3 4.75 22 </td></td></td>	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % % S-2 3.25 24 48 24 24 0 S-3 4.75 22	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % % S-2 3.25 24 48 24 24 0 8 S-3 4.75 22	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % <td>NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % %<td>NUMBER DEPTH MOISTURE CONTENT LIMIT % INDEX % % % .002 mm % % S-2 3.25 24 48 24 24 0 8 60 32 92 S-3 4.75 22 </td></td>	NUMBER DEPTH MOISTURE CONTENT LIMIT % LIMIT % INDEX % % <td>NUMBER DEPTH MOISTURE CONTENT LIMIT % INDEX % % % .002 mm % % S-2 3.25 24 48 24 24 0 8 60 32 92 S-3 4.75 22 </td>	NUMBER DEPTH MOISTURE CONTENT LIMIT % INDEX % % % .002 mm % % S-2 3.25 24 48 24 24 0 8 60 32 92 S-3 4.75 22

Layer: ORIGINAL EMBANKMENT FILL

Layer: ALLU	VIUM SILT A	ND CLAY									
BORING	SAMPLE	SAMPLE	NATURAL	LIQUID	PLASTIC	PLASTIC	GRAVEL	SAND	SILT	CLAY	SILT/0

BORING NUMBER	SAMPLE NUMBER	SAMPLE DEPTH	NATURAL MOISTURE	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX	GRAVEL	SAND	SILT	CLAY .002 mm	SILT/CLAY	USCS CLASSIFICATION
			CONTENT	%	%	%	%	%	%	%	%	
BAP-0901	S-15	22.75	30	NP	NP	NP	0	5	89	6	95	SILT ML
BAP-0901	S-16A	24.5										
BAP-0901	S-18	29.25	27	37	22	15	0	9	63	28	91	LEAN CLAY CL
BAP-0901	S-19A	31.25										
BAP-0901	S-19B	31.75	33	35	28	7	0	26	56	18	74	SILT with SAND ML
BAP-0901		32.25										
BAP-0902	S-14	21.25	26	NP	NP	NP	0	13	83	4	87	SILT ML
BAP-0902	S-15	22.75					1	22			78	
BAP-0903	S-10	21.75	35	34	21	13	0	29	51	19	70	LEAN CLAY with SAND CL
BAP-0904	S-15	22.75	26	NP	NP	NP	1	52	45	3	48	SILTY SAND SM
BAP-0904	S-17	25.75	22	NP	NP	NP	0	8	86	5	91	SILT ML
BAP-0905	S-11	21.75	38	38	23	15	2	36	47	15	62	SANDY LEAN CLAY CL
BAP-0906	S-15	24.75	31	NP	NP	NP	0	5	89	7	96	SILT ML
BAP-0906	S-16A	26.25					4	41			55	
BAP-0906	S-17	27.25	22	NP	NP	NP	5	20	70	5	75	SILT with SAND ML
												•

Sample Size	15	10	4	4	4	12	12	10	10	12
Minimum	21	22	34	21	7	0	5	45	3	48
Maximum	32.25	38	38	28	15	5	52	89	28	96
Mean	25.73	29.0	36.0	23.5	12.5	1.1	22.2	67.9	11.0	76.8
Median	24.75	29	36	23	14	0	21	67	7	77
Mode	22.75	26	#N/A	#N/A	15	0	5	89	5	91
Std Dev	-	5.4	1.8	3.1	3.8	1.7	15.2	17.8	8.5	15.9

NP - Non Plastic

Layer: ORGANIC CLAYEY SILT

BORING	SAMPLE	SAMPLE	NATURAL	LIQUID	PLASTIC	PLASTIC	GRAVEL	SAND	SILT	CLAY	SILT/CLAY	USCS
NUMBER	NUMBER	DEPTH	MOISTURE	LIMIT	LIMIT	INDEX				.002 mm		CLASSIFICATION
			CONTENT	%	%	%	%	%	%	%	%	
BAP-0901	S-20	34.25	42	34	27	7	0	22	62	16	78	ORGANIC SILT with SAND OL
BAP-0901	S-21	36.75	40	45	29	16	11	30			59	SANDY ORGANIC SILT OL
BAP-0901	S-22	39.25	42	40	23	17	0	18	59	22	81	ORGANIC CLAY with SAND OL
BAP-0902	S-18	27.25	54	NP	NP	NP	0	15	69	16	85	ORGANIC SILT OL
BAP-0902	S-19	28.75	43	NP	NP	NP	0	25	61	13	74	ORGANIC SILT with SAND OL
BAP-0902	S-20	32.25	38	36	28	8	2	23	59	16	75	ORGANIC SILT with SAND OL
BAP-0903	S-6	9.25	49	41	38	3	0	33	52	15	67	SANDY ORGANIC SILT OL
BAP-0903	S-7	14.25	43	NP	NP	NP	0	29	56	15	71	ORGANIC SILT with SAND OL
BAP-0903	S-8	16.75	43	37	24	13	0	24	57	19	76	ORGANIC CLAY with SAND OL
BAP-0903	S-9	19.25	44	35	24	11	0	39	45	16	61	SANDY ORGANIC CLAY OL
BAP-0904	S-13	19.75	28	NP	NP	NP	0	8	87	5	92	ORGANIC SILT OL
BAP-0904	S-18	27.25	38	38	24	14	0	21	58	21	79	ORGANIC CLAY with SAND OL
BAP-0904	S-19	28.75	47	42	30	12	0	22	62	17	79	ORGANIC SILT with SAND OL
BAP-0905	S-8	14.25	45	43	27	16	0	19	60	21	81	ORGANIC SILT with SAND OL
BAP-0905	S-9	16.75	42	40	25	15	0	16	60	24	84	ORGANIC CLAY with SAND OL
BAP-0906	S-19	31.75	34	33	22	11	0	19	63	18	81	ORGANIC CLAY with SAND OL
BAP-0906	S-20	34.25	43	50	30	20	0	3	53	44	97	ORGANIC SILT OH
BAP-0906	S-21	36.75	38	43	26	17	1	7	65	27	92	ORGANIC CLAY OL
BAP-0907	S-7	11.75					0	17	66	17	83	
BAP-0907	S-8	14.25	43	44	28	16	0	15	63	22	85	ORGANIC SILT with SAND OL
BAP-0907	S-9	16.75	44	45	29	16	0	15	64	21	85	ORGANIC SILT with SAND OL
BAP-0907	S-10	19.25	40	48	29	19	0	9			91	ORGANIC SILT OL
BAP-0907	S-11	21.75	39	30	24	6	1	43	44	12	56	SANDY ORGANIC SILT OL
												_
Samp	le Size	23	22	18	18	18	23	23	21	21	23	
Mini	mum	9	28	30	22	3	0	3	44	5	56	
Max	imum	39.25	54	50	38	20	11	43	87	44	97	
Me	ean	23.97	41.8	40.2	27.1	13.2	0.7	20.5	60.2	18.9	78.8	
Me	dian	21.75	43	41	27	15	0	19	60	17	81	
Mo	ode	14.25	43	45	24	16	0	15	62	16	81	
Std	Dev	-	5.2	5.4	3.7	4.7	2.3	9.8	8.8	7.4	10.6]

Layer: GLACIAL OUTWASH SAND AND GRAVEI

BORING NUMBER	SAMPLE NUMBER	SAMPLE DEPTH	NATURAL MOISTURE	GRAVEL	SAND	SILT	CLAY .002 mm	SILT/CLAY
			CONTENT	%	%	%	%	%
BAP-0902	S-22	37.25	22	0	70	22	8	30
BAP-0902	S-23	39.75	24	0	83	13	4	17
BAP-0902	S-24	42.25		4	82			14
BAP-0903	S-11	24.25		9	77			14
BAP-0904	S-21	36.75		0	76			24
BAP-0905	S-13	26.75		19	73			8
BAP-0906	S-24	44.25		56	38			7
BAP-0907	S-13	26.75		53	40			7
Samp	le Size	8	2	8	8	2	2	8
Mini	imum	24	22	0	38	13	4	7
Max	imum	44.25	24	56	83	22	8	30
M	ean	34.75	23.0	17.6	67.4	17.5	6.0	15.1
Me	dian	37.00	23	7	75	18	6	14
M	ode	26.75	#N/A	0	#N/A	#N/A	#N/A	14
Std	Dev	-	1.4	23.7	18.0	6.4	2.8	8.4



SOLUTIONS TO BUILD ON	Project/Proposal No. <u>011.//497.</u> 0/3	Calculated By MITR	Date	6-29-09
Cincinnati (513) 771-8471 Cleveland (216) 901-1000	Project/Proposal Name CARDINAL ASH PIND	Checked By MAR	Date	7-2-09
Columbus (614) 793-2226 Dayton (937) 424-1011	Subject STRUCKGTH ! PERM. PARAMETERS	Sheet/ of8		

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1	JAUFAC Z	DESIGN.	ESTIMATE MANUAL MPACTED:	7.2 Usine	D STREET	BUE 1	142UE	5 FR. YPICAL	estan.
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1	JAUFAC Z	DESIGN.	MANUAL	7.2 Usine	D STRE	NGTH L	142 US	5 FR UPICAL	ola
1	JAUFAC Z	DESIGN.	MANUAL	7.2 Usine	D STREE	BLIE 1	142 USS	5 FR APICAL	DIA .
	JAUFAC I	OF CO	MAXWAL MPACTED:	7,2 Using	4 7A	BUE 1	- 'T	YPICAL	
+ GRA	NAUFAC I. PROPERTIES	OF CO	MANUAL MARCTEDS:	J.Z USING	CAL CUI	BUE 1	- T	F GR	quel)
+ GRA	NAUFAC I. PROPERTIES	OF CO	MAXWAL MPACTED:	J.Z USING	CAL CUI	BUE 1	- T	F GR	quel)
+ GRA	NOUTES NULAR FOR	OF CO	MANUAL MATER: MATER LAYIER	J.Z USING SOILS' SOILS' CALAC	CIAL CXI	RUE 1	54~10 ERAIN	F GRI	quel)
+ GRA	NOUTES NULAR FOR	OF CO	MANUAL MARCTEDS:	J.Z USING SOILS' SOILS' CALAC	CIAL CXI	RUE 1	54~10 ERAIN	F GRI	quel)
+ GRA:	HOPERTIES NULAR FOR ESTIMATE 6'= -15.	DESUGN OF CO MINATION OF BAS 4 (NGO)	MANUAL MATER LAVIER ED ON SI	TIZ USING SOILS' SOILS' HANTALIAK	HAL CXIT	WASH UCHIDA	54~1D E,RAIN	F GAN	quel) ANALY
+ GRA:	NOUTES NULAR FOR	MATO S' 305 4 (N60)	MANUAL MATER LAVIER ED ON SI	J.Z USING SOILS' SOILS' CALAC	HAL CXIT	WASH UCHIDA	54~1D E,RAIN	F GAN	quel) ANALY
+ GRA:	NOLAR FOR ESTIMATE COMPARE	MINATION BOS	MANUAL MATTERS: MATTE	TIZ USING SOILS' SOILS' HANTALIAK YPICAL VAN	ATIONS A AND	WASH UCHIDA	54~1D E,RAIN	F GAN	quel) ANALY
+ GRA:	PROPERTIES ALLAR FOR ESTIMATE COMPARE TABLE 7.1	MATION SON	MANUAL MATTER: MANUAL MATTER: AND ONE SI THEO ONE SI	TIZ USING SOILS' SOILS' HANTALIAK YPICAL VAN	HAL CUT ATIONS HA ANI) WES ES	BUE 1 -WASH - AND - WASH - AND - WASH - AND	5.4~1D E.R.A.M 1996	F GRI V SIZE SCHRU	ANALY
+ GRA:	PROPERTIES PROPERTIES AULAR FOR ESTIMATE COMPARE TABLE 7.1 Relative Den.	MATO OF CO MATO A' 30S H (NGO) FEQNI Relative	MANUAL MATTER: MATTE	TIZ USING SOILS' SOILS' HANTALIAK YPICAL VAN HOSIONIOSS SCI	HAL CUT ATIONS ANI) WES ES	AND OCHIDA, APPROXIME	SAND ERAIN 1994 D A	SCHRU	ANALY
+ GRA:	PROPERTIES ALLAR FOR ESTIMATE COMPARE TABLE 7.1	MATO OF CO MATO A' 30S H (NGO) FEQNI Relative	MANUAL MATTER: MANUAL MATTER: AND ONE SI THEO ONE SI	TIZ USING SOILS' SOILS' HANTALIAK YPICAL VAN HOSIONIOSS SCI	HAL CUT ATIONS HA ANI) WES ES	AND OCHIDA, APPROXIME	5.4~1D E.R.A.M 1996	SCHRU	ANALY SEAER SET
+ GRA:	PROPERTIES PROPERTIES PROPERTIES AULAR FOR ESTIMATE G' = -15. COMPARE TABLE 7.1 Relative Den. Designation Very loose	MATO OF CO MATO A' 30S H (NGO) FEQNI Relative	MANUAL MATER MATER AND AND AND AND AND AND AND AN	TIZ USING SOILS' SOILS' HANTALIAK YPICAL VAN HOSIONIOSS SCI	CIAL CXII ATIONS CA ANI COES ES COIIS Standard Con Resistant 0-4	AND OCHIDA, APPROXIME	SAND ERAIN 1996 DIN cimate Anof Soil ϕ ,	SCHRO	AMALY SEAER SET
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Project/Proposal No. O///497.0/3 Calculated By MTR Date 6-29-09
Project/Proposal Name CARNINAL ASH POND Checked By ML12 Date 1-2-09
Subject 578624GTH 5 PERM PARAMETERS Sheet 2 of 8

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Project/Proposal No. O//. //497. 0/3

Calculated By MTR

Date 6-29-09

Project/Proposal Name CARDWAL ASH POND

Checked By MIR

Date 1-1-09

Subject STRENGIN S PEUM PARAMETERS

Sheet U of 8

- DESCRIPTION: STIPPE TO HARD RECOUNT MOTTLED WITH CHAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROPETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONFORM CAVE = 50 KR BASED ON RELATION OF THIS LAYER - RESULTS: \$\frac{1}{9} = 30^\text{**} (SEE CHART THIS APPENDEX) 2) \$\frac{1}{9} VRS PI - RESULTS: FOR PI = 24 , \$\frac{1}{9} = 30^\text{**} (SEE CHART THIS APPENDEX) 3) \$N/A FOR FILL SOILS 4) NAVERC TARKE GROUP SOIL TYPE CL TNONGANIC CHAIS OF LOW C'= ZTOPSE TO MICH PLASTICITY DESIGN STRENGTH PARAMETER: C'= 100 PIF, \$\frac{1}{9} = 30^\text{**} - PERMICABILITY: FLEX WALL PERMEABLING TEST PERMOLYMED ON SAMPLE ST-6A OF BOSING BAP-0907 - RESULTS KV = T.42 × 10^\text{**} CM/SEC - DESIGNS: USE KV = 1410^7 CM/SEC TO ACCOUNT FOR PERM ON A MACKO SCALE. 57 KV ABUSTED TO 5 × 10^\text{**} CM/SEC - UTH KH/KL = 5 DURING SIERBAGIE ANALYSIS						111						
- DESCRIPTION : STIPPE TO HARD BROWN MUTTLED WITH CHMY SILTY CLAY (USCS: LEAN CLAY) - NANID PENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH FARAMETER: 1) CORRELATION TO STORK CHARTS CONSIDER Gr' = 50 KR BOSED ON RELATION OF THIS LAYER FO THE PANDER PLANE. - RESULTS: G' = 30° (SEE CHART THIS LAYER - RESULTS: FOX PI = 24 , d' = 30° (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS 4) NAVEAC TABLE GROUP SOIL THE CL INDEGRANIC CHAIS OF LOW C' = TO PSF TO THED PLASTICITY DESIGN STRENGTH PARAMETER: C' = 100 PSF , d' = 30° - PERMICABILITY: FLEX WALL PIERMEABILITY TEST PERFOLITIED ON SAMPLE ST-10A OF BOXING BAP-0907 - RESULTS KV = 7.42 × 10-8 CM/SEE - DESIGN: USE KV = 142 × 10-8 CM/SEE TO MICH SCALE.			100	cincly Siz	ENACIE	ANIALY	SIS					
- DESCRIPTION: STIPE TO HARD BROWN MUTTLED WITH CHAY SILTY CLAY (USCS: LEAN CLAY) - MANID BENETROMETER FARGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CURRELATION TO STAKE CHARTS CONSIDER CAVE FOR BOOK ON RELATION OF THIS LAYER TO THE FARMET PRANTE. - RESULTS: \$\frac{1}{2} \cdot 20^\circ (SEE CHART MISS APPRICADE) 2) \$\frac{1}{2}' \text{ FIX PI = 2H \$\frac{1}{2}' \cdot 30^\circ (SEE CHART MISS APPRICADE) 3) N/A FIX FILL SOILS 4) NAVERC TABLE GROUP SOIL THE CL INORGANIC CHARS OF LOW C'= TOPSE TO MISD PLASTICITY \$\frac{1}{2} \cdot 20^\circ DESIGN STRENGTH PARAMETER: C'= 100 PSF , \$\frac{1}{2} \cdot 30^\circ - PERMICABILITY: FLEX WALL PERMEABULITY TEST PERMOLATED ON SAMPLE ST-10A OF BOOKING BAP-0907 - RESOLTS KV = T.42 × 10^8 cm/sec - DESIGN: USE KV = 140^7 cm/sec TO ACCOUNT FIX PERM ON A MACKO SCALE.			=7 KV	ADJUSTE	er a	5×10	5-8 cm	lsec	WITH	KH/Kr	-5	
DESCRIPTION: STIPS TO HARD BROWN MOTTLES WITH CHAM SILTY CLAY (USCS: LEAN CLAY) - HAND REMETROMETER RANGE: 1.5 - 4.5 tsp - STRENGTH PARAMETER: 1) CURRELATION TO STOCK CHARTS CONTIDER ON'S = 50 KM BASED ON RELATION DIP THIS CAYER TO THE FAILURE PLANTE. - RESULTS: Q'S = 30° (SEE CHART THIS APPRILL - RESULTS: FOR PI = 24 , 0' = 30° (SEE CHART THIS APPRIL 3) N/A FOR FILL SOILS 4) NAVEAC TARLE GROUP SOIL THINE CL TNORGANIC CLAYS OF LOW C'= TOPSE TO MED PLASTICITY DESIGN STRENGTH PARAMETER: C'=100 PSP, 0'= 30° - PERMICABILITY: FLEX WALL PERMEABILITY TEST PERMINIMED ON SAMPLE ST-60A OF BOKING BAP-0907 - RESULTS KV = T.42 × 10°8 cm/sec - DESIGN: USE KV = 1 × 10° Cm/sec TO ACCOUNT FOR PERM ON A										4+		
- DESCRIPTION: STIFF TO HARD BROWN MOTTHER WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - HAND BENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STOCK CHARTS CONSIDER GNO = 50 KP BASED ON RELATION DIE THIS CAYER TO THE FAILURE PLANE. - RESULTS: \$\frac{1}{2} \text{ SO'S}\$ (SEE CHART THIS APPEAL 2) \$\frac{1}{2}\$ (PS PI - RESULTS: FOR PI = 24 , \$\frac{1}{2}\$ ' = 30' (SEE CHART THIS APPEAL 3) N/A FOR FILL SOILS 4) NAVEAC TABLE GROUP SOIL THE CL TNOKGANIC CHARS OF LOW C'= TOPSE TO THE PLANTICITY DESIGN STRENGTH PARAMETER: C'=100 PSF, \$\frac{1}{2} \text{ 30'}\$ - PERMICABILITY: FLEX WALL PERMEABULITY TEST PERMULTIKED ON SAMPLE ST-60A OF BORNG BAP-0907		DESIGN:	USE K	(v = 1 x L	0-7 cm	1/sec	TO AC	COUNT	FUX P	ERM.	ON A	1
- DESCRIPTION: STIFF TO HARD BROWN MUTTHED WITH CHAY SILTY CLAY (USCS: LEAN CLAY) - NAME DENETROMETER RANGES: 1.5 - 4.5 FLP - STRENGTH PARAMETER: 1) CORRELATION TO STORK CHARTS CONSIDER GNO = 50 KR. BROWN ON RELATION OF THIS LAYER FOR THE FAILURE PLANTE. - RESULTS: \$\frac{1}{2} \cdot 20^{\text{C}}\$ (SEE CHART THIS APPENDIX) 2) \$\frac{1}{2}\$ VRS PI - RESULTS: FOR PI = 24, \$\frac{1}{2}\$ \cdot 30^{\text{C}}\$ (SEE CHART THIS APPENDIX) 3) \$N/A FIR FILL SOILS H) NAVEAC TRAILE GROUP SOIL THRE CL THORGANIC CLASS OF LOW C!= 2TO PSF TO MIKD PLASTICITY DESIGN STRIENGTH PARAMETER: C'=100 PSF, \$\frac{1}{2}\$ \cdot 20^{\text{C}}\$ - PERMICABILITY: FLEX WALL PERMICABILITY TEST PERMINATED ON SAMPLE ST-10A OF BOWING BAP-0907			-11-1-1-1								1	
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAME DESCRIPTION TO STORK CHARTS CONSIDER GIO' = 50 KP. BASED ON RELATION OF THIS LAYER TO THE FAILURE PLANTE. - RESULTS: Q' = 30° (SEE CHART THIS APPENDIX) 2) Q' YES PI - RESULTS: FOX PI = 2H, Q' = 30° (SEE CHART THIS APPENDIX) 3) H/A FOR FILL SOILS H) NAVEAC TABLE GROUP SOIL THRE CL INORGANIC CLAYS DIE LOW C' = 70 PSF TO MED PLASTICITY DESIGN STRIENGTH PARAMETER: C' = 100 PSF, Q' = 30° - PERMICABILITY:					- 8	1						
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - HAND RENETROMETER RANGE: 1.5 - 4.5 tep - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER GA'S - 50 KR BASED ON RELATION OF THIS LAYER TO THE FAILURE PLANE. - RESULTS: GO: 30° (SEE CHART THIS APPENDIX) 2) G' IRS PI - RESULTS: FOR PI = 2H , G' = 30° (SEE CHART THIS APPENDIX) 3) H/A FOR FILL SOILS H) NAVERIC TRAILE GROUP SOIL THRE CL TNORGANIC CHAYS OF LOW C'= 770 PSF TO MED PLASTICITY DESIGN STRIENGTH PARAMETER: C'= 100 PSF , G'= 30° - PERMICABILITY:		of Born	ALL PIERN NG BAP	MEABUIT -0907	1 TEST	PERF	OLME	No C	SAM	PLE ST	-6A	
- DESCRIPTION: STIFF TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGE: 1.5 - 4.5 tsp - STRENGTH PARAMETER: 1) CURRELATION TO STARK CHARTS CONSIDER GIVE = 50 KM BASED ON RELATION DIE THIS CAYER TO THE FAILURE PLANE. - RESULTS: G' = 30° (SEE CURRENATION THIS APPENDIX) 2) G' RS PI - RESULTS: FOK PI = 2H , G' = 30° (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS H) NAVEAC TRISLE GROUP SOIL TYPE CL INORGANIC CLAYS OF LOW C' = 270 psf TO MED PLASTICITY DESIGN STRIENGTH PARAMETER: C' = 100 psf , G' = 30° - PERMIEABILITY:												
- DESCRIPTION: STORE TO HARD BROWN MOTTLIED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGOLE: 1.5 - 4.5 top - STRENGTH PARAMIETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANE. - RESULTS: OS = 30° (SEE CORRENATION THIS APPENDIX) 2) O' VRS PI - RESULTS: FOK PI = 2H, O' = 30° (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS H) NAVEAC TABLE GROUP SOIL TYPE CL INORGANIC CHARS OF LOW C' = 270 PSF TO MIED PLASTICITY O' = 28° 7 10°	- PERI	MEABILIT	Y:									I
- DESCRIPTION: STOPP TO HARD BROWN MOTTLIED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANE. - RESULTS: POK PI = 2H, Ø' = 30° (SEE CHART THIS APPENDIX) 2) Ø' VRS PI - RESULTS: FOK PI = 2H, Ø' = 30° (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS H) NAVEAC TABLE GROUP SOIL THE CL INORGANIC CHARS OF LOW C' = 270 PSF TO MED PLASTICITY Ø' = 28° 7 10°			NO COLIE	1 VICANIC	TIEN :	G =	100 ps.	- ,	x' = 3	3		
- DESCRIPTION: STIPTE TO HARD BROWN MOTTLED WITH CHAY SILTY CLAY (USCS: LEAN CLAY) - NAMED REMETROMETER RANGGE: 1.5 - 4.5 tsp - STRENGTH PARAMETER: 1) CORRELATION TO STOCK CHARTS CONSIDER ON = 50 KR BASED ON RELATION OF THIS LAYER TO THE FAILURE PLANE. - RESULTS: OB = 30° (SEE CONCENTION THIS APPENDIX) 2) O' VRS PI - RESULTS: FOR PI = 24, 0' = 30° (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS 4) NAVEAC TARLE GROUP SOIL TYPE CL INORGANIC CLANS OF LOW C'= TOPSE	7	ESIGN =										
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAME DENETROMETER RANGE: 1.5 - 4.5 tsp - STRENGTH PARAMIETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON' = 50 KR BASED ON RELATIONS OF THIS LAYER - RESULTS: OS: 30° (SEE CORREMATION THIS APPENDIX) 2) O' URS PI - RESULTS: FOK PI = 24, O' = 30° (SEE CHART THIS APPEND 3) N/A FOR FILL SOILS 4) NAVEAC TABLE			CL	TO M	HAMIC I	STICITY	OF=	LOW	C1= 2	70 psf		10
- DESCRIPTION: STIPPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER DVO = 50 KR BADED ON RICLATION OF THIS CAYER TO THE FAILURE PLANE. - RESULTS: \$\phi_{\text{s}}^2 = 30^\circ\$ (SEE CORREMATION THIS APPRINDLY) 2) \$\phi' vrs PI - RESULTS: FOK PI = 24, \$\phi' = 30^\circ\$ (SEE CHART THIS APPENDIX) 3) N/A FOR FILL SOILS			Ca 13					T F	STRE	midkahiri Mgti+	(cm	1se
- DESCRIPTION : STIPPE TO HARD BROWN MOTTLIED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAME DENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANTE. - RESULTS: OS SEE CHART THIS APPENDIX 2) O' VRS PI - RESULTS: FOR PI = 24, 0' = 30° (SEE CHART THIS APPENDIX)		4) NI	AVFAC	TABLE		11		+				.16
- DESCRIPTION: STAPE TO HARD BROWN MOTTLIED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANE. - RESULTS: OS: 30° (SEE COMPRENDIX) 2) O' VRS PI - RESULTS: FOR PI = 24, 0' = 30° (SEE CHART THIS APPENDIX)		3) HI	A FUR	FILL S	SILS							
- DESCRIPTION: STIPE TO HARD BROWN MOTTLIED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND RENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON' = 50 KR BASED ON RELATION OF THIS LAYER TO THE FAILURE PLANE. - RESULTS: OF: 30° (SEE CORREMATION THIS APPENDIX)			1 1 1			4, \$	1 - 3	0°	(SEE C	HART	THIS F	t PPIEM
- DESCRIPTION : STIPTE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAMED DEMETROMETER RANGGIE: 1.5 - 4.5 tsp - STRENGTH PARAMIETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANTE. - RESULTS: OF: 30° (SEE SUPPLIEDED N. THE ADDRESS - LAYER CAYER CAYER		2) \$'	NS2 LI				1 - 7					
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAMED DENETROMETER RANGGIE: 1.5 - 4.5 top - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS CONSIDER ON STARK CHARTS TO THE FAILURE PLANTE.				(SEE			200					
- DESCRIPTION: STIPTE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAMED DENETROMETER RANGGIE: 1.5 - 4.5 tsp - STRENGTH PARAMETER: 1) CORRELATION TO STARK CHARTS		10	TITE P	AILURE	PLAN	E.	• **			ie Titis	LA	YICR
- DESCRIPTION: STIPPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGE: 1.5 - 4.5 top - STRENGTH PARAMETER:		1) CURA	VELATION O	TO 5	TARK C	HARTS						1
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY) - NAND DENETROMETER RANGE: 1.5 - 4.5 top												1
- DESCRIPTION: STIPE TO HARD BROWN MOTTLED WITH GRAY SILTY CLAY (USCS: LEAN CLAY)						,,,	71.5		fiz			
- DESCRIPTION: STIPE TO HARD BROWN MOTTHER WITH STAN	1_1	HAND DE										
			S	ILTY CI	LAY	(uscs	: LEA	IN CL	AY)	WM	Gra	1
LAYER: ORIGINAL EMBANKINENT FILL (OLDER FILL)		DESCRIP	nal.	c n ec		. 1000						



Project/Proposal No. 01. 11497. 013 Calculated By MITR Date 6-29-09

Project/Proposal Name CARMAL ASH POND Checked By MAR Date 7-2-09 Subject STRENGTH & PERM. Sheet 5 of 8

DESC	SIDISON:	USRY LOO	SE 76	MED L	CAUSE	GRAY S	SILT, CON	ITAINS
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- 1-111-12	PENETRON	TETRIK:	J - Z,	S Est	ON	SICK 5	AMPLIES	
- STRE	LATH PARA	4METERS						
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		: \$\p'_{\beta_2} =		(5)	EE CORNE	ATKIN T	The Wallet	Lix
							Hes Miller	13.2)
2)	\$' URS F	I						
	- RESUL	TS: FUR	P1 =	15 ,	\$'	31.5°	(SEE CH	HART TH
		1 - 1					74 4	APPE
3)	HALL'S TI	HESIS						
	4:	ic = 36 -	0,2	665 (& CLAY)		
	FUN	2 CF = 10	,9,	PNC =	33°	[]		
4)	N/A FOR	NATURAL	SOILS	- u	SE TA	BLE 3.29	- COMMON	00.00
	OF COITEFO	WHIES SOIL	S (So	Neci				1 1-01 61
	- FUR 'LO	OSE INORELI	ANIC S	SILTS'	Ø'=	27°		
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Design	1 Strengt	h Param	eter:	Us	se o	NC = 30	, c'= (o psf
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- Permeo	Kilidu: P	Based on	soil de	scoi ati	C0			
	k,	= 1 ×10-5	cm/s	(tunic	ol oub	ished V	· lue	1



Project/Proposal No. 0//1/497.0/5 Calculated By MTR Date 6-29-09

Project/Proposal Name CARDMAL ASH FOULD Checked By More Date 1-2-9
Subject STRENGIA F PERMEABILITY Sheet 6 of 8

1 1	
	DESCRIPTION: VERY SOFT TO STIFF ORGANIC CLAYRY SILT, CONTAIN SCAMS OF VERY LOOSE DREATHIC SILT
v	LOSS ON IGNITION: RANGE = 7.9% TO 10.4% FROM 3
7	SAMPLES TESTED.
	HAND PENETROMETER: 0.0 - 1.25 Est
	STREMATH PARAMETER:
	SIRCHAIN FARAMETER.
	1) STARK CURRELATION: - CONSIDER GUS = 100 KPg AND 400 KPg WITH TENDANCY
	TOLLINED 100 KP.
	- RESULTS: \$\dis = 260 (SIEE CORRELATION THIS MODERIAL)
	2) &' URS PI
	- RESULTS: FUR PI = 16, 0'= 31° (SEE CHART THIS APPEND
	3) HALL'S THESIS
	d'ic = 36 - 0,2665 (20 CLAY) FOR CF = 16, pric = 31.7°
++	5) CU TRIAKIAL TIEST - SAMPLE WAS NOT DESCRISED IT 'ORGANIC', DESCRIPTION BEST MATCHES THIS LAYER
	RESULTS: \$'=36.9°, C'= 110' psf
	PERMEABILITY: DIS - LOW = 0.0015 KV = 5 KIO-6 CA HIGH = 0.005 (GIESSYNTEC , SKE APPEN
	AVG = 0.0023) (GICOSYNTEC, SEE APPER
	PER PHULA GEC 5, LOI & 20% SOIL
	PROPERTIES CONTROLLED 34 JON-URGANIC
	PURTION S. REGULAR CORRECATIONS OK



Project/Proposal No. 01/. 1/497.013 Calculated By MTR Date 6-29-09
Project/Proposal Name CARNWAL ASH POND Checked By MGR Date 1-2-09
Subject STRENGTH F PERM. Sheet 7 of 8

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7		DESCRIP	mon:	MEC	DEN PUE C	ACIAN STRAVE	C =	ENSE ANI	3. J.	in/E	1 Ar	ME	GM D S	Y F		70
+		DESCRIP	mon:	MED COA	DEN DEN PLE	ACIAN GRAVE	C = 200 C	ENSE ANI	34.	11/E	1 Ar	ME	GIRAS D SI ARLE 31-3	Y F		70
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		DESCRIP	TION:	MET COA	- GL DEN PRJE (ACIAN FRANE	C = 2	SAF	54. 52. 42.	77 6 2	DIE	ME	G/M b S/ ABLE 3/-3 74/ 36°	7.11 7.11 2°		
		DESCRIP NGO Î	TION:	MET COA	DEN PLE C	SCRAVE FRAVE 10 3 60211469 0903	L = 2	SAN S	52. 42.	72 77 60 2	DIS.09	ME	G/M b S/ ABLE 3/-3 74/ 36°	7 F		
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SOLUTIONS TO BUILD ON	Project/Proposal No. <u>0// //497, 0/3</u>	Calculated By MTR	Date 7/15/09
Cincinnati (513) 771-8471 Cleveland (216) 901-1000	Project/Proposal Name CARDILIAL ASH POND	Checked By	Date
Columbus (614) 793-2226 Dayton (937) 424-1011	Subject SEISMIC STRENGTH PARAMETERS	Sheet <u>8</u> of <u>8</u>	

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TO ASSUME IT WILL EXHIBIT					7177 C/E		EN
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Table 10.3 Summary of Soil Properties Used in Comparison of R and τ_{ff} vs. σ'_{fc} Strength Envelopes

Soil no.	Description and reference	Index properties	c' (psf)	φ' (deg)	c_R (psf)	$\phi_{\scriptscriptstyle R}$ (deg)	d ^a (psf)	ψ ^b (deg)
1	Sandy clay (CL) material from Pilarcitos Dam; envelope for low (0–10 psi) confining pressures. (Wong et al., 1983)	Percent minus No. 200: 60–70 Liquid limit: 45 Plasticity index: 23	u	SE C:	= 60 = 50 psf , P1 =	F Ø=.		24.4
2	Brown sandy clay from dam site in Rio Blanco, Colorado (Wong et al., 1983)	Percent minus No. 200: 25 Liquid limit: 34 Plasticity index: 12	200	31	700	15	782	16.7
3	Same as soil 1 except envelope fit to 0-100 psi range in confining pressure (Wong et al., 1983)	Percent minus No. 200: 60–70 Liquid limit: 45 Plasticity index: 23	0	34	300	15.5	327	16.8
4 ·	Hirfanli Dam fill material (Lowe and Karafiath, 1960)	Percent minus No. 200: 82 Liquid limit: 32.4 Plastic limit: 19.4	0	35	1400	22.5	1716	26.9

[&]quot;Intercept of au_{ff} vs. σ_{fc}' envelope—can be calculated knowing c', ϕ' , c_{R} , and ϕ_{R} .

^bSlope of τ_{ff} vs. σ'_{fc} envelope—can be calculated knowing c', ϕ' , c_R , and ϕ_R .

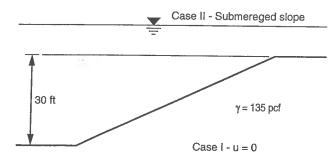


Figure 10.6 Slope used to compare simple, single-stage and rigorous, two-stage pseudostatic analyses.

Table 10.4 Summary of Pseudostatic Safety Factors Computed Using Simple Single-Stage and Rigorous Two-Stage Procedures

	Case dry si		Case submerge	
Soil	Single-stage analysis	Two-stage analysis	Single-stage analysis	Two-stage analysis
1	0.95	1.06	0.83	0.95
2	1.56	1.77	1.59	1.79
3	1.07	1.19	1.10	1.21
4	2.76 3.42		2.83	3.49

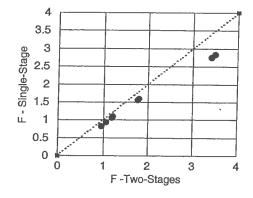


Figure 10.7 Comparison of factors of safety by simplified single-stage pseudostatic and more rigorous two-stage pseudostatic analyses.

used for cases where significant (more than 15 to 20%) strength losses are not anticipated.

POSTEARTHQUAKE STABILITY ANALYSES

Following an earthquake, the stability of a slope may be diminished because cyclic loading has reduced the shear strength of the soil. The reductions in shear strength are generally treated differently depending on whether or not liquefaction occurs. Stability follow-



Project No: 011-11497-014 Date: <u>5/29/09</u>

Project: Gavin Plant Bottom Ash Pond Investigation

Reference:

Drained Shear Strength Parameters for Analysis of Landslides. Timothy D. Stark; Hangseok Choi; and Sean McCone. Journal of Geotechnical Engineering, May 2005. pp 575 - 588

Purpose:

Estimate effective stress, or drained, shear strength parameters of cohesive soils through emperical correlations using laboratory index testing and the effective normal stress. Secant residual and secant fully softened friction angles can be estimated from charts developed by Stark et al.

Laboratory Data

Soil Layer: Newer Embankment Fill	
-----------------------------------	--

Statistical Results from _4_ Borings				% Passing	Clay Sized	
				#200 Sieve		
	<u>PI</u>	<u>LL</u>	<u>MC</u>	<u>(.075 mm)</u>	(.002 mm)	
Number in Statistical Sample	12	12	16	9	8	
Minimum	9	25	10	24	8	
Maximum	18	37	31	79	21	
Mean	12.1	30.3	16.3	41.6	12.1	
Median	11	28.5	14.5	31	11	
Mode	10	27	16	31	12	
Std Dev	3.2	4.5	5.4	18.9	4.6	
Design Value	10	27	_	-	12	

Adjustment Factor for ASTM Derived Values

$$\frac{\textit{ball-milled derived LL}}{\textit{ASTM derived LL}} = .003 \, (\textit{ASTM derived LL}) + 1.23$$

$$\text{LL}_{\text{ASTM}} = 27$$

$$\text{LL}_{\text{BM}} = 35.4$$

$$\frac{\textit{ball-milled derived CF}}{\textit{ASTM derived CF}} = 0.0003 \, (\textit{ASTM derived CF}) 2 - 0.037 \, (\textit{ASTM derived CF}) + 2.254$$

 $CF_{ASTM} = 12$ $CF_{BM} = 22.2$

where: LL = Liquid Limit

CF = Clay-sized Fraction



Soil Layer: Newer Embankment Fill

 $\begin{array}{c} \text{LL}_{\text{BM}} = & 35.4 \\ \text{CF}_{\text{BM}} = & 22.2 \end{array}$

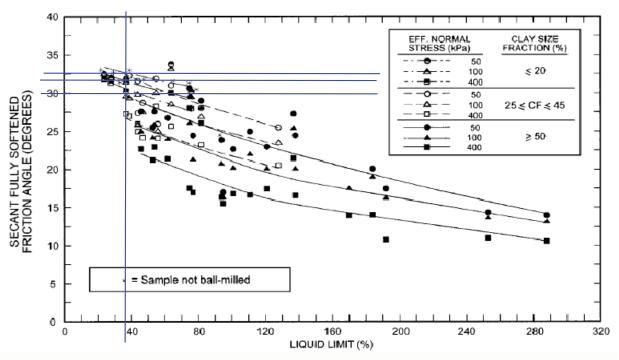


Fig. 5. Secant fully softened friction angle relationships with liquid limit, clay-size fraction, and effective normal stress

Secant Fully Softened Friction Angle

Effective Normal Stress

		50 kPa	100 kPa
Sized stion,	<i>CF</i> ≤ 20	32.5°	31.5°
Clay S Fracti	25 ≤ CF ≤ 45	32.5°	30°

Design Friction Angle Value	31°
-----------------------------	-----



Project No: 011-11497-014 Date: 5/29/09

Project: Gavin Plant Bottom Ash Pond Investigation

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Purpose:

Estimate effective stress, or drained, shear strength parameters of cohesive soils through emperical correlations using laboratory index testing and the effective normal stress. Secant residual and secant fully softened friction angles can be estimated from charts developed by Stark et al.

Laboratory Data

Soil Layer: Original Embankment Fill	
--------------------------------------	--

Statistical Results from 3 Borings				% Passing	Clay Sized	
				#200 Sieve	Fraction	
	<u>PI</u>	<u>LL</u>	MC	(.075 mm)	(.002 mm)	
Number in Statistical Sample	6	6	10	5	4	
Minimum	14	32	15	76	23	
Maximum	24	49	33	96	32	
Mean	19.8	43.3	22.5	86.2	28.0	
Median	20.5	47.5	22	86	28.5	
Mode	24	48	22	#N/A	#N/A	
Std Dev	4.4	7.4	5.1	8.1	3.7	
Design Value	24	48	_	_	28	

Adjustment Factor for ASTM Derived Values

$$\frac{\textit{ball-milled derived LL}}{\textit{ASTM derived LL}} = .003 \, (\textit{ASTM derived LL}) + 1.23$$

$$\text{LL}_{ASTM} = 48$$

$$\text{LL}_{BM} = 66.0$$

$$\frac{\textit{ball-milled derived CF}}{\textit{ASTM derived CF}} = 0.0003 \, (\textit{ASTM derived CF}) 2 - 0.037 \, (\textit{ASTM derived CF}) + 2.254$$

 $CF_{ASTM} = 28$ $CF_{BM} = 40.7$

where: LL = Liquid Limit $CF_{BM} = 40.$

CF = Clay-sized Fraction



Soil Layer: Original Embankment Fill $LL_{BM} = 66.0$ $CF_{BM} = 40.7$

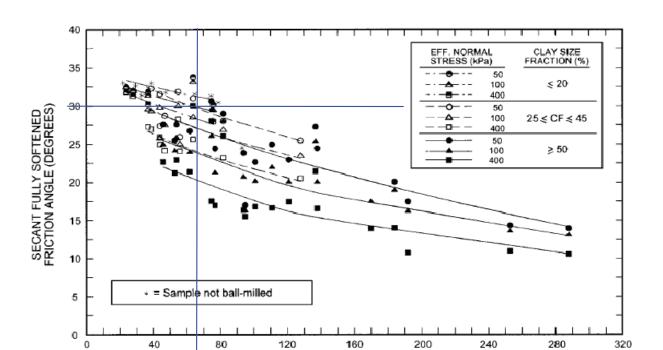


Fig. 5. Secant fully softened friction angle relationships with liquid limit, clay-size fraction, and effective normal stress

LIQUID LIMIT (%)

Effective Normal Stress, kPa	50
Secant Fully Softened Friction Angle	30°



Project No: 011-11497-014 Date: 5/29/09

Project: Gavin Plant Bottom Ash Pond Investigation

Reference:

Drained Shear Strength Parameters for Analysis of Landslides. Timothy D. Stark; Hangseok Choi; and Sean McCone. Journal of Geotechnical Engineering, May 2005. pp 575 - 588

Purpose:

Estimate effective stress, or drained, shear strength parameters of cohesive soils through emperical correlations using laboratory index testing and the effective normal stress. Secant residual and secant fully softened friction angles can be estimated from charts developed by Stark et al.

Laboratory Data

Statistical Results from 7 Borings			% Passing	Clay Sized	
	DI	- 11	MC	#200 Sieve	
	<u>PI</u>	<u>LL</u>	<u>MC</u>	<u>(.075 mm)</u>	(.002 mm)
Number in Statistical Sample	17	17	20	21	19
Minimum	3	30	34	56	12
Maximum	20	50	54	97	44
Mean	13.5	40.6	42.5	78.2	19.8
Median	15	41	43	81	18
Mode	16	45	43	81	16
Std Dev	4.6	5.3	4.4	10.7	7.0
Design Value	16	45	-	-	20.0

Adjustment Factor for ASTM Derived Values

$$\frac{\textit{ball-milled derived LL}}{\textit{ASTM derived LL}} = .003 \, (\textit{ASTM derived LL}) + 1.23 \qquad \qquad \text{LL}_{ASTM} = \qquad 45 \\ \text{LL}_{BM} = \qquad 61.4$$

$$\frac{\textit{ball-milled derived CF}}{\textit{ASTM derived CF}} = 0.0003 \, (\textit{ASTM derived CF}) 2 - 0.037 \, (\textit{ASTM derived CF}) + 2.254$$

 $CF_{ASTM} = 20.0$ $CF_{BM} = 32.7$

where: LL = Liquid Limit CF

CF = Clay-sized Fraction



DRAINED SHEAR STRENGTH PARAMETER CORRELATION

Soil Layer: Organic Clayey Silt $LL_{BM} = 61.4$ $CF_{BM} = 32.7$

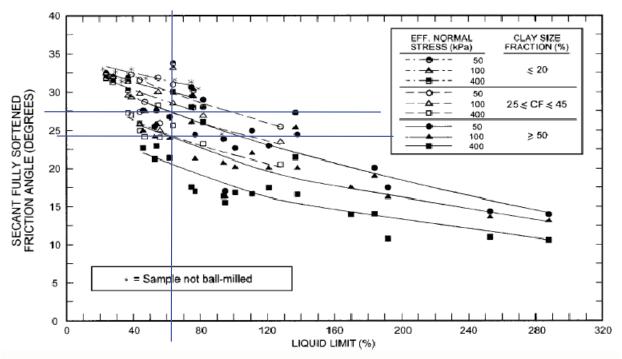


Fig. 5. Secant fully softened friction angle relationships with liquid limit, clay-size fraction, and effective normal stress

Secant Fully Softened Friction Angle

Effective Normal Stress

		100 kPa	400 kPa	
Clay Sized Fraction, %	<i>CF</i> ≤ 20	27.5°	24°	
	25 ≤ CF ≤ 45	-	-	

Design Friction Angle Value	26°



DRAINED SHEAR STRENGTH PARAMETER CORRELATION

Project No: 011-11497-014 Date: 5/29/09

Project: Gavin Plant Bottom Ash Pond Investigation

Reference:

Drained Shear Strength Parameters for Analysis of Landslides. Timothy D. Stark; Hangseok Choi; and Sean McCone. Journal of Geotechnical Engineering, May 2005. pp 575 - 588

Purpose:

Estimate effective stress, or drained, shear strength parameters of cohesive soils through emperical correlations using laboratory index testing and the effective normal stress. Secant residual and secant fully softened friction angles can be estimated from charts developed by Stark et al.

Laboratory Data

Statistical Results from 6 Bo	rings			% Passing	Clay Sized
	Dut			#200 Sieve	Fraction
	<u>PI*</u>	<u>LL*</u>	<u>MC</u>	<u>(.075 mm)</u>	(.002 mm)
Number in Statistical Sample	4	4	10	12	10
Minimum	7	34	22	48	3
Maximum	15	38	38	96	28
Mean	12.5	36.0	29.0	76.8	11.0
Median	14	36	28.5	76.5	6.5
Mode	15	#N/A	26	91	5
Std Dev	3.8	1.8	5.4	15.9	8.5
*Does not include results from 'Non-Plastic'	samples.				
Design Value	15	36	-	_	10.0

Adjustment Factor for ASTM Derived Values

$$\frac{\textit{ball-milled derived LL}}{\textit{ASTM derived LL}} = .003 \, (\textit{ASTM derived LL}) + 1.23$$

$$\text{LL}_{\text{BM}} = 36$$

$$\text{LL}_{\text{BM}} = 48.2$$

CF = Clay-sized Fraction



DRAINED SHEAR STRENGTH PARAMETER CORRELATION

Soil Layer: Alluvium Silt and Clay $LL_{BM} = 48.2$ $CF_{BM} = 19.1$

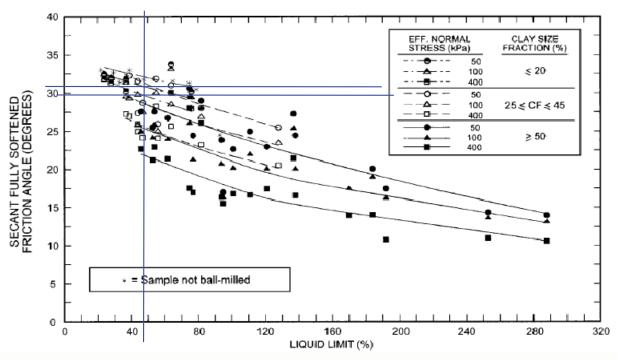


Fig. 5. Secant fully softened friction angle relationships with liquid limit, clay-size fraction, and effective normal stress

Secant Fully Softened Friction Angle

Effective Normal Stress

		100 kPa	400 kPa	
Clay Sized Fraction, %	<i>CF</i> ≤ 20	31°	29.5°	
	25 ≤ CF ≤ 45	-	-	

	0
Design Friction Angle Value	30°

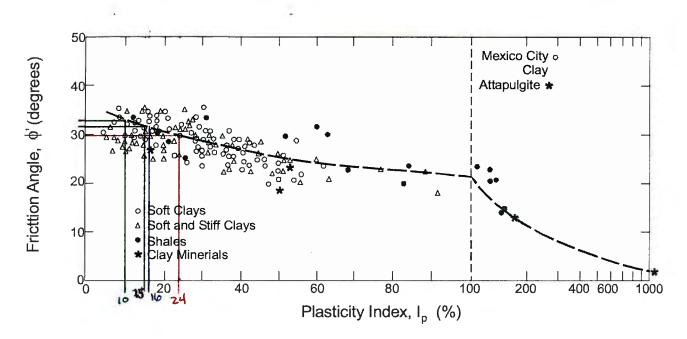


Figure 74. Relationship between ϕ' and PI (Terzaghi, Peck, and Mesri, 1996).

Report No. FHWA-IF-02-034 Geotechnical Engineering Circular No. 5 Evaluation of Soil and Rock Properties April, 2002

LAYER	<u> 7.1.</u>	4'
- EMBANKMENT EXPANSION FILL	10	33°
- ORIGINAL EMBANKMENT FILL	24	30°
- ALLUNUM SILTAND CLAY	15	31.5
ORGANIC CLAYEY SILT	16	31°

TABLE 3.28 COMMON PROPERTIES OF COHESIONLESS SOILS $\frac{13/62}{62.427}$									
Material	Compactness	D _R , %	N*	γ dry,† dry g/cm³ (ρος)	Void ratio	YSAT (PCT)	Strength‡ ø		
GW:well-graded	Dense	75	90	2.21 138	0.22	149	40		
gravels, gravel-	Medium dense	50	55	2.08 129.8	0.28	143.5	36		
sand mixtures	Loose	25	<28	1.97 123	0.36	139.5	32		
GP: poorly graded	Dense	75	70	2.04 /27.4	0.33	/43	38		
gravels, gravel-	Medium dense	50	50	(1.92) 120	0.39	137.5	35		
sand mixtures	Loose	25	<20	1.83 //4.2	0.47	134	32		
SW: well-graded sands,	Dense	75	65	1.89 //8	0.43	136.8	37		
gravelly sands	Medium dense	50	35	(1.79) 111.7	0.49	132.2	34		
	Loose	25	<15	11.70 106.1	0.57	128.8	30		
SP: poorly graded	Dense	75	50	1.76 /09.9	0.52	131-3	36		
sands, gravelly	Medium dense	50	30	1.67 /0 4 . 2	0.60	127.6	33		
sands	Loose	25	<10	1.59 9 9.3	0.65	124	29		
SM: silty sands	Dense	75	45	1.65 /03	0.62	127	35		
	Medium dense	50	25	1.55 97	0.74	123.5	32		
	Loose	25	<8	1.49 93	0.80	120,7	29		
ML: inorganic silts, very	Dense	75	35	.1.49 93	0.80	120.7	33		
fine sands	Medium dense	50	20	1.41 88	0.90	117.6	31		
	Loose	25	<4	1.35 84.3	1.0	115.5	27		

^{*}N is blows per foot of penetration in the SPT. Adjustments for gradation are after Burmister (1962). See Table 3.23 for general relationships of D_R vs. N. †Density given is for $G_s = 2.65$ (quartz grains).

4Friction angle ϕ depends on mineral type, normal stress, and grain angularity as well as D_R and gradation (see Fig. 3.63).

D15 Range = 0,002 - 0,080

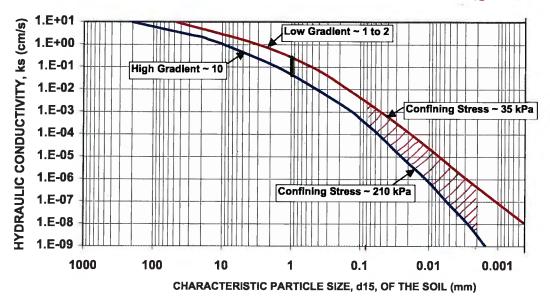


Figure 91. Range of hydraulic conductivity based on grain size (after GeoSyntec, 1991).

Considering the site geology, the laboratory and field data should be tabulated with other known data for the sample/test location and with depth, soil/rock type, grain size distribution, Atterberg limits, and water content. This table should also include important test information such as: stress conditions, gradients, and test method. Once this table is constructed it will be much easier to group like soil types and k values, to delineate distinct areas within the site, and to eliminate potentially erroneous data. Once these values have been grouped together and potentially erroneous values eliminated, it may be useful to compute an average value for each grouping. When averaging, the log of the hydraulic conductivity value must be taken before performing an arithmetic mean or incorrect results will be produced. First, the logarithm of each value should be taken. Second, an average value should be calculated from these logarithmic values. Finally, the antilog of this average value should be taken to calculate the average hydraulic conductivity value. Table 35 illustrates how to calculate the mean of the log of k data and compares this value with an incorrect direct arithmetic mean.

Glacial outwash sand and gravel.

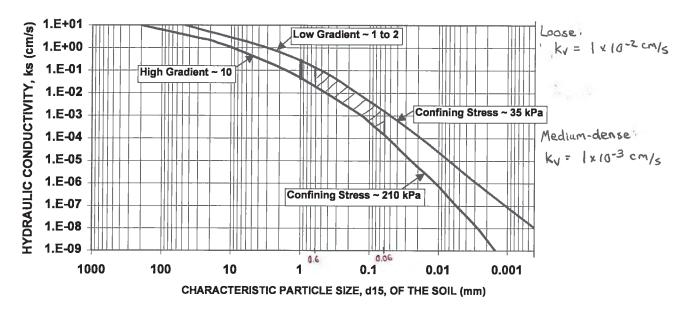


Figure 91. Range of hydraulic conductivity based on grain size (after GeoSyntec, 1991).

Considering the site geology, the laboratory and field data should be tabulated with other known data for the sample/test location and with depth, soil/rock type, grain size distribution, Atterberg limits, and water content. This table should also include important test information such as: stress conditions, gradients, and test method. Once this table is constructed it will be much easier to group like soil types and k values, to delineate distinct areas within the site, and to eliminate potentially erroneous data. Once these values have been grouped together and potentially erroneous values eliminated, it may be useful to compute an average value for each grouping. When averaging, the log of the hydraulic conductivity value must be taken before performing an arithmetic mean or incorrect results will be produced. First, the logarithm of each value should be taken. Second, an average value should be calculated from these logarithmic values. Finally, the antilog of this average value should be taken to calculate the average hydraulic conductivity value. Table 35 illustrates how to calculate the mean of the log of k data and compares this value with an incorrect direct arithmetic mean.

Geotechnical Engineering Circular No. 5 Evaluation of Soil and Rock Properties.

Method: Geosyntec

Source: FHWA GEC No 5: pg 184

Equation: Graphic

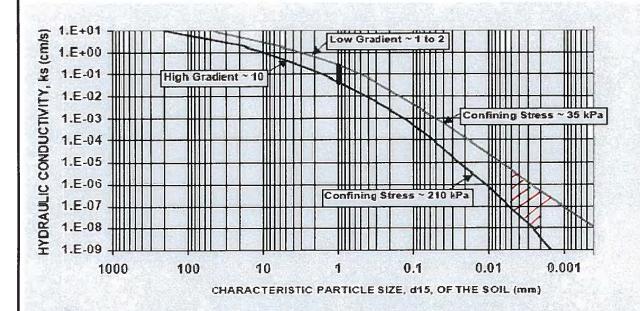


Figure 91. Range of hydraulic conductivity based on grain size after GeoSyntec, 1991).

LAYER: ORGANIC CLAYEY SILT

dis range = 0.0015 mm - 0.005 mm

AVA dis = 0.002 mm

USE $KV = 5 \times 10^{-16}$ cm/SEC BANEA ON INCLUSION SIE SILT SEAMS

PERMEABILITY TEST DATA AND COMPUTATION SHEET





Job Number:	011.11497.013	Date:	5/6-7/2009	Maximum Dry Density:	
Project Name:	Cardinal Ash Pond Investigation	Boring:	CD-PZ-BAP-0907	Optimum Moisture Content:	
Project Location:	Brilliant, Ohio	Sample:	ST-6A Sec. II	% Compaction.:	
Tested By:	PJM	Depth:	8.5' to 9.9'	Optimum +/-:	
Remarks:				Natural:	Х
Material:	FILL: Hard brown, gray and dark-gra	v silty clay	inter-mixed with organic s	ilt, trace Remolded:	

Material: FILL: Hard brown, gray and dark-gray silty clay inter-mixed with organic silt, trace

fine to coarse sand.

Sample:

Initial Length: ____ 5.5945 in = 14.210 cm Final Ave. Length (L): _____ 5.6042 in = 14.235 cm

> Diameter: 2.8765 in = 7.31 cm Area (A): 6.499 sq in = 41.93 sq cm

Volume (V): 36.356 cu in = 595.77 cu cm

Wet Wt.: 1144.17 grams Unit Wet Wt.: 119.90 pcf Unit Dry Wt.: 93.99 pcf

Test Conditions:

Chamber Pressure: 62 psi Back Pressure: 58 psi Confining Pressure: ____ 4 psi

Temp. @ Start: 22.5 °C Temp. @ End: 22.5 °C

Average Temp.: 22.5 °C B Parameter: 0.96

Bottom Pipette: 58 psi = 4079.6 cm

Before Test Moisture Content: After Test Pan No. = D D Wet Wt. + Pan = 1144.17 1157.03 Dry Wt. + Pan = 896.92 896.92 Wt. of Pan = 0.00 0.00 Wt. of Dry Soil = 896.92 896.92

Wt. of Water = 247.25 260.11 % Moisture = 27.57 29.00

2.7000

Pipette Pressures During Test:

Top Pipette: 60 psi = 4220.3 cm S.G.(est) = 93.80 98.30

Pipette:

Area (a): 0.3435 sq in = 0.8725 sq cm

Calculations:

 $2 \cdot A \cdot \Delta t$

where:

k = Hydraulic Conductivity

a = Pipette Cross-Sectional Area

L = Length of Sample

A = Sample Cross-Sectional Area

 Δt = Time Interval (t_2 - t_1)

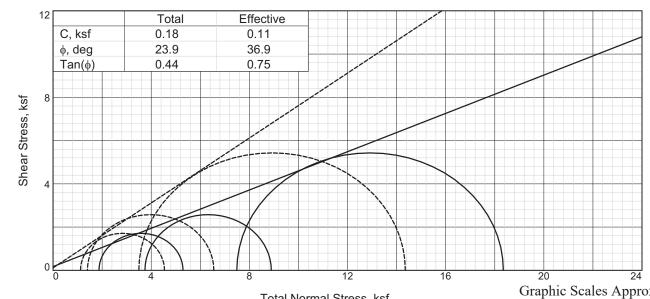
h₁ = Head Loss Across Permeameter/Specimen at t₁ h₂ = Head Loss Across Permeameter/Specimen at t₂

% SATURATION

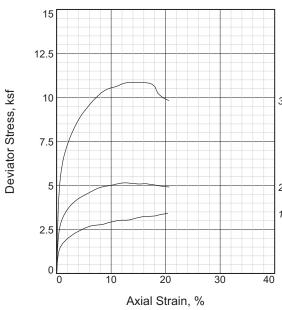
In = Natural Logarithm (Base e = 2.71828)

			+	Hydraulic Head				Temp. Corr.	
		Time Interval	Тор	Headwater	Bottom	Tailwater	Head Loss		Permeability
	Time	Δt	Pipette	H ₁	Pipette	H ₂	$h = H_1 - H_2$		k
Date	Readings	Seconds	CC	cm	cc	cm	cm	ℓn (h ₁ /h ₂₎	cm/sec
5/6/2009	9:45 AM	0.00	48.45	4092.08	14.20	4272.01	-179.93	_	-
5/6/2009	10:51 AM	3,960	48.40	4092.14	14.45	4271.73	-179.59	0.00191	6.740E-08
5/6/2009	12:15 PM	5,040	48.20	4092.36	14.65	4271.50	-179.13	0.00256	7.077E-08
5/6/2009	1:45 PM	5,400	48.05	4092.54	15.00	4271.09	-178.56	0.00320	8.280E-08
5/6/2009	3:17 PM	5,520	47.85	4092.77	15.25	4270.81	-178.04	0.00289	7.312E-08
5/7/2009	8:21 AM	61,440	45.60	4095.34	18.00	4267.66	-172.31	0.03272	7.431E-08
					•				
		_							
							-		

Time Weighted Average, k [cm/sec] = 7.423E-08



Total Normal Stress, ksf -Effective Normal Stress, ksf ----- Graphic Scales Approximate



	Sar	mple No.	1	2	3	
		Water Content, %	35.1	43.8	31.9	
	_	Dry Density, pcf	83.0	76.2	85.0	
	Initial	Saturation, %	92.2	97.7	87.6	
	ī	Void Ratio	1.0297	1.2123	0.9833	
3		Diameter, in.	2.90	2.85	2.90	
		Height, in.	5.59	5.59	5.59	
		Water Content, %	33.3	38.9	31.0	
	7,	Dry Density, pcf	86.9	82.6	90.3	
	At Test	Saturation, %	95.6	101.0	96.5	
	_ _	Void Ratio	0.9402	1.0401	0.8674	
2		Diameter, in.	2.86	2.78	2.85	
		Height, in.	5.49	5.42	5.43	
1	Stra	ain rate, in./min.	0.00	0.00	0.00	
	Bad	ck Pressure, psi	40.00	40.00	40.00	
	Cel	l Pressure, psi	53.00	66.00	92.00	
	Fai	I. Stress, ksf	3.4	5.1	10.9	
	Т	otal Pore Pr., ksf	6.5	8.1	9.8	
	Ult.	Stress, ksf	3.4	4.9	9.8	
	Т	otal Pore Pr., ksf	6.5	8.0	9.9	
	$\overline{\sigma}_1$	Failure, ksf	4.5	6.6	14.4	
	$\overline{\sigma}_3$	Failure, ksf	1.1	1.4	3.5	

Type of Test:

CU with Pore Pressures Sample Type: Shelby Tube

Description: Gray mottled with dark-gray and brown clayey silt, some fine sand, trace medium to

LL= 35 **PL=** 28 **PI=** 7 **Assumed Specific Gravity= 2.7**

Remarks:

Client:

Project: Cardinal Plant Ash Pond Investigation

Brilliant, Ohio

Location: CD-PZ-BAP-0901

Sample Number: ST-19A **Depth:** 31.0' to 32.8'

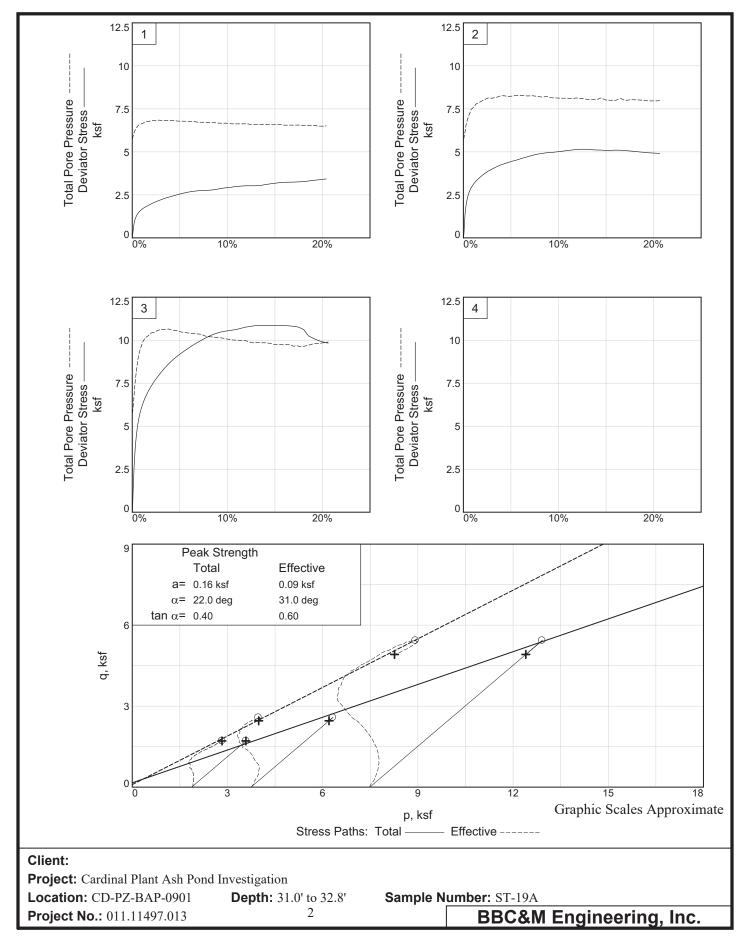
Proj. No.: 011.11497.013

Date Sampled: 5/1/09

TRIAXIAL SHEAR TEST REPORT

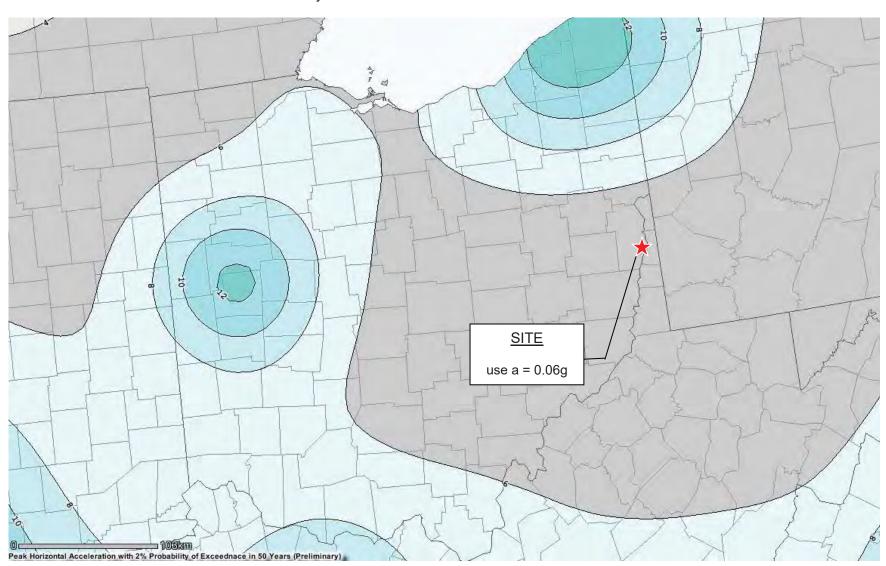
BBC&M Engineering,

Tested By: PJM Checked By: JJ PLATE 31

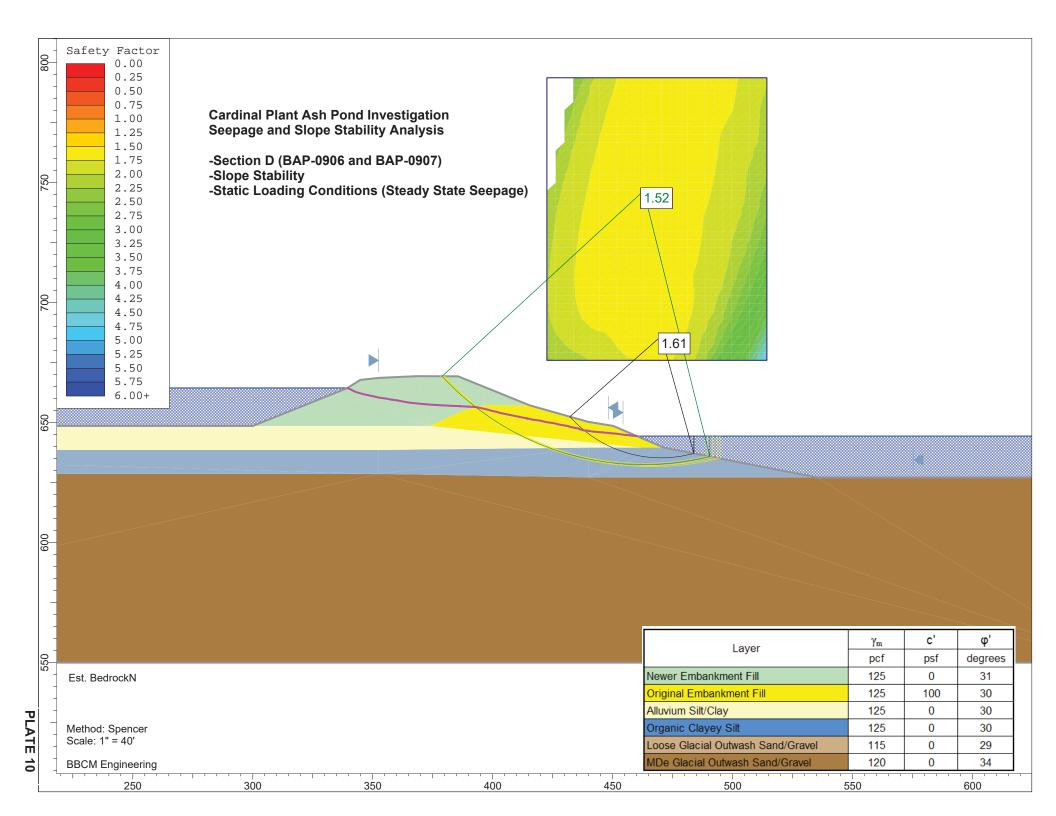


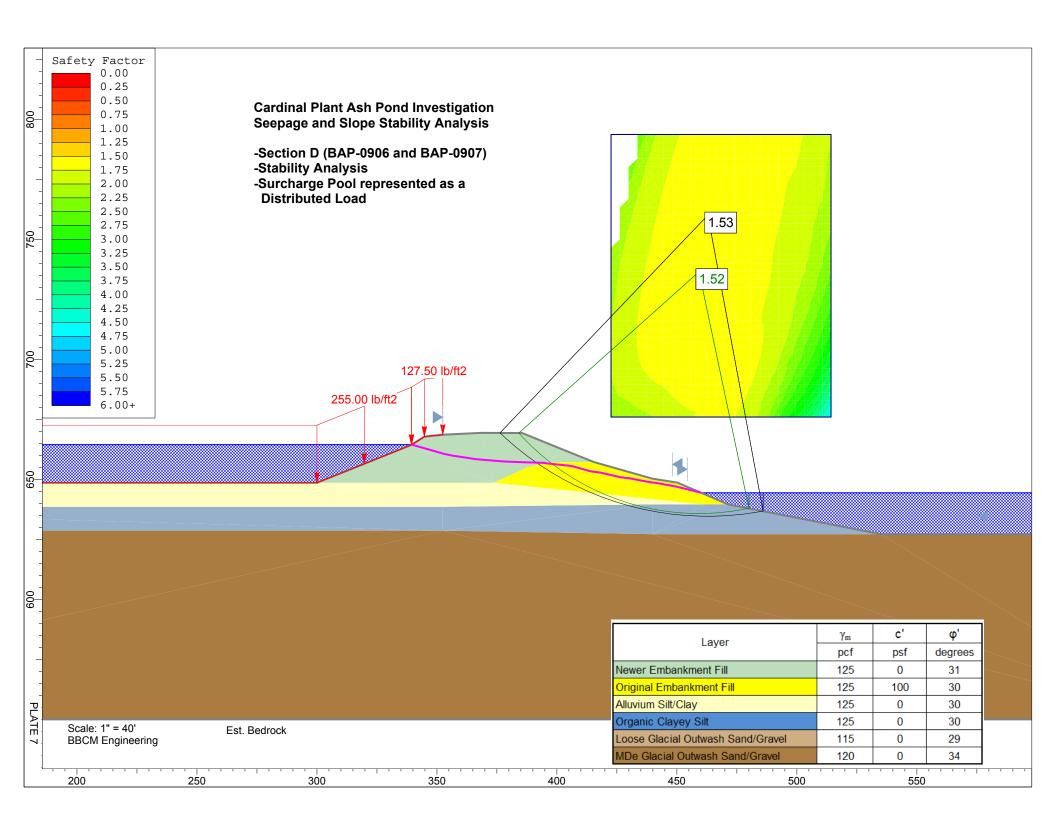
USGS National Seismic Hazard Maps - 2008

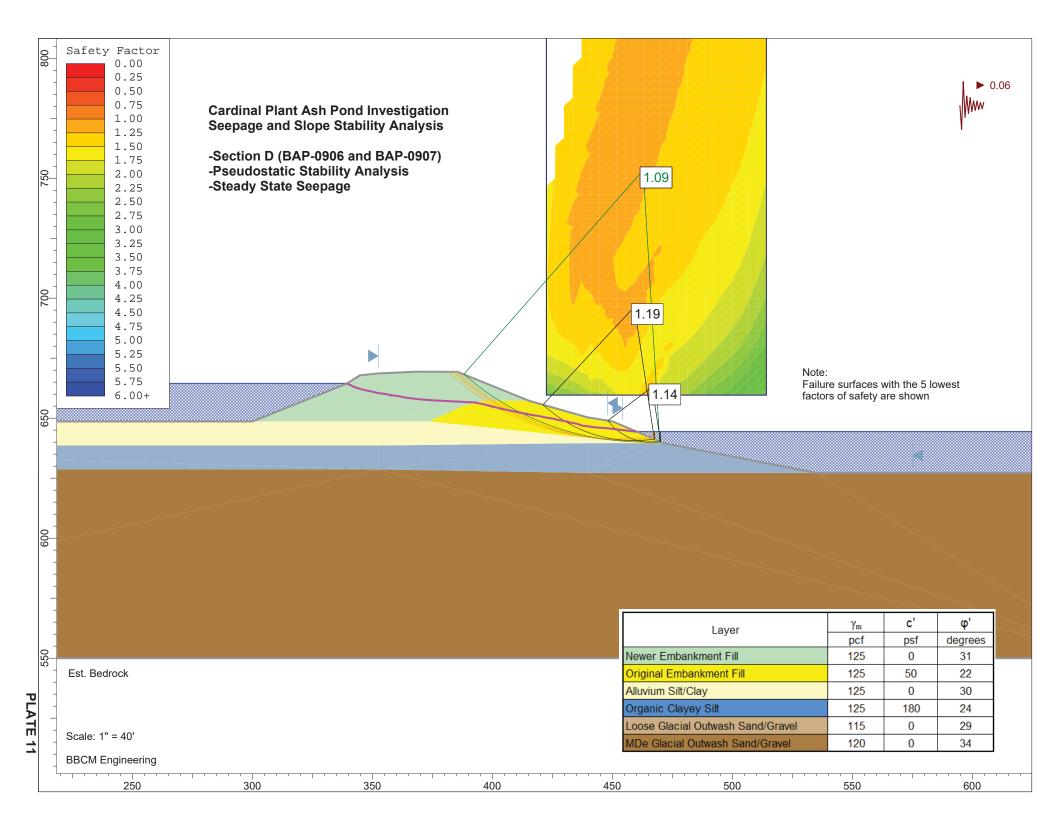
Peak Horizontal Acceleration with 2% Probability of Exceedence in 50 Years

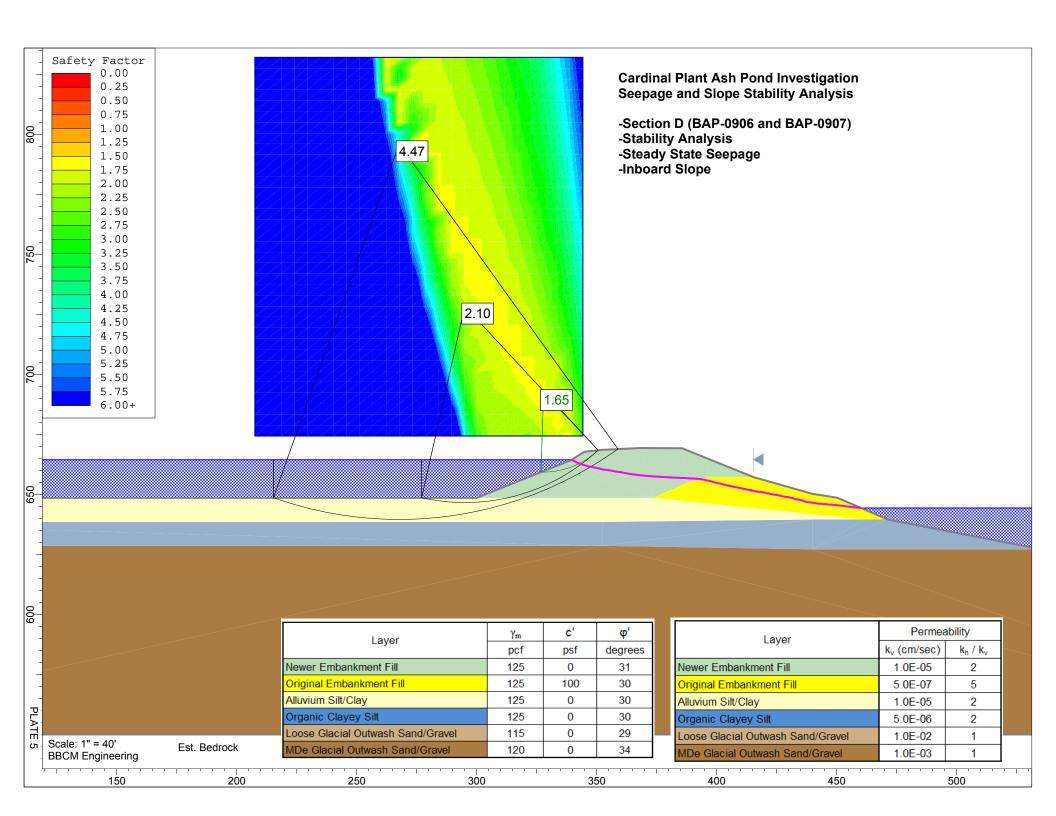


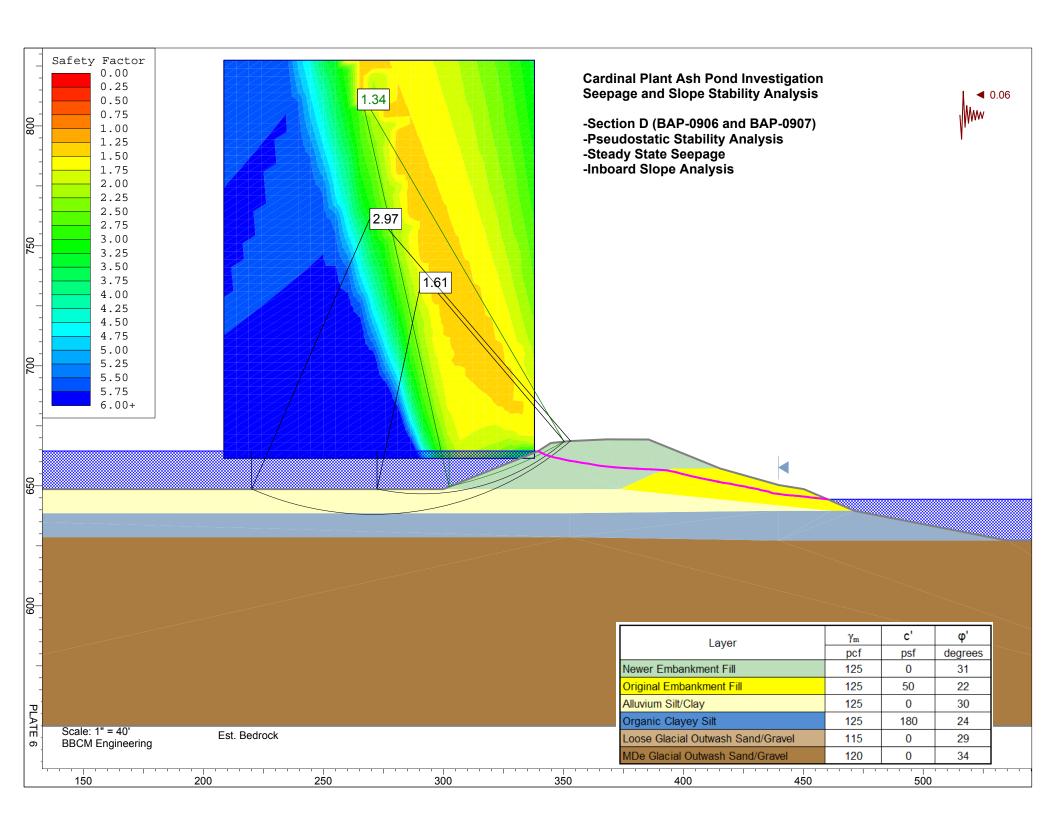
Appendix IV – Limit Equilibrium Analysis

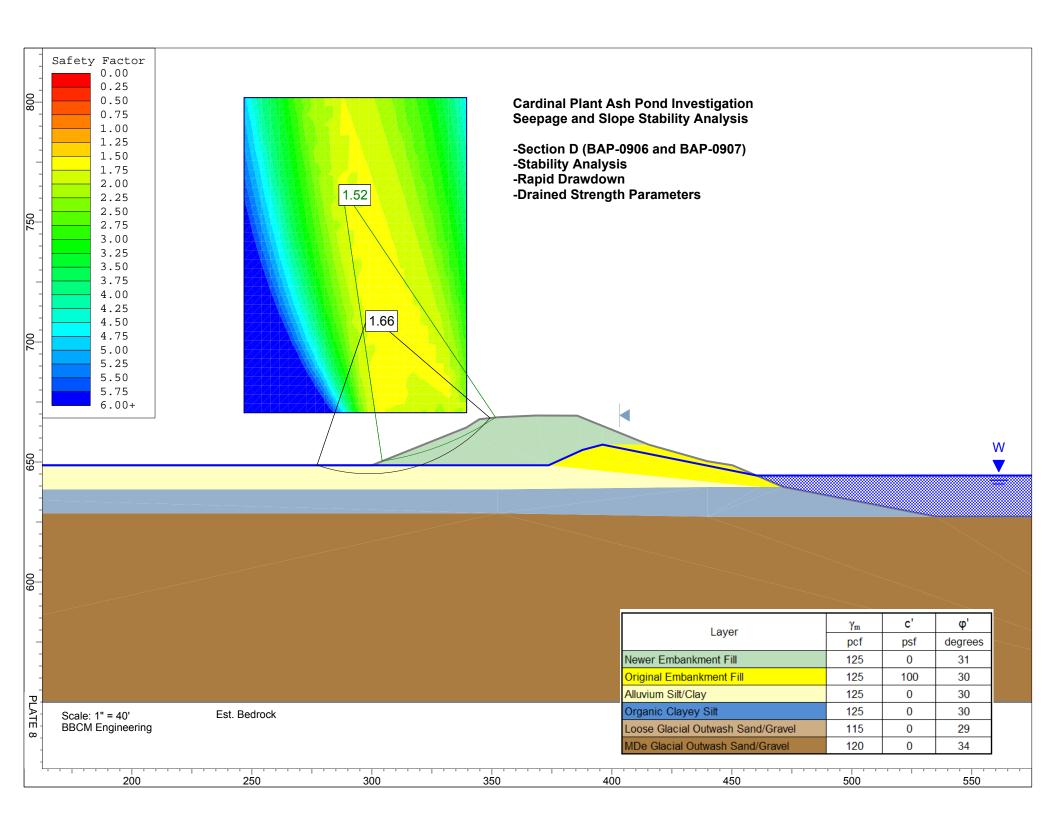












Fine Grained Soil Liquefaction Screening Cardinal Bottom Ash Pond

Layer: NEWER EMBANKMENT FILL

BORING	SAMPLE	SAMPLE	NATURAL	LIQUID	PLASTIC	PLASTIC	GRAVEL	SAND	SILT	CLAY	CLAY	SILT/CLAY	USCS
NUMBER	NUMBER	DEPTH	MOISTURE	LIMIT	LIMIT	INDEX				.005 mm	.002 mm		CLASSIFICATION
			CONTENT	%	%	%	%	%	%	%	%	%	
BAP-0901	S-5	7.75	16	28	18	10							
BAP-0901	S-9	13.75	13	27	17	10							
BAP-0901	S-12	18.25	14	37	24	13	7	32	49	23	12	61	SANDY LEAN CLAY CL
BAP-0902	S-11	16.75	24	37	19	18							
BAP-0902	S-12	18.25	21	35	17	18	8	37	33	28	21	54	SANDY LEAN CLAY CL
BAP-0902	S-13	19.75	31	29	17	12	1	20	62	28	17	79	LEAN CLAY with SAND CL
BAP-0904	S-9	13.75	16	35	21	14							
BAP-0906	S-3	4.75	15	27	17	10							
BAP-0906	S-8	12.75					30	40	22	13	9	31	
BAP-0906	S-11	17.25	14	31	19	12	18	44	26	18	12	38	CLAYEY SAND with GRAVEL SC

Fines Content	and Plasticity I	ndex Screening	
	% Passing	Is Soil Sample Liquefiable	
LL < 35	0.005 < 15	WC < 0.9LL	(meets all three criteria)
Yes	-	Yes	-
Yes	-	Yes	-
No	No	Yes	No
No	-	Yes	No
No	No	Yes	No
Yes	No	No	No
No	-	Yes	No
Yes	-	Yes	-
-	Yes	-	-
Yes	No	Yes	No

Layer: ORIGINAL EMBANKMENT FILL

5.05.010					B	B. 10510	0.5.11/51	0.1115		A. 117		AU =/AL 41/	
BORING	SAMPLE	SAMPLE	NATURAL	LIQUID	PLASTIC	PLASTIC	GRAVEL	SAND	SILT	CLAY	CLAY	SILT/CLAY	USCS
NUMBER	NUMBER	DEPTH	MOISTURE	LIMIT	LIMIT	INDEX				.005 mm	.002 mm		CLASSIFICATION
			CONTENT	%	%	%	%	%	%	%	%	%	
BAP-0903	S-2	3.25	24	48	24	24	0	8	60	45	32	92	LEAN CLAY CL
BAP-0903	S-5	7.75	20	36	20	16	0	14	58	38	28	86	LEAN CLAY CL
BAP-0905	S-3	4.75	17	32	18	14	0	25	53	30	23	76	LEAN CLAY with SAND CL
BAP-0905	S-5	7.75	22	48	24	24							
BAP-0907	S-5	7.75	23	49	26	23							
BAP-0907	S-6A	9.25	28	47	29	18	0	5	67	43	29	96	SILT ML

Fines Content	and Plasticity I	ndex Screening	
	% Passing		Is Soil Sample Liquefiable
LL < 35	0.005 < 15	WC < 0.9LL	(meets all three criteria)
No	No	Yes	No
No	No	Yes	No
Yes	No	Yes	No
No	-	Yes	No
No	-	Yes	No
No	No	Yes	No

Appendix V – 2009 Investigation Report Text

August 4, 2009 011-11497-013



Mr. Pedro Amaya, P.E. American Electric Power 1 Riverside Plaza Columbus, OH 43215

Re: Subsurface Investigation and Analysis

Bottom Ash Pond Embankments

AEP Cardinal Plant Brilliant, Ohio

Dear Mr. Amaya:

In accordance with our proposal dated March 23, 2009, and our signed contract dated March 25, 2009, BBC&M Engineering, Inc. (BBCM) has completed a geotechnical assessment of the embankment separating the Bottom Ash Complex from the Ohio River at the Cardinal Generating Plant in Brilliant, Ohio.

BBCM's scope of work, as developed by AEP, consisted of obtaining subsurface data at a total of four cross-sections through the bottom ash pond an recirculation pond embankments, and performing seepage and slope stability analyses to provide an indication as to the level of safety provided by the embankments. The following report is a summary of our investigation.

We appreciate having been given the opportunity to be of service on this project. If you have any questions, please do not hesitate to contact this office.

Respectfully submitted,

BBC&M ENGINEERING, INC.

Columbus, Ohio

Michael T. Romanello, E.I.

Staff Engineer

Michael G. Rowland, P.E.

Senior Engineer

Submitted: 4 bound copies

1 electronic copy on CDROM

Cardinal Generating Plant Bottom Ash Pond Investigation

Brilliant, Ohio

Report to

American Electric Power Service Corp. Columbus, Ohio

Prepared by

BBCM Engineering, Inc. Dublin, Ohio

August, 2009

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INTRODUCTION

The Cardinal Generating Plant is located along the Ohio river between Brilliant, Ohio and Tiltonsville, Ohio, as shown on the Vicinity Map, included as Plate 1 of Appendix A. The Bottom Ash Pond Complex is located along the west bank of the river just to the south of the main plant area. The Bottom Ash Complex consists of two components: the Bottom Ash Pond and the Recirculation Pond. The Bottom Ash Pond is located north of the Recirculation Pond and they are separated by an earthen embankment. The crest elevation for all of the embankments is approximately the same, but vary in Elevation from 668.6' to 669.4' at the surveyed cross sections. The total length of the exterior embankment along the Ohio River is approximately 2,000 feet. For comparison, the normal pool for this stretch of the Ohio River is El. 644. Both ponds are isolated from exterior surface water inflow.

SCOPE OF WORK

The purpose of this Geotechnical Assessment was to provide an indication as to the level of safety provided by the dam separating the ponds from the Ohio River. The work which was performed as part of the limited subsurface investigation consisted of 1) review of the original plans; 2) the performance of two soil borings each at four different locations (one at the crest and one at the toe); 3) conversion of four soil borings into observation wells; 4) the completion of laboratory testing on the recovered samples; and, 5) engineering analyses of the existing embankments with consideration to seepage, steady-state slope stability and seismic slope stability.

REVIEW OF HISTORICAL PLANS

The Site Development Plan for the Ash Storage Area and the corresponding Sections Plan (drawings numbers 3-3017-5 and 3-3027-3, respectively) from the ash pond vertical expansion in the 1970s were made available for review. The plans were developed in 1973 and include 'Record Drawing' information through 1978. The ash pond complex is believed to have been originally constructed in the 1960s when the plant was first brought online. BBCM also received an electronic drawing file of the plant, including topographic data, as depicted in the Plan of Borings presented as Plate 2 in Appendix A. The aerial survey used to develop the drawing file was performed in 1994.

Based on the historical cross-sections extending through both the Bottom Ash Pond and the Recirculation Pond from the vertical expansion, the original ash pond embankments along the Ohio River ranged in height from 4 to 6 feet above the bottom of the ash pond. Historical Sections 'A-A' and 'C-C' detail the vertical expansion plans for the embankment which was assessed during this investigation. These cross-sections are presented as Plates 1 and 2 of Appendix C. Based on the sections, the original embankment was raised by approximately 10 to 12 feet by constructing an earthen embankment on the inboard slope of the original embankments. The construction was intended to raise the crest from an approximate elevation of 658.0 feet to Elevation 670.0 feet. The approximate boundary of the original ash pond embankment is depicted on the historical cross-sections as well as the seepage and stability analysis graphic output.

GEOLOGY

The natural soils at the site generally consist of a layer of alluvium silt, clay and fine sand over glacial outwash deposits of variable thickness overlying the bedrock surface. The alluvium clays and silts were deposited in the backwater of the Ohio River, while the outwash materials typically consist of sand, gravel and silt deposits deposited during the last ice age. Based on geological literature, the glacial outwash extends to the bedrock surface, estimated to be roughly 60 feet below the natural ground surface at the pond. The upper most bedrock most likely consists of shale and/or sandstone belonging to the Conemaugh Group of Pennsylvanian Age.

FIELD WORK

Site Reconnaissance

On March 20, 2009, a Senior Engineer and a Project Engineer from our office performed a Dam and Dike Condition Survey and results were presented in the 2009 Inspection Report for the Ash Impoundment. During the condition survey, the locations of the critical cross sections determined by AEP were observed, and the proposed borings were staked in these areas. Additional information concerning the visual condition of the dam may be found in this report.

Soil Borings

During the period of April 6 through April 10, 2009, BBCM was on site and performed a total of seven (7) soil borings, designated CD-BAP-0901 through CD-BAP-0907, that were extended to depths ranging from 30.0 to 60.5 feet below existing grade. A 'PZ' designation was added to Borings CD-PZ-BAP-0902, 0904, and 0905 to indicate an observation well was installed within the borehole. For simplicity throughout this report, the borings are typically referred to with the 'BAP' (Bottom Ash Pond) designation only. Borings BAP-0901, 0902, 0904 and 0906 were located at the crest of the pond embankments and Borings BAP-0903, 0905, and 0907 were located at the outboard toe of the embankment slopes, and were placed to correspond with the crest borings. The boring location areas were selected by AEP and field located by BBCM. The boring locations are shown on the 'Plan of Borings' presented on a full size drawing as Plate 2 in Appendix A. All boring locations and elevations, as well as additional ground surface points near the borings were surveyed by AEP personnel to create surface profiles.

All borings were performed with either a truck-mounted drill rig or an all-terrain-vehicle (ATV) mounted drill rig and were advanced between sampling attempts using 3½-inch or 4½-inch I.D. hollow-stem augers. Disturbed, but representative samples were obtained by lowering a 2-inch O.D. split-barrel sampler to the bottom of the hole and driving it into the soil by blows from a 140-pound automatic hammer freely falling 30 inches (Standard Penetration Test, ASTM D1586). The automatic hammer used to advance the SPT sampler had previously been calibrated for energy transmission using dynamic pile monitoring methods. The energy calibration factor is included on the boring logs. SPT sampling was performed continuously through the embankment fill and at 2½-foot intervals once the native soil was encountered. Split barrel samples were examined immediately after recovery and representative portions of each sample were placed in air tight jars and retained for subsequent laboratory testing.

<u>Undisturbed Soil Samples</u>

In addition to the disturbed samples, thin-walled press tube samples ("Shelby" tubes) were also attempted at various depths in order to obtain relatively undisturbed soil samples for strength testing. The samples were collected by hydraulically pressing a 3-inch diameter thin-walled steel (Shelby) tube at the end of the drill rod stem into the soil at a uniform rate. The samples were preserved inside the Shelby tube sampler and sealed with wax. The sample collection was completed in accordance with ASTM D 1587 Method for Thin-Walled Tube Geotechnical Sampling of Soils. Two Shelby tube samples were obtained in Boring BAP-0901 and one Shelby tube sample was obtained in each of borings BAP-0903 and BAP-0906. It should be noted that several other attempts were made to obtain additional undisturbed samples but resulted in crushing the tube or no recovery.

Borehole Backfilling and Observation Wells

During and at the completion of drilling, groundwater readings were measured and recorded in each boring. In Borings CD-PZ-BAP-0902, 0904, and 0905, wells were installed to permit future groundwater readings. The wells consist of 2-inch diameter PVC, well casings and screens. Screens are nominal 10-foot lengths with 10-slotted openings. Quartz sand was used as a filter (where the surrounding soil does not consist of sand and gravel) and was placed to a level approximately 2 feet above the top of the well screen. A well seal consisting of approximately 2 feet of granular bentonite (3/8-inch hole plug) was set above the filter pack and the remainder of the annular space was filled with a bentonite slurry (benseal). A lockable steel cover was installed over the well and a 3 foot by 3 foot concrete pad was constructed to protect the exposed portion of the well which extends above the ground surface. Three to four steel bollards were installed around each concrete pad to protect the well.

During the installation of the wells, a surge block was used to densify the sand pack. Upon completion, each well was developed. Well development includes an attempt to hand bail 10 well volumes of groundwater from each well. Well Completion Diagrams are presented as Plates 23 though 25 of Appendix A. BBCM understands that all follow up groundwater level measurements will be obtained by AEP personnel. It is also understood that AEP will formally survey in the top of pipe for the three wells.

Recording of Field Data

In the field, the following procedures and specific duties were performed by a Staff Engineer or a Field Geologist from our office:

- examined all samples recovered from the borings;
- cleaned soil samples of cuttings and preserved representative portions in airtight glass iars:
- made seepage observations and measured the water levels in the borings;
- prepared a log of each boring;
- made hand-penetrometer measurements in soil samples exhibiting cohesion; and,
- provided liaison between the field personnel and the Project Manager so that the field investigation could be modified in the event that unexpected subsurface conditions were encountered.

At the completion of drilling, all samples were transported to the BBCM laboratory for further examination and testing.

LABORATORY TESTING

Index Testing

Laboratory testing was performed on selected representative soil samples obtained during the field investigations to determine natural moisture content (ASTM D2216), liquid and plastic limits (BBCM adjustment to ASTM D4318), and grain size analyses (ASTM D422). The results of these and other tests permit an evaluation of the strength, compressibility and permeability characteristics of the soils encountered at this site.

The results of the moisture content testing and of the liquid and plastic limits are graphically displayed on the individual boring logs presented in Appendix A. The results of all grain size analyses are also displayed graphically and presented as Plates 10 through 66 in Appendix B. All laboratory test results and a summary of laboratory test results are presented in Appendix B.

Table 1 summarizes the results of the index testing for the each layer except for the glacial outwash sand and gravel, where only a limited number of index testing was performed. For a comprehensive summary of all index testing performed, see Plates 3 through 7 of Appendix C.

Table 1. Summary of index values

Newer Embankment Fill

Statistic	MC	LL	PL	PI	CF
Sample Size	16	12	12	12	8
Minimum	10	25	16	9	8
Maximum	31	37	24	18	21
Mean	16.3	30.3	18.3	12.1	12.1
Median	15	29	17	11	11
Mode	16	27	17	10	12
Standard Deviation	5.4	4.5	2.3	3.2	4.6

Original Embankment Fill

Statistic	MC	LL	PL	PI	CF
Sample Size	10	6	6	6	4
Minimum	15	32	18	14	23
Maximum	33	49	29	24	32
Mean	22.5	43.3	23.5	19.8	28.0
Median	22	48	24	21	29
Mode	22	48	24	24	N/A
Standard Deviation	5.1	7.4	4.0	4.4	3.7

Alluvium Silt and Clay

Statistic	MC	LL	PL	PI	CF
Sample Size	10	4	4	4	10
Minimum	22	34	21	7	3
Maximum	38	38	28	15	28
Mean	29.0	36.0	23.5	12.5	11.0
Median	29	36	23	14	7
Mode	26	N/A	N/A	15	5
Standard Deviation	5.4	1.8	3.1	3.8	8.5

Organic Clayey Silt

Statistic	MC	LL	PL	PI	CF
Sample Size	22	18	18	18	21
Minimum	28	30	22	3	5
Maximum	54	50	38	20	44
Mean	41.8	40.2	27.1	13.2	18.9
Median	43	41	27	15	17
Mode	43	45	24	16	16
Standard Deviation	5.2	5.4	3.7	4.7	7.4

MC = Moisture Content; LL = Liquid Limit; PL = Plastic Limit; PI = Plasticity Index;

Specialty Testing

In addition to the above index tests, a three-point isotropically consolidated-undrained (CU) triaxial shear test (ASTM D4767) and a flex wall permeability test was performed on undisturbed soil samples obtained from Shelby Tube sampling. Results of all laboratory testing are included in Appendix B. Difficulties were encountered in obtaining undisturbed samples within the newer embankment fill due to the granular nature of the material. The CU triaxial test and permeability test were performed on undisturbed samples obtained within the alluvium and original embankment fill layers, respectively.

GENERAL SUBSURFACE CONDITIONS

Stratigraphy

Based on the descriptions of the samples recovered in the borings and laboratory testing, the subsurface stratigraphy for each section can generally be described in descending order from the top of the embankment as follows:

• The four borings which were performed from the crest of the embankments encountered 1.0 to 3.0 feet of roadway base consisting of bottom ash/boiler slab at the ground surface overlying 18.0 to 22.0 feet of embankment fill consisting of very stiff to hard silty clay and medium-dense to dense fine to coarse sand and gravel. Hand penetrometer measurements on samples exhibiting cohesion within this layer ranged from 2.5 to 4.5+ tons per square foot (tsf), while SPT N-values (corrected for 60% energy) ranged from 6 to 50 with an average of 26. Index testing results, including liquid limit and plasticity index of samples tested within this stratum are summarized in Table 1 of the previous section. The material was predominantly classified as Lean Clay (CL) to Clayey Gravel

CF = Clay-sized Fraction (% finer than 0.002 mm)

with Sand (GC) under the Unified Soil Classification System. Boring CD-PZ-BAP-0901 encountered a 4.5 foot thick zone of very-soft to very-stiff silty clay at the bottom of the fill. Hand penetrometer measurements within this zone ranged from 0.0 to 2.25 tsf.

- The three borings which were performed from the outboard toe of the embankments encountered 8.5 to 11.5 feet of embankment fill consisting of very-stiff to hard brown mottled with gray silty clay. The fill encountered in these borings is believed to be associated with the original pond embankments, and is denoted throughout this report as the 'Original Embankment Fill'. Hand penetrometer measurements on samples within this layer ranged from 1.6 to 4.5+ tons per square foot (tsf), while SPT N-values (corrected for 60% energy) ranged from 11 to 48 with an average of 22. Index testing results, including liquid limit and plasticity index of samples tested within this stratum are summarized in Table 1 of the previous section. The material was predominantly classified as Lean Clay (CL) under the Unified Soil Classification System.
- Underlying the embankments, the borings encountered 4.5 to 10.5 feet of alluvium consisting of very-loose to loose silt with few zones of stiff to hard silty clay and thin seams of very loose to loose fine to coarse sand. Hand penetrometer measurements on samples exhibiting cohesion within this layer ranged from 1.6 to 4.5+ tons per square foot (tsf), while SPT N-values (corrected for 60% energy) ranged from 0 to 33, with an average of 8. Index testing results, including liquid limit and plasticity index of samples tested within this stratum are summarized in Table 1 of the previous section.
- Beneath the alluvium silt and clay, the borings encountered 3.5 to 14.5 feet of very-soft to stiff organic clayey silt. Hand penetrometer measurements on samples exhibiting cohesion within this layer ranged from 0.0 to 1.25 tons per square foot (tsf), while SPT N-values (corrected for 60% energy) ranged from 0 to 20, with an average of 5. Index testing results, including liquid limit and plasticity index of samples tested within this stratum are summarized in Table 1 of the previous section. Loss on Ignition (LOI) values ranged from 7.9 to 10.4%. The material is predominantly classified as organic clay with sand (OL) under the Unified Soil Classification System. Throughout the report, this layer was identified as a clayey silt based on its consistency even though the PI often indicated the material would be classified as a silty clay
- All borings were terminated after penetrating 7.0 to 30.0 into feet very-loose to loose fine to coarse sand and/or medium-dense to dense brown fine to coarse sand and gravel. SPT N₆₀-values in the very-loose to loose sand ranged from 4 to 29 bpf with an average of 12. SPT N₆₀-values in the medium-dense to dense sand and gravel ranged from 14 to 69 bpf with an average of 32. The percent passing the 200 sieve ranged between 6 and 24, with an average of 12.2.

The newer embankment fill consisted of silty clay, sand, and gravel and was considered as a uniform stratum although the main descriptor varied based on the small variations in the percent by weight of each material. Strength parameters associated with this layer are discussed in the **Seepage and Stability Analysis** section. For a more detailed description of the stratigraphy, including the presence of minor variations and inclusions, the logs of the individual borings should be examined in conjunction with the summary above.

Groundwater

Groundwater observations were made as each boring was being advanced and measurements were made at the completion of drilling. The groundwater observations are graphically displayed on the boring logs and also noted at the bottom of the log. All water level readings indicated on the borings logs are referenced from the ground surface, as the top of pipes have not yet been formally surveyed. Extended groundwater measurements were made in the observation wells while on site and are summarized in Table 2.

Table 2: Extended Groundwater Measurements.

	Elevation During	Elevation at	Elevation on	Elevation on
Boring	Drilling	Completion	4-7/8-09	4-10-09
CD-BAP-0901	635.2	654.9		-
CD-PZ-BAP-0902	655.0	657.3	657.3	659.6
CD-BAP-0903	627.6	633.6		-
CD-PZ-BAP-0904	652.1	652.1		652.2
CD-PZ-BAP-0905	632.1	642.1	642.1	644.7
CD-BAP-0906	648.6	658.3		-
CD-BAP-0907	627.3	634.0		-

Elevation Datum: NAD 27 / NGVD 29

SEEPAGE AND STABILITY ANALYSIS

Embankment dams must exhibit adequate factors of safety against a slope stability failure for static and seismic conditions. As part of this project, BBCM considered four areas of the ash pond embankment along the river as deemed critical by AEP to analyze for stability. Each section was developed by performing one boring through the crest of the embankment and one boring at the outboard toe, with the exception of the southernmost section through the recirculation pond embankment, where the location of the proposed boring at the toe was inaccessible. The following sections of this report discuss the analyses that were performed, explain the rational supporting parameter selection and present the results.

Based on visual observations, the Recirculation Pond embankments appeared to be in 'Fair' condition while the Bottom Ash Pond appeared to be in "Good' Condition. The principal item which came out of this inspection relative to this report is that no evidence of slope failure or seepage was observed on the embankment slope between the pond and the river. It should be noted however, that the toe of the slope is inundated by the ordinary high water level of the Ohio River. The 2009 Inspection Report should be consulted for the complete assessment of the visual observations made for the Bottom Ash Complex.

Methodology

The seepage and stability analyses were performed with the aid of the computer program Slide (Version 5.0) developed by Rocscience, Inc. The program performs 2-D limit equilibrium slope stability analyses and steady-state unsaturated seepage analysis; the latter using the finite element method. Pore pressure values produced from the seepage analysis are used in the slope stability computations for each model.

Static and seismic slope stability analyses were performed on the outboard embankment slopes for Cross-Sections B and D using Spencer's method (Spencer, 1973) with a deterministic approach. Both methods provide solutions for given cross sections based on limit equilibrium theory. The five critical slip surfaces corresponding to the lowest factor-of-safety are shown in the graphical output. Seismic slope stability analyses were performed based on a pseudo-static slope stability approach. Stability calculations were performed in general accordance with the US Army Corps of Engineer's Engineering Manual 1110-2-1902 entitled *Slope Stability*.

Cross Sections

Cross-sections showing the general subsurface conditions encountered in the borings were developed based on the survey data provided by AEP. Table 3 summarizes the borings used to develop the four cross sections, which are shown individually on the Subsurface Cross Sections shown on a full size plan sheet as Plate 3 of Appendix A. Two cross-sections were chosen to carry out the seepage and stability analysis, and are considered representative of the cross-sections not used. It should be noted that no bathymetric data was available. As such, the portion of the slope located below the Ohio River normal pool was estimated. If bathymetric information becomes available in the future, it is recommended that the analysis cross-sections be reviewed.

Table 3: Cross Section Data

Cross-Section	Location	Crest Boring	Toe Boring
Section A	Recirculation Pond	CD-BAP-0901	-
Section B	Recirculation Pond	CD-PZ-BAP-0902	CD-BAP-0903
Section C	Bottom Ash Pond	CD-PZ-BAP-0904	CD-PZ-BAP-0905
Section D	Bottom Ash Pond	CD-BAP-0906	CD-BAP-0907

Although four separate cross-sections were examined, the parameters selected to represent the permeability and strength of both the original and newer embankment fill layers were kept the same between sections. Although there are minor differences when comparing the two layers between borings, it is believed that there is insufficient evidence to support delineating the parameters from section to section. Therefore, for the purposes of the seepage and slope stability analyses, the permeability and shear strength parameters used to represent the fill layers were based on the totality of test data available for the embankment across the entire site.

The natural alluvium soils underlying the pond embankments are somewhat variable, consistent with the depositional environment of such soils. As with the embankment fill, it is difficult to justify developing specific parameters for an individual cross-section, as the properties of this stratum may vary over short distances. As such, the parameters used to represent the alluvium, and similarly the organic clayey silt and glacial outwash layers, were based on the totality of test data available for these layers across the entire site.

At the time of the survey performed March 27, 2009, the pool levels in the recirculation pond and bottom ash pond were at EL. 663.1, and EL. 664.4, respectively. The resulting freeboard from the surveyed pool levels range from 4.3 - 5.1 feet and 5.6 - 5.8 feet for the recirculation and bottom ash ponds, respectively. It is understood that these levels represent the approximate normal operating pool level. The pool level in the Ohio River was recorded as Elevation 644.4 feet. The ordinary high water level of the river is believed to be EL. 644 at the site.

Seepage Analysis

The location of the groundwater table within the embankments was estimated based on extended groundwater readings taken from the observations wells and conditions encountered during drilling. Groundwater conditions used in the finite element model were then calibrated to match the observed conditions. Results from the seepage analysis provided pore pressure values within the model to be used in the Stability Analysis.

Hydraulic Properties

As previously indicated, the same modeled permeability values for the various soil layers were taken for both cross-sections based on the totality of information available for the site. A flex wall permeability test was performed on an undisturbed sample obtained within the original embankment fill layer yielding a vertical permeability of 7.4x10⁻⁸ cm/sec. The design value for permeability was increased to 5x10⁻⁷ cm/sec as a result of the calibration of the seepage models. Permeability values for the other strata were estimated from typical published values based on material description or correlations to grain size. Permeability values and anisotropic ratios were then adjusted during the seepage analysis to best match the observed groundwater conditions. Supporting calculations for the development of the permeability values are included in the *Slope Stability Shear Strength and Permeability Parameter Justification* section of Appendix C.

Permeability values assigned to the model layers are shown in the table below. Several layers were modeled with anisotropic permeability functions. The horizontal permeability (k_h) of the original embankment fill soils were estimated as 10 times the vertical permeability (h_v), to best model the stratification of the soil as a result of compacting the fill in horizontal lifts (Casagrande, 1937), but was adjusted to a ratio of 5 times during the analysis. Similarly, a k_h/k_v ratio of 2 was used for the newer embankment fill soils. The alluvium and organic clayey silt foundation layer were modeled with a horizontal permeability twice the vertical permeability to simulate the natural stratification and inclusion of fine sand seams. The remaining soil layers were defined as a granular material and were assigned isotropic permeability functions.

Table 4: Permeability Values

Material Description	Permeability		Reference	
iviateriai Description	k _v (cm/sec)	k_h / k_v	Releience	
Newer Embankment Fill	1x10 ⁻⁵	2	Grain Size Correlation	
Original Embankment Fill	5x10 ⁻⁷	5	Permeability Test	
Alluvium Silt and Clay	1x10 ⁻⁵	2	Typical Published Values	
Organic Clayey Silt	5x10 ⁻⁶	2	Typical Published Values	
Loose to Med Dense Glacial Outwash Sand and Gravel	1x10 ⁻²	1	Grain Size Correlation	
Med Dense - Dense Glacial Outwash Sand and Gravel	1x10 ⁻³	1	Grain Size Correlation	

Hydraulic Boundary Conditions

Topographic contours from the most recent survey as well as from historical construction drawings were used to expand the surface profile created from the AEP survey in order to develop a full scale model. The following boundary conditions were assigned to the finite element based models.

- A 'Constant Head' boundaries of 663.0 and 664.5' were used to represent the level of water in the recirculation pond and ash pond, respectively.
- The model was extended on the downstream side to the approximate middle of the Ohio River, and a 'Constant Head' boundary of 644.4' was used to represent the normal flow level of the river at this point (water level recorded by AEP).
- A 'No-Flow' boundary was placed on the upstream end of the model, as flow should become predominantly downward near the middle of the pond.
- A 'No-Flow' boundary was placed on the bottom of the model at Elevation 550' representing the approximate bedrock surface, which is assumed impermeable for this analysis.
- 'Unknown' boundary conditions were set on the remainder of the model to allow the program freedom to calculate values at these locations. These locations include the downstream slope face and the downstream ground surface.
- For Section D, the Constant Head Boundary of 644.4' was extended up the downstream slope to the location of the toe boring in an effort to model the observed groundwater conditions within the original embankment fill.

Finite Element Discretization and Mesh

The following steps were performed during the development of the seepage model:

- 6 Noded Triangles were used to generate the finite element mesh for the models (see Plates 2 and 7 of Appendix D).
- The density of nodes was manually increased to minimize the number of 'Poor Quality Elements' based on the Mesh Quality function available in Slide.
- Poor quality elements were defined as elements with one of the following characteristics:
 - 1. Maximum side length to minimum side length ratio greater than 10.
 - 2. Minimum interior angle less than 20 degrees.
 - 3. Maximum interior angle greater than 120 degrees.
- Prior to final computational runs, a sensitivity analysis was performed to determine if an adequate number of total finite element nodes were used in the analysis.
- A sensitivity analysis was performed on the tolerance of the computational iteration.

Seepage Analysis Models and General Results

Graphical output from the seepage analyses for Sections B and D are presented in Appendix D as Plates 3 and 4 for Section and B and Plates 8 and 9 for Section D. The calibrated seepage models produced phreatic surface shapes close to what was expected based on the water levels measured in the observation wells.

Although a typical phreatic surface extending from the ash pond level to the Ohio River was generated, much of the seepage emanating from the ponds is moving downward through the newer embankment fill and thin stratum of alluvium soils and into the glacial outwash sand and gravel stratum.

Stability Analyses

Shear Strength Parameters

In order to perform slope stability analyses, it was necessary to estimate appropriate parameters to represent the existing soils. The shear strength and unit weight values used for the slope stability analyses were based on a combination of the laboratory index test results, triaxial shear tests, published values and judgment, and are intended to be representative of long-term conditions. Table 5 lists the strength parameters used in both static and seismic analyses for each stratum. Supporting calculations for the development of these strength values are presented in the *Slope Stability Shear Strength Parameter Justification* section of Appendix C.

The percent of organic content in the Organic Clayey Silt layer was determined by performing Loss on Ignition (LOI) tests; results ranged from 7.9 to 10.4 percent. For LOI-values of less than 20 percent, the soil properties are controlled by the non-organic portion of the soil (FHWA, 2002).

Table 5: Strength \	Values for	Static	Conditions
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Material Description	Y _{wet} (pcf)	Strength		Reference
		φ'	c' (psf)	Reference
Newer Embankment Fill	125	31°	0	SPT and Index Testing Correlations
Original Embankment Fill	125	30°	100	Index Testing Correlations
Alluvium Silt and Clay	125	30°	0	Index Testing Correlations
Organic Clayey Silt	125	30°	0	Index Testing Correlations and CU Triaxial Test (BBCM 2009)
Very Loose to Loose Glacial Outwash Sand and Gravel	115	29°	0	SPT and Grain Size Correlations
Medium Dense Glacial Outwash Sand and Gravel	120	34°	0	SPT and Grain Size Correlations

In addition to the static steady-state stability analyses, strength parameters were developed for use with the pseudo-static seismic analyses. With respect to seismic loading, it is believed that the newer embankment fill soil is sufficiently granular that drained strengths values will be exhibited during seismic loading. However, as the original embankment fill is more cohesive in nature, it will likely exhibit an undrained response. As the embankment fill has come to equilibrium under the present steady-state seepage conditions, the shear strength envelope used in the analysis was based on the "R" test, as recommended in the Army Corps of Engineer's Manual 1110-2-1906 "Laboratory Soils Testing," and suggested by Duncan and Wright in their 2005 publication. This is essentially the slope and y intercept of the CU strength envelope. Unfortunately, CU triaxial tests were not performed in the newer embankment fill layer as all Shelby tubes attempted in this layer failed to recover an adequate sample size (however, a permeability test was performed). The seismic strength values for the newer embankment fill layer has been estimated based on values given by Duncan and Wright (2005) for soils with similar index properties (See Plate 16 of Appendix D). CU Triaxial test data was available for the Organic Clavev Silt laver, and the corresponding R envelope was used to model the shear strength. As there is a significant amount of sand within the alluvium strata, drained strength values were used for seismic loading.

Table 6: Strength Values for Seismic Conditions

Material Description	Ywet	Stre	ength	Reference
Material Description	(pcf)	ф	c (psf)	Reference
Newer Embankment Fill	125	31°	0	SPT and Index Testing Correlations
Original Embankment Fill	125	22°	50	Duncan and Wright (2005)
Alluvium Silt and Clay	125	30°	0	Index Testing Correlations
Organic Clayey Silt	125	24°	180	CU Triaxial Test (BBCM 2009)
Very Loose to Loose Glacial Outwash Sand and Gravel	115	29°	0	SPT and Grain Size Correlations
Medium Dense Glacial Outwash Sand and Gravel	120	34°	0	SPT and Grain Size Correlations

Analysis and Results

Static and seismic analyses were performed on Sections B and D to determine the factor of safety against rotational failure for the outboard slopes using drained soil strength parameters. The graphical computer outputs for these analyses have been included with this report in Appendix D.

Seismic analyses were performed using a pseudo-static analysis with a horizontal seismic coefficient of 0.06g. This coefficient was determined from the 2008 USGS National Seismic Hazard Maps for the "Peak Acceleration (%g) with 2% Probability of Exceedance in 50 Years". This chart is provided as Plate 33 of Appendix C.

Graphical results of the slope stability analysis for static and seismic conditions are shown in Appendix D. Table 7 summarizes the lowest factors of safety determined for each analysis case.

Table 7: Stability Analysis Summary

Analysis Coss	Required Minimum	Computed FS				
Analysis Case	Factor of Safety	Section B	Section D			
Static (Steady-State Seepage)	1.50	1.57	1.52			
Pseudo-Static	1.00	1.05	1.09			

The critical failure surfaces were located through a deterministic search, with no limitations on failure depth. The failure surface locations were restricted to find only surfaces associated with a global failure through the composite embankment (original plus newer embankment fill) or through the original embankment only. Shallow sloughing failures along the river bank were not considered for this analysis. The results are based on the pool level recorded at the time of the survey, extrapolated bathymetric data, and the groundwater measurements recorded from the observation wells.

CONCLUSIONS

As part of this report, BBCM examined the stability of the outboard embankment slopes at 4 locations under steady-state seepage and seismic loading conditions using the results of 7 soil borings. The analyses suggest that at the four cross sections examined, the embankments exhibit adequate factors of safety relative to those recommended by the US Army Corps of Engineers (COE).

REFERENCES

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Appendix VI – Excerpt from 2010 Follow-Up Investigation Report

INTRODUCTION

BBCM previously performed a limited subsurface investigation and slope stability analyses of the Cardinal Bottom Ash Pond Complex, the report of which was dated August 4, 2009. This report consisted of obtaining subsurface data at a total of four cross-sections through the bottom ash pond and recirculation pond embankments, and performing seepage and slope stability analyses to provide an indication as to the level of safety provided by the embankments.

The purpose of this follow-up work was to supplement the analyses performed as part of the original work in an attempt to fulfill the AEP action plan requirements in response to the USEPA inspection report. The follow-up slope stability analyses are solely based on existing subsurface data, as no additional field or laboratory work was performed as part of this project. Also as part of this follow-up work, hydraulic and hydrologic (H&H) analyses were performed to determine the capacity and freeboard of the Bottom Ash Pond related to current requirements. A summary of the work performed is contained in this report. This report should be considered an addendum to our August 4, 2009 Bottom Ash Pond Complex report.

SLOPE STABILITY ANALYSIS

Follow-Up Embankment Stability Analysis

Additional slope stability analyses were performed on Sections B and D to determine the factor of safety against rotational failure for the following conditions:

- 1.) Inboard slopes under steady-state seepage conditions;
- 2.) Pseudo-static seismic analyses under steady-state seepage conditions for the inboard slopes;
- 3.) Surcharge pool conditions (outboard slopes); and,
- 4.) Rapid drawdown analyses for the inboard slope.

The previously developed cross-section (B and D) geometry, permeability values, and shear strength parameters were used in the follow-up analysis. Please refer to the 'Subsurface Investigation and Analysis – Bottom Ash Pond Embankments' report by BBCM dated August, 2009 for a complete discussion of these parameters.

Seismic analyses for the inboard slopes were performed using a pseudo-static analysis with a horizontal seismic coefficient of 0.06g, consistent with the original report. The surcharge pool was modeled using a distributed line surcharge load, as it is not expected that the phreatic surface within the embankment will change during this temporary loading condition.

A rapid drawdown analysis was also completed for the bottom ash pond inboard embankment slopes utilizing the previously developed cross-sections. It is the understanding of BBCM that the ponds are typically filled with ash which would tend to support the inboard slopes. However, on an occasional basis, during times of ash removal and subsequent re-filling, a full pool of water could be established and a rapid drawdown scenario could occur if the pond were suddenly emptied. While not impossible, a large scale rapid drawdown event with unsupported interior slopes is unlikely. Notwithstanding, a rapid drawdown analysis was completed using the conventional method whereby the phreatic surface is positioned at the ground surface (inside the pond) and extended up into the slowly-draining embankment layers to the normal pool elevation. Drained strength parameters are used in this scenario. The drawdown level for the

Addendum: Ash Pond Investigation Cardinal Generating Plant Brilliant, Ohio BBC&M Engineering, Inc. analysis was considered to occur from the normal operating pool EI. 664.4 down to the natural ground surface on the inboard side of the embankment. During the subsurface investigation it was determined that there are two types of fill present in the embankments, identified as *newer embankment fill* and *original embankment fill*. The *newer embankment fill* contains a high percentage of sand and gravel (58%), as determined from previous laboratory testing. While pockets of this layer are cohesive and will exhibit a slowly-draining response during a rapid drawdown event, the layer as a whole likely will not maintain a consistent phreatic surface on the inboard slope. As a result, the phreatic surface was modeled to maintain its elevated level only within the *original embankment fill* and not within the *newer embankment fill*. Please see the analysis of the *newer embankment fill* layer submitted in Appendix B.

Graphical results of the slope stability analysis for static and seismic conditions are shown in Appendix A. Table 1 summarizes the lowest factors of safety determined for each analysis case.

Table 1: Stability Analysis Summary

Analysis Case	Required Minimum	Computed FS				
Allalysis Case	Factor of Safety	Section B	Section D			
Static (Steady-State Seepage) – Inboard Slope	1.50	1.70	1.65			
Pseudo-Static – Inboard Slope	1.00	1.39	1.34			
Maximum Surcharge Pool – Outboard Slope	1.40	1.55	1.52			
Rapid Drawdown – Inboard Slope	1.30	1.55	1.52			

The critical failure surfaces were located through a deterministic search, with no limitations on failure depth. The failure surface locations were restricted to find only surfaces associated with a global failure through the embankment. Shallow sloughing failures along the river bank were not considered for these analyses.

Liquefaction of Foundation Alluvium

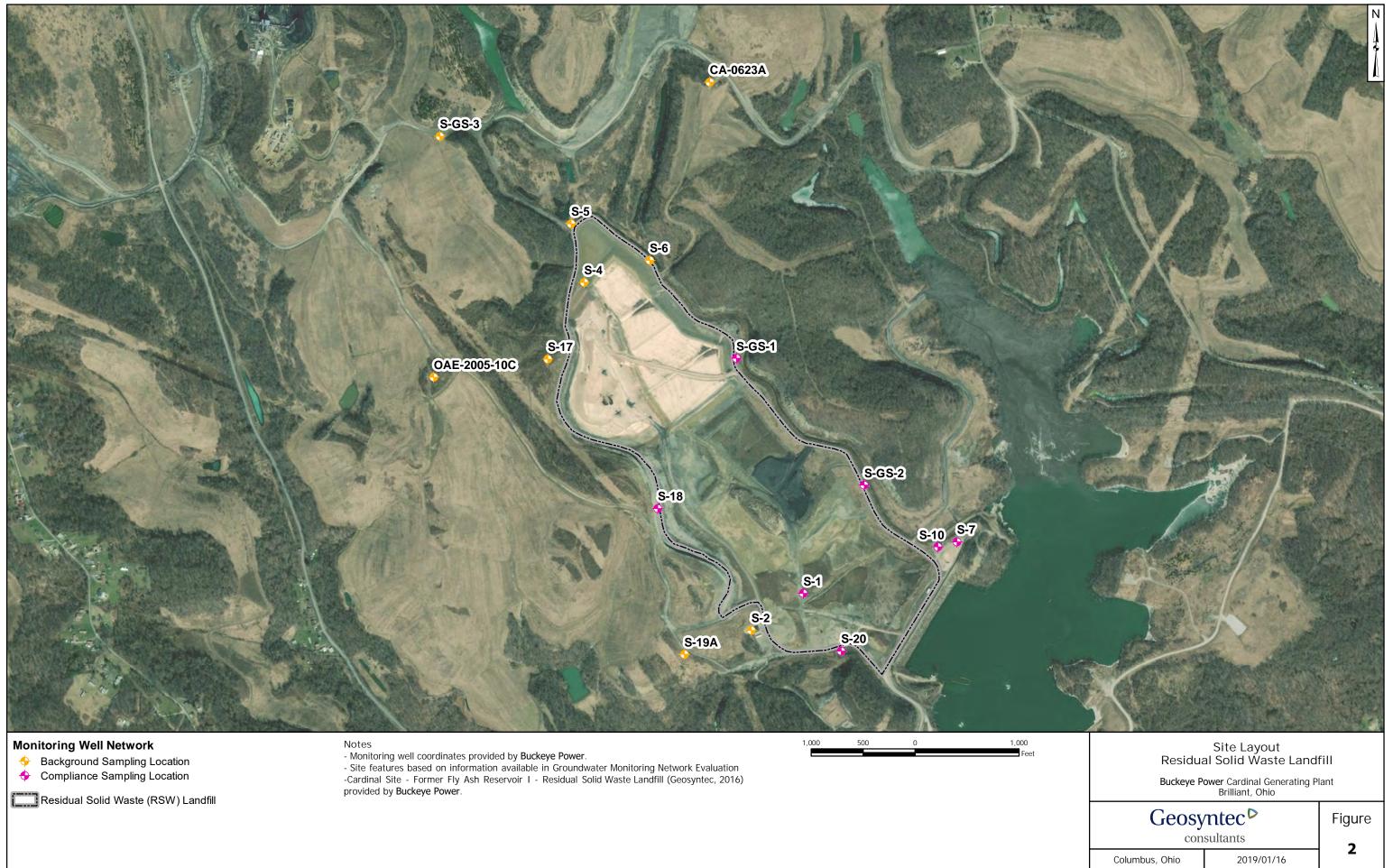
A liquefaction screening analysis was performed for the soft alluvium soils underlying the embankments. There is concern that areas of this layer could potentially liquefy during seismic excitation and ultimately cause a failure of the embankments. The screening analysis was performed using the five techniques listed in the Federal Highway GEC No. 3:

- 1.) Geologic Age and Origin,
- 2.) Fines Content and Plasticity Index,
- 3.) Saturation,
- 4.) Depth Below Ground Surface, and
- 5.) Soil Penetration Resistance.

The five screening techniques are described in detail in the hand calculations provided in Appendix B. Due to the fines content and plasticity index, as well as the geologic age and origin, the screening analysis suggests that liquefaction will not occur for the alluvium silt and clay layer.

FAR I RSW Landfill 40 CFR 257.101 (f)(1)(iv)(B)(2)(i)

Maps of Groundwater monitoring well locations in relation to CCR Unit



FAR I RSW Landfill

40 CFR 257.101 (f)(1)(iv)(B)(2)(ii)

Well construction diagrams and drilling logs for all groundwater monitoring wells

LOG OF BORING

Monitoring Well: CA-0623A

	Λ	7	\mathbf{E}	
3A			8	

JOB	NUM	BER _				-		LO	GO	P BOKING Monitoring we	i. C <i>F</i>	A-0023A
				N ELECTRIC						ORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SH		
				L LANDFILL						DRING START 4/10/06 BORING FINISH		
				6,291.1 E 2,5						EZOMETER TYPE WELL TYPE		
GRO	UND	ELEVAT	ION	1159.2 sy	STEM					GT. RISER ABOVE GROUND 2.281 DIA		
Wate	er Lev	el, ft	$\overline{\triangle}$	Ţ		Ā				PTH TO TOP OF WELL SCREEN354.9BOTTOM		
TIME	Ξ									ELL DEVELOPMENT BACKFILI		
DAT	E								FIE	ELD PARTY DLB / MCR / MWJ RIG	<u>D</u>)-120
		CAN	וחו ר	STANDARD		DOD						
SAMPLE	SAMPLE		IPLE PTH			RQD	DEPTH	GRAPHIC LOG	S	SOIL / ROCK	4	DRILLER'S
AMF	AMF	IN F	EET	PENETRATION RESISTANCE	SEST	%	IN	RA O	n s c	IDENTIFICATION	WELL	NOTES
ω z	S	FROM	TO	BLOWS / 6"	REGET		FEET	Ō	_			
		0.0	10.0				5	-				GROUNDING PROCEDURES NOT IN USE ON THIS BORING. BLIND DRILLED FROM GRADE TO 10' WITH 3 7/8" ROLLER BIT & SET 3" PVC CASING. STARTED CORING AT 10.0'
1	NQ	10.0	13.9		3.3		10 -			HARD N8 VERY LIGHT GRAY LIMESTONE w/ 1/2" clay bands in bottom 0.3'		
2	NQ	13.9	18.9		5.0		- 15 - -	- 1		HARD N8 VERY LIGHT GRAY LIMESTONE SOFT 5G 6/1 GREENISH GRAY SHALE		
FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ	18.9	23.9 • OF C	ASING USED	4.7		-			HARD 5R 4/2 GRAYISH RED SHALE Continued Next Page		
글	T					\dashv	DIES ::		-		- 05	
AN PER		NQ-2 R0 6" x 3.25	HSA	TNE			PIEZOM SI (E: PT = OPEN TUBE POROUS TIP, SS : SCREEN, G = GEONOR, P = PNEUMATIC	= OP	EN IUBE
9		9" x 6.25		VANCER	4"	-					4. ~	NEOMON!
8		NW CAS		VANUEN	3"		WELL T	YPE:	-0	W = OPEN TUBE SLOTTED SCREEN, GN	/I = G	BEUMUN
<u>ы</u> —	+	SW CAS			6" 8"					RECORDER		

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBE	=R			g			
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. CA-0622	DATE_ 7/17	'/15 SHEE	T 2	OF _	16
PROJECT	CARDINAL LANDFILL	BORING START	/10/06 BO	RING FINISH	6/1/06		

NUMBER	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-			5G 6/1 GREENISH GRAY LIMESTONE fractured throughout		
	NQ	23.9	33.9		9.7		-			5GY 6/1 GREENISH GRAY SHALE 5B 5/1 MEDIUM BLUISH GRAY SHALE		
							25 –			fractured N7 LIGHT GRAY LIMESTONE		
							-					
							-			5G 6/1 GREENISH GRAY SHALE 5G 6/1 GREENISH GRAY LIMESTONE fractured		
							30 –			5G 6/1 GREENISH GRAY SHALE		
							-			HARD 5B 5/1 MEDIUM BLUISH GRAY SHALEY LIMESTONE		
5	NQ	33.9	43.9		9.8		35 -			HARD 5B 5/1 MEDIUM BLUISH GRAY SHALEY LIMESTONE fractured in bottom 1.5'		
							-					
							40 -					
6	NQ	43.9	46.9		3.0		-			HARD 5B 5/1 MEDIUM BLUISH GRAY		
	110	70.8	-0.8		3.0		45 -			SHALEY LIMESTONE		

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEET <u>3</u> OF <u>16</u> PROJECT CARDINAL LANDFILL BORING START 4/10/06 BORING FINISH 6/1/06

NUMBER	SAMPLE	SAM DEF	PLE	STANDARD PENETRATION	STH VERY	RQD	DEPTH	GRAPHIC LOG	C S	SOIL / ROCK	Ë	DRILLER'S
NON S	SAM	IN F	TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	RECO	%	IN FEET	GRAF	S N	IDENTIFICATION	WELL	NOTES
7 N	NQ	46.9	53.9		7.0		-					
							50 -			5B 5/1 MEDIUM BLUISH GRAY SHALE	-	
8 N	NQ	53.9	63.9		9.6		55 -			HARD 55 5/1 MEDIUM BLUISH GRAY SHALEY LIMESTONE HARD N5 MEDIUM GRAY SHALEY LIMESTONE		
							60 -					
9 N	NQ	63.9	73.9		10.0		65 –			HARD 5B 5/1 MEDIUM BLUISH GRAY to N6 MEDIUM LIGHT GRAY SHALE		
							70 -			HARD N4 MEDIUM DARK GRAY SHALE small coal band @ 73.8	_	

LOG OF BORING

JOB NUMBER

Monitoring Well: CA-0623

1	V =	D
BA	\exists	-

COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 4 OF 16 BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH **DEPTH** F0G SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 10.0 HARD N6 MEDIUM LIGHT GRAY SHALE 10 NQ 73.9 83.9 w/ coal band @ 74.4, angle fracture @ 75.7 75 SOFT N4 MEDIUM DARK GRAY SHALE 80 HARD N2 GRAYISH BLACK SHALE COAL HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE 11 NQ 83.9 93.9 10.0 HARD N5 MEDIUM GRAY SHALE 85 90 HARD 5B 7/1 LIGHT BLUISH GRAY MIXED w/ **N6 MEDIUM LIGHT GRAY SHALE** w/ limestone nodules CD FGD LANDFILL.GPJ AEP.GDT 7/17/15 12 NQ 93.9 103.9 10.0 HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE 95

JOB	NUM	BER						LO	GΟ	F BORING		M	onitoring \	Well:	CA-0		
			ERICA	N ELECTRIC	POV	VER			во	RING NO. CA-0	0622	DATE 7	7/17/15	SHE	ET (5 OF	16
										RING START							
SAMPLE	SAMPLE	SAM DEF IN F	IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS		SOIL / F			- L	WELL	DRILLE NOTE	
		FROM	TO	BLOWS / 6"	<u> </u>					HARD 5B 7/1	LICHT BLU	IISH GDA	V SHALE				
							100 -			w/ sandstone s							
								-									
13	NQ	103.9	113.9		10.0		105 -			HARD N6 MEI w/ sandstone s							
							110 -			N8 VERY LIGI	HT GRAY LI	IMESTO	NE .				
							-			HARD N3 DAI							
							-	- 1		N7 LIGHT GR w/ 0.2 5B 5/1 111.6			shale band @				
14	NQ	113.9	123.9		10.0		-			N7 LIGHT GR							
							115 -			HARD 5GY 4/ SHALE							
										5GY 4/1 DARI							
. 1	1	I	I	I	1	1		-		IJAKU IND INE	PIOINI FIGUI	I GRAI	JIIALE	- 1			

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

w/ sandstone streaks

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER ___ DATE_<u>7/17/15</u>___ SHEET <u>6</u>__ OF ___ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** LOG SOIL / ROCK DRILLER'S WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO NQ 123.9 133.9 10.0 HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE bottom 0.8 N3 dark gray carbonious 125 130 **N5 MEDIUM GRAY FINE GRAIN SANDSTONE** w/ shale band NQ 133.9 143.9 10.0 HARD N5 MEDIUM GRAY SHALE 16 135 COAL w/ hard shale bands N4 MEDIUM DARK GRAY SHALE w/ 0.5 of carbonious shale at 142.0, bottom 1.9 hard 17 NQ 143.9 153.9 10.0 HARD N6 MEDIUM LIGHT GRAY SHALE **N8 VERY LIGHT GRAY LIMESTONE** FGD LANDFILL.GPJ AEP.GDT 7/17/15 145 HARD N6 MEDIUM LIGHT GRAY SHALE **N8 VERY LIGHT GRAY LIMESTONE** w/ 0.3 shale bands @ 147.8 & 152.4 8

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **7** OF BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL SOIL / ROCK DRILLER'S WELL LOG SC IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 153.9 163.9 6.2 68 HARD N6 MEDIUM LIGHT GRAY LIMESTONE SWL 21.4' on 18 NQ 04/17/06 w/ NQ 155 HOLE TO 153.9'. USED ±4.000 GALS. WATER TO THIS **POINT** HARD N6 MEDIUM LIGHT GRAY FRACTURED LIMESTONE HARD N5 MEDIUM GRAY SHALE/LIMESTONE SOFT N5 MEDIUM GRAY SHALE/LIMESTONE **LOST ALL WATER RETURN AT 157.8'.** HARD N5 MEDIUM GRAY SHALE/LIMESTONE HYD. PUSH - NO **ROTATION FROM** 163.9' - 165.9' 160 (VOID) NQ 163.9 168.9 VOID 19 1.9 84 165 SOFT 5B 5/1 MEDIUM BLUISH GRAY SHALE 20 NQ 168.9 170.9 1.3 0 SOFT N5 MEDIUM GRAY SHALE wet 170 Stopped after going through mine void. LANDFILL.GPJ AEP.GDT 7/17/15 NQ HARD N6 MEDIUM LIGHT GRAY SHALE Started drilling HW 21 170.9 178.9 7.9 67 casing and cleaning SOFT N4 MEDIUM DARK GRAY SHALE inside of casing w/ 4" fractures throughout roller bit. At 155', roller bit broke off inside casing. It was decided to abandon HARD N6 MEDIUM LIGHT GRAY SHALE and grout this boring. fractured Moved east +/- 5" 175 and started drilling FGD new boring w/6" air 8

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **8** OF BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 __ BORING FINISH 6/1/06 **BORING START** SAMPLE **STANDARD** RQD SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION SOIL / ROCK DRILLER'S WELL LOG SC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO HARD N7 LIGHT GRAY SHALE hammer and inserted HW casing to bottom old mine floor @ 173.3'. This boring was drilled through mine piller; no camera work done on this 178.9 186.9 SOFT N7 LIGHT GRAY SHALE 22 NQ 6.6 56 boring. Coal seam w/ fracture estimated @ +/-180 SOFT N6 MEDIUM LIGHT GRAY SHALE 165.0'-17 SOFT N6 MEDIUM LIGHT GRAY SHALE w/ fracture, wet HARD N7 LIGHT GRAY SHALE dry **N7 LIGHT GRAY CLAY SHALE** HARD N7 LIGHT GRAY CLAY SHALE 185 N4 MEDIUM DARK GRAY SHALE NQ 186.9 189.4 2.5 88 **VERY HARD N6 MEDIUM LIGHT GRAY** Resumed coring and SHALE logging core @ w/ trace of fine limestone 186.9' **N5 MEDIUM GRAY SHALE** 24 NQ 189.4 194.4 5.0 40 fracture, wet 190 **N6 MEDIUM LIGHT GRAY** SHALE/LIMESTONE **SOFT MEDIUM GRAY SHALE** MEDIUM LIGHT GRAY SHALE **SOFT N5 MEDIUM GRAY SHALE** moist 204.4 10.0 5B 5/1 MEDIUM BLUISH GRAY SHALE 25 NQ 194.4 83 195 HARD N5 MEDIUM GRAY SHALE fracture FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N5 MEDIUM GRAY SHALE 200 8

LOG OF BORING

Monitoring Well: CA-062

JOR MOMBE	<u> </u>		IV	onitoring well:	CA-062	5A —	
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. CA-0622	2 DATE 7	7/17/15 SHE	ЕТ 9	OF _	16
PROJECT _	CARDINAL LANDFILL	BORING START	1/10/06	BORING FINISH	6/1/06		

PRU	JECT	_CAI	DINA	_ LANDFILL						RING START <u>4/10/06</u> BORING FINISH	' -	0/1/00	
SAMPLE NUMBER	SAMPLE	SAM DEF IN F	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S	3
							-			N5 MEDIUM GRAY SHALE \[\text{fracture, wet} \] HARD N5 MEDIUM GRAY SHALE			
26	NQ	204.4	214.4		8.7	64	205 -			HARD N4 MEDIUM DARK GRAY SHALE			
							-			5G 6/1 GREENISH GRAY SHALE w/trace of fine imestone, wet N2 GRAYISH BLACK SHALE			
							-			fractured SOFT N4 MEDIUM DARK GRAY SHALE N2 GRAYISH BLACK SHALE			
							-			fracture N5 MEDIUM GRAY SHALE			
							210 -			fracture, wet			
							-			5G 6/1 GREENISH GRAY SHALE			
							-			5G 6/1 GREENISH GRAY SHALE wet			
27	NQ	214.4	219.4		5.0	66	215 -			5GY 6/1 GREENISH GRAY SHALE/LIMESTONE			
							-			N5 MEDIUM GRAY SHALE SOFT 5YR 6/1 LIGHT BROWNISH GRAY SANDY SHALE			
28	NQ	219.4	229.4		9.9	81	220 –	, ,		HARD 5B 5/1 MEDIUM BLUISH GRAY SHALE w/limestone fractures			
							-			5B 5/1 MEDIUM BLUISH GRAY SHALE w/limestone			
							225 -						
										N4 MEDIUM DARK GRAY SHALE fractured, wet			
										Continued Next Page			

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 BORING FINISH 6/1/06 **BORING START** STANDALL
PENETRATION ZED SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S WELL LOG SC IN FEET **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO 5B 5/1 MEDIUM BLUISH GRAY SHALE/ NQ 229.4 238.8 29 LIMESTONE 230 fracture **N4 MEDIUM DARK GRAY SHALE** HARD MEDIUM DARK GRAY SHALE w/limestone 235 MEDIUM DARK GRAY LIMESTONE shale fractures HARD DARK GRAY LIMESTONE NQ 238.8 244.4 HARD N4 MEDIUM DARK GRAY SHALE 240 N2 GRAYISH BLACK COAL \fracture SOFT N4 MEDIUM DARK GRAY SHALE HARD N4 MEDIUM DARK GRAY SHALE/LIMESTONE NQ 244.4 254.4 **5B 5/1 MEDIUM BLUISH GRAY SHALE** 245 5B 5/1 MEDIUM BLUISH GRAY SHALE w/limestone fractures SOFT 5GY 6/1 GREENISH GRAY SHALE w/limestone, wet 250 N5 MEDIUM GRAY & 5YR 4/1 BROWNISH **GRAY SHALE** 5B 5/1 MEDIUM BLUISH GRAY SHALE

Continued Next Page

FGD LANDFILL.GPJ AEP.GDT 7/17/15

8

JOB	NUM	BER _				_		LO	00	Monitoring Well: CA-	0623A
COM	IPAN'	Y AM	ERICA	N ELECTRIC	POW	VER			ВС	DRING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEET _	11 OF 16
PRO	JECT	CAF	RDINA	L LANDFILL					ВС	DRING START 4/10/06 BORING FINISH 6/1	/06
								, ,			
SAMPLE	SAMPLE	DEI	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	N S C S	SOIL / ROCK IDENTIFICATION	DRILLER'S NOTES
32	NQ	254.4	264.4				255 -			SOFT MEDIUM BLUISH GRAY SHALE	
										HARD 5GY 6/1 GREENISH GRAY SHALE	
							000			w/fractures of limestone	
							260 -			5YR 4/1 BROWNISH GRAY RED SHALE	
										MEDIUM BLUISH GRAY SHALE	
								-		w/fractures of limestone	
33	NQ	264.4	274.4				265 -			N4 MEDIUM DARK GRAY SHALE	
										SOFT N4 MEDIUM DARK GRAY SHALE wet	
							270 -				
34	NQ	274.4	284.4				275 -			SOFT N4 MEDIUM DARK GRAY SHALE	
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15										N7 LIGHT GRAY & N4 MEDIUM DARK GRAY SHALE W/trace of limestone	
LANDFILL											
AEP CD_FG[Continued Next Page	

LOG OF BORING

Monitoring Well: CA-0623A

JOB NUMBER __ DATE <u>7/17/15</u> SHEET <u>12</u> OF __ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 PROJECT CARDINAL LANDFILL 4/10/06 ___ BORING FINISH _6/1/06 **BORING START** SAMPLE STANDARD RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION LOG SOIL / ROCK WELL DRILLER'S USC IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO N4 MEDIUM DARK GRAY SHALE/LIMESTONE HARD SHALE NQ 284.4 294.4 N4 MEDIUM DARK GRAY SHALE 285 w/fractures of limestone HARD N3 DARK GRAY SHALE 290 HARD N4 MEDIUM DARK GRAY SHALE 36 NQ 294.4 304.4 295 300 37 NQ 304.4 314.4 10.0 100 305

Continued Next Page

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

JOB NUMBER

LOG OF BORING

Monitoring Well: CA-0623A

				N ELECTRIC L LANDFILL	POV	/ER			ORING NO. <u>CA-0622</u> DATE <u>7/17/15</u> SHEE ORING START 4/10/06 BORING FINISH		
SAMPLE			IPLE PTH EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	<u>-</u>		DRILLER'S NOTES
							310 -				
38	NQ	314.4	324.4		10.0		315 -	-	N4 MEDIUM DARK GRAY SHALE N4 MEDIUM DARK GRAY & N6 MEDIUM LIGHT GRAY SHALE		
							320 -		N4 MEDIUM DARK GRAY SHALE w/traces of fine standstone lens N5 MEDIUM GRAY SHALE		
39	NQ	324.4	334.4		10.0		325 -		W/trace of fine sandstone HARD MEDIUM GRAY & MEDIUM DARK GRAY SHALE W/trace of coarse sandstone		
CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15							330 -		N5 MEDIUM GRAY COARSE GRAIN SANDSTONE HARD N3 DARK GRAY SHALE w/trace of sandstone		
AEP CD_FG									N5 MEDIUM GRAY COARSE GRAIN Continued Next Page	MOR	GANTOWN

JOB	NUMI	BER						LO	GΟ	F BORING Monitoring We	II: CA	A-0623A
			ERICA	N ELECTRIC	POV	VER			ВС	PRING NO. <u>CA-0622</u> DATE <u>7/17/15</u> S	HEET	14 OF 16
				LANDFILL						PRING START 4/10/06 BORING FINIS		
		I										
щЖ	щ	SAM		STANDARD	그돈X	RQD	DEPTH	2	S	COIL / DOCK	١.	DDILL EDIC
SAMPLE NUMBER	SAMPLE	DEF IN F		PENETRATION RESISTANCE	P S S S S S S S S S S S S S S S S S S S	%	IN	GRAPHIC LOG	၁ ၀	SOIL / ROCK	WELL	DRILLER'S
SAUN	SA	FROM	TO	BLOWS / 6"		90	FEET	GR L	⊃	IDENTIFICATION	>	NOTES
				22011070						SANDSTONE		SANDSTONE
							-			Morgantown sandstone starts @ 331.5'		STARTS @ 331.5'
							-					
40	NQ	334.4	344.4		10.0		335 -			N6 MEDIUM LIGHT GRAY SANSDSTONE		
							000			HARD N3 DARK GRAY SHALE W/trace of fine sandstone		
							-					
							_			N2 GRAYISH BLACK SHALE		
							-					
							_					
							340 -					
							-			N5 MEDIUM GRAY COARSE GRAIN SANDSTONE		
							-			HARD N2 GRAYISH BLACK SHALE	-	
										w/trace of fine sandstone		
							-					
41	NQ	344.4	354.4		9.8	92	345 -			N5 MEDIUM GRAY COARSE GRAIN		
							345			SANDSTONE W/trace of dark shale		
							-			HARD N4 MEDIUM DARK GRAY SHALE		
										w/trace of fine sandstone		
							-					
							-					
							-					
							350 -					
							-					
							-					
										MEDIUM GRAY SANDSTONE	-	
							-			w/dark shale fractures		

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

42 NQ 354.4

364.4

9.7

91

355

N6 MEDIUM LIGHT GRAY COARSE GRAIN

SANDSTONE

fracture

GRAYISH BLACK COAL

JOB	NUMI	BER _			, ,_			LO	G O	F BORING	Monitoring Wel	II: CA	-0623		
СОМ	PAN	/ <u>AM</u>	ERICA	N ELECTRIC	POV	VER			ВС	RING NO. <u>CA-0622</u> DATI	E_ 7/17/15 SH	HEET _	15	OF _	16
PRO	JECT	CAF	RDINA	L LANDFILL					ВС	RING START	BORING FINISH	⊣ _6/	1/06		
		SAM				RQD	DEPTH IN FEET								
PLE BER	PLE	DEF		PENETRATION	FER		DEPTH	일	C S	SOIL / ROCK	(크	D	RILLEF	R'S
SAMPLE NUMBER	SAM	IN F	EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"		%	IN	LO LO	USC	IDENTIFICATIO	NC	WELL		NOTES	3
0, 2	0,	FROM	TO	BLOWS / 6"	78		FEET								
										N6 MEDIUM LIGHT GRAY COA SANDSTONE	ARSE GRAIN				
							-			5, 11, 15 ° 1 ° 1 ° 1					
							360 -								
							300			N5 MEDIUM GRAY SHALE					
							-								
							_								
							-								
							-	V V							
43	NQ	364.4	373.4		10.0	90	365 -	× ×		N6 MEDIUM LIGHT GRAY SIL	TSTONE				
								× × × ×							
							-	X							
							-	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$							
								× × × × × ×							
							-	× × ×							
							-	× × × ×							
								× × ×							
							370 –	× × × × × ×							
							_	× × × ×							
								× ×							
							-	× × × ×							
							_			HARD N5 MEDIUM GRAY SHA	ALE				
44	NQ	373.4	383.4		10.0	81				HARD N3 DARK GRAY CLAY	SHALE				
							-								
							375 -								
							0.0								
							-			N2 GRAYISH BLACK CLAY SI	HALE SEAM				
							-								
								3		N1 BLACK COAL SEAM					
							-			HARD N5 MEDIUM GRAY CLA	AY SHALE				
							-								
							380 –	=							
1				I	I							1			

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

Continued Next Page

STOPPED BORING

LOG OF BORING

JOB NUMBER

Monitoring Well: CA-0623A

		ı		
	14	1	7	
Α				

__ DATE <u>7/17/15</u> SHEET <u>16</u> OF <u>16</u> COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0622 PROJECT CARDINAL LANDFILL BORING START 4/10/06 BORING FINISH 6/1/06 PENETRATION RESISTANCE BLOWS / 6" SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO @ 383.4'. SET 1" **GEOMON WELL** CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-17

Λ	<u> 35</u>

			BER _ ⁄ ΔM	FRICA	N ELECTF	RIC POV	_ VFR			R(DRING NO. <u>CA-0601</u> DATE <u>7/17/15</u> SH	HEET	1 OF 17
					L LANDFIL						DRING START 6/5/07 BORING FINISH		
С	OOF	RDIN	ATES _	N 833	3,612.2 E						EZOMETER TYPE N/A WELL TYPE		
G	RO	UND	ELEVAT	TON	1195.6	SYSTEM				Н	GT. RISER ABOVE GROUND 2.369 DIA	A _2	
٧	Vate	r Lev	el, ft	∇	Ţ		Ā	-		DE	EPTH TO TOP OF WELL SCREEN	1 <u>1</u>	99.8
Т	IME										ELL DEVELOPMENT YES BACKFIL		UICK GROUT
С	ATE	=								FII	ELD PARTY MCR / MWJ RIG	3 <u>D</u>)-120
Г			CAN	וחו ר	CTANDADI		DOD						
A MDI E	NUMBER	SAMPLE	DEF IN F		PENETRATION RESISTANCE	TOTAL TOTAL ENGTH	RQD %	DEPTH IN FEET	SRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
	1	NQ NQ	14.0 15.5	15.5 25.5	BLOWS / 6	3.6	42	5 - 10 - 15 -			SOFT 5Y 6/1 LIGHT OLIVE GRAY CLAY SOFT CLAYEY LIMESTONE HARD 10YR 7/4 GRAYISH ORANGE LIMESTONE		GROUNDING PROCEDURES NOT IN USE ON THIS BORING; WATER FOR DECONNING AND DRILLING FROM CARDINAL FIRE PROTECTION SYSTEM; DECONED RIG & TOOLS 05/05/07; BLIND DRILLED HW 4" CASING TO START CORING @ 14.0'; MOVED +/- 6' NORTH WHERE TOM DICK HAD THE BORING STAKED.
J AEP													RETURN @ 19.0'
FILL.GP					ASING US	ED					Continued Next Page		
ANDF			NQ-2 R0 6" x 3.25	OCK CO S HSA	RE						PEN TUBE		
1 09			9" x 6.25	HSA	VANCER	4"					SCREEN, G = GEONOR, P = PNEUMATIC W = OPEN TUBE SLOTTED SCREEN, GN		SEOMON!
8			NW CAS	SING	7/1140LI	3"		WELL T	/ı = C	DEUIVIUN			
AEP	Х		SW CAS AIR HAN			6" 8"					RECORDER		

LOG OF BORING

Monitoring Well: S-17

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 2 OF _ BORING NO. CA-0601 PROJECT CARDINAL LANDFILL BORING FINISH 6/12/07 BORING START 6/5/07

1 K	۳ ا	SAM		STANDARD PENETRATION RESISTANCE BLOWS / 6"	HH. HY	RQD	DEPTH	일	S	SOIL / BOCK		חסוו ו בסיס
NUMBER	SAMPLE	DEF IN F		RESISTANCE	OTA SOVE	0/_	IN	GRAPHIC LOG	SC	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
z	SA	FROM	ТО	BLOWS / 6"	THE H	70	FEET	RP _	Π	IDENTIFICATION	>	NOTES
							-	Ш				
							-	\Box				
							-	H				
							-	H				
							25 -					
3	NQ	25.5	35.5		2.1	48				5YR 6/4 LIGHT BROWN SANDY SHALE		
							-					
							-					
							-	Н		HARD N8 VERY LIGHT GRAY LIMESTONE		
							-			HARD N8 VERY LIGHT GRAY LIMESTONE		
							00	П		w/iron staining		
							30 –	H		SOFT 5YR 4/1 BROWNISH GRAY SHALE HARD N6 MEDIUM LIGHT GRAY LIMESTONE		
							-			THAT NO MEDICIN EIGHT GIVET EIMEGTONE		
							-					
							-	廿				
							-	Ħ				
							35 -			5YR 5/6 LIGHT BROWN SAND		
4	NQ	35.5	45.5							SOFT 5B 5/1 MEDIUM BLUISH GRAY SHALE		
							-					
							-					
							40 -					
							40			HARD N6 MEDIUM GRAY SHALE		
							-	H		HARD N6 MEDIUM GRAY SHALE w/iron staining		
							_	Ξ		HARD N7 MEDIUM LIGHT GRAY LIMESTONE		
								H				
							-	Щ				
							_	H				
								H				
							45 -	井				
5	NQ	45.5	52.5		6.5	42		ПÏ		5YR 7/2 GRAYISH ORANGE PINK		



JOB NUMBER _____ LOG OF BORING Monitoring Well: S-17

COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0601 DATE 7/17/15 SHEET 3 OF 17

PROJECT CARDINAL LANDFILL BORING START 6/5/07 BORING FINISH 6/12/07

PRO	JECI	CAR	IDINA	L LANDFILL					BO	RING START <u>6/5/07</u> BORING FINISI	1 <u>0</u>	112/01
SAMPLE NUMBER	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-			Vertical fracture w/iron staining SOFT 5G 6/1 GREENISH GRAY SHALE		
							50 –			SOFT 5G 6/1 GREENISH GRAY SHALE Wiron staining SOFT 5B 5/1 MEDIUM BLUISH GRAY SHALE		
							-			HARD 5B 5/1 MEDIUM BLUISH GRAY LIMESTONE		
6	NQ	52.5	58.5		6	52	-			w/ vertical fracture 5YR 4/4 MODERATE BROWN SANDY SHALE w/iron staining N5 MEDIUM GRAY SHALE		SWL DRY; NQ HOLE TO 52.5
							55 -			MEDIUM LIGHT GRAY LIMESTONE		
7	NQ	58.5	60.5		2	25	-			VERY SOFT MEDIUM GRAY SHALE HARD MEDIUM GRAY SHALE HARD MEDIUM LIGHT GRAY SHALE		
8	NQ	60.5	70.5		10	70	60 -			N5 MEDIUM GRAY SHALE		
							-			N5 MEDIUM GRAY SHALE w/ vertical fracture		
							65 –			LIMESTONE		
							-					
							70 -					
9	NQ	70.5	76.5		6	47	-			HARD N5 MEDIUM GRAY SHALE		

AEP CD_FG

LOG OF BORING

Monitoring Well: S-17

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 4 OF _ BORING NO. CA-0601 PROJECT CARDINAL LANDFILL BORING FINISH 6/12/07 BORING START 6/5/07

PRO	JECT	CAF	KUINA	L LANDFILL					ВО	RING START 6/5/07 BORING FINISH	6/1	2/07
SAMPLE NUMBER	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-			N5 MEDIUM GRAY LIMESTONE w/ vertical fracture		
10	NQ	76.5	85.5		9	44	75 — - -			N4 MEDIUM DARK GRAY SHALE Wiron staining N7 LIGHT GRAY LIMESTONE HARD N4 MEDIUM GRAY CLAY SHALE		
							80 -					
							- -			N4 MEDIUM GRAY CLAY SHALE w/ broken areas		
11	NQ	85.5	95.5		9	82	85 -			HARD N6 MEDIUM LIGHT GRAY CLAY SHALE		
							- - -	-				
							90 -			BROKEN CLAY SHALE HARD N6 BROKEN CLAY SHALE		
							-			BROKEN CLAY SHALE		
12	NQ	95.5	105.5		10	60	95 -			HARD N6 BROKEN CLAY SHALE w/vertical fracture HARD N5 MEDIUM GRAY SHALE		
							-			N5 MEDIUM GRAY BROKEN CLAY SHALE HARD 5YR 4/1 BROWNISH GRAY CLAY SHALE		

8

J	BC	NUMI	BER						LO	GΟ	F BORING	Monitoring Well: S-	17			
С	OM	PAN	_ _ AM	ERICA	N ELECTRIC	POV	/ER			ВО	RING NO. CA-0601	DATE 7/17/15 SI	HEET	5	OF	17
					L LANDFILL					ВО	RING START 6/5/07					
Z MDI E	NUMBER	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / F		WELL	С	NOTE:	
	12	NO	105 5	444.5		F.G.	- F4	100 -			5YR 4/1 BROWNISH GRASHALE					
		NQ	111.5	120.5		9	54	110 -			N1 BLACK COAL N2 GRAYISH BLACK CL. SOFT N4 MEDIUM DARK	AY SHALE				

115

120

10.3 51

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

15 NQ 120.5

130.5

Continued Next Page

N2 GRAYISH BLACK CLAY SHALE
N4 MEDIUM DARK GRAY CLAY SHALE

HARD N6 MEDIUM LIGHT GRAY CLAY

N4 MEDIUM DARK GRAY CLAY SHALE

N6 LIGHT GRAY LIMESTONE

N6 LIGHT GRAY LIMESTONE

SHALE

LOG OF BORING

Monitoring Well: S-17

AEP

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ DATE_**7/17/15**__ SHEET **_6**__ OF __ BORING NO. CA-0601 PROJECT CARDINAL LANDFILL 6/5/07 BORING FINISH 6/12/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO SOFT N5 MEDIUM DARK GRAY CLAY SHALE 125 -**N6 FINE GRAIN SANDSTONE N6 FINE GRAIN SANDSTONE & CLAY SHALE** 130 NQ 130.5 MEDIUM DARK GRAY SILTY CLAY SHALE 140.5 10 37 135 140 N7 LIGHT GRAY LIMESTONE 17 NQ 140.5 150.5 10 60 HARD N6 MEDIUM LIGHT GRAY CLAY SHALE SOFT N6 MEDIUM LIGHT GRAY CLAY SHALE 145 HARD N6 MEDIUM LIGHT GRAY CLAY SHALE

EP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-17

AEP

JOB NUMBER __ DATE <u>7/17/15</u> SHEET <u>7</u> OF __ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0601 PROJECT CARDINAL LANDFILL 6/5/07 BORING FINISH 6/12/07 **BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S POG WELL SC IN FEET **IDENTIFICATION NOTES FEET** FROM TO HARD N5 MEDIUM GRAY CLAY SHALE 18 NQ 150.5 160.5 10 80 155 N4 MEDIUM DARK GRAY SILTY CLAY SHALE 160 N4 MEDIUM DARK GRAY SILTY CLAY SHALE 19 NQ 160.5 170.5 10 11 SOFT N4 MEDIUM DARK GRAY SHALE **N5 MEDIUM GRAY SILTY CLAY SHALE** 165 N1 BLACK COAL N2 GRAYISH BLACK DARK CLAY SHALE 170 N1 BLACK COAL 20 NQ 170.5 180.5 10 71 N2 GRAYISH BLACK CLAY SHALE SOFT N2 GRAYISH BLACK CLAY SHALE HARD N4 MEDIUM DARK GRAY CLAY SHALE SOFT N4 MEDIUM DARK GRAY CLAY SHALE HARD N6 MEDIUM LIGHT GRAY LIMESTONE 175 SOFT N5 MEDIUM GRAY CLAY SHALE

FGD LANDFILL.GPJ AEP.GDT 7/17/15

8

AEP
Well: S-17

 LOG OF BORING

 Monitoring Well: S-17

 COMPANY
 AMERICAN ELECTRIC POWER
 BORING NO. CA-0601
 DATE 7/17/15
 SHEET 8 OF 17

 PROJECT
 CARDINAL LANDFILL
 BORING START 6/5/07
 BORING FINISH 6/12/07

PRU	JECI	CAI	DINA	L LANDFILL					вО	RING START 6/5/07 BORING FINISI	٦ <u>٥</u>	112/01
SAMPLE NUMBER	SAMPLE		IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	문 의	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-			N7 LIGHT GRAY LIMESTONE		
21	NQ	180.5	190.5		10	70	180 – - -			N5 MEDIUM LIGHT GRAY LIMESTONE		
							-			HARD N5 MEDIUM GRAY CLAY SHALE N7 LIGHT GRAY LIMESTONE		
							185 - -			N6 MEDIUM LIGHT GRAY LIMEY CLAY SHALE		
							- 190 —			HARD N5 MEDIUM GRAY CLAY SHALE SOFT N5 MEDIUM GRAY LIMEY CLAY SHALE		
22	NQ	190.5	200.5		10	18	-			SOFT N5 MEDIUM GRAY LIMEY CLAY SHALE		
							-			N4 MEDIUM DARK GRAY to BLACK CLAY SHALE SOFT N6 MEDIUM LIGHT GRAY LIMEY CLAY SHALE N4 MEDIUM DARK GRAY to BLACK CLAY		
							195 - - -			N1 BLACK COAL		
23	NQ	200.5	210.5		10	80	200 -			N2 GRAYISH BLACK CLAY SHALE N5 MEDIUM GRAY LIMEY CLAY SHALE		

EP CD_FG

LOG OF BORING Monitoring Well: S-17 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 9 OF _ BORING NO. CA-0601 PROJECT CARDINAL LANDFILL 6/5/07 BORING FINISH 6/12/07 **BORING START** STANDAKL PENETRATION PENETRATION PENETRATION PENETRATION PENEDRATION PENEDRATI SAMPLE SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK WELL DRILLER'S LOG SC IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **N6 MEDIUM LIGHT GRAY LIMESTONE** 205 SOFT N5 MEDIUM GRAY CLAY SHALE N7 LIGHT GRAY CLAYEY LIMESTONE w/pyrite 210 NQ 210.5 220.5 10 83 **N6 MEDIUM LIGHT GRAY LIMESTONE** 215 N4 MEDIUM DARK GRAY CLAY SHALE N7 LIGHT GRAY FINE GRAIN SILTSTONE N7 LIGHT GRAY CLAY SHALE/SILTSTONE

220

225

10

79

CD FGD LANDFILL.GPJ AEP.GDT 7/17/15

NQ 220.5

230.5

Continued Next Page

SOFT N4 MEDIUM DARK GRAY CLAY SHALE

N6 MEDIUM LIGHT GRAY SILTY CLAY SHALE

N6 LIGHT GRAY CLAY SHALE/SILTSTONE N6 MEDIUM LIGHT GRAY SILTY CLAY SHALE

AFP

JOB NUMBER		LOC OF BOTAING	Monitoring Well: S-17	
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. CA-0601	DATE 7/17/15 SHEET 10	OF 17
PROJECT	CARDINAL LANDFILL	BORING START 6/5/0	7 BORING FINISH 6/12/07	7

11100				LANDFILL						RING START <u>6/5/07</u> BORING FINISH		
SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							230 –					
26	NQ	230.5	240.5		10	47	- - -	× × × × × × × × × × × × × × × × × × ×		N6 MEDIUM LIGHT GRAY CLAY SHALE/ SILTSTONE		
							_			N4 MEDIUM DARK GRAY CLAY SHALE		
							235 -			N6 MEDIUM LIGHT GRAY CLAY SHALE		
							233			w/ siltstone		
							-			SOFT N4 MEDIUM DARK GRAY CLAY SHALE		
										N4 MEDIUM DARK CLAY SHALE		
							-			SOFT N4 MEDIUM DARK GRAY CLAY SHALE		
										SOFT N4 WEDIOW DARK GRAT CLAT SHALE		
							-					
							240 -					
							240					
27	NQ	240.5	249.5		9	44	_	××		N6 MEDIUM LIGHT GRAY SILTSTONE		
										N7 LIGHT GRAY LIMEY CLAY SHALE		
							-					
							-					
							-					
							245 -			SOFT N5 MEDIUM GRAY CLAY SHALE		
							243					
							-			NE MEDIUM ODAY! IMEY OLAYOUALE		
										N5 MEDIUM GRAY LIMEY CLAY SHALE		
							-					
							-					
,							_			HARD N5 MEDIUM GRAY CLAY SHALE		
28							-					
28	NQ	249.5	255.5		6	55	250 -			N5 MEDIUM GRAY LIMEY CLAY SHALE		
							-			5RP 4/2 GRAYISH RED PURPLE CLAY		
										SHALE		
							-					
							-					
							_					
)												

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

AEP

											/12/07
SAMPLE	DEF	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
NQ	255.5	265.5		10	90	255 -	× × × × × × × × × × × × × × × × × × ×		N5 MEDIUM GRAY CLAY SHALE 5G 6/1 GREENISH GRAY LIMEY SILTSTONE W/clay shale		SWL 187.4'; NQ HOLE TO 255.5'
						-	× × × × × × × × × × × × × × × × × × ×				
						260 — - -			5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE w/ siltstone		
						265 -					
NQ	265.5	2/5.5		10	68	- - -			N6 MEDIUM LIGHT GRAY SILTY FINE GRAIN SANDSTONE		
						270 – -			SOFT BROWNISH GRAY SANDY CLAY		
						-			MEDIUM GRAY LIMEY CLAY SHALE VERY SOFT 5YR 4/1 BROWNISH GRAY CLAY SHALE		
NQ	275.5	285.5		10	53	275 -			5GY 6/1 GREENISH GRAY LIMEY CLAY SHALE 5RP 4/2 GRAYISH RED PURPLE RED CLAY		
						- - -			N5 MEDIUM GRAY SHALE		
	NQ	NQ 255.5 NQ 265.5	NQ 255.5 265.5 NQ 265.5 275.5	NQ 255.5 265.5 NQ 265.5 275.5	NQ 255.5 265.5 10 NQ 265.5 275.5 10	DEPTH PENETRATION SUBJECT PENETRATION PENETRATIO	NQ 255.5 265.5 10 90 255 - 260 - 260 - 260 - 265 - 275.5 10 68 275 - 275	NQ 255.5 265.5 10 90 255 260 260 260 260 260 270 270 275 275 275.5	NQ 255.5 265.5 10 90 255 —	NQ 255.5 265.5 10 90 255 10 90 255	NQ 255.5 265.5 10 90 255 255.5 265.5 10 90 265 265 265.5 2

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LOG OF BORING

Monitoring Well: S-17

JOB NUMBI	ER	LOG OF BORNING	Monitoring Well: S-17				
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. <u>CA-0601</u>	DATE 7/17/15 SHEET	12 OF	17		
PROJECT	CARDINAL LANDFILL	BORING START 6/5/0	7 BORING FINISH 6/1	12/07			

			, \	L LANDFILL						RING START	6/5/07	_ BORING FINISH		
NUMBER	SAMPLE	DEI	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS		SOIL / ROCK	N	WELL	DRILLER'S NOTES
							-			EDD 4/2 CDAV	'ISH RED PURPLE	E CLAV		
							-			SHALE	ISH KED PORPLE	CLAY		
							-			N6 MEDIUM LI	IGHT GRAY CLA	Y SHALE		
							285 -							
32	NQ	285.5	295.5		10	72	-			SHALE	YR 3/2 GRAYISH MEDIUM BLUISH			
							-			SHALE	MEDIUM BLUISH	GRAY CLAY		
							-							
							290 –							
							-							
							-							
							295 -							
33	NQ	295.5	305.5		10	84	-			N5 MEDIUM G	RAY CLAY SHAL	E		
							-			SOFT N5 MED	OIUM GRAY CLAY	SHALE		
							-							
							300 -			5YR 4/1 BROW	VNISH GRAY CLA	Y SHALE		
							-			SOFT N5 MED	DIUM GRAY CLAY	SHALE		
							-			N7 LIGHT GRA	AY LIMESTONE			
							305 -			HARD N5 MED	DIUM GRAY CLAY	'SHALE		
34	NQ	305.5	310.5		5	58								

LOG OF BORING

Monitoring Well: S-17

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 13 OF __ BORING NO. CA-0601 PROJECT CARDINAL LANDFILL **BORING START** 6/5/07 BORING FINISH 6/12/07

PRU	JECI	CAL	DINA	L LANDFILL				вс	DRING START <u>6/5/07</u> BORING FINIS	ын <u>о</u>	112/07
SAMPLE NUMBER	SAMPLE	SAM DEF IN F FROM		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							-	-	HARD 5YR 3/2 GRAYISH BROWN CLAY SHALE		
35	NQ	310.5	315.5		5	58	310 -		5R 4/2 GRAYISH RED RED CLAY SHALE		SWL 185.3'; NQ HOLE TO 310.5'; 50 hr reading
							315 -		N4 MEDIUM DARK GRAY CLAY SHALE		
36	NQ	315.5	325.5		10	95	-	-			
							320 -	× × × × × × × × × × × × × × × × × × ×	N6 MEDIUM LIGHT GRAY SILTSTONE W/limestone nodules		
37	NQ	325.5	335.5		10	100	325 - - -	X X X X X X X X X X X X X X X X X X X	HARD N6 MEDIUM LIGHT GRAY SILTSTONE w/limestone nodules	_	
							330 -	* * * * * * * * * * * * * * * * * * *			

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Monitoring Well: S-17 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 14 OF 17 BORING NO. CA-0601 PROJECT CARDINAL LANDFILL BORING FINISH 6/12/07 BORING START 6/5/07

		SAM	IPLE	STANDARD	>	RQD						
NUMBER	SAMPLE		PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL ENGTH COVER	%	DEPTH IN	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
	0)	FROM	ТО	BLOWS / 6"	고盟		FEET					
							-	× × × × × ×		HADD NZ LIGHT CDAY FINE CDAIN		
								× ×		HARD N7 LIGHT GRAY FINE GRAIN SANDSTONE		
							-	× × × × × × × ×		HARD N5 MEDIUM GRAY SILTSTONE		
							335 –	× × × × × ×				
38	NQ	335.5	345.5		10	97	-					
							-	× × × ×				
							_	X				
								× × × ×				
							-			MEDIUM LIGHT GRAY FINE GRAIN SANDSTONE		
							340 -			w/crossbedding throughout		
							-					
							-					
							_					
							-					
							345 -					
39	NQ	345.5	355.5		10	97	-			HARD N4 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
							-					
							-					
							-					
							350 –					
							-					
							-					
							-					
							-					
							355 -	1 1 1 1		N2 COAL PARTING GRAYISH BLACK		
40	NQ	355.5	365.5		10	94	-	<u> </u>		HARD N4 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
							-	:::::		JANUS I GIVE		
	1		I					10000				

LOG OF BORING

Monitoring Well: S-17

JOB NUMBE	ER		wormoring weil.	3-17	
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. CA-0601	DATE 7/17/15	SHEET 15 OF	= <u>17</u>
DDO IECT	CAPDINAL LANDEILI	PODING START 6/5/07	PODING EIN	1101 6/12/07	

SAMPLE NUMBER	SAMPLE	SAM DEF IN F	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							360 -			HARD N5 MEDIUM GRAY MEDIUM GRAIN STANDSTONE		
							- - 365			HARD N6 MEDIUM LIGHT GRAY FINE GRAIN SANDSTONE HARD N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE W/coal partings		
41	NQ	365.5	375.5		10	92	-			HARD N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
							370 -			GRAYISH BLACK COAL PARTING HARD N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE W/coal partings throughout HARD N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
42	NQ	375.5	385.5		10	92	375			N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE Wessesheddings throughout		SWL 190.7'; NQ HOLE TO 375.5
							380 —			W/crossbeddings throughout N5 MEDIUM GRAY MEDIUM GRAIN SANDSTONE		
							- - -			N4 MEDIUM DARK GRAY MEDIUM GRAIN SANDSTONE w/crossbeddings throughout		

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-17

 COMPANY
 AMERICAN ELECTRIC POWER
 BORING NO. CA-0601
 DATE 7/17/15
 SHEET 16
 OF 17

 PROJECT
 CARDINAL LANDFILL
 BORING START 6/5/07
 BORING FINISH 6/12/07
 6/12/07

•		0.			LANDINE						THING START OF THE BORNING FINISH		
RAMPIE	NUMBER	SAMPLE	DEI	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
								385 -			N6 MEDIUM LIGHT GRAY LIMESTONE		
	43	NQ	385.5	395.5		10	91	-			N5 MEDIUM GRAY FINE GRAIN SILTY SANDSTONE		
								-					
								390 -					
								-					
								-			HARD N6 MEDIUM LIGHT GRAY FINE GRAIN		
-	44	NQ	395.5	405.5		10	94	395 -			HARD N6 MEDIUM LIGHT GRAY FINE GRAIN		
								-			SANDSTONE		
								-					
								400 -					
								-					
								-					
פווווווווווווווווווווווווווווווווווווו	45	NQ	405.5	415.5		10	70	405 -			HARD MEDIUM LIGHT GRAY FINE GRAIN SANDSTONE		
ILL.GPJ AEP.								-					
_ reDLANDFILL.GFJ AEP.GDI //1//13								-			N4 MEDIUM DARK GRAY CLAY SHALE		
∟'ב					I								

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

JOB NUMBER

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LOG OF BORING Monitoring Well: S-17 JOB NUMBER DATE 7/17/15 SHEET 17 OF 17 COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0601 PROJECT CARDINAL LANDFILL 6/5/07 ____ BORING FINISH _6/12/07 **BORING START** PENETRATION RESISTANCE BLOWS / 6" RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO 415 STOPPED BORING @ 415.5'; FLUSHED W/~700 GALS WATER; GEO PHYSICAL LOGGED; **INSTALLED 1" GEOMON TYPE** WELL CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15



Monitoring Well: S-10

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JOB N	NUM	BER _					_		LO		DOMINO	,	Monito	oring Well: S	5-10	
COM	PAN	/ <u>AM</u>	ERIC/	N ELE	CTRIC	POW	VER			ВС	RING NO. C	A-0607	DATE_7	/17/15	SHEET	T_1_OF_5_
				L LAND							RING START	1/9/07		BORING FINIS	SH _	1/9/07
COOF	RDIN	ATES _	N 831	1,867.6	E 2,5	16,49	5.5			PII	EZOMETER T	YPE		WELL TY	PE _	
GRO	JND	ELEVAT	ION _	1002.5	SY	STEM				HC	ST. RISER AB	OVE GROUND	2.70	4	DIA _	2
Wate	r Lev	el, ft	$\overline{\mathbb{V}}$		lacksquare		1	-		DE	PTH TO TOP	OF WELL SC	REEN _	39.7 BOTTO	OM _	58.7
TIME								WI	ELL DEVELO	PMENT		BACKF	ILL _			
DATE	<u> </u>									FIE	ELD PARTY	MCR / ZLF	R	F	rig _l	D-120
										1						
SAMPLE	SAMPLE	DEF IN F		PENET RESIS	DARD RATION TANCE	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS		SOIL / F			WELL	DRILLER'S NOTES
	UGE	R 0.0	14.0	BLOV	VS / 6"	~		5			Deconned water used system. B	procedures not rig & tools 01/03 from cardinal p lind drilled 3.25 ed coring @ 14.	3/07. Dec plant fire p 5" HSA's fi	con & drilling protection		
1	NQ	14.0	19.0			1.8	72	- 15 - - -			MEDIUM H GRAY CLA	IARD 5B 5/1 M AY SHALE	EDIUM B	LUISH		
FGD_LANDFILL.GPJ AEP.GDT 7/17/15	NQ			ASING	USED	5.8	17	-			w/iron stair	DIUM LIGHT Ging and fracture	es Vext Pa	ge		DEN TURE
FGD_LANUI		NQ-2 R0 6" x 3.25 9" x 6.25 HW CAS	HSA HSA	VANCER	?	4"		PIEZOM SLO WELL T	ITTC	ED S	SCREEN, C	OPEN TUBE = GEONO TUBE SLO)R, P = 1	PNEUMATI	С	
8		NW CAS	SING		-	3"		VVELL I	IPE:		VV - OPEN		/ I I E D 3	JOINEEN, C	– ועונ	OLOIVIOIN
L Y		SW CAS				6" 8"					RECORD	er <u>MCR</u>				

LOG OF BORING

Monitoring Well: S-10

JOB NUMBER DATE 7/17/15 SHEET 2 OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0607 PROJECT CARDINAL LANDFILL 1/9/07 BORING FINISH 1/9/07 **BORING START** SAMPLE STANDARD PENETRATION PENETRATION RESISTANCE SAMPLE NUMBER SAMPLE DEPTH **DEPTH** SOIL / ROCK DRILLER'S LOG SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO MEDIUM HARD N5 MEDIUM GRAY CLAY SHALE HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY NQ 24.8 34 8 9.9 51 25 SHALE w/high angle fracture, iron staining throughout HARD 5B 5/1 MEDIUM BLUISH GRAY SANDSTONE w/high angle fracture, iron staining throughout HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE w/high angle fracture, iron staining throughout 30 HARD 5B 5/1 MEDIUM BLUISH GRAY SANDSTONE w/high angle fracture, iron staining throughout NQ 34.8 35 MEDIUM TO SOFT N5 MEDIUM GRAY CLAY 44.8 4.8 33 SHALE HARD N6 MEDIUM LIGHT GRAY LIMESTONE MEDIUM TO SOFT N5 MEDIUM GRAY CLAY SHALE 40 LANDFILL.GPJ AEP.GDT 7/17/15 Lost all water return @ 43.7' HARD 5B 5/1 MEDIUM BLUISH GRAY CLAY NQ 44.8 54.8 9.8 54 45 FGD **SHALE** 8

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LOG OF BORING

Monitoring Well: S-10 JOB NUMBER DATE 7/17/15 SHEET 3 OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0607 PROJECT CARDINAL LANDFILL 1/9/07 BORING FINISH 1/9/07 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** LOG SOIL / ROCK WELL DRILLER'S USC IN FEET **IDENTIFICATION NOTES FEET** FROM TO w/iron staining 50 HARD FINE 5B 7/1 LIGHT BLUISH GRAY **WELL SEAMED SANDSTONE** w/iron staining 55 6 NQ 54.8 10.0 20 HARD 5GY 6/1 GREENISH GRAY SHALE 64.8 HARD 56 5/1 MEDIUM BLUISH GRAY FINE **SANDY SHALE** 60 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE 65 SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY NQ 64.8 74.8 10.0 55 SHALE FGD LANDFILL.GPJ AEP.GDT 7/17/15 HARD N5 MEDIUM GRAY SILTY CLAY SHALE w/fractures 70

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LOG OF BORING

Monitoring Well: S-10

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 4 OF BORING NO. CA-0607 PROJECT CARDINAL LANDFILL 1/9/07 BORING FINISH 1/9/07 BORING START

PROJECT C	ARDINAL	LANDFILL					RING START <u>1/9/07</u> BORING FINISH	BORING FINISH 1/9/07		
	SAMPLE DEPTH IN FEET DM TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	LENGTH RECOVERY	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES	
8 NQ 74	.8 84.8		9.7 4	75 -			HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY CLAY SHALE w/fractures throughout			
				80 -			SOFT CLAY SHALE AREA HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY CLAY SHALE			
							w/fractures throughout			
9 NQ 84	.8 90.3		5.5 4	2 85 -			HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY CLAY SHALE w/fractures			
10 NQ 90	.3 99.8		8.3	90 -			HARD N7 LIGHT GRAY LIMESTONE SOFT 5B 5/1 MEDIUM BLUISH GRAY CLAY SHALE			
				95 -			HARD 5B 5/1 MEDIUM BLUISH GRAY SILTY CLAY SHALE			

AEP CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

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LOG OF BORING Monitoring Well: S-10 JOB NUMBER DATE 7/17/15 SHEET 5 OF 5 COMPANY AMERICAN ELECTRIC POWER BORING NO. CA-0607 PROJECT CARDINAL LANDFILL BORING START 1/9/07 PENETRATION RESISTANCE BLOWS / 6" RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S DEPTH SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO Stopped boring @ 99.8' on 01/04/07. Plugged NQ hole from 99.8' to 61.0' w/ bentonite pellets. Built 2" well. CD_FGD_LANDFILL.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-1

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		IBER _					-) DOMING	,	IV	/ionito	ring vveii: S-1		
COM	1PAN	Y _ AN	IERIC/	AN ELE	CTRIC	POW	ER			В	ORING NO. 8	3502	D/	ATE_ 7	/17/15 SHE	ET1	1 OF 3
				L PLAN							ORING START	Τ _	12/9/85		BORING FINISH	12/1	2/85
COO	RDIN	NATES .	N 83	1,399.8	E 2,5	15,20	7.8			PI	EZOMETER 1	TYPE	<u> </u>		WELL TYPE	GM	
GRO	UND	ELEVA	ΓΙΟΝ _	999.6	SY	/STEM	STA	TE PLANE		Н	GT. RISER AE	BOVE	GROUND _	1.64	DIA	.75	
Wate	er Lev	vel, ft	$\overline{\nabla}$		Ţ		$ar{ar{ar{\Lambda}}}$			DI	EPTH TO TOP	OF	WELL SCRE	EN _	64.5 BOTTOM	68.5	i
TIME	E									W	ELL DEVELO	PME	:NT		BACKFILL	GRO	DUT
DAT	E									FI	ELD PARTY	M	CR-ML		RIG	B-61	1
SAMPLE	SAMPLE	DE	MPLE EPTH EEET TO	STAN PENET RESIS BLOV	IDARD RATION STANCE VS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	nscs			SOIL / RO			WELL	DRILLER'S NOTES
								5 -									
								10 -									
SI.GPJ AEP.GDT 7/17/15			OCK CC 5 HSA	CASING	USED			PIEZOM	ETER OTTE	TYFED	PE: PT = (OPE	ntinued Ne. EN TUBE F GEONOR	PORC	ge DUS TIP, SS = PNEUMATIC	OPEN	N TUBE
SIG	HW CASING ADVANCER 4" WELL						WELL T	YPE:	0	W = OPEN	I TU	JBE SLOT	TED S	SCREEN, GM :	= GEC	OMON	
8		NW CAS				3" 6"	\dashv										
AEP											RECORD	י⊏К					

LOG OF BORING Monitoring Well: S-1 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 2 OF _ BORING NO. 8502 PROJECT CARDINAL PLANT 12/9/85 **BORING START** STANDARD
PENETRATION PLOUS
SISTANCE SAMPLE RQD SAMPLE NUMBER GRAPHIC LOG SAMPLE DEPTH USCS DEPTH SOIL / ROCK WELL DRILLER'S IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO 25 28.8 TOP OF SEAL. 30 34.0 TOP OF SAND. 35

40 CD SI.GPJ AEP.GDT 7/17/15 45 Continued Next Page

LOG OF BORING

Monitoring Well: S-1

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JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 3 OF BORING NO. 8502 PROJECT CARDINAL PLANT **12/9/85** BORING FINISH **12/12/85 BORING START** STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** SOIL / ROCK WELL DRILLER'S SCS IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO 50 LIGHT GREEN GRAY MEDIUM GRAY DARK NQ 50.0 60.0 9.5 69 **GRAY SOME RED CLAY SHALE** Calcareous, fissile, soft, fresh, partings sandy. 55 60 NQ 60.0 61.0 .6 33 2 MEDIUM TO DARK GRAY CLAYEY **LIMESTONE** Hard, fresh except oxidized orange 3 NQ 61.0 70.0 8.5 78 ∖on joints at 66.2. MEDIUM GREEN GRAY CLAY SHALE Fissile, calcareous with sand size limestone nodules, poorly cemented, soft, fresh. 63.9 CHECK VALVE. 64.5 TOP OF 65 SCREEN. CD SI.GPJ AEP.GDT 7/17/15 68.5 BOTTOM OF SCREEN. 70

JOB NUM	IBER		_	Monitoring Well: S-2	· ·			
COMPAN	Y AMERICA	AN ELECTRIC POW	/ER	ORING NO. <u>8503</u> DATE <u>7/17/15</u> SHEET _	1 OF 4			
PROJECT	CARDINA	L PLANT		ORING START <u>12/12/85</u> BORING FINISH <u>12/</u>	17/85			
COORDIN	NATES N 83	1,038.2 E 2,514,71	4.2	IEZOMETER TYPE WELL TYPE	1			
GROUND	ELEVATION	1038.6 SYSTEM	STATE PLANE	GT. RISER ABOVE GROUND 1.29 DIA .75				
Water Le	vel, ft	▼	Ā	EPTH TO TOP OF WELL SCREEN 80.5 BOTTOM 84.	5			
TIME	VOI, 11 <u>-</u>	<u>-</u>		/ELL DEVELOPMENT BACKFILL GR	OUT			
				IELD PARTY MCR-ML RIG B-6				
DATE				e	-			
SAMPLE NUMBER SAMPLE	SAMPLE DEPTH IN FEET FROM TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	RQD DEPTH DHAW DEPTH OF THE PROPERTY OF THE PR	SOIL / ROCK IDENTIFICATION	DRILLER'S NOTES			
			5					
			10 -					
/15			15					
SI.GPJ AEP.GDT 7/17/15	TYPE OF C	ASING USED		Continued Next Page				
[6]					NITURE			
A. X	NQ-2 ROCK CC 6" x 3.25 HSA	/N⊏ 	PIEZOMETER SI OTT	PE: PT = OPEN TUBE POROUS TIP, SS = OPE SCREEN, G = GEONOR, P = PNEUMATIC	N IUBE			
GP.	9" x 6.25 HSA	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\						
SD SI.	HW CASING AE NW CASING	OVANCER 4" 3"	WELL TYPE:	DW = OPEN TUBE SLOTTED SCREEN, GM = GE	OMON			
AEP C	SW CASING	6"		RECORDER				
₹∟	AIR HAMMER	8"						

LOG OF BORING

Monitoring Well: S-2

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 2 OF BORING NO. 8503 PROJECT CARDINAL PLANT 12/12/85 BORING FINISH 12/17/85 **BORING START** SAMPLE **STANDARD** RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE USCS **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK WELL DRILLER'S IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** BLOWS / 6" FROM TO 25 30 35 40 40.5 TOP OF SEAL. CD SI.GPJ AEP.GDT 7/17/15 MEDIUM BLUE GRAY CLAY SHALE NQ 44.3 50.0 2.9 38 Calcareous, portions sandy with laminations fine 45 grain light gray sand to 47.1. limestone nodules and streaks limestone, most

LOG OF BORING

Monitoring Well: S-2

JOB NUMBE	ER		wonitoring weil		
COMPANY	AMERICAN ELECTRIC POWER	BORING NO. <u>8503</u>	DATE 7/17/15	SHEET 3	OF
PROJECT	CARDINAL PLANT	BORING START 12	2/12/85 BORING FI	INISH 12/17/85	

PRO	ROJECT CARDINAL PLANT					ВО	RING START	12/12/85	BORING FINISH	_1:	2/17/85		
SAMPLE NUMBER	SAMPLE	로 DEPTH PENETRATION 로등의 및 IN FEET RESISTANCE ON % 비			RQD DEPTH	ZAPH LOG	nscs		SOIL / ROCK IDENTIFICATION		WELL	DRILLER'S NOTES	
						50				rtions moderately har oderately hard, fresh.			46.5 TOP OF SAND.
2	NQ	50.0	60.0		10.0	92 55							
3	NQ	60.0	70.0		10.0	60	-		Fissile, lenses light gray quar all fresh no join to be machine shale grades of	DARK BLUE GRAY S and laminations of v tZ sandstone, portior nts visible, all core po breaks. down to carbonaceouses, sandy portions ha	ery fine grain us calcareous, ortions appears s shale to 69',		
4 5	NQ NQ	70.0	71.5		1.3	87 70			fresh. DARK BLUE (streaks and no of shale with >	CLAYEY LIMESTON GRAY SHALE Block odules, limestone, sid 50% limestone, fresl E GRAY SHALE CA	y, calcareous , erite portions		

AEP CD SI.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-2

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<u> </u>		

JOB NUMBER DATE **7/17/15** SHEET **4** OF COMPANY AMERICAN ELECTRIC POWER BORING NO. 8503 PROJECT CARDINAL PLANT 12/12/85 BORING FINISH 12/17/85 **BORING START** STANDARD
PENETRATION PENETRATI SAMPLE RQD SAMPLE NUMBER DEPTH SAMPLE GRAPHIC S **DEPTH** LOG SOIL / ROCK DRILLER'S SCS WELL IN IN FEET **IDENTIFICATION NOTES FEET** FROM TO WITH LAMINATIONS, FINE GRAIN WHITE SAND, FRESH MODERATELY HARD. **DARK GRAY LIMESTONE** Microcrystalline, ∖fresh, hard. 75 MOSTLY LIGHT BLUE GRAY TO LIGHT **GREEN GRAY CLAY SHALE** Calcareous blocky, portions dark gray at 75' and 75.7', all soft, fresh, fresh slickenslided surfaces at various orientations. all calcareous with fine sand size to gravel size limestone nodules lenses shaley limestone at 75.-76.3 and 84.5-85.0. 80 79.9 CHECK VALVE. 80.0 10.0 NQ 90.0 . 80.5 TOP OF SCREEN. 84.5 BOTTOM OF 85 **SCREEN LIGHT BLUE GRAY SANDY SHALE** Fissile with laminations light gray fine grain quartz sand portions calcareous with streaks and nodules limestone, fresh. moderately hard. 90 CD SI.GPJ AEP.GDT 7/17/15

LOG OF BORING

Monitoring Well: S-4

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	JOB NUMBERCOMPANY AMERICAN ELECTRIC POWER										BORING NO. 88-5-6 DATE 7/17/15 SHEET 1 OF 9					
							POW	EK								
					L PLAN		12 05	າ າ				PRING START <u>8/11/88</u> EZOMETER TYPE				
			_						TE PLANE			GT. RISER ABOVE GROUND SE				
г												PTH TO TOP OF WELL SCREEN				
H			el, ft	∑ 20 -		▼ 20		Ā				ELL DEVELOPMENT				
H	ГІМЕ				20		15					ELD PARTY MCR-TJH				
	DATE			8-16	6-88	8-1	7-88					LDTARTT WOR-TOTT	1110		/-01	
II IONAGO	SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH EET	PENET RESIS	DARD RATION TANCE VS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	USCS	SOIL / ROCK IDENTIFICATION		WELL	DRILLER'S NOTES	
	1	SS	2.5	4.0		7-6	.5		5 -			GRAY CLAY With limestone and of (fill).	coal fragments		WATER IN CREEK pH 7.3 TESTED BY CARDINAL PLANT LAB PERSONAL.	
	2	SS	7.5	9.0	7-6	6-5	.4		10 -						7.8 LOST WATER IN CASING.	
	3	SS	12.5	12.5	50	W.5	0		15 -							
7/15	4	SS	18.3	19.8	7-	7-6	.2		10			YELLOW CLAY With coal and lim fragments.	estone		17.1 DRILLED 2 15/16" ROLLER BIT FROM 17.1 TO 18.3 THROUGH LIMESTONE BOLDERS.	
SI.GPJ AEP.GDT 7/17/15	TYPE OF CASING USED										ı	Continued Next Pa		<u>.1∓</u> V/	1	
P.GD.	X NQ-2 ROCK CORE PIEZO							\dashv	DIEZO:	ICTCC	T) (C)				EN TUDE	
J AE	FIEZOM 6" x 3.25 HSA SLO						ZOMETER TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE SLOTTED SCREEN, G = GEONOR, P = PNEUMATIC									
SI.GP	9" x 6.25 HSA HW CASING ADVANCER 4" WELL T							WELL T			W = OPEN TUBE SLOTTED		= 0	SEOMON		
S =	X NW CASING 3"						\Box	vv⊏LL I	175			JUINEEN, GIVI	- 0	PLOIVIOIN		
AEP	SW CASING 6" AIR HAMMER 8"							\dashv				RECORDER TJH				

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING Monitorine

ORY
Monitoring Well: S-4

JOB NUMBER ______ BORING NO. 88-5-6 DATE 7/17/15 SHEET 2 OF 9
PROJECT CARDINAL PLANT BORING START 8/11/88 BORING FINISH 8/16/88

TROULG	ROJECT CARDINAL PLANT									RING START <u>0/11/00</u> BURING FINIS		
SAMPLE NUMBER SAMPLE	J LINICO	SAM DEF IN FI FROM	PLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
5 SS		22.2	23.4	19-27-50/.2	.4		-	000000000000000000000000000000000000000		GRAY LIMESTONE FRAGMENTS		
6 SS	S	27.5	29.0	8-11-15	.5		25 - - - - -			GRAY CLAY With clay shale and limestone fragments.		27.0 TOP OF SEAL
7 SS	S	32.5	34.0	5-12-13	.7		35 -					32.0 TOP OF SANE
8 SS	S	37.5	39.0	6-9-13	.2		- - - 40 –	00 00 00 00 00		SANDSTONE FRAGMENTS		
9 88	S	42.5	42.9	50/.4	.4		-			.2 YELLOW SANDSTONE .2 LIMESTONE		
							45 -			Continued Next Page	• • • • • • • • • • • • • • • • • • • •	

LOG OF BORING

Monitoring Well: S-4

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE **7/17/15** SHEET **3** OF BORING NO. 88-5-6 PROJECT **CARDINAL PLANT** 8/11/88 ____ BORING FINISH 8/16/88 **BORING START** SAMPLE **STANDARD** RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH S **DEPTH** PENETRATION TOTAL LENGTH RECOVE SOIL / ROCK DRILLER'S USC WELL IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 10 SS 47.5 49.0 20-15-16 50 **GREEN AND GRAY SANDY SHALE** Partially SS 52.5 53.3 27-50/.3 .2 cemented. 55 12 SS 57.5 57.8 50/.3 .2 **RED CLAY SHALE** 58.0 SET CASING 13 NQ 58.0 61.6 .8 RED AND GRAY CLAYSTONE Soft. 60 **GRAY CLAYEY SANDSTONE** Hard, 14 NQ 61.6 65.0 3.4 88 calcareous, grading to fine grain hard sandstone. **GRAY SANDSTONE** Fine, hard. 65 NQ 65.0 71.7 6.6 89 15 67.0-67.6 LIGHT BROWN 68.5-70.0 LIGHT BROWN CD SI.GPJ AEP.GDT 7/17/15 70 71.9-73.0 LIGHT BROWN



JOB NUMBER
LOG OF BORING

COMPANY
AMERICAN ELECTRIC POWER
BORING NO. 88-5-6
DATE 7/17/15
SHEET 4 OF 9

PROJECT
CARDINAL PLANT
BORING START 8/11/88
BORING FINISH 8/16/88

PROJECT CARDINAL PLANT							BORING START <u>8/11/88</u> BORING FINISH <u>8/16/88</u>						
SAMPLE	SAMPLE	DEI IN F FROM	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / ROCK IDENTIFICATIO		WELL	DRILLER'S NOTES
16	NQ	71.7	75.0		3.3	67	-						1.7-75.0 LOST 50% DRILL WATER.
17	NQ	75.0	85.0		9.7	60	75 - - -			77.2-77.6 LIGHT BROWN			
							80 —			79.3-79.7 BROKEN WITH IRON SOFT. GRAY CLAYSTONE Calcareous			79.4 CHECK VALVE. 80.0 TOP OF SCREEN.
18	NQ	85.0	95.0		9.8	46	- 85 - -			GRAY CLAYSTONE Calcareous			82.0 BOTTOM OF SCREEN. 84.0 BOTTOM OF SAND.
							90 — - -			GRAY LIMESTONE Hard. GRAY CLAYSTONE Calcareous	s, soft.		
19	NQ	95.0	105.0		9.8	62	- 95 - -			GRAY LIMESTONE Hard.			

P CD SI.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER

Monitoring Well: S-4

AEP

				AN ELECTRIC	POV	VER				ORING NO. 88-5-6 DATE 7/17/15 SHEET 5 OF 9
PRO	JECT	CAF	RDINA	L PLANT					ВС	RING START 8/11/88 BORING FINISH 8/16/88
SAMPLE	SAMPLE	DE	MPLE PTH EEET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK ☐ DRILLER'S ☐ NOTES
							100 -			GRAY CLAY SHALE Soft with some red and
20	NQ	105.0	115.0		10.0	68	- 105 -			gray layers.
							- 110 –			GRAY CLAYSTONE Soft. GRAY LIMESTONE Changing to brown at
24	NO	115.0	125.0		5.2	22	. 115 –			114.6. SOFT CLAYSTONE LAYERS AT 112.1-112.3 AND 112.7-112.09
21	NQ	115.0	125.0		5.2	33	-			RED CLAYSTONE Calcareous, soft.
CD SI.GPJ AEP.GDT 7/17/15							120 -			
AEP CD		1			•				•	Continued Next Page

LOG OF BORING

Monitoring Well: S-4

AEP

DATE **7/17/15** SHEET **6** OF COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-5-6 PROJECT CARDINAL PLANT BORING START 8/11/88 BORING FINISH 8/16/88 STANDARD
PENETRATION PLOOP
SISTANCE SAMPLE RQD SAMPLE NUMBER DEPTH SAMPLE **DEPTH** LOG SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM TO 125 22 NQ 125.0 135.0 9.5 **GRAY CLAYSTONE** Soft, calcareous, with gray 36 clay shale layers from 126.4-128.1 **GRAY SHALEY LIMESTONE** Hard. 130 135 23 NQ 135.0 145.0 9.9 73 **GRAY CLAYSTONE** Soft with limestone nodules. 140 **GRAY SHALEY SANDSTONE** Soft, calcite seams. **GRAY SANDSTONE** Fine grain. 145 24 NQ 145.0 10.0 100 155.0 146.5 T SAND. 146.5 TOP OF

AEP CD SI.GPJ AEP.GDT 7/17/15

JOB NUMBER

LOG OF BORING

Monitoring Well: S-4

AEP

COMPANY AMERICAN ELECTRIC POWER _ DATE <u>7/17/15</u> SHEET <u>7</u> OF _ BORING NO. 88-5-6 PROJECT CARDINAL PLANT 8/11/88 BORING FINISH 8/16/88 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER DEPTH USCS SAMPLE **DEPTH** PENETRATION SOIL / ROCK WELL DRILLER'S IN IN FEET RESISTANCE **IDENTIFICATION NOTES** FEET FROM BLOWS / 6" TO 155 NQ 155.0 165.0 10.0 160 165 NQ 165.0 175.0 10.0 170 175 NQ 175.0 185.0 10.0 98

AEP CD SI.GPJ AEP.GDT 7/17/15

JOB NUMBER

JOB	NUMI	BER			, ,_	0		LOC	3 O	F BORING	Mor	nitoring Well: S	6-4	/_ \ -	
COM	PAN	/ <u>AM</u>	IERIC <i>A</i>	N ELECTRIC	POV	VER			ВС	RING NO. <u>88-5</u>	5-6 DATE	7/17/15	SHEET	8 OF9	_
PRO.	JECT	CAF	RDINA	L PLANT					ВС	RING START	8/11/88	_ BORING FINIS	SH <u>8/</u>	16/88	-
SAMPLE	SAMPLE	DEI	IPLE PTH EEET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	nscs		SOIL / ROCK		WELL	DRILLER'S NOTES	_
							- - 180 — -								
28	NQ	185.0	195.0		10.0	100	- 185 — - -								
							190 —							191.9 CHECK	
29	NQ	195.0	205.0		9.9	95	- 195 — - -			GRAY SHALE	<u>ey sandstone</u> s	Soft.		VALVE. 192.5 TOP OF SCREEN. 194.5 BOTTOM OF SCREEN. 196.5 BOTTOM OF SAND.	
							200 —								

AEP CD SI.GPJ AEP.GDT 7/17/15



Monitoring Well: S-4 JOB NUMBER DATE 7/17/15 SHEET 9 OF COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-5-6 PROJECT **CARDINAL PLANT** BORING START 8/11/88 BORING FINISH 8/16/88 STANDARD
PENETRATION RESISTANCE ON THE PENETRATION PEN RQD RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH USCS DEPTH SOIL / ROCK WELL DRILLER'S IN FEET **IDENTIFICATION** NOTES FEET FROM TO 205 CD SI.GPJ AEP.GDT 7/17/15

LOG OF BORING

JOB NUMBER

Monitoring Well: S-5

COMPANY AMERICAN ELECTRIC POWER										BORING NO. <u>88-7-8</u> DATE <u>7/17/15</u> SHEET <u>1</u> OF <u>8</u>													
										BORING START 8/8/88 BORING FINISH 8/10/88													
COORDINATES N 834,917.6 E 2,513,916.2										PIEZOMETER TYPE WELL TYPE GM													
GRC	UND	ELEVAT	TION _	1000.2	SY	STEM	ST	ATE PLANE		HC	HGT. RISER ABOVE GROUND SEE NOTE DIA 1.0												
Wat	er Lev	el, ft	<u>⊽</u> 6	.4	▼ 10	0.2	Ā	22.0	22.0 DEPTH TO TOP OF WELL SCREEN SEE N								SEE NOTE						
TIME 7:10 7:20					3:00		WI	ELL DEVELO	PMENT		BAC	KFILL	GROUT										
DAT	E			-88		0-88		8-11-8	8		ELD PARTY	MCR=TJ	H		RIG	B-61	61						
				1				1								1							
щЖ	щ		/IPLE PTH		DARD		RQD	DEPTH IN FEET	⊋	S		COII /	DOCK				DDILLE	210					
SAMPLE	SAMPLE		EET	RESIS	TANCE	NG S	%	IN	AP. LOG	SC		IDENTIF	ROCK		ļ	WELL	DRILLER						
δ N	S		то	BLOV	RATION TANCE VS / 6"		70	FEET	GR _	Π		IDENTIF	ICATION		,	>	NOTES						
		TTOW		DEG.	.070																		
									[
									7														
									-100														
											CDAVELY ASH PROMINICIAY												
1	SS	2.7	4.2 10-12-14	2-14	.9					GRAY FLY ASH BROWN CLAY													
									[-]														
								5 -															
								,	7														
2	ss	7.7	9.2	14-	-3-5	.5			- 2		GRAY FLY	ASH AND A	SPHALT I	FRAGMENT									
								40															
								10 -								₹//							
									2														
		12.7	14.2	13-	3-9-9	.5					LIMESTONE AND GRAVEL FRAGMENTS												
3	SS								7.7														
									J (
									0														
								15 -															
									D::_(
									18														
									2:::														
4	SS	17.7	19.2	9-1	1-11	.1			- 0.		LIMESTO	NE FRAGMEN	<u>ITS</u>										
									\mathbb{Q}														
									-0														
c[/]) · · (
		TYPE	E OF C	ASING	USED							Continued	Next Pa	age									
. X	<u> </u>	NQ-2 R	OCK CO	RE				PIEZOM	IFTED	TVP		OPEN TUB			SS = 1	OPFN	TURF						
		6" x 3.25	5 HSA									G = GEON				○ 1	. 000						
<u>5</u>		9" x 6.25 HW CAS		VANCEF	₹	4"										- (250)	MON						
S X		NW CAS	SING			3"		WELL T	rPE:			TUBE SLO	טוו⊏ט	SUREEN	i, GIVI =	- GEUI	VIOIN						
SW CASING 6"						_				RECORD	ER TJH												

LOG OF BORING

JOB NUMBER

CD SI.GPJ AEP.GDT 7/17/15

AEP

Monitoring Well: S-5

AEP

COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 2 OF BORING NO. 88-7-8 PROJECT CARDINAL PLANT 8/8/88 BORING FINISH 8/10/88 **BORING START** SAMPLE STANDARD RQD GRAPHIC LOG SAMPLE NUMBER DEPTH SAMPLE S **DEPTH** PENETRATION TOTAL LENGTH RECOVER SOIL / ROCK DRILLER'S USC WELL IN IN FEET RESISTANCE **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 20.0 TOP OF SEAL. **LIMESTONE FRAGMENTS** SS 22.7 24.2 5 11-6-9 .3 24.6 TOP OF SAND. **LIMESTONE FRAGMENTS** SS 27.7 29.2 10-10-13 .5 6 30 SS 32.7 32.7 50/0 7 0 **LIMESTONE FRAGMENTS** 8 SS 37.7 39.2 8-12-9 .1 39.2 67.5 39.2 No samples were taken from 39.2' 40 to 67.5'. material consisted of bolderss and soil. NQ core barrel was used to cut bolders and advanced casing. 45

LOG OF BORING

Monitoring Well: S-5

JOB NUMBER COMPANY AMERICAN ELECTRIC POWER BORING NO. <u>88-7-8</u> DATE <u>7/17/15</u> SHEET <u>3</u> OF _ PROJECT CARDINAL PLANT BORING START 8/8/88 BORING FINISH 8/10/88

SAMPLE	SAMPLE	SAM DEF IN FE	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
							50 -				
							55 -	-			
							60 -			•	• [
10	NO	67.5	60.9		22	0	65 -	-	LICHT CREEN SANDSTONE Fine grain		
	NQ NQ	69.8	69.8 73.2		1.7	0	70 -		LIGHT GREEN SANDSTONE Fine grain changing to gray at 69.8'. GRAY CLAYSTONE Soft.		70.0 CHECK VALVE 70.6 TOP OF SCREEN.

LOG OF BORING

Monitoring Well: S-5

AEP

JOB NUMBER DATE **7/17/15** SHEET **4** OF COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-7-8 PROJECT CARDINAL PLANT 8/8/88 BORING FINISH 8/10/88 **BORING START** STANDARD
PENETRATION
PENETRATI SAMPLE SAMPLE NUMBER SAMPLE DEPTH **DEPTH** F0G SOIL / ROCK WELL DRILLER'S SCS IN FEET **IDENTIFICATION NOTES FEET** FROM TO 72.6 BOTTOM OF SCREEN. 12 NQ 73.2 74.8 1.5 0 **LIGHT BROWN SILTY LIMESTONE** Hard. 74.6 BOTTOM OF 75 13 NQ 74.8 77.0 **GRAY CLAYSTONE** Soft. SAND. NQ 77.0 79.8 2.4 **GRAY CLAYSTONE** Soft with limestone nodules. 15 NQ 79.8 82.0 2.1 80 16 NQ 82.0 83.2 1.0 0 NQ 17 83.2 84.8 1.3 0 RED AND GRAY CLAYSTONE Soft, with calcite seams 90.0-93.0. 85 18 NQ 84.8 87.2 2.1 19 NQ 87.2 89.8 2.6 0 90 20 NQ 89.8 94.8 4.9 92 **GRAY CLAY SHALE** Soft. CD SI.GPJ AEP.GDT 7/17/15 21 NQ 94.8 99.8 4.7 95 78

LOG OF BORING

JOB COM			IERIC <i>A</i>	AN ELECTRIC	POV	VER			BORING NO. <u>88-7-8</u> DATE <u>7/17/15</u> SHEET <u>5</u> OF <u>8</u>							
PRO	JECT	CAF	RDINA	L PLANT					ВС	ORING START 8/8/88 BORING FINISH 8/10/88						
SAMPLE	SAMPLE	SAMPLE DEPTH IN FEET FROM TO		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	۱	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL/ ROCK						
-22	NQ	99.8			2.1	0	100 -			GRAY CLAYSTONE Soft.						
23 24	NQ NQ				.5 2.1	0 51	-			GRAY LIMESTONE Hard.						
- 25	NQ	104.8	109.8		5.0	76	105 -			GRAY CLAYSTONE Soft, calcareous, changing to red at 105.1.						
-26 -	NQ-	109.8	114.8		4.6	46	110 -									
-27	NQ	114.8	124.8		10.0	48	115 -			GRAY CLAYSTONE Soft with calcite seams. GRAY SANDY SILTSTONE Hard.						
1 7/17/15							- 120 -			GRAY CLAYSTONE Soft, with limestone nodules 121.3-124.1.						
AEP CD SI.GPJ AEP.GDT 7/17/15							-			Continued Next Page						

LOG OF BORING Monitoring Well: S-5 JOB NUMBER COMPANY AMERICAN ELECTRIC POWER DATE 7/17/15 SHEET 6 OF BORING NO. 88-7-8 PROJECT CARDINAL PLANT **BORING START** 8/8/88 ___ BORING FINISH <u>8/10/88</u> STANDARD
PENETRATION PENETRATI SAMPLE SAMPLE NUMBER SAMPLE DEPTH DEPTH LOG SOIL / ROCK WELL DRILLER'S USC IN IN FEET **IDENTIFICATION NOTES** FEET FROM TO 125 28 NQ 124.8 134.8 9.8 83 **GRAY SANDY CLAYSTONE** Calcareous grading to light fine grain sandstone at 143.7. 130 134.7 TOP OF 135 29 NQ 134.8 144.8 10.0 100 SAND. 140 **GRAY SANDSTONE** Hard, fine grain, well cemented. 145 30 NQ 144.8 154.8 10.0 89 CD SI.GPJ AEP.GDT 7/17/15

ОВ	NUMI	BER _				_		LO	G OF BOR	ING		Monito	ring Well:	S-5				
СОМ	PAN	AM	ERICA	N ELECTRIC	POV	VER			BORING N	O. <u>88-7</u>	'- 8	DATE 7	/17/15	SHEET _	EET <u>7</u> OF <u>8</u>			
PRO	JECT	CAF	RDINA	L PLANT					BORING S	ΓART	8/8/88		BORING FI	NISH <u>8/1</u>	0/88			
SAMPLE	SAMPLE	SAM DEF IN F FROM	EET	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPH	S C S C		SOIL / F			WELL	DRILLER'S NOTES			
31	NQ	- 154.8	164.8		10.0	90	155 -											
							160 -											
32	NQ	164.8	174.8		10.0	98	165 -											
							170 -											
33	NQ	174.8	184.8		10.0	84	175 -											

AEP CD SI.GPJ AEP.GDT 7/17/15



LOG OF BORING Monitoring Well: S-5 JOB NUMBER ____ DATE <u>7/17/15</u> SHEET <u>8</u> OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-7-8 PROJECT CARDINAL PLANT BORING START 8/8/88 BORING FINISH 8/10/88 RQD RQD SAMPLE STANDARD GRAPHIC LOG PENETRATION PENETRATION RESISTANCE SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S USC WELL IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO 180 180.1 CHECK VALVE. 180.7 TOP OF SCREEN. 182.7 BOTTOM OF **GRAY CLAYSTONE** Soft, calcareous. SCREEN 184.7 BOTTOM OF 34 NQ 184.8 194.8 10.0 79 185 **GRAY CLAYEY LIMESTONE** SAND. **GRAY CLAYSTONE** Calcareous, soft. **GRAY SHALEY SANDSTONE** Calcareous. 190 -CD SI.GPJ AEP.GDT 7/17/15



JOB NUMBER

Monitoring Well: S-6

- /4\	42	

COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL PLANT									BORING NO. <u>88-9-10</u> DATE <u>7/17/15</u> SHEET <u>1</u> OF <u>10</u> BORING START <u>7/28/88</u> BORING FINISH <u>8/4/88</u>									
						13,679	.4										<u>, </u>	
		_						TE PLANE		PIEZOMETER TYPE WELL TYPE WELL TYPE HGT. RISER ABOVE GROUND _ SEE NOTE DIA 1.0								
Wat	er Lev	vel, ft	∑ 22	2.0	▼ 2:	2.8	1	26.4		DE	PTH TO TOF	OF WELL	SCREEN	SEE NOT	Б М	SEE I	NOTE	
	TIME 1:15 7:10				+-	7:05		WI	ELL DEVELO	PMENT		BACK	FILL .	GRO	<u>JT</u>			
DAT	DATE 8-1-88 8-2-88					8-3-88		FIE	ELD PARTY	MCR-TJ	H		RIG	B-61				
SAMPLE	SAMPLE	DE	MPLE PTH EEET TO	PENET RESIS	IDARD RATION TANCE VS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs			/ ROCK FICATION	l	II.W	\$	DRILLEI NOTE:	:S
145								5								CAS DRII PON BY 0	LED NW ING TO S L WATE CARDINA NT PERS	53.2'. ER IN FESTED AL
T 7/17/15									1	Continue	l Next P	age	<u> </u>	VA				
X X		NQ-2 R 6" x 3.25 9" x 6.25	OCK CO 5 HSA 5 HSA	RE				SLC	OTTE	Continued Next Page TER TYPE: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE TTED SCREEN, G = GEONOR, P = PNEUMATIC								
CD SI.O		HW CA	SING AD SING	VANCEF	Κ	4" 3"		WELL TY	YPE:	0	W = OPEN	TUBE SI	OTTED	SCREEN,	GM =	GEO	MON_	
SW CASING 6" AIR HAMMER 8"							RECORDER											

AEP

JOB NUMBER Monitoring Well: S-6

COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-9-10 DATE 7/17/15 SHEET 2 OF 10

PROJECT CARDINAL PLANT BORING START 7/28/88 BORING FINISH 8/4/88

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

ring Well: S-6

 JOB NUMBER
 LOG OF BORING
 Monitoring Well: S-6

 COMPANY AMERICAN ELECTRIC POWER
 BORING NO. 88-9-10
 DATE 7/17/15
 SHEET 3 OF 10

 PROJECT CARDINAL PLANT
 BORING START 7/28/88
 BORING FINISH 8/4/88

۱ ا	
WELL	DRILLER'S NOTES

AEP CD SI.GPJ AEP.GDT 7/17/15

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING

JOB	NUMI	BER _				_		LO	G O	r boning		Monitoring W	/ell: S-6			
COM	IPAN\	/ <u>AM</u>	ERICA	N ELECTRIC	POV	VER			ВО	RING NO. <u>88-9</u>	-10	DATE 7/17/15	SHEET	4	OF _	10
PRO.	JECT	CAF	RDINA	L PLANT						RING START	7/28/88	BORING	FINISH _	8/4/88		
SAMPLE	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL / R		WELL	D	RILLER NOTES	
5	NQ	75.0	85.0		9.8	49	75 -			GRAY SANDY RED AND BRO	OWN CLAYS	STONE Soft.				
6	NQ	85.0	95.0		9.8	34	80 - - - 85 -			GRAY AND RI nodules, calcit		ONE Limestone				
							90 -							91.4 W		
7	NQ	95.0	105.0		9.8	83	95 -							91.7 T SCREI 93.7 B SCREI	OP OF EN. OTTOM EN. OTTOM	

Continued Next Page

AEP CD SI.GPJ AEP.GDT 7/17/15

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



LOG OF BORING Monitoring Well: S-6 JOB NUMBER DATE **7/17/15** SHEET **5** OF _ COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-9-10 PROJECT CARDINAL PLANT BORING START 7/28/88 BORING FINISH 8/4/88 STANDARD
PENETRATION YES SAMPLE RQD GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH DEPTH SOIL / ROCK DRILLER'S SCS WELL IN FEET **IDENTIFICATION NOTES FEET** FROM BLOWS / 6" TO **GRAY CLAYSHALE** Soft, changing to red and gray at 103.3. 100 **GRAY CLAYSTONE?** Soft. 105 NQ 105.0 108.0 1.9 0 1.3' of GRAY LIMESTONE Hard. NQ 108.0 115.0 1.6 0 108 PULLED NQ RODS TO REPAIR LANDING RING IN CORE BARREL. 110 REASON FOR LOST CORE. 115 NQ 115.0 125.0 10.0 **GRAY AND RED CLAYSTONE** Soft, 68 calcareous.

CD SI.GPJ AEP.GDT 7/17/15

Continued Next Page

GRAY CLAYSTONE Soft, limestone nodules,

GRAY SILTY SANDSTONE Hard.

RED AND GRAY CLAYSTONE Hard.

120

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

JOB	NUM	BER _				_		LO	GΟ	F BORING	Monit	oring Well:	S-6			
CON	1PAN	AM	ERICA	N ELECTRIC	POV	VER			во	RING NO. 88-9-10	DATE	7/17/15	SHEET	6	OF	10
				L PLANT						RING START 7/28/88						
SAMPLE	SAMPLE	SAM DEF IN F FROM		STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	70	DEPTH IN FEET	GRAPHIC LOG	nscs	SOIL / F	CATION		WELL	С	RILLEF NOTES	
										grading to fine grain clayey	y sandsto	one at133.4.				
11	NQ	125.0	135.0		8.2	57	125 - - - 130									
12	NQ	135.0	145.0		9.9	90	- - 135 –			GRAY CLAYEY SANDST	' ONE Hai	rd, fine grain.				
					0.0		- - 140 - - -			GRAY SANDSTONE Hard cemented.	d, fine gra	ain, well				

145

10.0

90

AEP CD SI.GPJ AEP.GDT 7/17/15

13 NQ 145.0

155.0

Continued Next Page

145.6 BOTTOM OF SEAL.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING Maniforing



LOG OF BORINGMonitoring Well: S-6COMPANYAMERICAN ELECTRIC POWERBORING NO. 88-9-10DATE 7/17/15SHEET 7OF 10PROJECTCARDINAL PLANTBORING START7/28/88BORING FINISH8/4/88

PRU	JECI	CAR	KUINA	L PLANT					BO	RING START BORI	NG FINISH <u>8</u>	4/88
SAMPLE	SAMPLE	SAM DEF IN F FROM	PTH	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
14	NQ	155.0	165.0		10.0	87	155 —			158.0-158.6 DARK GRAY FINE GRAIN		
15	NQ	165.0	175.0		10.0	90	- 165 — - -					
16	NQ	175.0	180.0		4.9	70	170 — - - - - 175 —					

AEP CD SI.GPJ AEP.GDT 7/17/15

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY LOG OF BORING Manifesting



SAMPLE	SAMPLE	SAM DEF IN F	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPHIC LOG	USCS	SOIL / ROCK IDENTIFICATION	WELL	DRILLER'S NOTES
17	NQ	180.0	185.0		5.0	98	180 -					
18	NQ	185.0	189.1		4.1	76	- 185 - - -					* * * * * * * * * * * * * * * * * * *
19	NQ	189.1	195.0		5.9	81	- 190 – -			GRAY SANDY CLAYSTONE Soft, calcite seams.		191.0 CHECK VALVE.
20 21 22 23	NQ NQ NQ NQ	195.0 195.2 195.4 196.4	195.2 195.4 196.4 205.0		.2 .2 .9 8.5	0 0 0 96	- 195 – -			DARK GRAY SANDSTONE Hard, v-fine grain. GRAY SANDSTONE Fine grain, calcareous.		191.6 TOP OF SCREEN. 193.6 BOTTOM OF SCREEN. 195.6 BOTTOM OF SAND.
							200 - -			GRAY SANDY SILTSTONE Hard, calcareous.		

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

ОВ	NUMI	BER _						LOC	G OI	F BORING		Monito	oring Well	: S-6			
			ERICA	N ELECTRIC	POV	VER			во	RING NO. <u>88-9</u>)-10	DATE_7	7/17/15	SHEET	9	OF _	10
PRO	JECT	CAF	RDINA	L PLANT					во	RING START	7/28/8	8	BORING F	INISH 8	4/88		
SAMPLE	SAMPLE	DEF	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD	DEPTH IN FEET	GRAPHIC LOG	nscs		SOIL /			WELL		RILLER	
24	NQ	205.0	210.0		8.0	76	205 -										
25	NQ	210.0	215.0		5.0	86	210 -										
26	NQ	215.0	220.0		5.0	100	215 -			CDAV SII TS	TONE Horde						
27	NQ	220.0	230.0		10.0	70	220 -			GRAY SILTS	<u>IONE</u> riaiu						
							225 -			GRAY CLAYS	STONE Soft	t, calcarec	us.				

AEP CD SI.GPJ AEP.GDT 7/17/15

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



LOG OF BORING Monitoring Well: S-6 JOB NUMBER __ DATE_<u>7/17/15</u> SHEET <u>10</u> OF __ COMPANY AMERICAN ELECTRIC POWER BORING NO. 88-9-10 PROJECT **CARDINAL PLANT** BORING START 7/28/88 BORING FINISH 8/4/88 STANDAKD
PENETRATION RESISTANCE
RESISTANCE RQD RQD SAMPLE GRAPHIC LOG SAMPLE NUMBER SAMPLE DEPTH USCS DEPTH SOIL / ROCK WELL DRILLER'S IN FEET **IDENTIFICATION** NOTES FEET FROM TO 230

AEP CD SI.GPJ AEP.GDT 7/17/15

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING

JOB NUMBER

Monitoring Well: S-7



COMP	PANY	OHIO PO	OWER COMP	ANY					BORING NO. 90CA22-S DATE SHEET 1 OF 2
PROJE	ЕСТ Т	<u>IDD ASI</u>	H POND SITE	INVES	TIGAT	ION			BORING START <u>08/13/90</u> BORING FINISH <u>08/14/90</u>
COOR	RDINATE	S <u>N 83</u>	1,920.2 E 2,	516,676	5.4				PIEZOMETER TYPE WELL TYPE
GROU	IND ELE	/ATION _	1008.5 S	YSTEM _	STAT	TE PI	LAN	E	HGT. RISER ABOVE GROUND 1.94 DIA 1.0
WATE	R LEVEL	⊻ 5	2.7 ₹		<u>v</u>				DEPTH TO TOP OF WELL SCREEN 66.2 BOTTOM 68.2
TIME					=		\dashv		WELL DEVELOPMENT BACKFILLBENSEAL
DATE		+			 				FIELD PARTY MCR-JF RIG B-61
DAIL			1		J				
SAMPLE NUMBER	SAMPLE	AMPLE DEPTH N FEET M TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	독등	%	EPTH IN EET	GRAPH	8080	SOIL / ROCK
		4 05 0				5 10 15			NO SPT SAMPLES TAKEN SEATED CASING AT 18.1. LOST WATER DRILL NW CASING AT 9.7. NO WATER RETURN DURING DRILLING. NOT A GOOD SEAL AT CASING ROCK INTERFACE.
1 N	IQ 18.	1 25.6		5.0		20 -			GRAY SILTY CLAYSHALE Calcareous, vertical cracks 20.8-21.1, 21.6-21.8
		-			- 1	25 -			GRAY SHALEY LIMESTONE Hard.
2 N	IQ 25.6	28.6		2.6	0	-			GRAY SILTY SANDSTONE V-fine grain.
3 N	JQ 28.6	35.6		7.0	80	30 -			GRAY LIMESTONE Hard, stain on joints and vertical cracks.
						_	昌道		GRAY TO BLACK CLAYSHALE
4 N	IQ 35.6	45.6		?		35 -			GRAY SILTY SANDSTONE F-fine grain. 33.1 TOP OF SEAL. vertical cracks
					-	40 -			GRAY LIGHT GRAY CLAYSHALE Slightly sandy, calcareous. LIGHT GRAY SANDSTONE Sitt crossbedding
						-	= =		throughout, thin bedding at 43.1
5 N	IQ 45.6	50.6		?		45 — - -			GRAY TO LIGHT TO DARK GRAY CLAYSHALE Broken slightly calcareous. LIGHT GRAY LIMESTONE Vertical fracture from 46.0-46.9, calcite filled. GRAY SANDY CLAYSHALE Broken, silty,
,	TYI	E OF C	ASING USED						Continued Next Page
X	6" x 3	ROCK 3.25 HS/ 5.25 HS/	4	4"		ZOMI SLC	TTE	D S	E: PT = OPEN TUBE POROUS TIP, SS = OPEN TUBE CREEN, G = GEONOR, P = PNEUMATIC W = OPEN TUBE SLOTTED SCREEN, GM = GEOMON
X		CASING		3"	-			Ť	
1		CASING		6"					RECORDER JD

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY

LOG OF BORING Monitoring Well: S-7



JOB NUMBER	C	
COMPANY OHIO POWER COMPANY	BORING NO. 90CA22-S DATE SHEET 2	_ OF 2
PROJECTTIDD ASH POND SITE INVESTIGATION	BORING START	/90

SAMPLE NUMBER	SAMPLE	DE	IPLE PTH EET TO	STANDARD PENETRATION RESISTANCE BLOWS / 6"	TOTAL LENGTH RECOVERY	RQD %	DEPTH IN FEET	GRAPH	S C S O	SOIL / ROCK IDENTIFICATION	MELL	DRILLER'S NOTES
6	NQ NQ	50.6 52.0	52.0 57.0	,	1.1 4.6	31 58	55 -			slightly calcareous. DEEP MAROON PURPLE CLAYSHALE Blocky, slightly calcareous, slightly weathered. DIGHT GREEN TO LIGHT GRAY CLAYSHALE Slightly broken.		PLUGGED OFF.
8	NQ	57.0	65.6		8.6	100	60 -			LIGHT TAN TO LIGHT GRAY SANDSTONE Fine grain, silt bedding throughout.		PLUGGED OFF. AFTER PULLING NQ RODS SWL 52.7.
9	NQ	65.6	70.6		5.0	43	65 -			RUST BROWN CLAYSHALE Iron precipitate staining throughout, broken, slightly sandy to very sandy, fine grained sand.		66.0 CHECK VALVE.
							70 -			LIGHT GRAY SANDSTONE Very fine grain, silt partings and cross bedding throughout.		66.6 TOP OF SCREEN. 68.6 BOTTOM OF SCREEN

Z	GEOSYNTEC CONSULTANTS	Project:	AE FA	P - (R1,	Card FAR	inal 2 &	Plant: RWL	Log	of Bor Sheet	ing: M-20 1 of 8
Project	No:	Date:			8/15/0	06		Initial GI	VL at:	
Location	n: Brilliant, Ohio	Total Boreh	ole De	epth:	281 f	t.		Drilling (Company	Pennsylvania Drilling Co.
Logged	by: Scott Hayder	Surface Ele	vation	:				Driller:		Earl Dye
Drilling I	Method: HSA, RC	N:			E:			Drilling I	Machine:	
	ng Comments:									
Depth, ft.	SOIL DESCRIPTION		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	RQD (%)	Plastic Index	Comments
	Brown, silty <u>SAND</u> (SM); dry; with some fin- gravel; poorly sorted.	e to coarse		SS-1	12/24	5 4 6 7	•10			
] 2	Loose, black <u>COAL ASH</u> ; dry; medium grain	ned.		SS-2	13/24	32 29 34 50/4	63			
5-				\$\$-3	10/24	25 30 34 26	64			
6	Light brown, SILT to SAND (ML/SP); dry; o bedrock.			55-4	16/24	6 46 21 50/3	67			
10—	Soft, green brown to orange brown, <u>SANDS</u> medium grained; approx. 6 horizontal fractustaining along fractures.			SS-5	10/24	50/5 50/2	√ 50			
-				RC-1	74/84			31		
15-	Hard, light gray, <u>LIMESTONE</u> ; vertical fractift.; iron staining in fractures. Green brown to orange brown, <u>SANDSTON</u> grained; 7 vertical and horizontal fractures; along fractures.	IE; medium		RC-2	59/84			35		
20.5	Soft, gray, <u>CLAYSHALE</u> ; with high sand co Green brown to orange brown, <u>SANDSTON</u> staining. Hard, light gray, <u>LIMESTONE</u> ; vertical and leading to the same of t	IE ; iron								
27	fractures; iron staining along fractures. Hard, green gray, LIMESTONE.	nonzorital		RC-3	94/120			35		
35— 35	Horizontal fracture at 32.5 ft.; iron staining. Orange brown secondary mineral at 34.0 ft Iron staining at 34.5 to 35.0 ft. Hard, gray, <u>SANDSTONE</u> ; fine grained; ver at 36 ft.; iron staining at fracture; massive. Soft to moderately hard, greenish gray, <u>CLA</u> high sand content; iron staining.			RC-4	n16/120			60		

Ē	GEOSYNTEC CONSULTANTS	Project:	FAF	P - (R1, I	Card FAR	inal 2 &	Plant: RWL	Log	of Bo Shee	oring: M-20 t 2 of 8
Project	No:	Date:			8/15/0	06		Initial G	WL at:	
Location	n: Brilliant, Ohio	Total Boreh	ole De	pth:	281 ft	t.	The second secon	Drilling (Compa	Pennsylvania Drilling Co.
Logged	by: Scott Hayder	Surface Ele	vation:					Driller:		Earl Dye
Drilling I	Method: HSA, RC	N:			E:			Drilling I	Machin	e:
Samplin	g Comments:	A manufactured of the control of the								
Depth, ft.	SOIL DESCRIPTION		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	RQD (%)	Plastic Index	Comments
43	Soft to moderately hard, greenish gray, <u>CL</u> high sand content; iron staining. Soft, gray, <u>CLAYSHALE</u> ; massive. Vertical fracture with iron staining at 47.5 f			RC-5	116/120			68		
52.5	Hard, light gray, LIMESTONE. Vertical fracture at 55.0 ft. Vertical fracture at 56.5 ft. Soft, dark greenish gray, CLAYSHALE; brollimestone in upper 1 ft; massive.	oken		RC-6	113/120	LL.		61		S-20 screen from 60.0 - 85.0 ft,
62 65 67	Hard, gray, SANDSTONE ; fine grained. Hard, gray, SANDSTONE ; fine grained. Soft to moderately hard, dark greenish gray CLAYSHALE ; massive.	<i>I</i> .		RC-7	113/120			99		
75 75	Hard, gray, LIMESTONE; broken; gradation Soft to moderately hard, dark greenish gray black, (GLEY1-5G), CLAYSHALE; massiv Gray, medium to fine sandstone at 78.5 to Vertical fracture at 79.0 ft.	/ to gray to e		RC-8	119/120			54		

Z	GEOSYNTEC CONSULTANTS	Project:	AEF	2 - C	ard	inal 2 &	Plant: RWL	Log	of Bo Sheet	ring: M-20 3 of 8
Project No:		Date:			8/15/0	06		Initial G	WL at:	
Location:	Brilliant, Ohio	Total Boreh	ole De	pth:	281 fi	t.		Drilling	Compa	ny: Pennsylvania Drilling Co.
Logged by:	Scott Hayder	Surface Ele	vation:					Driller:		Earl Dye
Drilling Met	thod: HSA, RC	N:			E:			Drilling I	Machine	e:
Sampling C	Comments:									
Depth, ft.	SOIL DESCRIPTION		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	(%)	Plastic Index	S-20 screen from
	Soft to moderately hard, dark greenish gray olack, (GLEY1-5G), <u>CLAYSHALE;</u> massive			RC-9	120/120			48		60.0 - 85.0 ft.
!	ight gray, <u>LIMESTONE;</u> vertical fracture th init.	rough 2 ft.		RC-10	120/120			92		
101 S	Soft to moderately hard, dark greenish gray, CLAYSHALE; massive.			RC-11	120/120			67		
115-				RG-12	116/120			55		

	GEOSYNTEC CONSULTANTS	Project:	AEP - FAR1,	Carc	linal 2 &	Plant: RWL	Log of Boring: M-20 Sheet 4 of 8				
Project	No:	Date:		8/15/		Initial GWL at:					
Location	n: Brilliant, Ohio	Total Boreho	ole Depth	281 f	t.		Drilling (Compan	y: Pennsylvania Drilling Co.		
Logged	by: Scott Hayder	Surface Elev	vation:				Driller:		Earl Dye		
Drilling I	Method: HSA, RC	N:		E;			Drilling I	Machine	:		
Samplin	g Comments:										
Depth, ft.	SOIL DESCRIPTION	• • •	Strata Plot Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	RQD (%)	Plastic Index	Comments		
123	Soft to moderately hard, dark greenish gray CLAYSHALE; massive. Soft to moderately hard, indian red, (10R 3. CLAYSHALE.										
130-	Black between 129.0 and 131.5 ft. Coal at 131 ft.; pyrite crystals at 131 ft. Light gray, <u>LIMESTONE</u> .		RC-1	3 120/12(74				
135-134.5	Light gray to dark gray, SANDSTONE ; fine horizontal bedding in sandstone; pyrite crys ft.		RC-1	4 120/120			92				
140-	Light gray to dark gray, SANDSTONE ; fine horizontal bedding in sandstone.	grained;									
145-	Horizontal fracture at 145.0 ft.		RC-1	5 116/120	0		94				
150-											
155—	Light gray to dark gray, SANDSTONE ; med laminar bedding; minor coal streaks through			G 118/120			100				

Z	GEOSYNTECCONSULTANTS	Project:	AE FA	P - (R1,	Card FAR	inal 2 &	Plant: RWL	Log of Boring: M-20 Sheet 5 of 8				
Project	No:	Date:		8/15/0		Initial G						
Location	n: Brilliant, Ohio	Total Boreh	ole De	epth:	281 f	t.		Drilling (Compar	Pennsylvania Drilling Co.		
Logged	by: Scott Hayder	Surface Ele	vation	:				Driller:		Earl Dye		
Drilling I	Method: HSA, RC	N:			E:			Drilling I	Machine):		
Samplin	g Comments:											
Depth, ft.	SOIL DESCRIPTION	, .	Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	3QD (%)	Plastic Index	Comments		
165—	Light gray to dark gray, SANDSTONE; med laminar bedding; minor coal streaks through streaks between 154.5 to 156.5 ft. Fractures at 161.5 ft. Coal streaks between 171.0 to 174.0 ft.			RC-17	120/120			93		M-20 screen from 220.0 - 160.0 ft.		
175-				RC-18	120/120			98				
185—	Coal streaks at 188.5 ft			RC-19	120/120			76				
195	Coal streaks at 191.0 ft. Fractures at 191.5 ft.			RC-20	120/120			97				

GEOSYNTEC CONSULTA	NTS Project:	AE FA	P - (R1, l	Card FAR	inal 2 &	Plant: RWL	Log of Boring: M-2 Sheet 6 of 8						
Project No:	Date:			8/15/0	06		Initial G	Sheet 6 of 8 SWL at: Company: Pennsylvania Drilling Co.					
Location: Brilliant, Ohio	Total Boreh	Total Borehole Depth: 281 ft.							Pennsylvania Drilling Co				
Logged by: Scott Hayder	Surface Ele	vation	;			57.1	Driller:						
Drilling Method: HSA, RC	N:			E:			Drilling I	Machin	ie:				
Sampling Comments:													
8A	- A 161			ć.		Standard		×					
ين SOIL		lot	No	η,	6 in	Penetration Resistance		Inde	ınts				
€ DESCRIP		ta F	ple	ove	/S/	Blows/ft N-Value	(%)	stic) mue				
# DESCRIP		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.		80	Plas	Com				
Light gray to dark gray, SANDSTO	NE; medium grained;					20 40 60 80)		Pennsylvania Drilling Co. Earl Dye e: ### April 19				
laminar bedding; minor coal streak streaks between 154.5 to 156.5 ft.							1		220.0 - 160.0 ft.				
- streaks between 154.5 to 156.5 π. 1" coal at 200.0 ft.													
							1						
205-													
-			RC-21	120/120			61						
1			. 1										
]													
7													
Coal streaks at 210.5 ft.													
+													
Sandstone conglomerate from 212 Coal streaks at 213.0 ft.	.5 to 213.0 ft.												
915													
Light gray to dark gray, SANDSTO laminar bedding; minor coal streak			RC-22	120/120			87						
coal streaks at 216 ft.	s unoughout.	,,,,,					\ \frac{\sqrt{\chi}}{\chi}						
- Hard, dark gray, CLAYSHALE; ma	ssive.												
-													
220													
							1						
Slicken slides at 222.0 ft.								1					
+													
Horizontal fracture at 225.0 ft.							l los						
]			RC-23	104/120			86						
Direct (and a second													
Plant fossils at 228.0 ft.													
30-													
1													
1 1													
35													
-			RC-24	1207120			87						
Slicken slides at 237 0 ft													
Slicken slides at 237.5 ft.							Ì						
Slicken slides at 238.0 ft													

Z	GEOSYNTEC CONSULTANTS	Project:	FA	P - (R1, I	ard	inal 2 &	Plant: RWL	Log of Boring: M- Sheet 7 of 8			
Project	No:	Date:			8/15/0		Initial GWL at:				
Locatio	n: Brilliant, Ohio	Total Boreh	ole De	epth:	281 ft	t.		Drilling (Company	Pennsylvania Drilling Co.	
Logged	by: Scott Hayder	Surface Ele	vation	:				Driller:		Earl Dye	
Drilling	Method: HSA, RC	N:			E:			Drilling I	Machine:		
Samplin	ng Comments:										
Depth, ft.	SOIL DESCRIPTION		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value	RQD (%)	Plastic Index	Comments	
245-	Hard, dark gray, <u>CLAYSHALE</u> ; massive. Horizontal fracture at 246.0 ft.; slicken slide Horizontal fracture at 246.5 ft. Slicken slides at 248.0 ft.	es at 246.0 ft.		RC-25	120/120			59			
251.5 252 252.5 252.5 254.5	tt. Hard, dark gray, CLAYSHALE from 252.0 t	to 252.5 ft.;		RC-26	119/120			95			
265				RC-27	119/120			96			
271 272.5 274 275— 276	Gray, SANDSTONE; medium to fine. Hard, dark gray, CLAYSHALE; massive. COAL. Hard, dark gray, CLAYSHALE.			RC-28	120/120			58			

Æ G	EOSYNTEC CONSULTANTS	Project:	AE FA	P - (R1,	Card	linal 2 &	Plant: RWL	Log	ing: M-20 8 of 8	
Project No:		Date:			8/15/			Initial G		
Location:	Brilliant, Ohio	Total Boreh	ole De	epth:	281 f	t.		Drilling (Company	Pennsylvania Drilling Co.
Logged by:	Scott Hayder	Surface Ele	vation	1				Driller:		Earl Dye
Drilling Method:	HSA RC	N:			E:			Drilling I	Machine:	
Sampling Comm	ents:									
Depth, ft.	SOIL DESCRIPTION ark gray, CLAYSHALE		Strata Plot	Sample No.	Recovery, in.	Blows / 6 in.	Standard Penetration Resistance Blows/ft N-Value 20 40 60 80	(%)	Plastic Index	Comments
	End of Boring at 281.0 ft.		7777							
295-										



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-1

Page: 1 of 6

Drilling Start Date: 03/08/2016 16:15
Drilling End Date: 03/09/2016 10:30

Drilling Method: Rock Core

Drilling Company:

Drilling Equipment: CS1500 Wireline Rig

Layne Drilling

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 102

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,012.81
Top of Casing Elev. (ft): 1,014.57

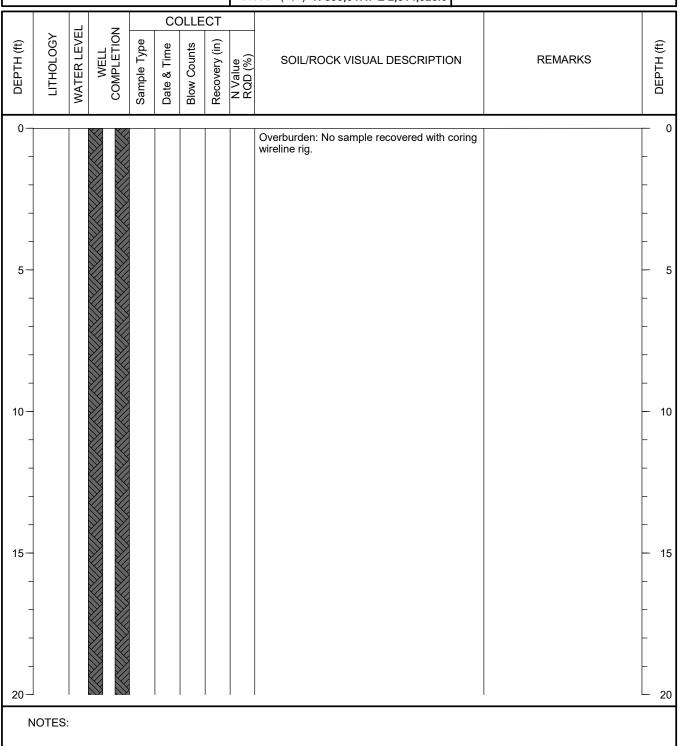
Location (X,Y): N 833,647.7 E 2,514,525.6

Well Depth (ft): **78**Well Diameter (in): **2**

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand





Client: **AEP-Cardinal** Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-1

Page: 2 of 6

03/08/2016 16:15 **Drilling Start Date:** Drilling End Date: 03/09/2016 10:30 Drilling Company: **Layne Drilling**

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack** Logged By: **Doug Mateas**

102 Boring Depth (ft): Boring Diameter (in):

Sampling Method(s): **Rock Core**

6

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,012.81

Top of Casing Elev. (ft): 1,014.57

Location (X,Y): N 833,647.7 E 2,514,525.6

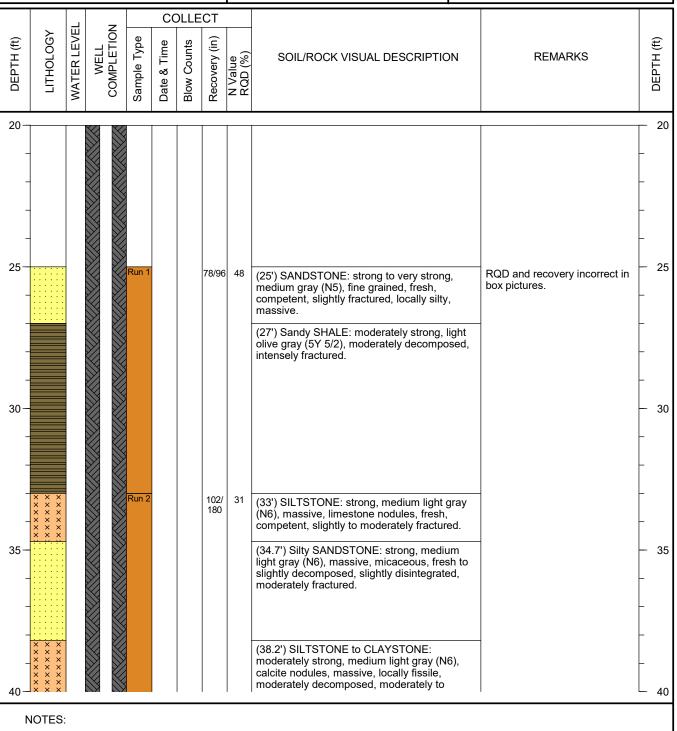
Well Depth (ft): 78

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:

Bentonite Pellets Seal Material(s): Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. S-GS-1

Page: 3 of 6

Drilling Start Date: 03/08/2016 16:15
Drilling End Date: 03/09/2016 10:30

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,012.81
Top of Casing Elev. (ft): 1,014.57

Location (X,Y): N 833,647.7 E 2,514,525.6

102

Rock Core

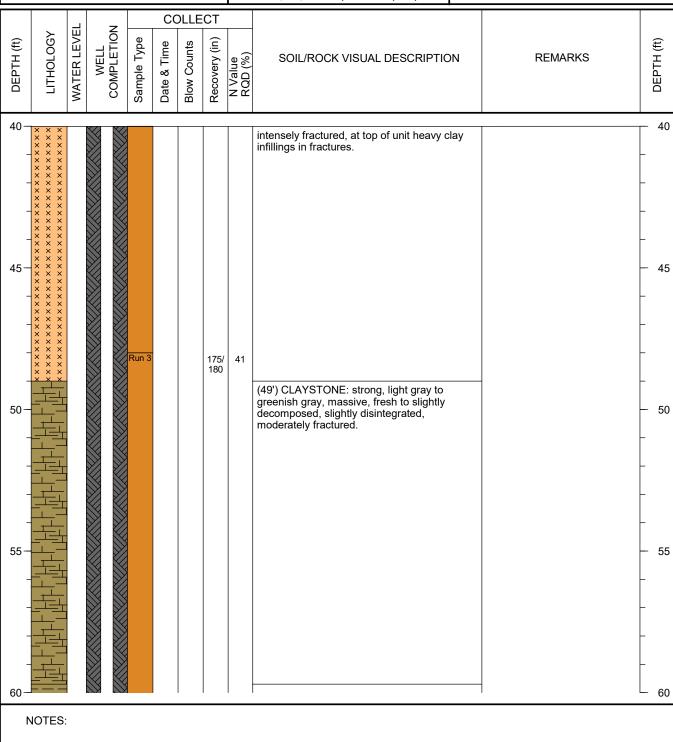
6

Well Depth (ft): 78

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: **AEP-Cardinal** Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Sampling Method(s):

DTW During Drilling (ft):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. S-GS-1

Page: 4 of 6

03/08/2016 16:15 **Drilling Start Date:** Drilling End Date: 03/09/2016 10:30

Drilling Company: Layne Drilling

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack**

Ground Surface Elev. (ft): 1,012.81

Top of Casing Elev. (ft): 1,014.57

Location (X,Y): N 833,647.7 E 2,514,525.6

102

Rock Core

6

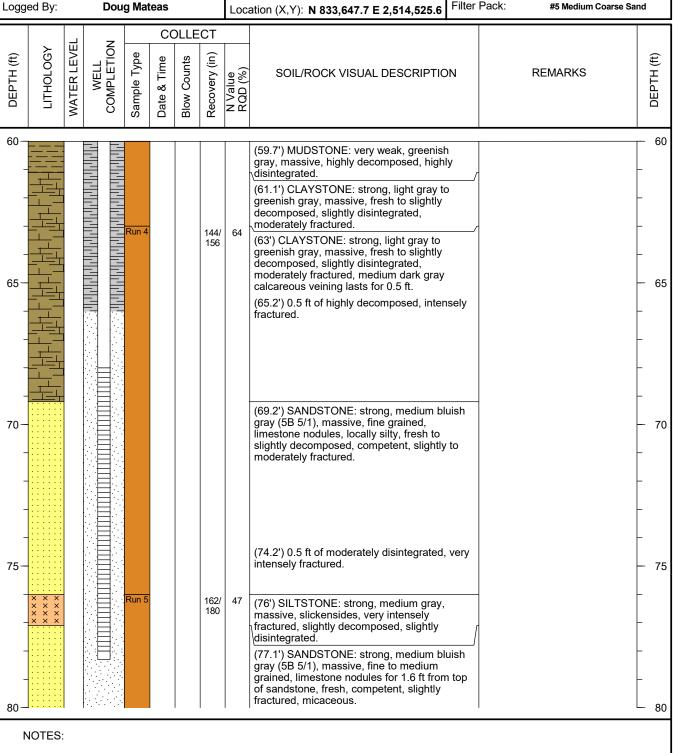
Well Depth (ft): 78

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material:

Bentonite Pellets Seal Material(s): Filter Pack: #5 Medium Coarse Sand





Client: **AEP-Cardinal** Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-1

Page: 5 of 6

03/08/2016 16:15 **Drilling Start Date: Drilling End Date:** 03/09/2016 10:30 Drilling Company: **Layne Drilling**

Drilling Method: **Rock Core**

Drilling Equipment: CS1500 Wireline Rig

Driller: **Bill Womack**

Logged By: **Doug Mateas**

102 Boring Depth (ft): Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,012.81 Top of Casing Elev. (ft): 1,014.57

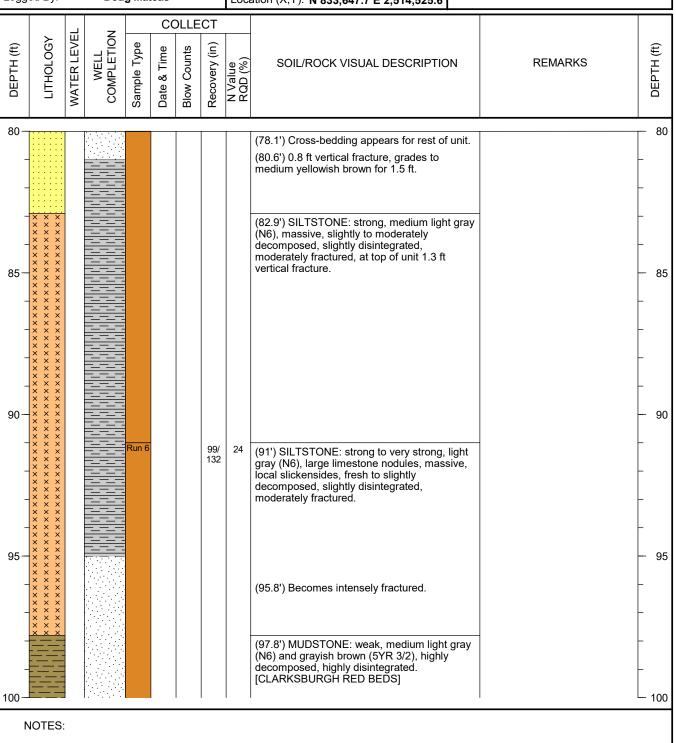
Location (X,Y): N 833,647.7 E 2,514,525.6

Well Depth (ft): 78

Well Diameter (in): 2 Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Pre-packed Sch 40 PVC Screen Material: Seal Material(s): **Bentonite Pellets** Filter Pack: #5 Medium Coarse Sand





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-1

Page: 6 of 6

Drilling Start Date: 03/08/2016 16:15

Drilling End Date: 03/09/2016 10:30
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 102

Boring Diameter (in): 6

Sampling Method(s): Rock Core
DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,012.81
Top of Casing Elev. (ft): 1,014.57

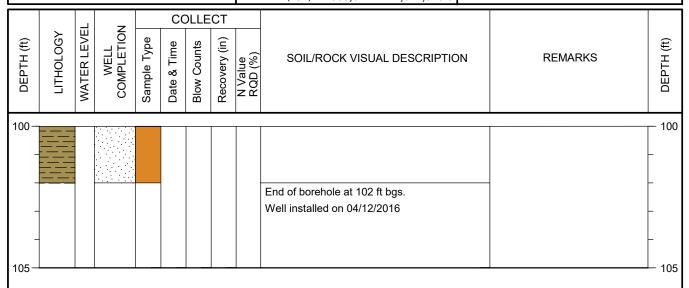
Location (X,Y): N 833,647.7 E 2,514,525.6

Well Depth (ft): 78

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand



NOTES:



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-2

Page: 1 of 5

Drilling Start Date: 03/09/2016 13:20

Drilling End Date: 03/09/2016 18:00
Drilling Company: Layne Drilling

Drilling Method: Rock Core

ROCK Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 89

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): **1,009.07**Top of Casing Elev. (ft): **1,011.75**

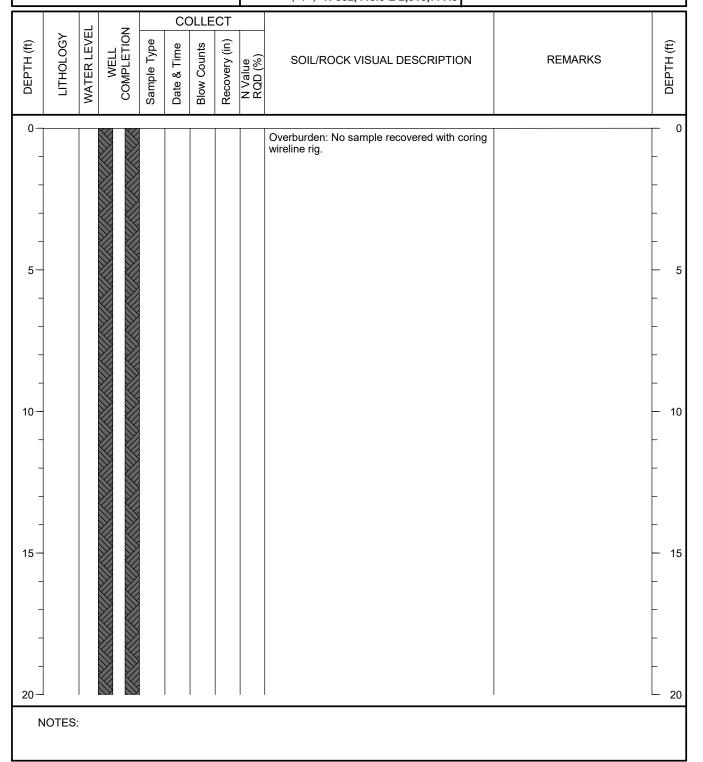
Location (X,Y): N 832,448.3 E 2,515,777.5

Well Depth (ft): 84

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-2

Page: 2 of 5

Drilling Start Date: 03/09/2016 13:20

Drilling End Date: 03/09/2016 18:00
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 89

Boring Diameter (in): 6
Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,009.07
Top of Casing Elev. (ft): 1,011.75

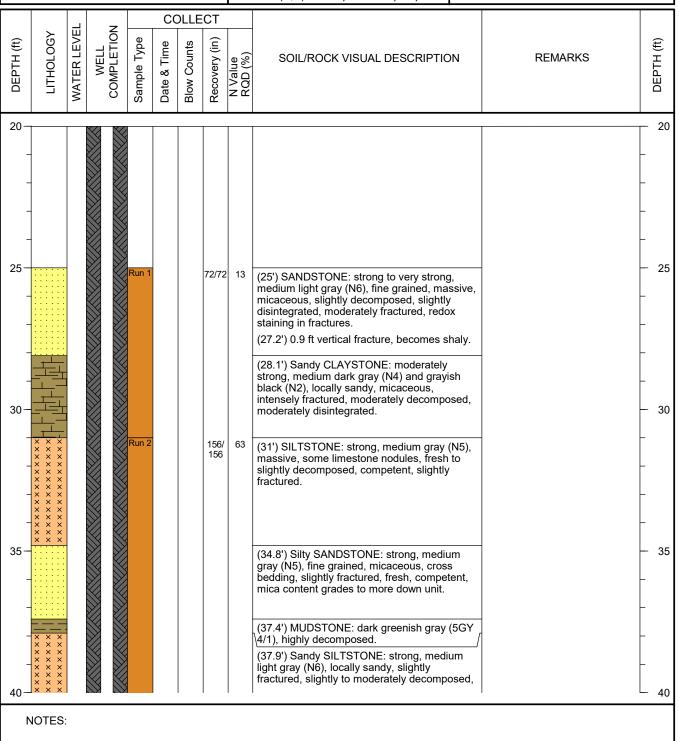
Location (X,Y): N 832,448.3 E 2,515,777.5

Well Depth (ft): 84

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets





Client: AEP-Cardinal Project: CHE8126L

Boring Depth (ft):

Boring Diameter (in):

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. S-GS-2

Page: 3 of 5

Drilling Start Date: 03/09/2016 13:20
Drilling End Date: 03/09/2016 18:00

Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Sampling Method(s): Rock Core
DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,009.07
Top of Casing Elev. (ft): 1,011.75

Location (X,Y): N 832,448.3 E 2,515,777.5

89

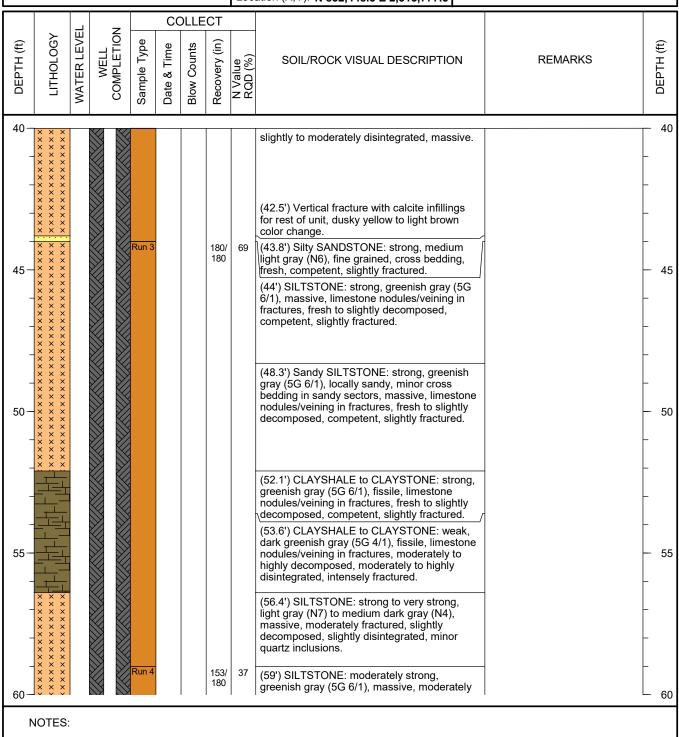
6

Well Depth (ft): 84

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Drilling Method:

Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-2

Page: 4 of 5

Drilling Start Date: 03/09/2016 13:20
Drilling End Date: 03/09/2016 18:00

Drilling Company: Layne Drilling

Drilling Equipment: CS1500 Wireline Rig

Rock Core

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 89

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,009.07
Top of Casing Elev. (ft): 1,011.75

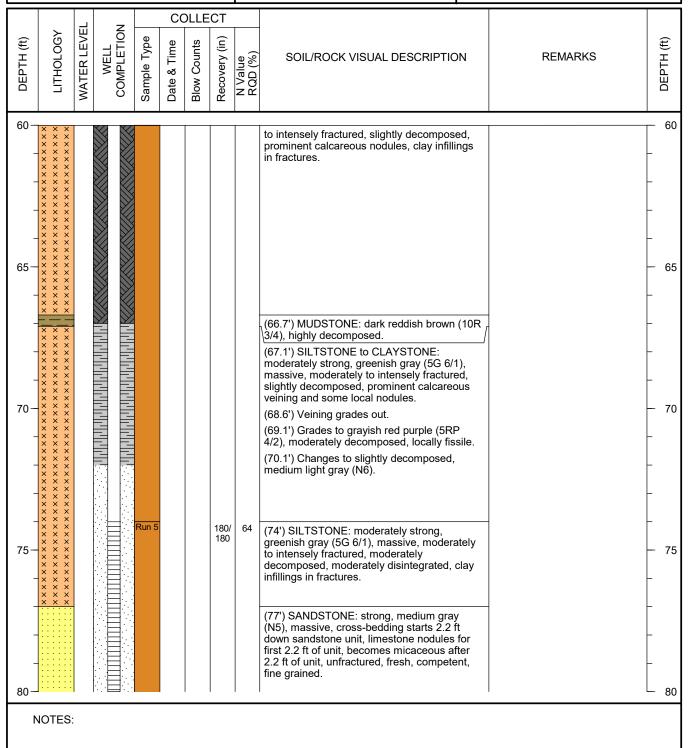
Location (X,Y): N 832,448.3 E 2,515,777.5

Well Depth (ft): 84

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC
Seal Material(s): Bentonite Pellets





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-2

Page: 5 of 5

Drilling Start Date: 03/09/2016 13:20

Drilling End Date: 03/09/2016 18:00
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack
Logged By: Doug Mateas

Boring Depth (ft): 89

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,009.07
Top of Casing Elev. (ft): 1,011.75

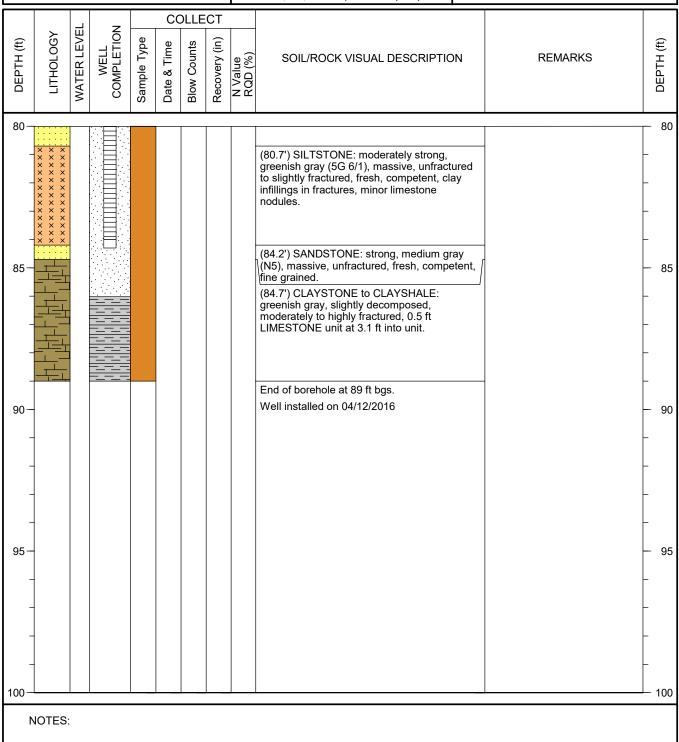
Location (X,Y): N 832,448.3 E 2,515,777.5

Well Depth (ft): 84

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-3

Page: 1 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller:

Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93
Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

Well Depth (ft): 140

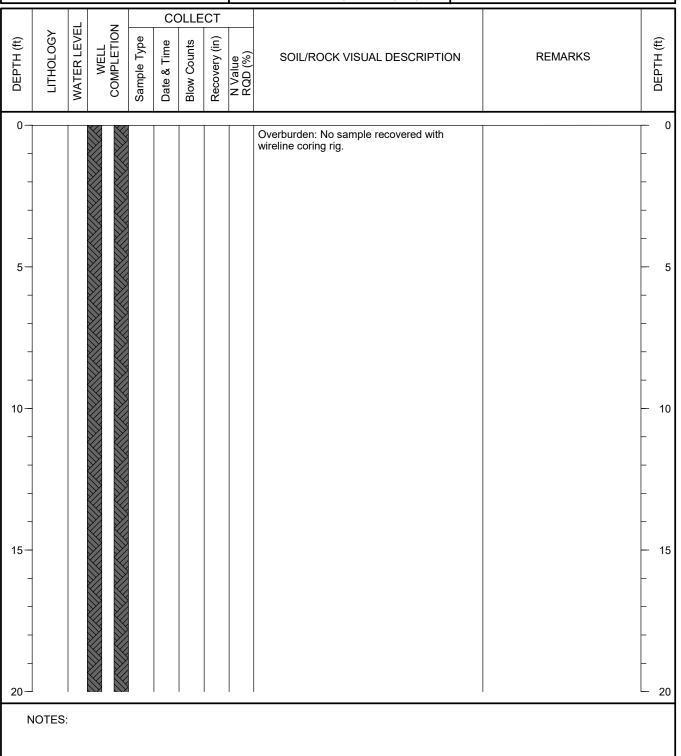
Well Diameter (in): 2

Screen Slot (in):

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

0.010





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-3

Page: 2 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93
Top of Casing Elev. (ft): 1,039.42

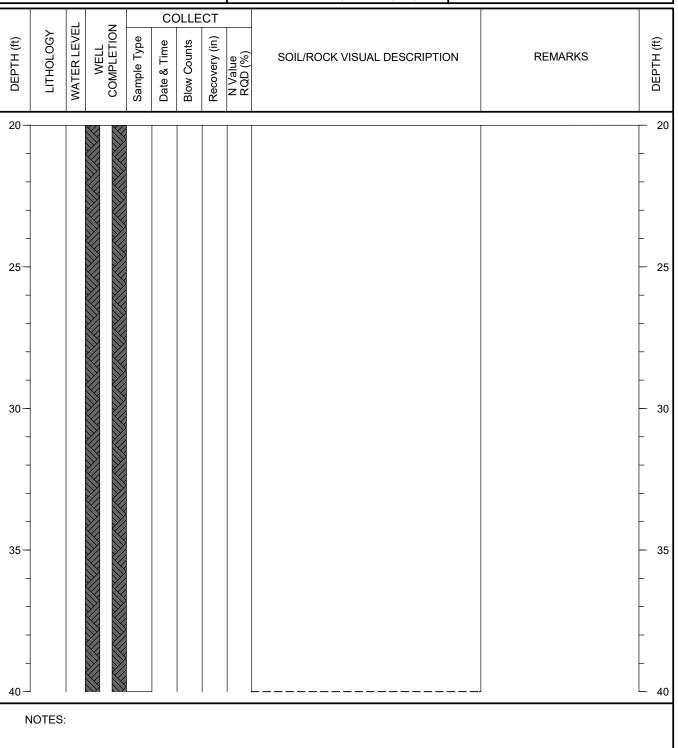
Location (X,Y): N 835,737.2 E 2,511,639.3

Well Depth (ft): 140

Well Diameter (in): 2

Screen Slot (in): 0.010
Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-3

Page: 3 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Drillor: Bill Womank

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93

Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

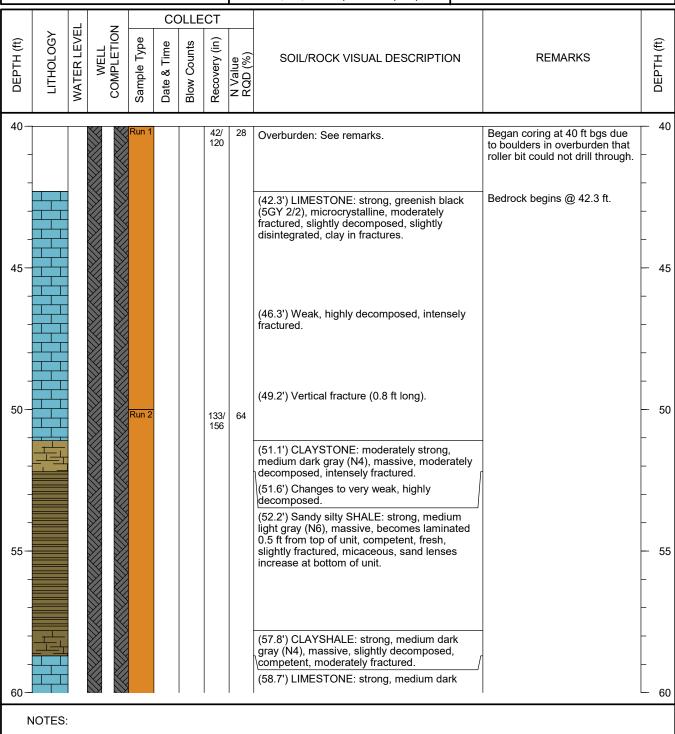
Well Depth (ft): 140

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-3

Page: 4 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93

Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

Well Depth (ft): 140

Well Diameter (in): 2

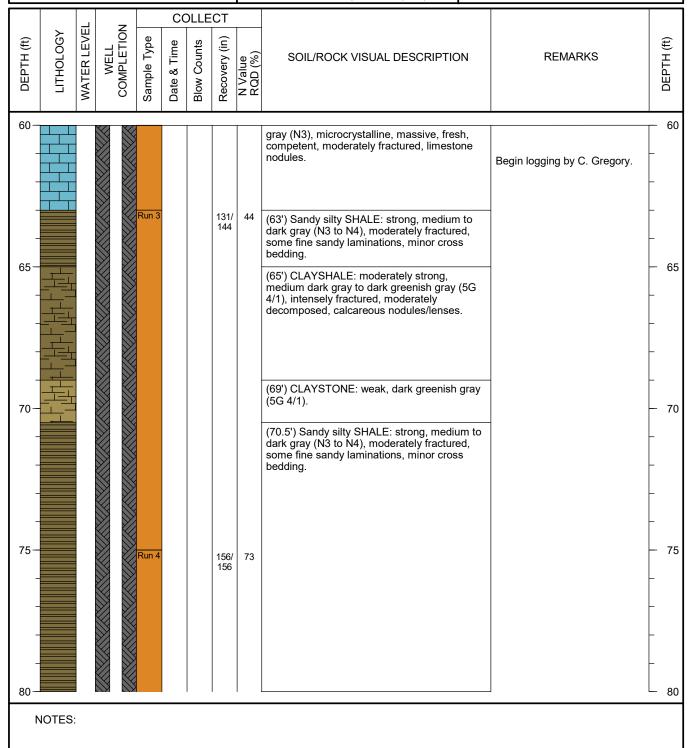
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

Seal Material(s): Bentonite Pellets

Filter Pack: #5 Medium Coarse Sand





Client: **AEP-Cardinal** Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG

Boring/Well No. S-GS-3

Page: 5 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15

Drilling Method: **Rock Core**

Drilling Company:

Drilling Equipment: CS1500 Wireline Rig

Layne Drilling

Driller: **Bill Womack**

Logged By:

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): **Rock Core**

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93 Top of Casing Elev. (ft): 1,039.42

D. Mateas & C. Gregory | Location (X,Y): N 835,737.2 E 2,511,639.3 | Filter Pack:

Well Depth (ft): 140

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

Seal Material(s): **Bentonite Pellets** #5 Medium Coarse Sand

Logged	ву:		D. IVI	ateas	s & C	. Gre	gory	Loca	ation (X,Y): N 835,737.2 E 2,511,639.3	Pack. #5 Medium Coarse San	10
			_		CC	DLLE	СТ				
DEPTH (ft)	LITHOLOGY	WAIEKLEVEL	WELL COMPLETION	Sample Type	Date & Time	Blow Counts	Recovery (in)	N Value RQD (%)	SOIL/ROCK VISUAL DESCRIPTION	REMARKS	DEPTH (ft)
80		· V									– 80
× 3				Run 5			58/60 168/ 180	79	(80') Silty SANDSTONE: strong, light gray (N7), moderately fractured, fresh, cross-bedded, thinly bedded, very fine lgrained. (81') SILTSTONE: strong, medium dark gray (N4). (82') CLAYSHALE: medium dark gray (N4), silty slightly pyritic, with dark greenish gray claystone lenses. (84-85') Very intensely fractured. (86') Silty SHALE: strong, medium dark gray, moderately fractured. (87-88') Calcareous. (88') LIMESTONE: strong, greenish gray (5G 6/1), massive, intensely fractured, slightly decomposed. (90.5') Silty CLAYSTONE: moderately strong, medium dark gray to dark greenish gray, moderately decomposed. (93') CLAYSTONE and CLAYSHALE: moderately strong, medium dark gray, moderately decomposed, moderately fractured, quartz veins (yellowish gray 5Y 8/1).		- 80 - 80 - 80 - 80 - 85 - 85 - 90 - 95 100 - 6
NOT	TES:										\Box



Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-3

Page: 6 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93

Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

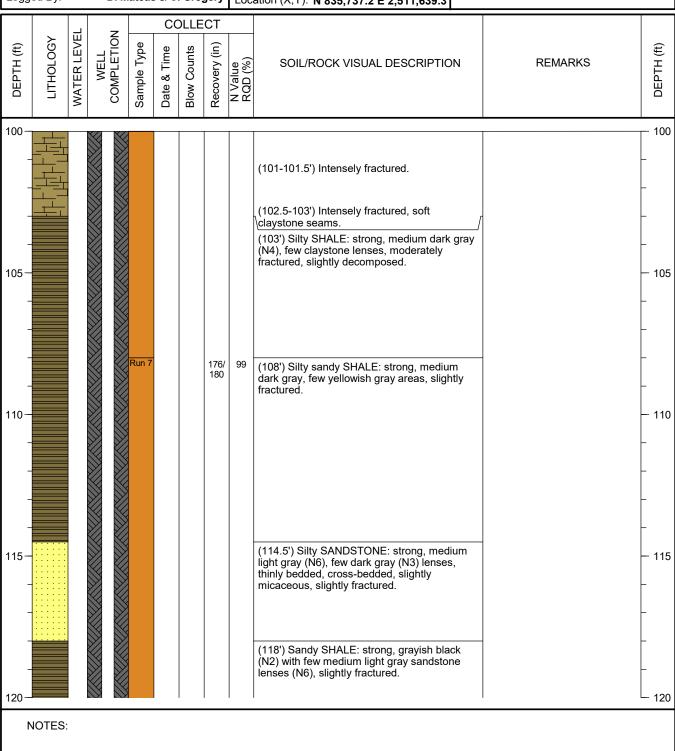
Well Depth (ft): 140

Well Diameter (in): 2

Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC





Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG Boring/Well No. S-GS-3

Page: 7 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93
Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

Well Depth (ft): 140

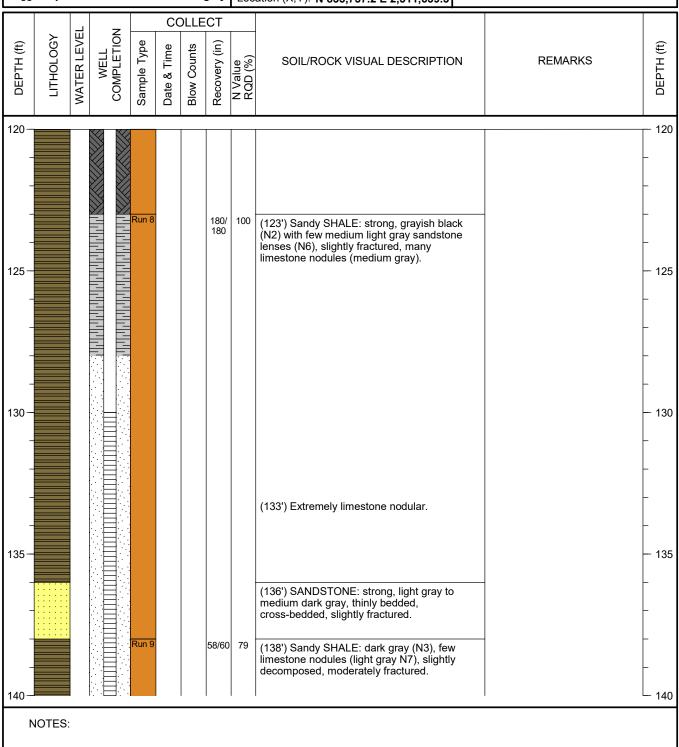
Well Diameter (in): 2

Screen Slot (in):

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

0.010





engineers | scientists | innovators

Client: AEP-Cardinal Project: CHE8126L

Address: 3202 Twp Rd 163, Brilliant, OH

BORING LOG
Boring/Well No. S-GS-3

Page: 8 of 8

Drilling Start Date: 03/16/2016 10:45

Drilling End Date: 03/21/2016 16:15
Drilling Company: Layne Drilling

Drilling Method: Rock Core

Drilling Equipment: CS1500 Wireline Rig

Driller: Bill Womack

Logged By: D. Mateas & C. Gregory

Boring Depth (ft): 143

Boring Diameter (in): 6

Sampling Method(s): Rock Core

DTW During Drilling (ft):

Ground Surface Elev. (ft): 1,036.93
Top of Casing Elev. (ft): 1,039.42

Location (X,Y): N 835,737.2 E 2,511,639.3

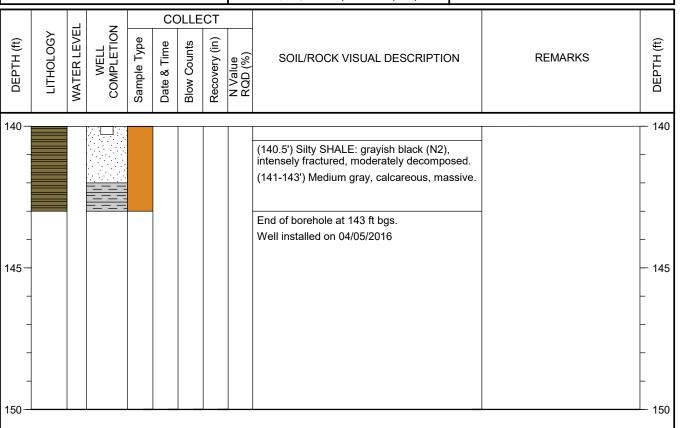
Well Depth (ft): 140

Well Diameter (in): 2
Screen Slot (in): 0.010

Riser Material: Sch 40 PVC

Screen Material: Pre-packed Sch 40 PVC

Seal Material(s): Bentonite Pellets
Filter Pack: #5 Medium Coarse Sand



NOTES:

APPENDIX D WELL CONSTRUCTION LOGS



JOB NUMBER

COMPANY AMERICAN ELECTRIC POWER WELL No. <u>CA-0623A</u> BORING No. <u>CA-0623A</u> INSTALLED <u>8/16/16</u> PROJECT CARDINAL LANDFILL COORDINATES N 836,300.1 E 2,514,227.5 SYSTEM State Plane using NAD27/29 TOP RISER: 1162.72 FT. GROUND ELEVATION 1159.62 FT. GROUT SEAL: BENTONITE CHIPS TOP BENTONITE SEAL: 1012.62 FT. BENTONITE SEAL: PELLETS SCREEN: 2" dia., U-PACK .10 SLOT, 10.0' **GRAVEL PACK:** TOP GRAVEL PACK: 1005.62 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 1005.62 FT. SPACERS, DEPTH: 20',60',100',140' BOTTOM SCREEN: 995.62 FT. BOTTOM WELL: 995.62 FT. BOTTOM GRAVEL PACK: 995.62 FT. BOTTOM BORING: 995.62 FT.

GEOMCNST CD_FGD_LANDFILL BORINGS & WELLS.GPJ AEP.GDT 8/22/16

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-1** BORING No. **8502** INSTALLED **12/12/85** PROJECT CARDINAL PLANT COORDINATES N 831,399.8 E 2,515,207.8 SYSTEM STATE PLANE TOP RISER: 1001.23 FT. GROUND ELEVATION 999.59 FT. GROUT SEAL: CEMENT\BENTONITE TOP BENTONITE SEAL: 970.70 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., POROUS POLYETHYLENE, 4.0 GRAVEL PACK: # 4 OHIO QUARTZ TOP GRAVEL PACK: 965.50 FT. ELEV. CHECK VALVE: 935.69 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 935.09 FT. SPACERS, DEPTH: 4' GEOMON BOTTOM SCREEN: 931.09 FT. BOTTOM WELL: 931.00 FT.

BOTTOM GRAVEL PACK: 929.50 FT.

BOTTOM BORING: 929.50 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. <u>S-10</u> BORING No. <u>CA-0607</u> INSTALLED <u>1/9/07</u> PROJECT CARDINAL LANDFILL COORDINATES N 831,867.6 E 2,516,495.5 SYSTEM TOP RISER: 1005.19 FT. GROUND ELEVATION 1002.48 FT. GROUT SEAL: 75 GALS QUICK GROUT TOP BENTONITE SEAL: 980.38 FT. BENTONITE SEAL: 100# BENONITE PELLETS SCREEN: 2' dia., 0.20 SLOT, 19' GRAVEL PACK: #4 QUARTZ 500 LBS TOP GRAVEL PACK: 973.68 FT. RISER PIPE: 1", dia., PVC TOP SCREEN: 962.78 FT. SPACERS, DEPTH: 51', 21' -SWL @ INSTALL 44.8' -DRILLED W/6" AIR HAMMER -FLUSHED W/700 GALS WATER -DECONNED TOOLS 01/08/07 -3' SS Pump Type -Pump intake @ 56.1' BOTTOM SCREEN: 943.78 FT. BOTTOM WELL: 943.38 FT. BOTTOM GRAVEL PACK: 941.08 FT.

BOTTOM BORING: 902.68 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ BORING No. CA-0601 ___ INSTALLED 6/12/07 WELL No. **S-17** PROJECT CARDINAL LANDFILL COORDINATES N 833,612.2 E 2,512,715.1 SYSTEM TOP RISER: 1198.00 FT. GROUND ELEVATION 1195.63 FT. GROUT SEAL: ~200 GALS QUICK GROUT TOP BENTONITE SEAL: 1013.83 FT. BENTONITE SEAL: 75 LBS 3/8" PELLETS SCREEN: 2" dia., .020 SLOT, 10.0' GRAVEL PACK: 300 LBS #4 QUARTZ TOP GRAVEL PACK: 1008.43 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 1005.33 FT. SPACERS, DEPTH: 170',120',80',40' NOTES: -Decon 07/11/07 -Drilled w/6" Air Hammer -SWL @ Install 193.4' -Hydrated Pellets -3' SS Pump Type -Pump intake @ 199.5' BOTTOM SCREEN: 995.83 FT. BOTTOM WELL: 995.13 FT. BOTTOM GRAVEL PACK: 993.33 FT.

BOTTOM BORING: 780.13 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER ___ BORING No. CA-0603 ___ INSTALLED 8/22/07 WELL No. **S-18** PROJECT CARDINAL LANDFILL COORDINATES N 832,194.6 E 2,513,596.2 SYSTEM TOP RISER: 1155.37 FT. GROUND ELEVATION 1153.26 FT. GROUT SEAL: ~250 GALS QUICK GROUT TOP BENTONITE SEAL: 1012.86 FT. BENTONITE SEAL: 100 LBS 3/8" PELLETS SCREEN: 2" dia., .020 SLOT, 10' GRAVEL PACK: 250 LBS #4 QUARTZ TOP GRAVEL PACK: 1003.26 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 999.46 FT. SPACERS, DEPTH: 110', 30' NOTES: -Decon 07/09/07 -Drilled w/6" Air Hammer -SWL @ Install 155.2' -Hydrated Pellets -3' SS Pump Type -Pump intake @ 163' BOTTOM SCREEN: 989.96 FT. BOTTOM WELL: 989.26 FT. BOTTOM GRAVEL PACK: 987.86 FT.

BOTTOM BORING: 987.86 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. <u>S-19A</u> BORING No. <u>CA-0606A</u> INSTALLED <u>7/2</u>8/11 PROJECT CARDINAL LANDFILL COORDINATES N 830,793.8 E 2,514,074.6 SYSTEM TOP RISER: 1098.60 FT. GROUND ELEVATION 1095.98 FT. GROUT SEAL: 750 LBS HOLE PLUG TOP BENTONITE SEAL: 1015.98 FT. BENTONITE SEAL: 200 LBS 3/8" PELLETS SCREEN: 2" dia., .020 SLOT, 9.7' GRAVEL PACK: 600 LBS #4 QUARTZ TOP GRAVEL PACK: 1001.08 FT. RISER PIPE: 2", dia., TOP SCREEN: 995.98 FT. SPACERS, DEPTH: N/A NOTES: -Replacement well for S-19 -Decon 07/27/11 / High-pressure wash / Billiant water -Drilled w/6" air hammer -SWL @ install 109.0' -Hole would not grout up BOTTOM SCREEN: 985.98 FT. BOTTOM WELL: 985.28 FT. BOTTOM GRAVEL PACK: 984.98 FT.

BOTTOM BORING: 986.28 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-2** BORING No. **8503** INSTALLED **12/17/85** PROJECT CARDINAL PLANT COORDINATES N 831,038.2 E 2,514,714.2 SYSTEM STATE PLANE TOP RISER: 1039.89 FT. GROUND ELEVATION 1038.60 FT. GROUT SEAL: CEMENT\BENTONITE TOP BENTONITE SEAL: 998.10 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., POROUS POLYETHLENE, 4.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 992.10 FT. ELEV. CHECK VALVE: 958.70 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 958.10 FT. SPACERS, DEPTH: 4' GEOMON BOTTOM SCREEN: 954.10 FT.

BOTTOM WELL: 954.10 FT.

BOTTOM BORING: 948.60 FT.

BOTTOM GRAVEL PACK: 948.60 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER __ BORING No. CA-0619 INSTALLED 8/24/06 WELL No. S-20 PROJECT CARDINAL LANDFILL COORDINATES N 830,850.2 E 2,515,582.3 SYSTEM TOP RISER: 1005.88 FT. GROUND ELEVATION 1003.43 FT. GROUT SEAL: BENTONITE SLURRY TOP BENTONITE SEAL: 963.13 FT. BENTONITE SEAL: PELLETS SCREEN: 1" dia., .020 SLOT, 25.0' GRAVEL PACK: FILTER PRO TOP GRAVEL PACK: 957.93 FT. RISER PIPE: 2", dia., PVC TOP SCREEN: 943.43 FT. SPACERS, DEPTH: NOTES:
-Surface Seal: Cement
-Annular Sealant: Bentonite Slurry, Tremie Pipe
Installation, Overnight Setting Time
-Bentonite Seal: Poured Slowly, One Hr Setting Time
-Sand Pack: Poured Slowly -3' PVC Pump Type -Pump intake @ 84.5' BOTTOM SCREEN: 918.43 FT. BOTTOM WELL: 918.43 FT. BOTTOM GRAVEL PACK: 916.43 FT.

BOTTOM BORING: 916.43 FT.



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-4** BORING No. **88-5-6** INSTALLED **8/16/88** PROJECT CARDINAL PLANT COORDINATES N 834,352.3 E 2,513,052.2 SYSTEM STATE PLANE TOP RISER: 0.00 FT. GROUND ELEVATION 1010.90 FT. GROUT SEAL: BENTONITE TOP BENTONITE SEAL: 983.90 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PVC SCH 40 20 SLOT, 2.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 978.90 FT. ELEV. CHECK VALVE: 931.90 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 0.00 FT. SPACERS, DEPTH: WELLS S-4 AND M-2 IN SAME BORING. GEOMON BOTTOM SCREEN: 0.00 FT. BOTTOM WELL: 928.90 FT.

BOTTOM GRAVEL PACK: 926.90 FT.

BOTTOM BORING: 805.90 FT.

AMERICAN ELECTRIC POWER SERVICE CORPORATION AEP CIVIL ENGINEERING LABORATORY



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-5** BORING No. **88-7-8** INSTALLED **8/10/88** PROJECT CARDINAL PLANT COORDINATES N 834,917.6 E 2,513,916.2 SYSTEM STATE PLANE TOP RISER: 0.00 FT. GROUND ELEVATION 1000.20 FT. GROUT SEAL: BENTONITE TOP BENTONITE SEAL: 980.60 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PVC SCH 40 20 SLOT, 2.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 975.60 FT. ELEV. CHECK VALVE: 930.60 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 0.00 FT. SPACERS, DEPTH: WELLS S-5 AND M-3 IN SAME BORING. GEOMON BOTTOM SCREEN: 0.00 FT. BOTTOM WELL: 927.60 FT.

BOTTOM GRAVEL PACK: 925.60 FT.

BOTTOM BORING: 805.40 FT.



MONITORING WELL CONSTRUCTION JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-6** BORING No. **88-9-10** INSTALLED **8/4/88** PROJECT CARDINAL PLANT COORDINATES N 834,577.4 E 2,513,679.4 SYSTEM STATE PLANE TOP RISER: 0.00 FT. GROUND ELEVATION 1010.90 FT. GROUT SEAL: BENTONITE TOP BENTONITE SEAL: 971.20 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PVC SCH 40 20 SLOT, 2.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 966.20 FT. ELEV. CHECK VALVE: 920.20 FT. RISER PIPE: 0.8, dia., PVC SCH 80 TOP SCREEN: 0.00 FT. SPACERS, DEPTH: WELL S-6 AND M-4 IN SAME HOLE. GEOMON BOTTOM SCREEN: 0.00 FT. BOTTOM WELL: 917.20 FT.

BOTTOM GRAVEL PACK: 916.20 FT.

BOTTOM BORING: 780.90 FT.



JOB NUMBER COMPANY AMERICAN ELECTRIC POWER WELL No. **S-7** BORING No. **90CA22-S** INSTALLED **8/14/90** PROJECT CARDINAL PLANT COORDINATES N 831,920.2 E 2,516,676.4 SYSTEM STATE PLANE TOP RISER: 1010.98 FT. GROUND ELEVATION 1008.52 FT. GROUT SEAL: BENSEAL TOP BENTONITE SEAL: 975.42 FT. BENTONITE SEAL: PI PELLETS SCREEN: 1.25 dia., PVC SCH 40 20 SLOT, 2.0 GRAVEL PACK: #4 OHIO QUARTZ TOP GRAVEL PACK: 969.92 FT. ELEV. CHECK VALVE: 942.92 FT. RISER PIPE: 1.0, dia., PVC SCH 80 TOP SCREEN: 942.32 FT. SPACERS, DEPTH: **GEOMON** BOTTOM SCREEN: 940.32 FT. BOTTOM WELL: 939.92 FT. BOTTOM GRAVEL PACK: 937.92 FT.

BOTTOM BORING: 937.92 FT.



WELL CONSTRUCTION LOG ABOVE GROUND COMPLETION

Drilling Drillers:	D. (LOCID): S-G Company: Layr Danny Allen st/Engineer: D. I	ie	. Muenich
2.2	Height Above Land Surface		
0 60	DEPTH BLS Land Surface —		Measuring Pt. 1014.57 Elevation (MPELEV)
66	Seal End Depth (SBDEPTH) - Screen Begin Depth (SBDEPTH) -		Seal 6 Length 2 Screen Length Filter Pack Length
78			(SCRLENGTH) 15 (FPL)
		7	422
78.3	Total Depth (TOTDEPTH)		Sump 4"
81			2.7
01		Borehole	
95		Diameter 6"	
107			
	rilled depth = 107	7'; backfille	d to 81' with sand
and chip	os.		

Site: <u>AEP – Cardinal</u> Project Number: <u>CHE8126</u> Installation Method: <u>HSA</u> Casing Installation Date (INSDATE): <u>4/12/16</u> Well Type (WTCCODE): <u>Monitoring Well</u> Well Completion Method (WCMCODE): <u>Above Grade</u> Geologic Completion Zone (GZCODE):	
Well Completion 2 Guard Posts (Y / N) Date: Surface Pad Size: 2 ft x 2 ft x 6" Protective Casing or Cover Diameter/Type: 4" locking flip-top Depth BGS: 2.5 Weep Hole (Y / N) Grout Composition/Proportions: 150 lbs Haliburton Benton	
Quick Grout / 100 gal. H ₂ O; 15 x 50 lb bags	
Placement Method: pressure tremie	
Seal Date: <u>4/12/16</u>	
Type: 3/8" coated bentonite pellets; 2 x 5 gal buckets	
Source: Pel-Plug Western Bentonite	
Set-up/Hydration Time: 30 mins	
Placement Method: poured gravity	
Vol. Fluid Added: N/A - submerged	
Filter Pack	
Type: #5 filter sand	
Source: Flat Rock Bagging, Sparta, MI	
Amount Used: 30 x 50 lb bags	
Placement Method: Poured gravity	
Well Riser Pipe	
Casing Material (CMACODE): <u>Sch. 40 PVC</u>	
Casing Inside Diameters (CASDIAM): 2.0	in.
Screen	
Material: Sch. 40 PVC	
Inside Diameter (SCRDIAM): 2.0	in.
Screen Slot Size: (SOUA): 0.010 10-slot	in.
Percent Open Area (PCTOPEN):	ш.
Sump or Bottom Cap (Y) N)	
Type/Length: 4" Sch. 40 PVC	
Backfill Plug (Y (N)	
Material: 3/8" med. crushed bentonite chips	_
Placement Method: poured gravity	
Set-up/Hydration Time:	
Total Water Volume During Construction Introduced (Gal): 0 Recovered	

Reviewed By: J. Neil Couch Date: 4/22/2016



WELL CONSTRUCTION LOG ABOVE GROUND COMPLETION

Well I.D. (LOCID): S-GS-2	Site: AEP – Cardinal Project Number: CHE8126L
Drilling Company: Layne	Installation Method: HSA
Drillers: Danny Allen	Casing Installation Date (INSDATE): 4/12/16
Geologist/Engineer: D. Mateas / M. Muenich	
Signature:	
	Geologic Completion Zone (GZCODE):
3 Height Above	
Land Surface	
	Well Completion
Measuring Pt. 100	2 Guard Posts (Y / N) Date:
	1.75 Surface Pad Size: 2 ft x 2 ft x 6"
Elevation	Protective Casing or Cover
0 DEPTH BLS (MPELEV)	Diameter/Type: 4" locking flip-top
Land Surface	Depth BGS: 2 Weep Hole (Y/N)
	Grout
	Composition/Proportions: 150 lbs Haliburton Bentonite
INTERVAL LENG	Quick Grout / 100 gal. H ₂ O
67 INTERVAL LENG	Placement Method: pressure tremie
	Seal Date: 4/12/16
Seal 5	Seal Date: 4/12/16 Type: 3/8" coated bentonite pellets
72 Seal End Depth	Source: Pel-Plug Western Bentonite
/2 Seal End Depth (SBDEPTH)	Set-up/Hydration Time: 30 mins
	Placement Method: poured gravity
Screen 2	Vol. Fluid Added: N/A - submerged
//4 Begin Depth (SBDEPTH)	Filter Pack
	Type: #5 filter pack sand
	Source: Flat Rock Bagging, Sparta, MI
Screen	Amount Used: 10 x 50 lb bags
Length	Filter Pack Placement Method: poured gravity
	Well Riser Pipe
(SCRLENGTH)	14 Casing Material (CMACODE): Sch. 40 PVC
	Casing Inside Diameters (CASDIAM): 2.0 in.
84	(FPL) Screen
	Material: Pre-packed Sch. 40 PVC
94.2 Sump 4"	Inside Diameter (SCRDIAM): 2.0 in.
84.3 Total Depth Length	Screen Slot Size: (SOUA): 0.010 10-slot in.
(TOTDEPTH)	Percent Open Area (PCTOPEN):
86	Sump or Bottom Cap (Y) N)
	Type/Length: 4" Sch. 40 PVC
Borehole Diameter	Backfill Plug (Y) N)
94 6"	Material: 3/8" coated bentonite pellets
	Placement Method: poured gravity
	Set-up/Hydration Time: 45 mins
Comments	Total Water Volume During Construction
Total boring depth = 94 ft; backfilled with chips to	ĕ
86'.	(Gal): -
<u> </u>	Reviewed By: J. Neil Couch Date: 4/22/2016



WELL CONSTRUCTION LOG ABOVE GROUND COMPLETION

Orillers: <u>Danny Allen</u> Geologist/Engineer: <u>J. B</u> Gignature:	Sannantine
2.7 Height Above Land Surface	
0 DEPTH BLS Land Surface —	Measuring Pt. 1039.42 Elevation (MPELEV)
123	INTERVAL LENGTH
Seal End Depth (SBDEPTH) –	Seal 5 Length 2
Begin Depth (SBDEPTH)	Screen Length 10 Filter Pact Length (SCRLENGTH) 14
140	(SCRLENGTH) (FPL)
40.3 Total Depth (TOTDEPTH)	Sump 4''
42	Borehole Diameter
03.5	6''

ite: <u>AEP – Cardinal</u> Project Number: <u>CHE8126L</u>
nstallation Method: HSA/Rotary
asing Installation Date (INSDATE): 4/5/16
Vell Type (WTCCODE): Monitoring Well
Vell Completion Method (WCMCODE): Above Grade
Geologic Completion Zone (GZCODE):
Well Completion
2 Guard Posts (Y / N) Date:
Surface Pad Size: 2 ft x 2 ft x 6"
Protective Casing or Cover
Diameter/Type: 4" locking flip-top
Depth BGS: 2 Weep Hole (Y/N)
Grout
Composition/Proportions: <u>150 lbs Haliburton Bentonite</u>
Quick Grout / 100 gal. H ₂ O
Placement Method: <u>pressure tremie</u>
G1 D. 4/5/16
Seal Date: <u>4/5/16</u>
Type: 3/8" coated bentonite pellets
Source: Pel-Plug Western Bentonite
Set-up/Hydration Time: 30 mins
Placement Method: poured gravity
Vol. Fluid Added: <u>N/A - submerged</u> Filter Pack
Type: #5 med. coarse sand
Source: Flat Rock, Sparta, MI Amount Used: 8 x 50 lb bags
Placement Method: poured gravity
r racement witchod. poured gravity
Well Riser Pipe
Casing Material (CMACODE): Sch. 40 PVC
Casing Inside Diameters (CASDIAM): 2.0 in.
Screen
Material: Pre-packed Sch. 40 PVC
Inside Diameter (SCRDIAM): 2.0 in.
Screen Slot Size: (SOUA): 0.010 10-slot in.
Percent Open Area (PCTOPEN):
Sumpor Bottom Cap (Y) N)
Type/Length: 4" Sch. 40 PVC
Backfill Plug (Y) N)
Material: 3/8" med. crushed bentonite chips
Placement Method: poured gravity
Set-up/Hydration Time:
Total Water Volume During Construction
Introduced (Gal):0 Recovered
(Gal):
Reviewed By: J. Neil Couch Date: 5/3/2016

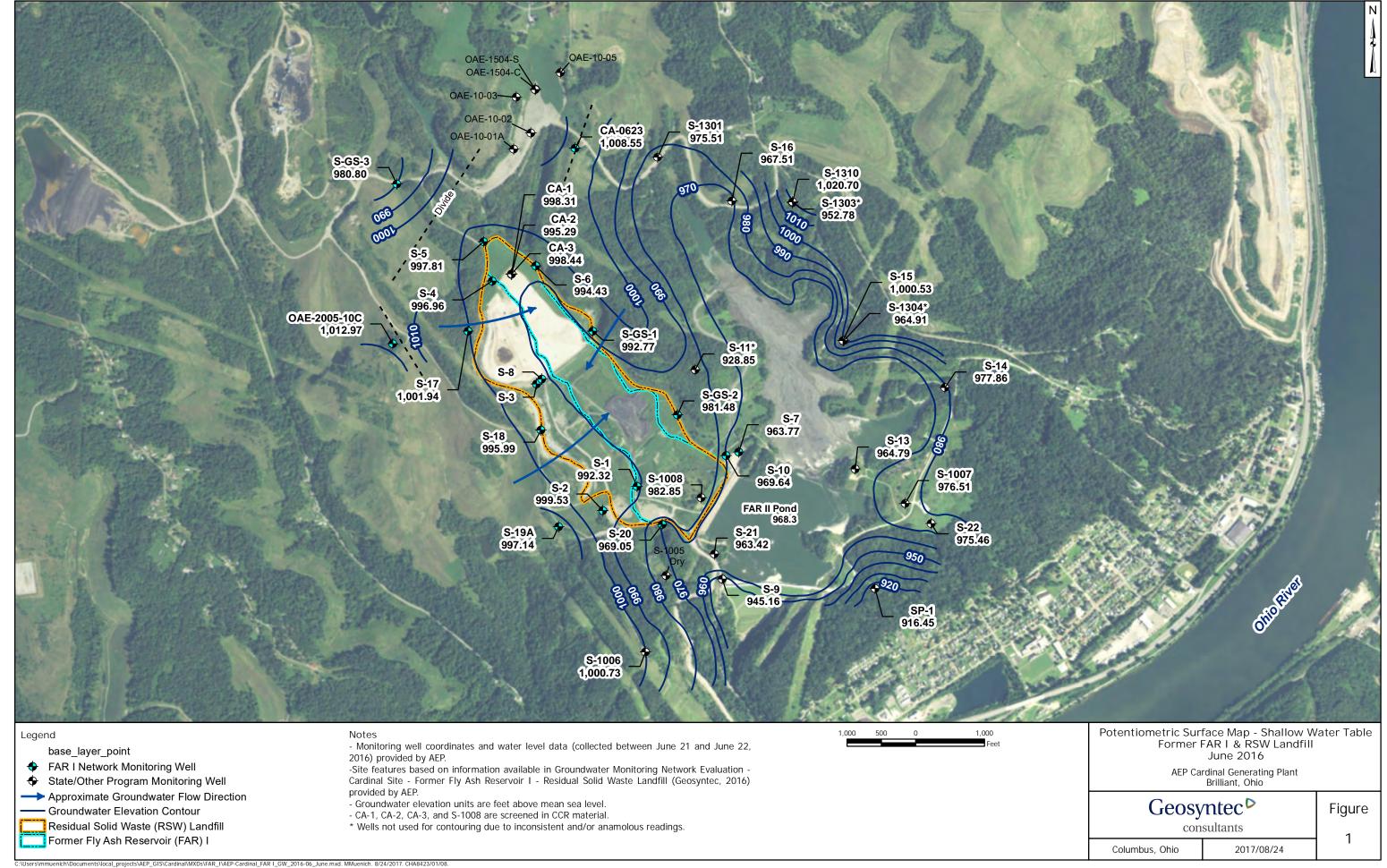


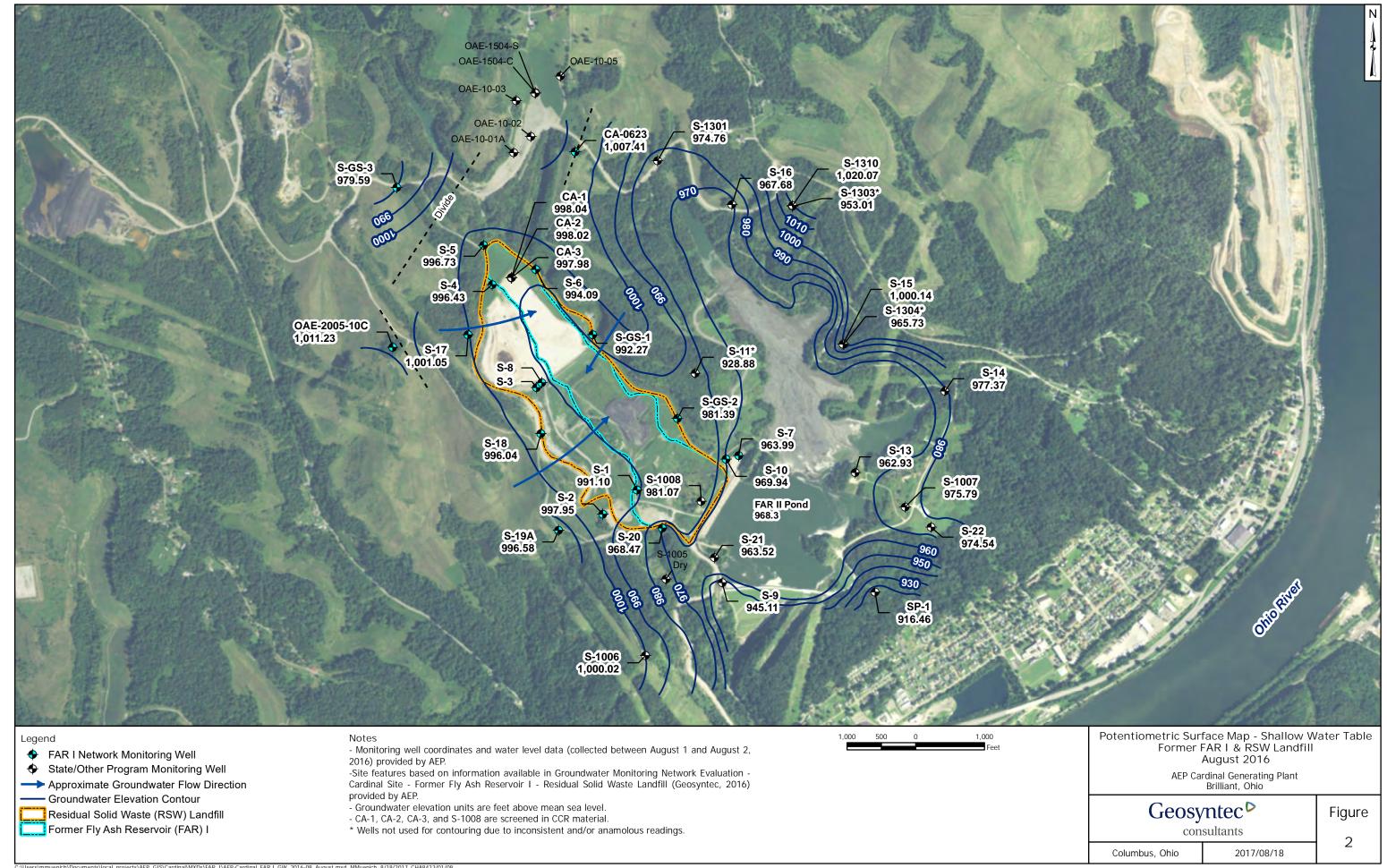
JOB NUMBER WELL No. OAE 0610-CBORING No. INSTALLED 2/16/06 COMPANY AMERICAN ELECTRIC POWER PROJECT CARDINAL FLY ASH DAM WELL NO. OAE 2005 10C COORDINATES N 833 417.3 E 2,511, 621.5 SYSTEM TOP RISER: FT. 1240.85 GROUND ELEVATION 0.00 FT. 1237.93 GROUT SEAL: ~350 Gals Grout 1013.83 TOP BENTONITE SEAL: -224.10 FT. BENTONITE SEAL: 110 lbs Pellets SCREEN: 1" dia., GEOMON, 5.0' GRAVEL PACK: 150 lbs. #4 Quartz 1008.23 TOP GRAVEL PACK: -229.70 FT. 1002.53 TOP SCREEN: -235.40 FT. RISER PIPE: 2", dia., PVC SPACERS, DEPTH: 200', 100' NOTES:
-Set 1" Geomon on 2" riser pipe
-SWL @ 52.4'
-Drilled w/6" air hammer
-Deconned 02/13/06 / No bailing
-Flushed w/1,000 gals water
-Set protector / poured pad WELL LOG CARD_FA_DAM.GPJ AEP.GDT 6/12/08 997,53 BOTTOM SCREEN: -240.40 FT. 997.43 MONITORS BOTTOM WELL: -240.50 FT. NO. 8 COAL BOTTOM GRAVEL PACK: -241.10 FT. 996.83 996.83 BOTTOM BORING: -241.10 FT.

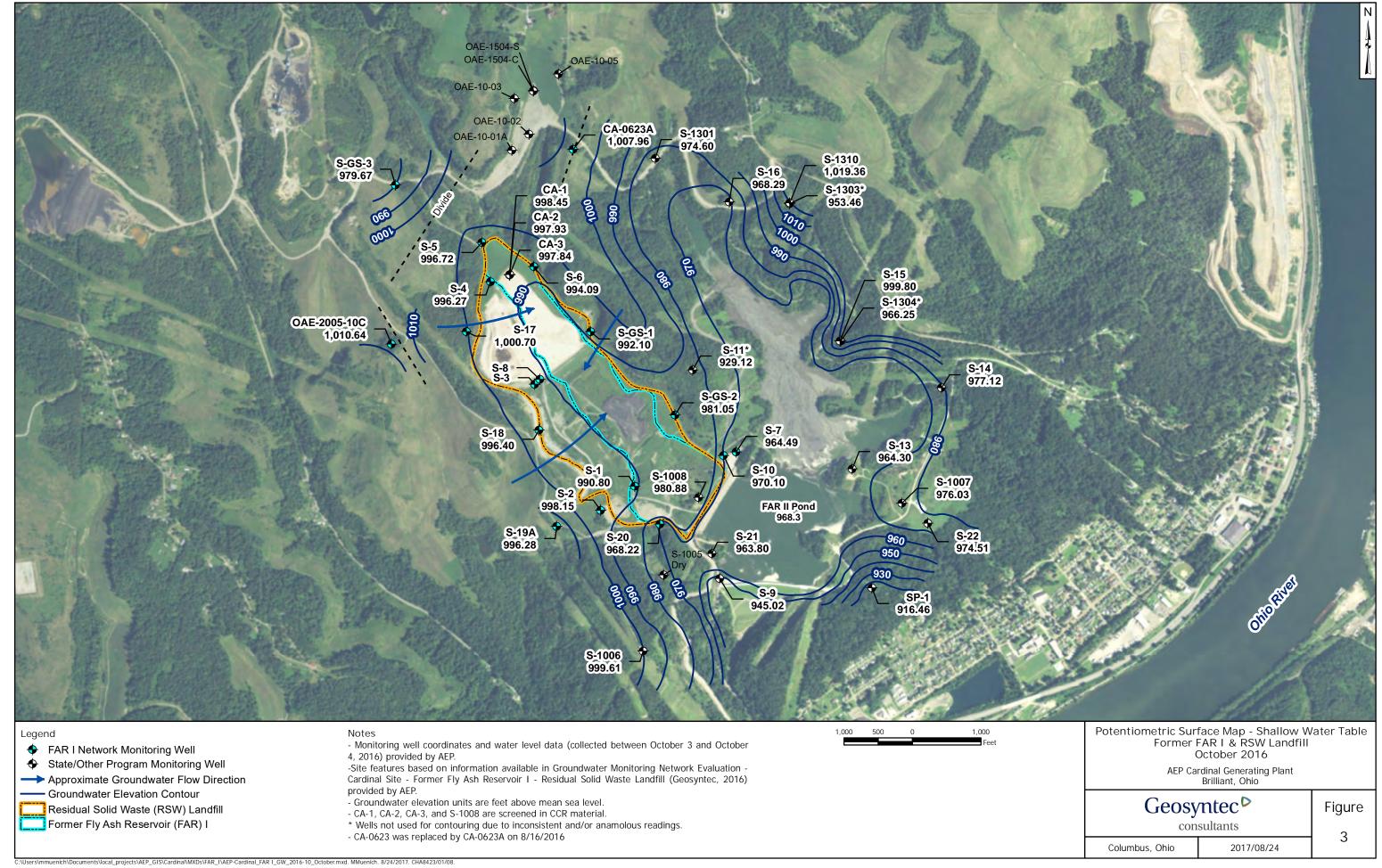
FAR I RSW Landfill

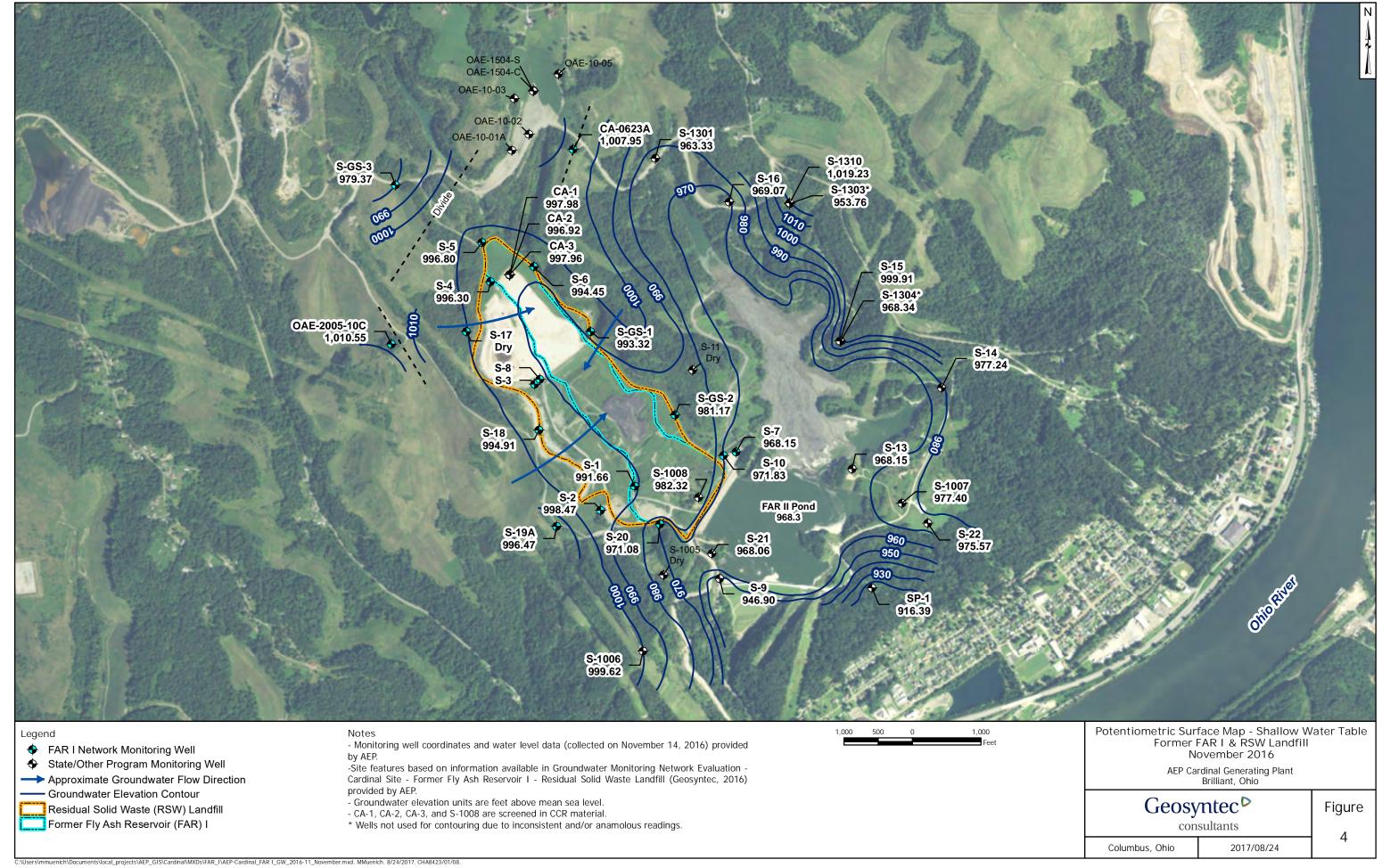
40 CFR 257.101 (f)(1)(iv)(B)(2)(iii)

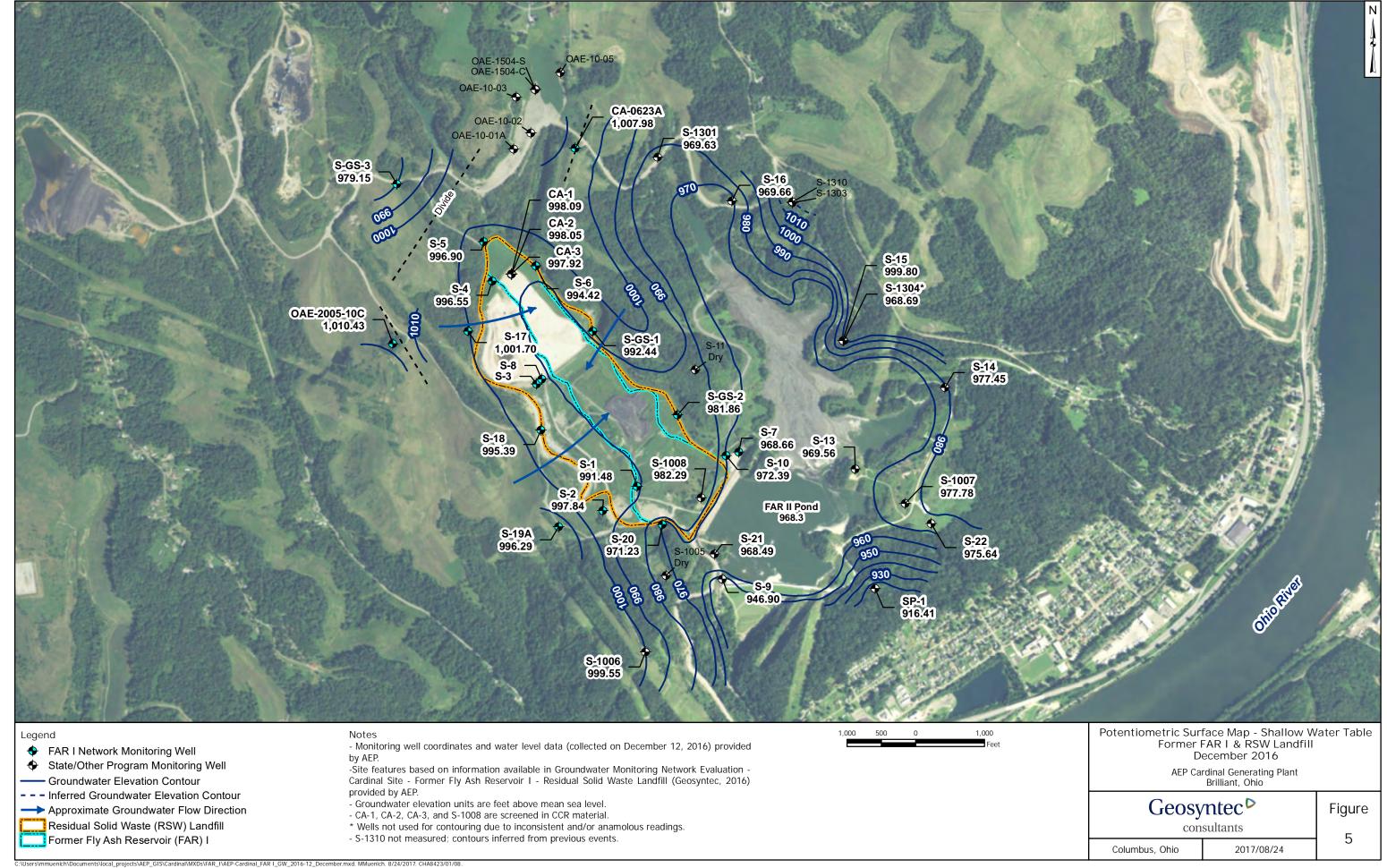
Maps that characterize the direction of groundwater flow accounting for seasonal variations

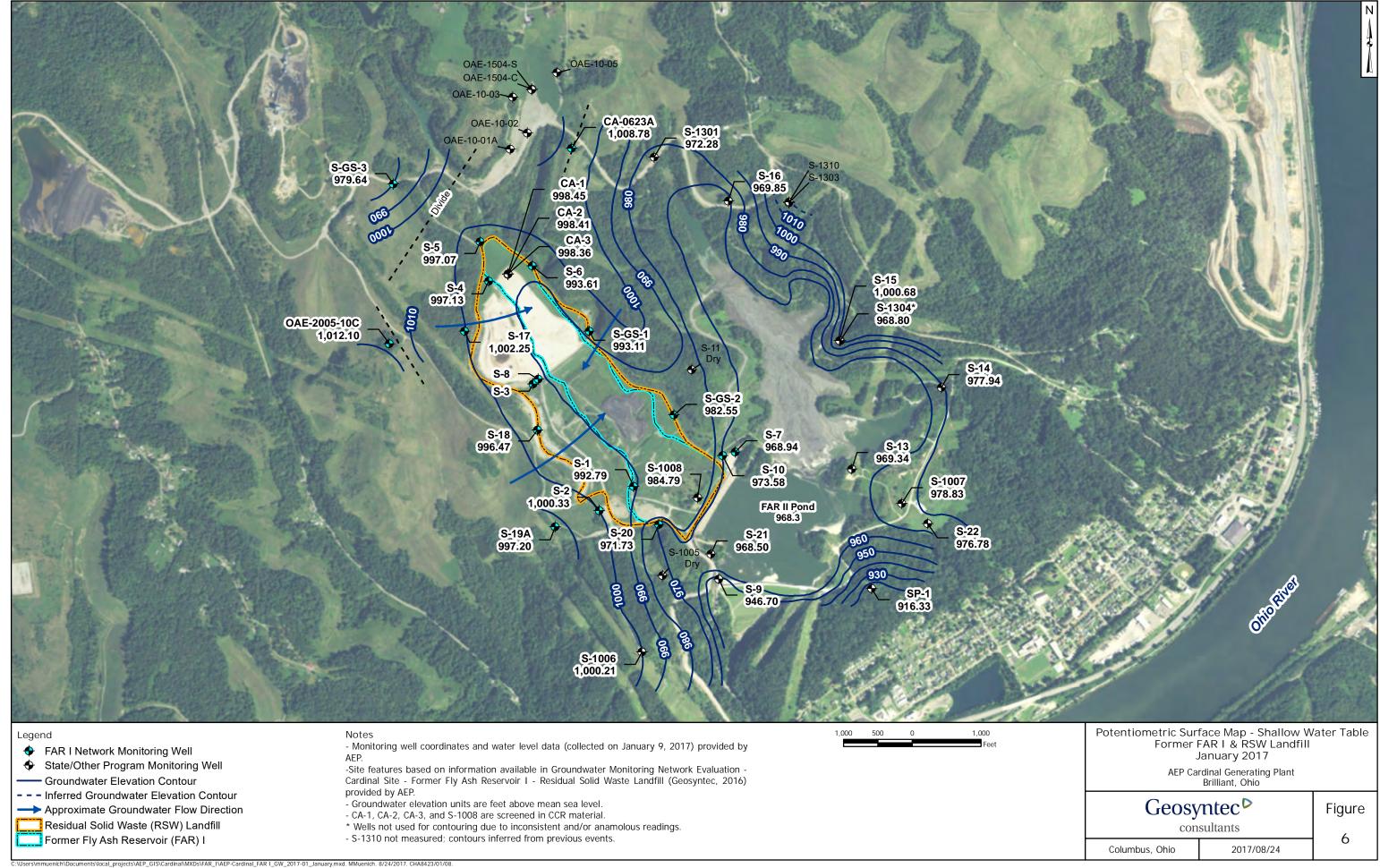


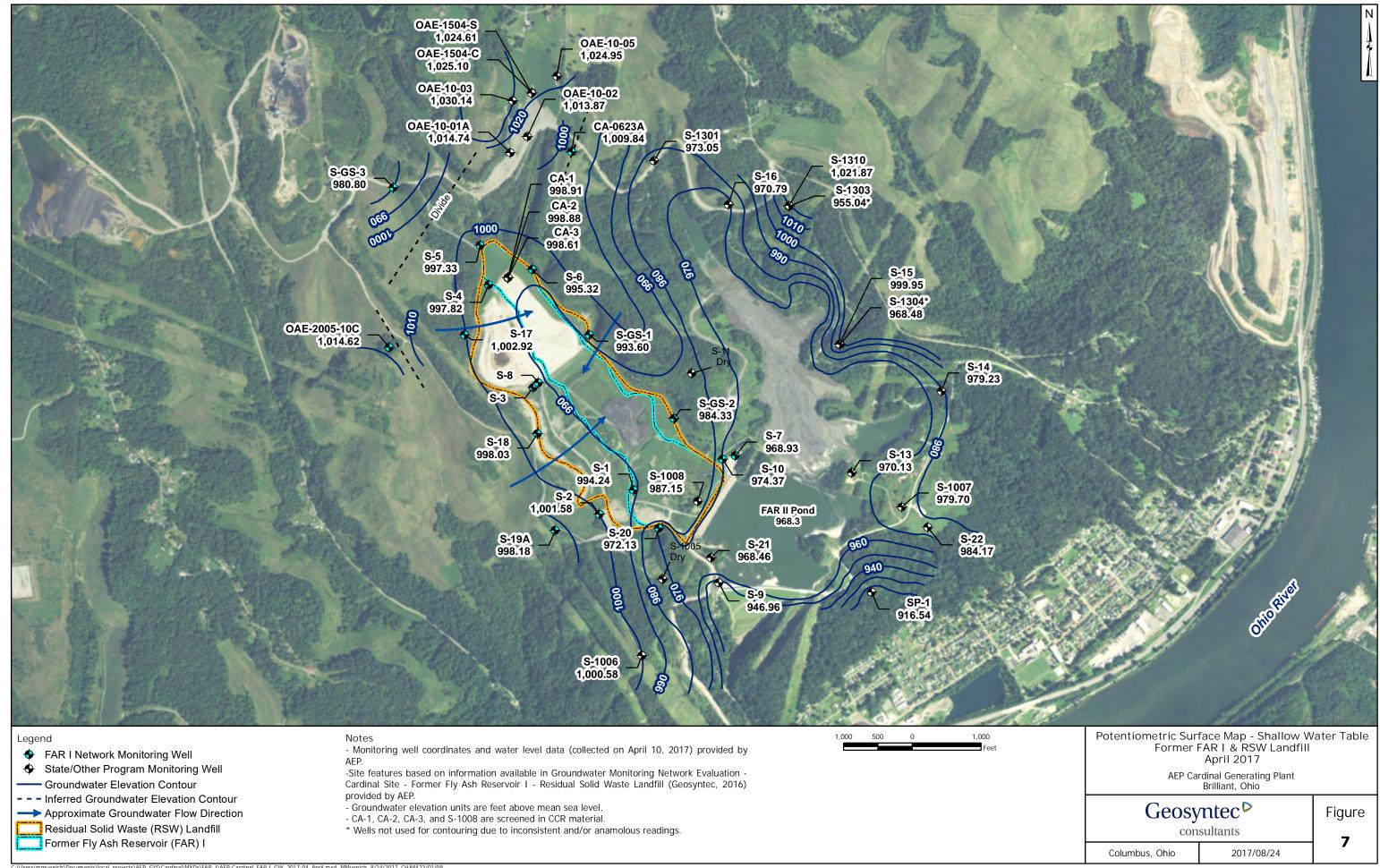


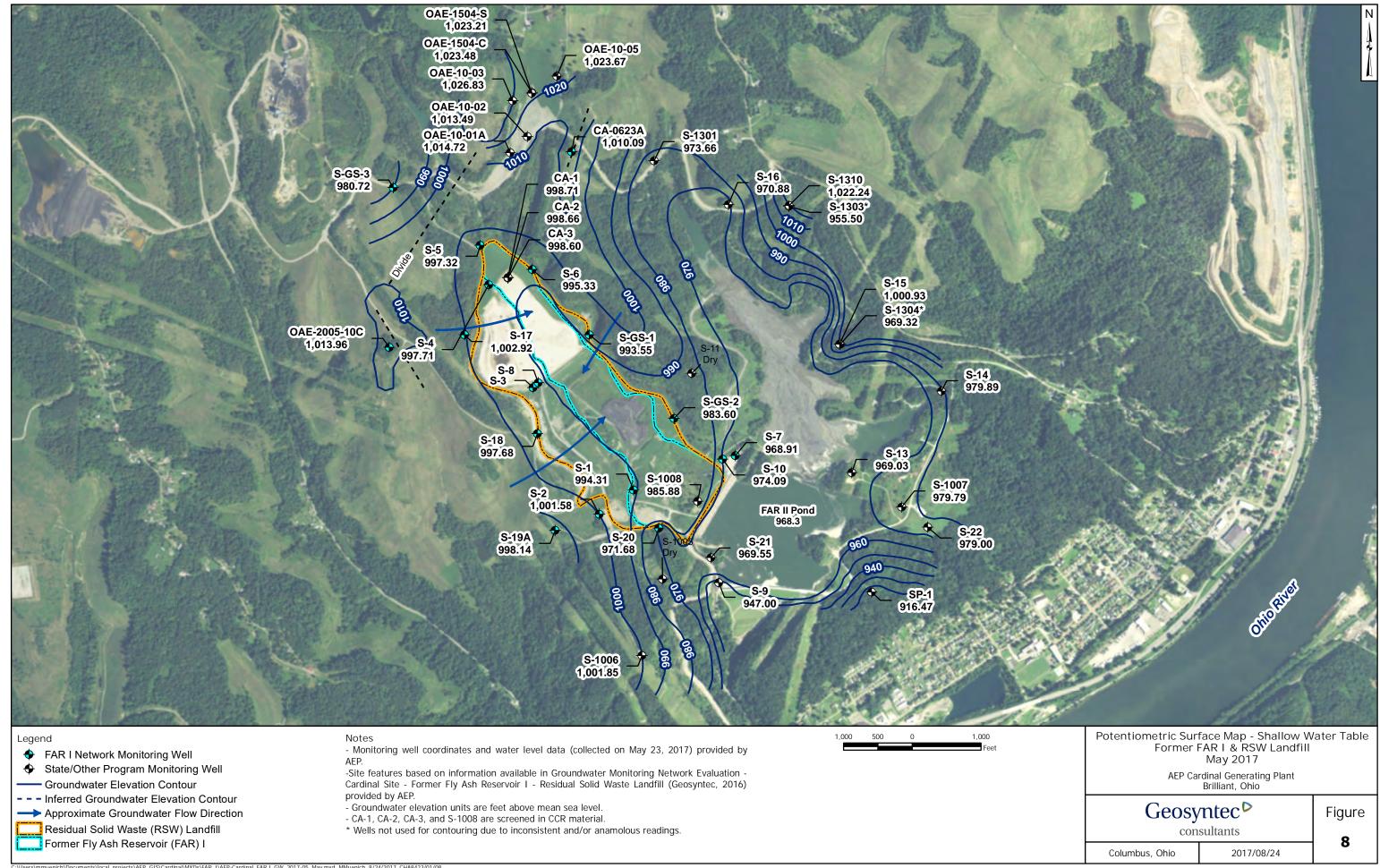












FAR I RSW Landfill 40 CFR 257.101 (f)(1)(iv)(B)(3)

Constituent concentrations, summarized in table form, at each groundwater monitoring well monitored during each sampling event

								Curun	iai i iaiit -	Danaini										
						CA-0623A									OAE-2	005-10-С				
Parameter	Unit	10/13/2016	11/15/2016	12/14/2016	1/10/2017	4/18/2017	5/25/2017	6/21/2017	7/27/2017	9/27/2017	6/29/2016	8/9/2016	12/14/2016	1/12/2017	2/9/2017	4/17/2017	5/31/2017	6/22/2017	7/27/2017	10/3/2017
					Backg	round				Detection]	Backgroun	d				Detection
Antimony	μg/L	0.02J	0.01J	0.01J	0.05U	0.02J	0.05U	0.05U	0.01J	-	0.29	-	0.36	0.07	0.03J	0.04J	0.03J	0.05J	0.28	-
Arsenic	μg/L	0.54	0.34	0.36	0.27	0.4	0.26	0.24	0.26	-	0.92	-	7.87	0.97	0.85	0.67	0.52	0.63	0.96	-
Barium	μg/L	20.1	21.1	21.5	21.2	23	22.5	20.6	21	-	26.6	-	209	45.3	29.5	31.9	32	27.6	41.1	-
Beryllium	μg/L	0.009J	0.006J	0.02U	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.02U	-	0.588	0.041	0.008J	0.007J	0.01J	0.02U	0.04J	-
Boron	mg/L	0.442	0.487	0.388	0.434	0.418	0.474	0.454	0.422	0.488	0.332	-	0.343	0.389	0.451	0.422	0.438	0.437	0.486	0.425
Cadmium	μg/L	0.02U	0.02U	0.02U	0.02U	0.02U	0.02U	0.02U	0.03	-	0.04	-	0.5	0.04	0.01J	0.02J	0.01J	0.007J	0.04J	-
Calcium	mg/L	1.55	1.37	1.22	1.16	1.12	1.15	1.14	1.14	1.11	8.18	-	142	12.4	7.55	7.16	6.19	5.99	10.5	5.52
Chloride	mg/L	24.8	25.4	25.7	24	16.4	14.6	16.2	23.1	24.5	35.9	-	12.2	10.5	10.5	11.4	11.1	11.2	12.3	12.3
Chromium	μg/L	4.1	0.548	0.829	0.12	0.112	0.515	0.062	0.143	-	0.6	-	36.3	3.86	0.824	0.86	0.737	0.244	2.95	-
Cobalt	μg/L	0.116	0.045	0.047	0.026	0.022	0.02	0.022	0.02J	-	0.162	-	12.8	0.852	0.167	0.207	0.151	0.06	0.801	-
Combined Radium	pCi/L	0.587	0.587	0.6	0.344	0.656	0.855	1.031	0.359	-	0.2248	-	2.3997	0.678	0.168	0.244	1.18	2.708	0.095	-
Fluoride	mg/L	2.12	1.98	1.93	1.88	2.02	2.04	2.09	2.03	1.98	1.01	-	0.72	0.82	0.84	0.88	0.87	0.89	0.9	0.96
Lead	μg/L	0.164	0.056	0.064	0.031	0.047	0.024	0.032	0.118	-	0.412	-	475	20.3	4.87	6.84	6.5	3.96	29.3	-
Lithium	mg/L	0.02	0.02	0.02	0.024	0.02	0.025	0.026	0.021	-	0.042	-	0.052	0.039	0.036	0.029	0.025	0.032	0.035	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	-	0.006	0.004J	0.005U	0.003J	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	0.99	0.73	0.6	1.61	0.72	0.81	0.43	0.53	-	10.9	-	3.05	1.27	1.06	1.97	3.99	1.79	2.72	-
Selenium	μg/L	0.06J	0.1U	0.03J	0.1U	0.1U	0.1U	0.1U	0.1U	-	0.1	-	3.1	0.1	0.04J	0.04J	0.1U	0.1U	0.06J	-
Total Dissolved Solids	mg/L	600	666	642	658	666	651	657	644	669	1660	-	1780	1490	1650	1470	1500	1430	1540	1350
Sulfate	mg/L	37.7	44.3	47.9	51.7	43.4	40.3	41.6	36.3	35.6	681	-	482	248	421	442	437	430	403	393
Thallium	μg/L	0.05U	0.05U	0.05U	0.05U	0.05U	0.05U	0.05U	0.02J	-	0.03J	-	0.124	0.02J	0.01J	0.05U	0.01J	0.05U	0.02J	-
рН	SU	8.92	8.7	8.71	8.72	8.93	7.89	8.31	8.5	8.55	8.15	8.12	8.49	8.07	7.66	8.21	8.21	8.01	7.93	8.35

Notes:

mg/L: milligrams per liter μg/L: micrograms per liter pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

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Parameter	Unit	6/30/2016	8/4/2016	10/11/2016	1/11/2017	4/11/2017	4/20/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017	6/24/2016	8/4/2016	10/6/2016	1/17/2017	4/19/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017
						Background					Detection				Backg	round				Detection
Antimony	μg/L	0.02J	0.02J	0.04J	0.02J	0.04J	-	0.03J	0.03J	0.1U	-	4.23	3.6	1.01	0.66	0.53	0.23	0.29	3.57	-
Arsenic	μg/L	0.44	0.35	0.46	0.48	0.4	-	0.42	0.42	0.41	-	13.6	10.9	5.57	5.78	5.99	2.7	3.93	5.87	-
Barium	μg/L	24.8	24.6	23.1	23.6	23.2	•	23.4	22.9	22.6	-	19.5	18.4	18.1	20.3	17.3	16.8	17.4	16.8	-
Beryllium	μg/L	0.02U	0.02U	0.02U	0.02U	0.02U	-	0.02U	0.04U	0.04U	-	0.04J	0.02J	0.03J	0.044	0.03J	0.02J	0.03J	0.02J	-
Boron	mg/L	0.924	0.937	0.921	0.856	0.874	•	0.897	0.895	0.855	0.834	1.66	1.91	2.74	2.32	1.8	1.9	1.77	2.09	2.73
Cadmium	μg/L	0.01J	0.04	0.04	0.04	0.1	•	0.06	0.06	0.05	-	0.62	1.04	0.29	0.27	0.04J	0.04U	0.03J	0.24	-
Calcium	mg/L	335	291	296	293	279	ı	314	320	319	305	424	386	421	463	403	420	353	391	395
Chloride	mg/L	6.36	6.1	6.13	6.03	5.69	-	5.77	5.73	5.65	5.39	7.91	6.7	5.76	7.56	6.88	7.62	6.82	6.35	4.98
Chromium	μg/L	0.3	0.1	0.3	0.193	0.02J	•	0.05J	0.1J	0.405	-	1.1	0.3	0.9	1.03	0.2J	0.05J	0.385	0.526	-
Cobalt	μg/L	0.824	0.771	0.623	0.811	0.919	•	0.622	1.17	1.04	-	3.87	4.97	4.21	5.29	4.8	3.67	3.96	5.21	-
Combined Radium	pCi/L	1.418	1.345	0.562	1.962	0.244	•	1.345	0.68	1.915	-	0.627	1.237	1.149	2.96	0.166	0.696	2.422	0.982	-
Fluoride	mg/L	0.2	0.19	0.24	0.17	0.16	ı	0.17	0.14	0.14	0.16	0.4	0.36	0.35	0.32	0.28	0.3	0.29	0.28	0.31
Lead	μg/L	0.101	0.162	0.089	0.229	0.383	-	0.154	0.145	0.11	-	10.1	8.64	7.53	3.77	0.546	0.2	1.02	5	-
Lithium	mg/L	0.05	0.033	0.033	0.039	0.036	-	0.047	0.039	0.042	-	0.074	0.067	0.066	0.092	0.088	0.09	0.079	0.083	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.006	0.005U	0.005	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	2.92	2.23	3.7	2.69	2.25	-	3.06	2.78	3.1	-	20.4	21.2	27.2	24.3	22.4	17.3	19	26.4	-
Selenium	μg/L	0.1	0.1U	0.05J	0.1U	0.1U	ı	0.05J	0.2U	0.2U	-	1.3	3	0.09J	0.1	0.1J	0.2U	0.3U	0.7	-
Total Dissolved Solids	mg/L	1750	1760	1670	1820	-	1860	1830	1880	1840	1820	3000	2870	2920	3260	3060	2970	2820	2890	2990
Sulfate	mg/L	947	928	885	996	-	970	954	1050	1080	1000	1880	1800	1820	2020	1880	1820	1800	1910	1880
Thallium	μg/L	0.054	0.02J	0.01J	0.03J	0.01J	-	0.01J	0.1U	0.02J	-	1.06	1.72	0.254	0.505	0.1J	0.199	0.303	1.22	-
pН	SU	7.1	7.1	7.26	7.23	6.81	6.98	6.82	7.05	6.79	7.08	7.55	7.91	7.33	6.92	6.89	6.82	7.05	6.9	7.89

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

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						S	-4									S-:	5				
Parameter	Unit	6/28/2016	8/5/2016	10/12/2016	1/11/2017	4/11/2017	4/26/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017	6/29/2016	10/12/2016	11/15/2016	1/11/2017	4/11/2017	4/26/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017
						Background	l				Detection				В	Background					Detection
Antimony	μg/L	0.41	0.17	0.43	0.31	0.25	-	0.2J	0.16	0.17	-	0.21	0.14	0.13	0.02J	0.03J	-	0.05J	0.13	0.02J	-
Arsenic	μg/L	15.7	6.82	4.84	9.1	5.37	-	12.1	5.9	6.76	-	11.6	11.8	15.8	0.68	0.6	-	1.04	4.29	0.64	-
Barium	μg/L	116	63.3	175	466	255	-	595	521	620	-	97.9	74.1	101	19.5	19.7	-	26.7	50.4	20.1	-
Beryllium	μg/L	3.13	1.24	0.217	0.506	0.192	-	0.691	0.276	0.334	-	0.771	0.7	1.19	0.01J	0.01J	-	0.051	0.221	0.021	-
Boron	mg/L	0.405	0.301	0.196	0.276	0.329	-	0.45	0.381	0.308	0.242	0.009J	0.031	0.026	0.019	0.034	-	0.087	0.032	0.03	0.02
Cadmium	μg/L	15.1	3.61	4.99	4.05	2.55	-	0.91	0.53	0.47	-	0.84	1.18	1.82	0.03	0.03	-	0.1	0.3	0.03	-
Calcium	mg/L	709	508	353	423	383	-	437	471	473	406	297	271	292	251	250	-	273	259	272	243
Chloride	mg/L	8.13	7.73	6.62	5.34	-	5.71	5.91	6.38	6.66	5.98	6.99	6.87	6.95	6.6	-	6.8	6.85	6.82	6.85	6.66
Chromium	μg/L	71.1	28.7	8	19.9	9.49	-	22.2	10.8	13.2	-	6.1	4.8	7.04	0.298	0.166	-	0.444	1.95	0.298	-
Cobalt	μg/L	116	37.1	34.9	26	18.7	-	18.1	14.1	14.2	-	7.77	6.76	9.5	0.27	0.229	-	0.718	2.76	0.296	-
Combined Radium	pCi/L	2.587	1.455	1.901	14.144	4.021	-	0.957	1.798	1.33	-	2.47	0.78	1.143	0.629	0.941	-	0.924	0.64	0.961	-
Fluoride	mg/L	0.26	0.23	0.22	0.23	-	0.26	0.24	0.21	0.21	0.2	0.11	0.1J	0.1	0.1	-	0.1	0.1	0.08	0.08	0.09
Lead	μg/L	128	29.8	5.64	13.5	7.85	-	19.1	7.7	10.2	-	24.8	28.9	39.8	0.444	0.499	-	1.91	8.38	0.769	-
Lithium	mg/L	0.14	0.072	0.047	0.076	0.064	-	0.088	0.077	0.078	-	0.014	0.011	0.023	0.014	0.011	-	0.013	0.01	0.017	-
Mercury	μg/L	0.03	1.01	0.022	0.059	0.03	-	0.003J	0.023	0.006	-	0.005U	0.005U	0.172	0.005U	0.005U	-	0.005U	0.002J	0.005U	-
Molybdenum	μg/L	9.15	3.65	6.29	5.06	3.85	-	4.04	3.62	3.72	-	1.22	0.97	1.23	0.43	0.18	-	0.32	0.67	0.31	-
Selenium	μg/L	10	4.8	1	2.1	1.1	-	2.7	1.1	0.5	-	4.7	4.8	7.6	0.06J	0.08J	-	0.3	1.6	0.08J	-
Total Dissolved Solids	mg/L	2870	3010	2280	2930	-	2690	3390	2780	2710	2310	1250	1250	1270	1240	-	1280	1310	1330	1300	1280
Sulfate	mg/L	1680	1580	1210	1400	-	1510	1500	1700	1640	1380	697	662	678	670	-	685	675	726	713	691
Thallium	μg/L	2.02	0.692	0.505	0.658	0.431	-	0.36	0.208	0.224	-	0.247	0.216	0.325	0.02J	0.05U	-	0.02J	0.096	0.02J	-
pН	SU	7.19	6.96	6.71	7.19	7.31	7.02	6.92	6.75	6.7	7.71	7.1	7.39	7.28	7.54	7.15	7.63	7.12	7.2	7.2	8.11

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

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Parameter	Unit	6/28/2016	8/5/2016	10/12/2016	1/11/2017	4/11/2017	4/20/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017	6/30/2016	8/4/2016	10/11/2016	1/11/2017	4/11/2017	4/20/2017	5/24/2017	6/21/2017	7/26/2017	10/4/2017
						Background	ł				Detection					Background	d				Detection
Antimony	μg/L	0.07	0.08J	0.05J	0.05J	0.1U	-	2U	0.01J	0.1U	-	0.07	0.1	0.09	0.08	0.1	-	0.06J	0.06J	0.06J	-
Arsenic	μg/L	0.54	0.72	0.75	0.81	0.6	-	0.8J	0.59	0.51	-	0.52	0.49	0.71	0.56	0.59	-	0.5	0.76	0.51	-
Barium	μg/L	15.4	23.7	26.5	15	13.6	•	14.3	12.7	15.1	1	13.7	13.2	14	13.3	13.3	-	14.1	12.8	13.5	-
Beryllium	μg/L	0.005J	0.04U	0.01J	0.04U	0.04U	-	0.3J	0.02U	0.04U	-	0.01J	0.008J	0.01J	0.007J	0.01J	-	0.008J	0.04U	0.009J	-
Boron	mg/L	1.65	1.97	2.06	2.15	1.69	-	1.72	1.54	1.44	1.67	1.77	1.78	1.81	1.75	1.84	-	1.82	1.74	1.79	1.88
Cadmium	μg/L	0.6	0.4	0.52	0.14	0.13	-	0.6U	0.08	0.14	-	0.03	0.07	0.09	1.16	0.43	-	0.03J	0.02J	0.02J	-
Calcium	mg/L	241	308	352	393	254	-	263	219	203	251	271	247	255	245	240	-	251	239	259	244
Chloride	mg/L	32	37.4	41.1	39	-	40.6	35.7	32.2	29.3	37	34.1	33.9	35.7	33.7	-	32.6	34.7	35.2	35.2	36.5
Chromium	μg/L	0.1	0.4	0.3	0.226	0.03J	-	0.3J	0.065	0.318	-	0.3	0.1	0.1	0.086	0.069	-	0.132	0.05J	0.329	-
Cobalt	μg/L	0.268	0.383	0.437	0.404	0.279	-	0.3J	0.226	0.208	-	0.132	0.142	0.14	0.144	0.147	-	0.14	0.149	0.159	-
Combined Radium	pCi/L	0.752	2.328	0.936	0.981	0.075	•	0.811	1.416	1.5629	1	0.996	1.149	0.235	0.824	0.437	-	1.05	0.71	1.071	-
Fluoride	mg/L	0.28	0.22	0.21	0.14	-	0.16	0.23	0.23	0.23	0.2	0.18	0.16	0.17	0.16	-	0.16	0.16	0.14	0.13	0.1
Lead	μg/L	0.737	0.862	0.717	1.4	0.211	-	0.2J	0.158	0.344	-	1.36	3.58	4.23	5.03	6.17	-	1.34	0.617	0.928	-
Lithium	mg/L	0.042	0.029	0.03	0.033	0.03	•	0.032	0.027	0.031	1	0.09	0.081	0.087	0.093	0.084	-	0.09	0.087	0.092	-
Mercury	μg/L	0.003J	0.005U	0.002J	0.005U	0.005U	•	0.005U	0.005U	0.005U	1	0.005U	0.005U	0.005U	0.005	0.005U	-	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	0.5	0.53	0.32	1.04	0.54	•	3.08	0.62	1.08	1	5.45	7.53	7.95	8.1	6.19	-	7.6	7.7	7.61	-
Selenium	μg/L	0.1U	0.2U	0.2U	0.06J	0.2U	-	3U	0.1U	0.2U	-	0.3	0.1	0.2	0.1	0.3	-	0.2	0.2J	0.1J	-
Total Dissolved Solids	mg/L	1040	2270	2260	2520	-	2470	22100	766	1890	2200	1850	1820	1760	1820	-	1850	1900	1890	1900	1860
Sulfate	mg/L	1110	1340	1350	1440	-	1420	1200	1220	1070	1250	1040	1020	988	1060	-	961	1010	1140	1090	1020
Thallium	μg/L	0.04J	0.04J	0.04J	0.04J	0.1U	-	2U	0.05U	0.03J	-	0.02J	0.02J	0.02J	0.03J	0.02J	-	0.02J	0.02J	0.03J	-
рН	SU	7.44	7.19	7.69	7.08	7.38	7.55	6.95	7.46	7.2	7.89	7.25	7.69	7.22	7.23	7.51	7.44	7	6.98	6.93	7.41

Notes:

mg/L: milligrams per liter μ g/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

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						S-10									S	-17				
Parameter	Unit	6/23/2016	8/3/2016	10/20/2016	1/17/2017	5/2/2017	5/25/2017	6/27/2017	7/26/2017	9/26/2017	6/29/2016	8/9/2016	10/12/2016	2/9/2017	3/9/2017	4/17/2017	5/31/2017	6/22/2017	8/1/2017	10/3/2017
					Backg	round				Detection]	Background	d				Detection
Antimony	μg/L	0.05	0.05J	0.06	0.05J	0.05J	0.06	0.07J	0.06J	-	0.02J	0.12	-	0.07	0.02J	0.02J	0.06	0.01J	0.07	-
Arsenic	μg/L	0.65	0.4	0.66	0.74	0.72	0.88	1.11	0.94	-	5.23	9.37	-	10.1	9.28	9.41	11.6	9.62	10.7	-
Barium	μg/L	20.2	13.1	12.9	20.6	19.9	25.6	19.4	20	-	37.7	41.2	-	31.3	31	33.7	43.3	23.7	43	-
Beryllium	μg/L	0.02J	0.04U	0.01J	0.01J	0.01J	0.02J	0.02J	0.02J	-	0.01J	0.157	-	0.042	0.01J	0.006J	0.043	0.006J	0.057	-
Boron	mg/L	1.27	0.853	0.896	1.22	1.33	1.5	1.4	1.29	0.825	0.127	0.151	-	0.173	0.157	0.153	0.175	0.221	0.165	0.168
Cadmium	μg/L	0.02J	0.01J	0.01J	0.01J	0.04U	0.03	0.04U	0.06	-	0.07	0.91	-	0.09	0.02	0.02U	0.11	0.02	0.12	-
Calcium	mg/L	246	303	285	252	251	238	264	249	295	235	237	-	254	203	201	195	138	190	212
Chloride	mg/L	25.7	25	24.3	26.4	29.3	27.8	25.3	25.1	24.6	3.52	3.98	-	3.02	2.55	2.98	3.22	2.36	2.68	4.71
Chromium	μg/L	0.2	0.02J	0.077	0.106	0.06J	0.04J	0.09J	0.372	-	0.6	4.3	-	1.17	0.479	0.03J	1	0.139	1.55	-
Cobalt	μg/L	0.111	0.043	0.079	0.077	0.068	0.311	0.292	0.26	-	0.518	1.69	-	3.43	1.27	0.769	1.17	1.1	1.11	-
Combined Radium	pCi/L	0.996	0.573	1.816	1.759	0.729	1.274	1.806	1.046	-	0.568	1.192	-	1.267	0.691	2.75	1.094	1.159	0.807	-
Fluoride	mg/L	0.17	0.21	0.23	0.16	0.16	0.15	0.15	0.13	0.17	0.24	0.22	-	0.16	0.19	0.19	0.2	0.2	0.22	0.22
Lead	μg/L	0.06	0.01J	0.034	0.067	0.048	0.056	0.047	0.057	-	0.393	6.43	-	1.79	0.471	0.008J	1.96	0.182	2.88	-
Lithium	mg/L	0.093	0.061	0.065	0.089	0.102	0.11	0.084	0.095	-	0.04	0.033	-	0.044	0.033	0.032	0.032	0.034	0.032	-
Mercury	μg/L	0.004J	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-	0.005U	0.015	-	0.006	0.005U	0.003J	0.002J	0.005U	0.005U	-
Molybdenum	μg/L	15.2	6.91	5.12	14.5	18.9	21.6	18.1	20.7	-	0.81	0.96	-	2.4	1.86	1.84	1.91	1.6	2.01	-
Selenium	μg/L	0.4	0.3	0.5	0.5	0.7	0.6	0.6	0.4	-	0.1	0.8	-	0.4	0.1	0.05J	0.3	0.04J	0.4	-
Total Dissolved Solids	mg/L	1670	1700	1690	1600	1640	1570	1660	1640	1730	1460	1410	-	1720	1370	1470	1440	1170	1420	1460
Sulfate	mg/L	967	998	970	897	889	867	979	989	1060	813	797	-	1180	740	823	821	643	850	784
Thallium	μg/L	0.05U	0.1U	0.116	0.02J	0.1U	0.05U	0.1U	0.03J	-	0.094	0.369	-	0.131	0.05J	0.03J	0.091	0.01J	0.116	-
pН	SU	6.88	6.88	6.99	6.97	7	7.12	7.13	6.92	7.74	7.04	6.97	8	6.39	-	6.8	6.73	6.84	8.08	7.14

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

								Caran	ai i iaiit -	Landini									
						S-18									S-19A				
Parameter	Unit	6/29/2016	8/9/2016	10/6/2016	1/12/2017	4/17/2017	5/31/2017	6/22/2017	7/27/2017	10/3/2017	6/24/2016	8/8/2016	10/5/2016	1/12/2017	4/17/2017	5/31/2017	6/22/2017	7/27/2017	10/3/2017
					Back	ground				Detection				Back	ground				Detection
Antimony	μg/L	0.03J	0.05	0.06	0.03J	0.09J	0.03J	0.02J	0.1U	-	0.1U	0.1U	0.1U	0.1U	0.05J	0.2U	0.2U	0.2U	-
Arsenic	μg/L	1.4	2.29	2.24	0.9	0.98	1.41	0.8	0.74	-	1.84	2.46	2.78	2.3	2.15	2.63	2.07	2.19	-
Barium	μg/L	20.9	68.9	64.5	22.1	21.7	25.9	18.3	18.3	-	18.2	18.6	15.2	15.7	20.7	19.6	18.7	19.6	-
Beryllium	μg/L	0.009J	0.092	0.103	0.008J	0.01J	0.02	0.04U	0.04U	-	0.01J	0.01J	0.01J	0.04U	0.03J	0.02J	0.06U	0.02J	-
Boron	mg/L	0.611	0.658	0.568	0.561	0.558	0.538	0.573	0.517	0.556	0.33	0.34	0.378	0.335	0.364	0.331	0.348	0.311	0.388
Cadmium	μg/L	0.06	0.04	0.04	0.02J	0.01J	0.14	0.02J	0.05	-	0.01J	0.04U	0.04U	0.04U	0.02J	0.1U	0.06U	0.06U	-
Calcium	mg/L	218	207	179	202	142	189	154	156	178	419	405	338	373	413	401	407	388	388
Chloride	mg/L	2.2	1.77	1.82	1.23	2.36	1.36	1.5	1.55	1.18	3.64	4.01	6.9	6.32	4.68	4.02	3.97	3.84	3.47
Chromium	μg/L	0.5	1.9	1.8	0.184	0.143	0.371	0.09J	0.421	-	0.1	0.1	0.1J	0.162	0.06J	0.2J	0.502	0.485	-
Cobalt	μg/L	0.248	0.606	0.834	0.35	0.334	0.215	0.151	0.227	-	0.095	0.087	0.463	0.182	0.099	0.127	0.05J	0.071	-
Combined Radium	pCi/L	1.036	2.05	2.108	1.173	0.522	1.351	2.577	0.434	-	1.769	1.239	1.759	1.821	0.572	1.565	0.948	2.26	-
Fluoride	mg/L	0.36	0.33	0.35	0.36	0.35	0.32	0.31	0.3	0.29	0.35	0.36	0.39	0.32	0.33	0.32	0.36	0.29	0.32
Lead	μg/L	0.329	1.87	2.48	0.236	0.175	0.877	0.07	0.116	-	0.041	0.01J	0.089	0.03J	0.06J	0.106	0.066	0.05J	-
Lithium	mg/L	0.074	0.063	0.057	0.073	0.057	0.056	0.06	0.057	-	0.061	0.058	0.061	0.065	0.062	0.06	0.066	0.061	-
Mercury	μg/L	0.002J	0.009	0.005U	0.002J	0.003J	0.006	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.003J	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	11.5	12.1	13.7	10.7	7.63	10.9	7.92	8.4	-	3.49	4.83	7.8	6.77	5.12	6.89	5.35	5.31	-
Selenium	μg/L	0.1	0.3	0.4	0.06J	0.3	0.07J	0.2U	0.2U	-	0.2U	0.09J	0.1J	0.2U	0.4U	0.5U	0.3U	0.3U	-
Total Dissolved Solids	mg/L	1540	1520	1600	1760	1610	1490	1530	1510	1520	3290	3190	2780	2910	3230	6730	3330	3080	3050
Sulfate	mg/L	910	859	928	1040	908	872	899	842	799	2070	1980	1700	1730	2040	2090	2130	2120	1930
Thallium	μg/L	0.095	0.02J	0.03J	0.069	0.02J	0.03J	0.1U	0.02J	-	0.1U	0.1U	0.1U	0.1U	0.05J	0.2U	0.2U	0.03J	-
рН	SU	6.98	7.01	7.35	6.87	6.97	6.76	6.93	6.98	7.12	6.83	6.85	6.85	6.83	6.89	6.64	6.8	6.96	6.97

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter

pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

									Carum	ai i iaiit -	Lanum										
							S-20										SGS-1				
Parameter	Unit	6/29/2016	8/3/2016	10/6/2016	1/17/2017	4/12/2017	4/19/2017	5/2/2017	5/30/2017	6/21/2017	8/1/2017	9/26/2017	6/23/2016	8/5/2016	10/20/2016	1/16/2017	5/2/2017	5/24/2017	6/22/2017	7/27/2017	10/4/2017
						Backgr	round					Detection				Backg	round				Detection
Antimony	μg/L	0.13	0.09J	0.06J	0.04J	0.22	-	-	0.05J	0.04J	0.1U	-	0.05J	0.04J	0.03J	0.07J	0.03J	0.02J	0.03J	0.02J	-
Arsenic	μg/L	4.91	4.55	3.11	2.67	2.35	-	-	2.42	2.16	2.06	-	1.42	1.71	2.47	3.17	2.22	2.16	2.18	2.73	-
Barium	μg/L	16.5	16.3	16.6	16.1	16.4	-	-	16.1	15.4	14.6	-	34	32.3	31.7	38.4	32.9	34.7	33.1	35.1	-
Beryllium	μg/L	0.03	0.03J	0.01J	0.01J	0.01J	-	-	0.01J	0.01J	0.01J	-	0.04U	0.02U	0.02U	0.08U	0.04U	0.04U	0.04U	0.04U	-
Boron	mg/L	0.172	0.2	0.22	0.22	0.254	-	-	0.214	0.273	0.294	0.293	0.989	0.948	1.12	0.853	0.831	0.912	0.877	0.866	0.934
Cadmium	μg/L	0.02U	0.04U	0.05	0.02U	0.02U	-	-	0.04U	0.04U	0.04U	-	0.04U	0.005J	0.02U	0.02J	0.04U	0.04U	0.04U	0.04U	-
Calcium	mg/L	345	342	386	344	323	-	-	305	326	322	339	189	170	156	129	118	126	119	115	110
Chloride	mg/L	3.04	2.62	2.72	2.59	-	2.76	2.81	2.84	2.72	2.59	2.53	28.6	25.5	24.1	23.6	24.3	24	24	23.8	23.4
Chromium	μg/L	5.5	1	0.3	0.212	3.5	-	-	2.32	0.508	0.384	-	0.2	0.1	0.245	0.2J	0.1J	0.223	0.13	0.355	-
Cobalt	μg/L	4.51	5.58	6.7	5.77	5.14	-	-	5.4	5.17	5.21	-	1.91	1.04	0.885	0.872	0.61	0.516	0.544	0.555	-
Combined Radium	pCi/L	0.504	1.89	1.005	2.0464	0.43	-	-	0.861	1.32	4.988	-	0.778	1.893	1.792	4.035	0.259	1.207	1.043	1.273	-
Fluoride	mg/L	0.22	0.26	0.25	0.25		0.25	0.26	0.23	0.24	0.26	0.26	0.42	0.47	0.51	0.57	0.59	0.58	0.57	0.56	0.55
Lead	μg/L	0.462	0.348	0.375	0.164	0.128	-	-	0.145	0.089	0.02J	-	0.041	0.02J	0.009J	0.05J	0.01J	0.03J	0.02J	0.02J	-
Lithium	mg/L	0.054	0.033	0.034	0.036	0.038	-	-	0.03	0.037	0.039	-	0.026	0.027	0.031	0.027	0.026	0.031	0.024	0.029	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.003J	-	-	0.005U	0.005U	0.005U	-	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	0.91	0.47	1.67	0.5	0.72	-	-	0.9	4	1.92	-	10.2	3.16	30.1	15.5	2.07	1.93	2.18	2.11	-
Selenium	μg/L	0.2	0.2U	0.06J	0.04J	0.08J	-	-	0.2J	0.2U	0.2U	-	0.09J	0.07J	0.05J	0.2J	0.2U	0.06J	0.2U	0.2U	-
Total Dissolved Solids	mg/L	1740	2030	2060	2010	-	1920	1940	1840	1960	1900	1950	1970	1890	1920	1820	1820	1810	1810	1810	1800
Sulfate	mg/L	1030	1140	1170	1070	-	1040	1030	1030	1070	1230	1180	1030	1010	947	918	916	891	947	959	922
Thallium	μg/L	0.03J	0.1U	0.1U	0.02J	0.01J	-	-	0.1U	0.04J	0.1U	-	0.1U	0.04J	0.03J	0.585	0.02J	0.1U	0.1U	0.1U	-
pН	SU	6.57	6.58	6.97	6.64	6.7	7.26	6.64	6.57	6.3	7.94	7.74	6.86	6.98	6.72	7.08	7.44	7.05	7.62	8.86	7.99

Notes:

mg/L: milligrams per liter $\mu g/L$: micrograms per liter pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

	Carumai I fant - Langini																		
		SGS-2									SGS-3								
Parameter	Unit	6/23/2016	8/4/2016	10/20/2016	1/16/2017	5/2/2017	5/24/2017	6/21/2017	7/27/2017	10/4/2017	6/27/2016	8/3/2016	10/7/2016	1/10/2017	4/17/2017	5/30/2017	6/22/2017	7/27/2017	10/3/2017
		Background Detection								Background							Detection		
Antimony	μg/L	0.74	0.31	0.13	0.2J	0.23	0.22	0.15	0.11	-	0.63	0.32	0.31	0.53	0.24	0.74	0.1J	0.11	-
Arsenic	μg/L	2.65	4.36	8.13	16.5	16.7	26.8	17.1	17.8	-	1.11	1.27	2.04	3.61	2.69	3.48	3.14	3.11	-
Barium	μg/L	50.5	49.3	50.9	67.5	63.7	82.7	71.6	78.4	-	74.3	104	189	253	156	208	155	152	-
Beryllium	μg/L	0.04U	0.02U	0.006J	0.08U	0.04U	0.04U	0.04U	0.04U	-	0.02J	0.01J	0.072	0.204	0.008J	0.09	0.06U	0.04U	-
Boron	mg/L	0.87	0.869	0.851	0.612	0.655	0.59	0.587	0.612	0.67	0.209	0.236	0.452	0.29	0.283	0.299	0.306	0.283	0.302
Cadmium	μg/L	0.14	0.04	0.009J	0.08U	0.04U	0.04U	0.04U	0.04U	-	0.008J	0.008J	0.007J	0.04	0.006J	0.11	0.06U	0.01J	-
Calcium	mg/L	104	70.3	44.8	23.2	26	12.1	16.1	15.1	13	16.4	15	9.98	24.6	6.04	12.2	4.98	4.96	4.82
Chloride	mg/L	80.9	77.5	63.3	102	70.5	97.7	68	111	62.2	149	248	279	297	313	346	311	298	314
Chromium	μg/L	0.3	0.6	0.544	0.423	0.281	0.279	0.264	0.505	-	0.8	1.5	1	6.64	0.293	3.23	0.223	0.4	-
Cobalt	μg/L	1.04	0.788	0.218	0.155	0.142	0.097	0.105	0.11	-	0.702	1.09	0.644	3.06	0.4	1.95	0.325	0.335	-
Combined Radium	pCi/L	0.799	1.316	1.087	1.34	0.988	1.115	0.563	0.552	-	0.237	1.659	1.673	0.448	0.506	1.293	0.563	0.672	_
Fluoride	mg/L	1.6	1.74	2.1	2.01	2.24	2.36	2.42	2.61	2.33	1.18	0.73	2.02	1.74	1.81	1.92	2.04	1.91	2.04
Lead	μg/L	0.131	0.192	0.13	0.164	0.266	0.19	0.184	0.15	-	0.358	0.382	0.71	5.13	0.279	3.43	0.15	0.184	-
Lithium	mg/L	0.044	0.035	0.039	0.034	0.032	0.034	0.03	0.034	-	0.047	0.022	1.96	0.032	0.02	0.02	0.027	0.025	-
Mercury	μg/L	0.005U	0.005U	0.005U	0.005U	0.002J	0.004J	0.003J	0.003J	-	0.005U	0.005U	0.005U	0.005U	0.002J	0.005U	0.005U	0.005U	-
Molybdenum	μg/L	31.1	22.8	15.5	3.82	5.31	1.82	7.18	7.28	-	24.2	30.2	12.6	11.6	12.2	13.4	12.6	10.6	-
Selenium	μg/L	0.8	0.2	0.09J	0.4U	0.1J	0.1J	0.08J	0.09J	-	0.3	0.2	0.3	0.7	0.1U	0.2	0.3U	0.2U	-
Total Dissolved Solids	mg/L	2050	1980	1790	1960	1830	1820	1740	1750	1700	1570	1550	1600	1690	-	1730	3390	1700	1630
Sulfate	mg/L	832	799	491	365	344	127	201	73.2	131	494	56.2	100	93.2	63.6	83.4	103	126	118
Thallium	μg/L	0.03J	0.055	0.02J	0.2U	0.1U	0.1U	0.1U	0.1U	-	0.02J	0.066	0.01J	0.06	0.05U	0.05J	0.2U	0.05J	-
рН	SU	7.41	7.91	7.87	7.91	7.89	8.17	8.15	8.84	8.28	7.9	8.21	8.22	8.06	7.94	7.83	7.9	7.86	8.09

Notes:

mg/L: milligrams per liter

μg/L: micrograms per liter pCi/L: picocuries per liter

SU: standard unit

U: Component was not present in concentrations above method

detection limit and is reported as the reporting limit

J: Estimated value. Component was detected in concentrations below the reporting limit

Table 1: Groundwater Data Summary Cardinal Plant - Landfill

		CA-0623A		OAE-2005-10-C			S-1		S	-2
Parameter	Unit	5/15/2018	10/16/2018	5/15/2018	10/16/2018	5/16/2018	10/9/2018	11/19/2018	5/16/2018	10/9/2018
		2018-D1	2018-D2	2018-D1	2018-D2	2018-D1	2018-D2	2018-D2-V1	2018-21	2018-D2
Boron	mg/L	0.546	0.513	0.476	0.502	0.888	0.97	0.961	1.09	2.5
Calcium	mg/L	1.64	1.18	6.3	5.44	315	321	-	271	385
Chloride	mg/L	14.1	20.7	9.69	12	5.42	6.4	-	8.09	5.6
Fluoride	mg/L	2.08	2.3	1.09	1.1	0.19	0.23	-	0.42	0.2
pН	SU	8.59	8.72	8.37	8.55	6.84	7.46	-	7.54	7.53
Total Dissolved Solids	mg/L	609	642	1410	1410	1880	1840	-	2110	3060
Sulfate	mg/L	33.1	30.9	357	377	1030	1020	-	1340	1840

		S-4		S-5		S	-6			S-7		
Parameter	Unit	5/16/2018	10/12/2018	5/16/2018	10/12/2018	5/16/2018	10/12/2018	1/24/2018	2/15/2018	5/16/2018	10/9/2018	11/19/2018
		2018-D1	2018-D2	2018-D1	2018-D2	2018-D1	2018-D2	2017-D1-V1	2017-D1-V1	2018-D1	2018-D2	2018-D2-V1
Boron	mg/L	0.255	0.307	0.051	0.0222	1.57	1.73	1.90	2.12	1.93	2.16	-
Calcium	mg/L	593	459	268	258	209	236	-	-	251	263	-
Chloride	mg/L	5.37	6	6.95	8.1	30.9	34.7	-	-	34.7	38.4	31.9
Fluoride	mg/L	0.27	0.27	0.11	0.13	0.28	0.25	-	-	0.18	0.17	-
pН	SU	6.92	7.94	7.37	8.77	7.23	8.02	-	-	7.1	7.61	-
Total Dissolved Solids	mg/L	3260	606	1260	1280	1950	2040	-	-	1870	1890	-
Sulfate	mg/L	1580	1600	704	743	1100	1200	-	-	1090	1080	-

Notes:

mg/L: milligrams per liter

SU: standard unit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

Not sampled

2017-D1-V1: Verification sampling for initial detection monitoring event (initial detection event occurred in 2017)

2018-D1: First semi-annual detection monitoring event of 2018

2018-D1-V1: Verification sampling, first semi-annual detection monitoring event

2018-D2: Second semi-annual detection monitoring event of 2018

2018-D2-V1: Verification sampling, second semi-annual detection monitoring event

Table 1: Groundwater Data Summary Cardinal Plant - Landfill

				S-10			S-	17		S-18	
Parameter	Unit	1/24/2018	5/22/2018	8/7/2018	10/15/2018	11/19/2018	5/15/2018	10/16/2018	5/15/2018	10/8/2018	11/19/2018
		2017-D1-V1	2018-D1	2018-D1-V1	2018-D2	2018-D2-V1	2018-D1	2018-D2	2018-D1	2018-D2	2018-D2-V1
Boron	mg/L	-	1.87	1.37	1.74	1.88	0.229	0.212	0.573	0.586	-
Calcium	mg/L	-	196	-	178	-	143	140	172	164	-
Chloride	mg/L	-	25.1	-	22.8	-	3.21	5.7	1.64	2.9	1.7
Fluoride	mg/L	-	0.100 J	-	0.16	-	0.23	0.28	0.36	0.39	0.36
pН	SU	-	7.27	-	7.18	-	6.87	7.13	7.05	7.05	-
Total Dissolved Solids	mg/L	-	1450	-	1480	-	1210	514	1320	1250	-
Sulfate	mg/L	894	849	-	834	-	671	775	743	772	-

	S-19A				S-20				SGS-1		SG	S-2	SG	S-3
Parameter	Unit	5/15/2018	10/16/2018	5/17/2018	8/7/2018	10/8/2018	11/19/2018	5/22/2018	8/7/2018	10/15/2018	5/22/2018	10/15/2018	5/15/2018	10/15/2018
		2018-D1	2018-D2	2018-D1	2018-D1-V1	2018-D2	2018-D2-V1	2018-D1	2018-D1-V1	2018-D2	2018-D1	2018-D2	2018-D1	2018-D2
Boron	mg/L	0.398	0.409	0.34	0.264	0.267	-	0.923	-	0.911	0.556	0.55	0.418	0.319
Calcium	mg/L	419	385	315	-	319	-	118	-	107	9.96	7.94	5.3	5
Chloride	mg/L	3.57	3.6	2.93	-	3.9	2.7	23.8	-	23.2	91.2	99.7	375	401
Fluoride	mg/L	0.40	0.35	0.27	-	0.31	0.22	0.72	0.62	0.64	2.75	2.6	2.08	1.9
pН	SU	6.94	7.26	6.75	-	6.83	-	7.23	-	7.17	7.98	7.98	8.05	8.19
Total Dissolved Solids	mg/L	3210	3100	1480	-	1860	-	1800	-	1820	1700	1700	1750	1900
Sulfate	mg/L	2080	2080	1040	-	1060	-	906	-	935	81.1	73.8	123	126

Notes:

mg/L: milligrams per liter

SU: standard unit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

2017-D1-V1: Verification sampling for initial detection monitoring event (initial detection event occurred in 2017)

2018-D1: First semi-annual detection monitoring event of 2018

2018-D1-V1: Verification sampling, first semi-annual detection monitoring event

2018-D2: Second semi-annual detection monitoring event of 2018

2018-D2-V1: Verification sampling, second semi-annual detection monitoring event

Table 1 - Groundwater Data Summary Cardinal Plant - Landfill and Fly Ash Reservoir I

		CA-0	623A	23A OAE-2005-10-C			S	-1		S	-2	S-4	
Parameter	Unit	3/26/2019	10/1/2019	4/2/2019	10/7/2019	2/7/2019	3/29/2019	5/1/2019	10/3/2019	3/29/2019	10/3/2019	3/29/2019	10/9/2019
		2019-D1	2019-D2	2019-D1	2019-D2	2018-D2-R2	2019-D1	2019-D1-R1	2019-D2	2019-D1	2019-D2	2019-D1	2019-D2
Boron	μg/L	441	440	468	489	960	938	-	834	1,280	2,590	312	263
Calcium	μg/L	929	1,070	4,850	6,180	-	333,000	-	341,000	318,000	404,000	499,000	478,000
Chloride	mg/L	11.9	19.9	12.6	19.3	-	5.00	-	4.50	6.70	4.50	4.40	4.30
Fluoride	mg/L	2.10	2.00	1.10	1.10	-	0.110	-	0.120	0.370	0.290	0.190	0.240
Total Dissolved Solids	mg/L	632	646	1,440	1,350	-	1,800	-	1,790	2,400	2,930	2,130	2,560
Sulfate	mg/L	32.1	18.7	363	421	-	1,400	940	992	1,290	1,910	1,400	1,440
pН	SU	8.76	8.71	7.80	7.61	7.19	7.19	7.11	7.43	7.05	7.37	7.34	7.60

	S-5 S-6			-6			S-7			S-10				
Parameter	Unit	3/28/2019	10/9/2019	3/28/2019	10/3/2019	1/7/2019	3/28/2019	5/1/2019	5/22/2019	10/3/2019	1/7/2019	3/28/2019	5/1/2019	9/30/2019
		2019-D1	2019-D2	2019-D1	2019-D2	2018-D2-R2	2019-D1	2019-D1-R1	2019-D1-R2	2019-D2	2018-D2-R2	2019-D1	2019-D1-R1	2019-D2
Boron	μg/L	22.2	20.5	1,510	2,190	1,900	1,980	1,940	1,860	2,000	1,600	1,080	-	608
Calcium	μg/L	302,000	287,000	182,000	418,000	-	252,000	-	-	255,000	-	268,000	-	278,000
Chloride	mg/L	6.90	7.20	27.1	37.6	29.9	32.1	-	-	31.4	19.8	23.1	-	19.4
Fluoride	mg/L	0.210	0.110	0.370	0.0720	-	0.260	0.0910	-	0.0880	-	0.290	0.0950	0.200
Total Dissolved Solids	mg/L	1,280	1,240	1,780	2,500	-	1,870	-	-	1,930	-	1,580	-	1,710
Sulfate	mg/L	739	689	973	1,360	-	1,100	-	-	984	-	966	-	946
pН	SU	8.11	7.93	7.59	7.00	7.48	7.72	7.42	7.22	7.68	7.40	7.23	7.16	7.03

Notes:

μg/L: micrograms per liter mg/L: milligrams per liter SU: standard unit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

2018-D2-R2: Second verification sampling, second semi-annual detection monitoring event of 2018

2019-D1: First semi-annual detection monitoring event of 2019

2019-D1-R1: Verification sampling, first semi-annual detection monitoring event

2019-D1-R2: Second verification sampling, first semi-annual detection event of 2019

2019-D2: Second semi-annual detection monitoring event of 2019

2019-D2-R1: Verification sampling, second semi-annual detection monitoring event

Table 1 - Groundwater Data Summary Cardinal Plant - Landfill and Fly Ash Reservoir I

		S-17		S-	18		S-1	19A		S-20	
Parameter	Unit	4/2/2019	4/2/2019	5/9/2019	10/7/2019	11/21/2019	3/28/2019	10/8/2019	3/28/2019	5/1/2019	10/3/2019
		2019-D1	2019-D1	2019-D1-R1	2019-D2	2019-D2-R1	2019-D1	2019-D2	2019-D1	2019-D1-R1	2019-D2
Boron	μg/L	182	558	-	549	-	388	381	253	-	287
Calcium	μg/L	159,000	112,000	-	146,000	-	457,000	452,000	361,000	-	304,000
Chloride	mg/L	4.20	2.60	2.10	1.80	-	3.00	3.30	2.70	-	2.70
Fluoride	mg/L	0.190	0.300	-	0.300	-	0.460	0.270	0.370	0.150	0.220
Total Dissolved Solids	mg/L	1,270	938	-	1,070	-	3,200	3,180	1,830	-	1,750
Sulfate	mg/L	644	444	-	688	-	2,090	2,180	1,130	-	1,120
рН	SU	7.03	7.08	-	6.63	6.99	7.07	6.95	6.65	7.08	6.75

			SGS-1		SG	S-2	SG	S-3
Parameter	Unit	3/28/2019	5/9/2019	10/2/2019	3/28/2019	10/2/2019	3/26/2019	10/2/2019
		2019-D1	2019-D1-R1	2019-D2	2019-D1	2019-D2	2019-D1	2019-D2
Boron	μg/L	870	-	855	502	473	302	288
Calcium	μg/L	111,000	-	110,000	6,920	6,380	5,600	5,540
Chloride	mg/L	23.1	-	22.7	90.9	93.2	404	363
Fluoride	mg/L	0.750	0.610	0.620	2.80	2.80	2.10	1.90
Total Dissolved Solids	mg/L	1,760	-	1,780	1,620	1,600	1,880	1,950
Sulfate	mg/L	954	-	991	58.3	38.1	174	184
рН	SU	7.36	-	7.45	8.02	8.02	8.40	8.21

Notes:

 μ g/L: micrograms per liter mg/L: milligrams per liter

SU: standard unit

J: Estimated value. Parameter was detected in concentrations below the reporting limit

-: Not sampled

2018-D2-R2: Second verification sampling, second semi-annual detection monitoring event of 2018

2019-D1: First semi-annual detection monitoring event of 2019

2019-D1-R1: Verification sampling, first semi-annual detection monitoring event

2019-D1-R2: Second verification sampling, first semi-annual detection event of 2019

2019-D2: Second semi-annual detection monitoring event of 2019

2019-D2-R1: Verification sampling, second semi-annual detection monitoring event

Table 1: Detection Monitoring Data Evalation Cardinal Plant - Landfill

Parameter	Unit	Description	S-1	S-7	S-10	S-18	S-20	SGS-1	SGS-2	SGS-2
r arameter	Oilit	Description	4/16/2020	4/14/2020	4/14/2020	4/14/2020	4/20/2020	4/14/2020	4/13/2020	6/3/2020
Boron	mg/L	Intrawell Background Value (UPL)	1.01	2.15	2.13	0.659	0.360	1.11	0.9	980
Bolon	mg/L	Detection Monitoring Result	0.830	1.96	1.55	0.549	0.245	0.794	0.467	
Calcium	mg/L	Intrawell Background Value (UPL)	353	275	342	246	390	198	32	2.6
Calcium	mg/L	Detection Monitoring Result	309	256	190	126	307	104	6.26	
Chloride	ma/I	Intrawell Background Value (UPL)	6.83	39.2	30.5	3.07	3.90	28.6	12	25
Cilioride	mg/L	Detection Monitoring Result	4.80	30.9	22.7	2.80	3.50	23.4	103	
Fluoride	ma/I	Intrawell Background Value (UPL)	0.267	0.258	0.293	0.411	0.362	0.788	3.	23
Fluoride	mg/L	Detection Monitoring Result	0.130	0.150	0.130	0.410	0.340	0.660	3.40	2.5
		Intrawell Background Value (UPL)	7.5	7.9	7.7	7.4	7.9	8.8	8	.8
pН	SU	Intrawell Background Value (LPL)	6.6	6.7	6.6	6.7	6.3	5.9	7	.2
		Detection Monitoring Result	7.2	7.7	7.2	6.9	6.6	7.1	8.0	
Total Dissolved	ma/I	Intrawell Background Value (UPL)	1960	1960	1830	1980	2250	1980	21	30
Solids (TDS)	mg/L	Detection Monitoring Result	1780	1850	1390	921	1720	1750	1530	
Sulfate	ma/I	Intrawell Background Value (UPL)	1400	1180	1100	1190	1260	1050	48	88
Sulfate	mg/L	Detection Monitoring Result	899	1010	694	475	1010	848	28.5	

Notes:

UPL: Upper prediction limit LPL: Lower prediction limit

Bold values exceed the background value.

Background values are shaded gray.

FAR I RSW Landfill 40 CFR 257.101 (f)(1)(iv)(B)(4)

A description of site hydrogeology including stratigraphic cross-sections



May	64.6	2.95
June	70.0	10.69
July	71.4	4.66
August	70.5	2.81
September	69.3	6.70
October	53.2	2.56
November	47.8	1.17
December	46.6	3.24

2.4.2 Regional and Local Geologic Setting

The geology at the former FAR I RSW Landfill and the vicinity consists of nearly horizontal sequences of lower Permian and upper Pennsylvanian sedimentary rock. The Permian-age Dunkard Group occurs only on the tops of some ridges above an elevation of approximately 1,250 feet (ft), northwest and west of landfill and FAR II sites.

The Monongahela Group is up to 230 feet thick in Jefferson County, consisting of shale, sandstone, limestone, coal claystone and siltstone. These rocks form much of the slopes above the current levels of the FAR I RSW Landfill and FAR II sites. Below the Monongahela Group is the Conemaugh Group, which is generally over 500 feet thick in Jefferson County. The Conemaugh Group consists of shale, sandstone, limestone, coal, and claystone, including the Morgantown Sandstone, which is a developed aquifer in the area. Beneath the Morgantown Sandstone is a sequence of the Conemaugh Group including the Elk Lick Limestone, the Skelly Limestone and shale, the Ames Limestone, several thick shale sequences, and the Cow Run Sandstone (AEP, 2005a).

2.4.3 Surface Water and Surface Water-Groundwater Interactions

The intermittent stream of the western branch of Blockhouse Hollow at the northwest end of the FAR I RSW Landfill was historically re-routed during surface mining operations and flows in a constructed stream channel along the bottom of the highwall slope north of the landfill and former FAR I. Blockhouse Hollow then drains into FAR II. Surface water northeast of the landfill flows to, or is collected and drained to, Blockhouse Hollow. Drainage from the highwall adjacent to Cells 1 & 2 of the landfill is collected in an engineered highwall drainage layer and conveyed through the landfill subsurface drainage layer and piping to a perimeter solid wall transmission pipe that discharges into the Blockhouse Hollow channel draining to FAR II (AEP, 2006; AEP, 2007). Perimeter landfill and final cover system surface water will be collected and conveyed in piping to either Blockhouse Hollow or piping that drains directly to FAR II. Landfill contact stormwater is collected and transferred to the landfill leachate collection system. Both surface stormwater and



leachate are transferred to FAR II as FAR II serves as the facilities sedimentation pond and leachate collection pond.

2.4.4 Water Users

According to water well records obtained from the Ohio Department of Natural Resources (ODNR), the nearest water supply well is located approximately 3,000 feet east of the landfill. Additionally, ODNR records indicate a series of water supply wells in the Tidd-Dale Subdivision of Brilliant, Ohio, approximately 4,000 to 5,000 feet southeast of the former FAR I RSW Landfill. These water supply wells are developed in the deeper Buffalo Sandstone, which underlies the uppermost aquifer.

Approximately one mile west of the former FAR I RSW Landfill, a series of water supply wells develop several limestone horizons, apparently the Arnoldsburg and Benwood Limestone units. These well logs report pumping rates ranging from approximately 1.0 gpm to 8.0 gpm with significant drawdown (AEP, 2006).

According to the Jefferson County Water and Sewer District, there are no surface water intakes supplying water to the town of Brilliant, Ohio. Brilliant's water source comes from two groundwater wells located at a water treatment plant approximately 1.25 mile east of the FAR I RSW Landfill. ODNR records indicate these wells are screened within the alluvial deposits of the Ohio River and exhibit pumping rates of up to 700 gpm.



3. MONITORING NETWORK EVALUATION

3.1 Hydrostratigraphic Units

3.1.1 Horizontal and Vertical Position relative to CCR Unit

The hydrogeology at the former FAR I RSW Landfill Facility is characterized by an uppermost aquifer system comprised of sandstone and limestone units, specifically the Connellsville Sandstone, Summerfield Limestone, and Bellaire Sandstone, which lie above the shale aquitard that caps the Morgantown Sandstone. The landfill is situated horizontally and vertically within the upper sandstone and limestone units and above the former FAR I. The landfill is separated from FAR I by a base liner system and five feet of geologic material. The existing monitoring network includes wells located upgradient and downgradient of the landfill facility that are screened within the uppermost aquifer system, referred to as the Shallow Aquifer. Geologic cross-sections illustrating the horizontal and vertical position of FAR II relative to the uppermost aquifer are provided in Appendix B.

3.1.2 Overall Flow Conditions

Based on monitoring well data in the vicinity of the former FAR I RSW Landfill site, the uppermost aquifer system is under water table conditions. This uppermost aquifer includes unconsolidated mine waste, sandstone, and limestone beds with a range of hydraulic conductivity from 1 x 10⁻¹ to 1 x 10⁻⁴ centimeters per second (cm/sec) (AEP, 2006). This water table zone generally flows toward the FAR I RSW Landfill from the east and west, while flowing south towards the Ohio River on the south side of the FAR I RSW Landfill. The shale aquitard where present above the Morgantown Sandstone has very low hydraulic conductivity values, in the range of 1 x 10⁻⁷ to 1 x 10⁻⁹ cm/sec. Contours depicting the groundwater elevations in the Shallow Aquifer are shown in Figure 3-1.

Historical groundwater elevation data for the Shallow Aquifer show water table elevations in the range of 1000 to 1010 ft upgradient and approximately 960 feet on the downgradient side of the FAR I RSW Landfill. The groundwater elevation data indicates a regular seasonal variation, with spring water levels up to several feet higher than fall water levels. Seasonal variation appears somewhat more pronounced on the upgradient side of the FAR I RSW Landfill (AEP, 2006).

3.2 Uppermost Aquifer

3.2.1 CCR Rule Definition

According to the 2015 CCR rule, the term "uppermost aquifer" has the same provisions as in §257.40: "The geologic formation nearest the natural ground surface that is an aquifer, as well as lower aquifers that are hydraulically interconnected with this aquifer within the facility's property boundary. This definition includes a shallow, deep, perched, confined, or unconfined aquifer, provided that it yields usable water" (40 CFR 257.60).

3-1



For the purposes of this report, it is assumed that the uppermost useable aquifer has the following characteristics: (1) groundwater production rate over a 24-hour period of at least 0.1 gallons per minute (gpm); and (2) groundwater quality with total dissolved solids (TDS) less than 10,000 milligrams per liter (mg/L).

3.2.2 Identified Onsite Hydrostratigraphic Unit

The FAR I RSW Landfill overlies the former FAR I reservoir, which had surface elevations from approximately 990 ft. to 1,020 ft. Based upon these elevations and the elevations of the material underlying the original FAR I topography, the uppermost aquifer consists of saturated unconsolidated material, limestone, and sandstone sedimentary units.

Based on ODNR water well logs, the nearest wells with a recorded pumping rate (not including wells screened in the alluvial sediments near the Ohio River) occur approximately one mile west of FAR I RSW Landfill. These wells are screened within limestone and shale units, and at a similar elevation to the upper aquifer system at the FAR I RSW Landfill. These wells have recorded pumping rates ranging from 1.0 to 8.0 gpm.

Based on the information gathered from ODNR, geological and hydrogeologic conditions at the FAR I RSW Landfill, the uppermost aquifer is considered to be the unconsolidated material, limestone, and sandstone sedimentary units (Shallow Aquifer) which lie above the shale aquitard and Morgantown Sandstone.

3.3 Review of Existing Monitoring Network

3.3.1 Overview

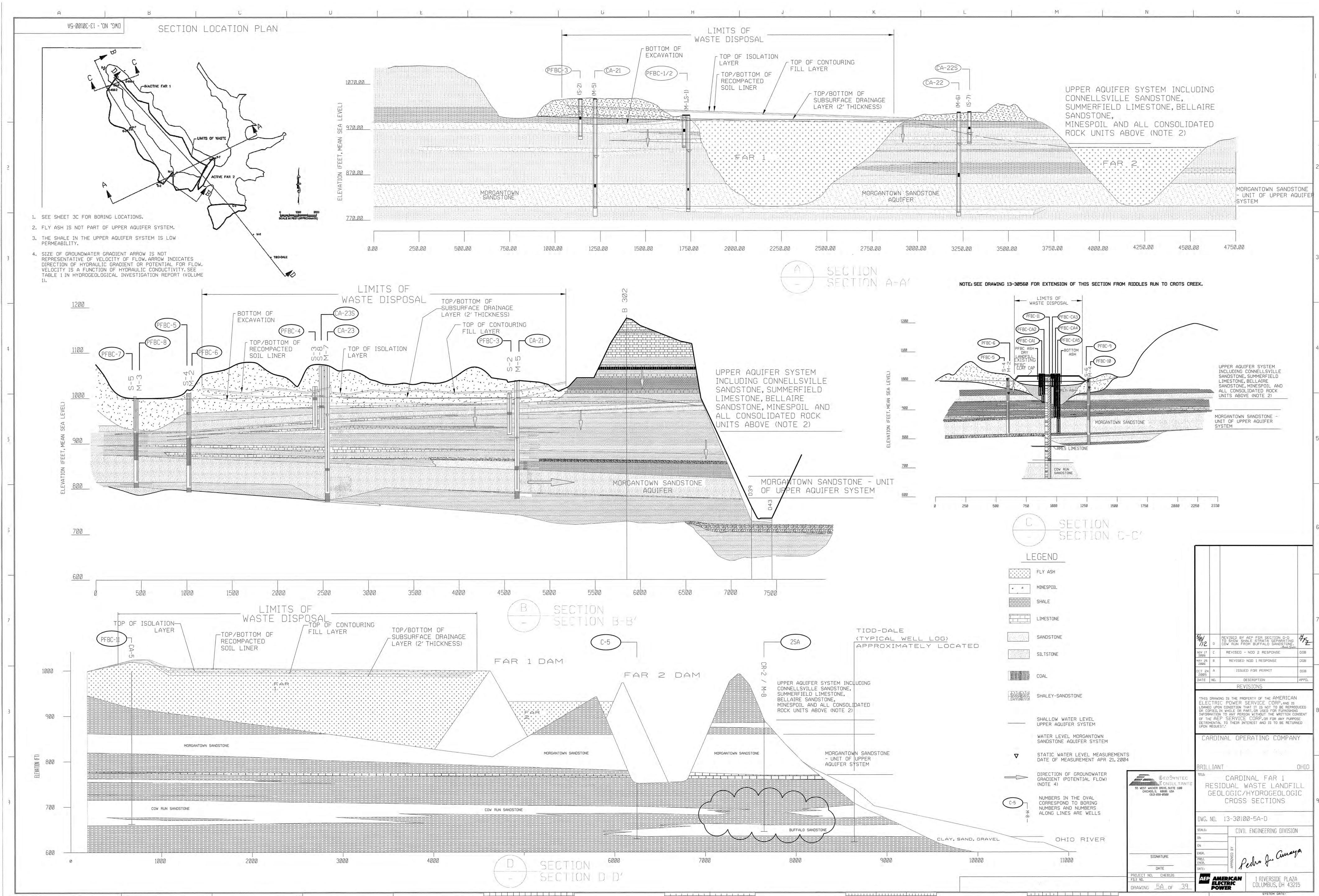
The groundwater monitoring network is shown in Figure 3-2 and consists of seven (7) wells located upgradient (0AE 2005 10C, CA-0623A, S-GS-3, S-4, S-5, S-6 and S-17) and nine (9) monitoring wells located downgradient (S-GS-1, S-GS-2, S-1, S-2, S-7, S-10, S-18, S-19 and S-20) of the former FAR I RSW Landfill. The network will provide detection monitoring for the uppermost aquifer (Shallow Aquifer). The number, spacing, and depth of groundwater monitoring wells included in the groundwater monitoring network are based on site-specific geochemical, geologic and hydrogeologic information and span the full thickness of the uppermost aquifer system. Well construction details are summarized in Table 3-1. Boring and well construction logs for the groundwater monitoring well network wells are provided in Appendix C and Appendix D, respectively.

3.3.2 Compliance Assessment

Review of the existing groundwater monitoring well network in relation to the geologic and hydrogeologic conditions in the area of the former FAR I RSW Landfill indicates that the monitoring well network consists of a sufficient number of wells installed at the appropriate depths to collect groundwater samples from the uppermost aquifer system that accurately represent the



groundwater quality upgradient and downgradient of the former FAR I RSW Landfill. groundwater monitoring well network is also capable of providing upgradient background groundwater quality and downgradient detection monitoring for a potential contaminant release to the uppermost aquifer (Shallow Aquifer) nearest the waste boundary. Based on the above review, the groundwater monitoring network around the Cardinal former FAR I RSW Landfill meets the requirements of 40 CFR 257.91.



FAR I RSW Landfill 40 CFR 257.101 (f)(1)(iv)(B)(5)

Any corrective measures assessment conducted as required at 40 CFR 257.96

Not applicable. The FAR I RSW Landfill is in Detection Monitoring

FAR I RSW Landfill

40 CFR 257.101 (f)(1)(iv)(B)(6)

Any progress reports on corrective action remedy selection and design and the report of final remedy selection required at 40 CFR 257.97(a)

Not applicable. The FAR I RSW Landfill is currently in Detection Monitoring.

FAR I RSW Landfill

40 CFR 257.101 (f)(1)(iv)(B)(7)

The most recent structural stability assessment required at 40 CFR 257.73(d)

Not applicable to Landfills

FAR I RSW Landfill 40 CFR 257.101 (f)(1)(iv)(B)(8)

The most recent safety factor assessment required at 40 CFR 257.73(e)

Not Applicable to Landfills